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EXECUTIVE DOCUMENTS

PRINTED BY ORDER OF

-THE HOUSE OF REPRESENTATIVES

DURING THE

SECOND SESSION OF THE FORTIETH CONGRESS,

1867-'68.

IN TWENTY VOLUMES.

Volume	2	No. 1. War: Parts 1 and 2.
Volume	3	No. 1. Interior: Parts 1 and 2.
Volume	4	No. 1. Navy, Postmaster General.
Volume	5	No. 2 and 3.
Volume	6	No. 4 and 5.
Volume	7	No. 6 to 57, except No. 23.
	8	
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Volume	10	No. 96. Patents: Parts 1, 2 and 3.
Volume	11	No. 97 to 156, except No. 99.
Volume	12	No. 99. Ordnance.
Volume	13	No. 157 to 180, except No. 160.
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WASHINGTON:
GOVERNMENT PRINTING OFFICE.
1868.



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MESSAGE

OF THE

PRESIDENT OF THE UNITED STATES

AND

ACCOMPANYING DOCUMENTS.

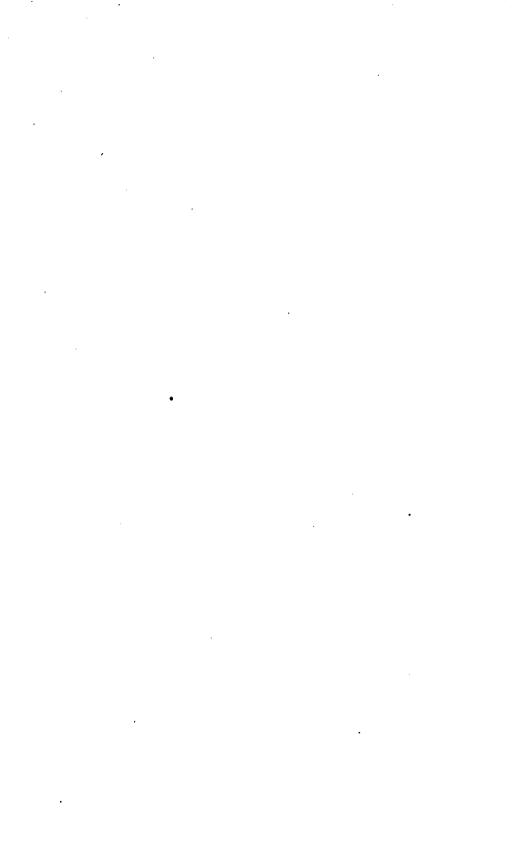
TO THE

TWO HOUSES OF CONGRESS

AT THE

COMMENCEMENT OF THE SECOND SESSION OF THE FORTIETH CONGRESS.

WASHINGTON: GOVERNMENT PRINTING OFFICE. 1867.

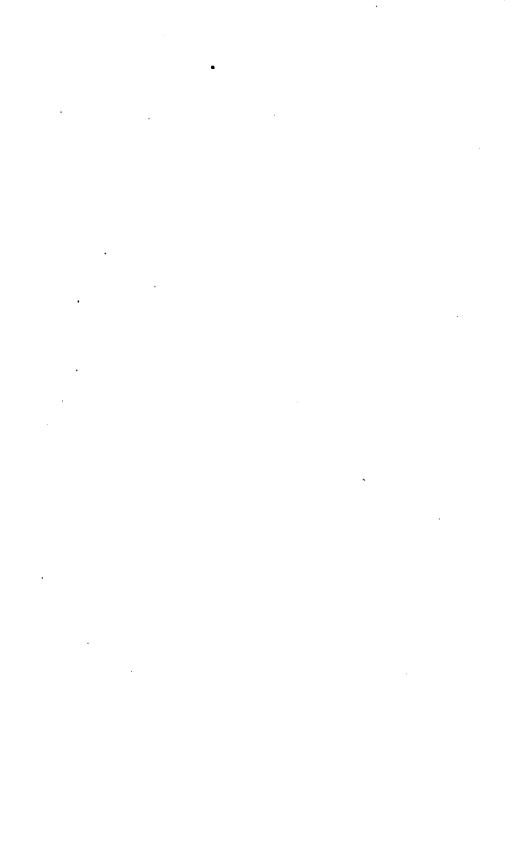


REPORT

OF

THE SECRETARY OF WAR.

PART II.



REPORT

0 F

THE CHIEF OF ENGINEERS.

HEADQUARTERS CORPS OF ENGINEERS,

Washington, October 21, 1867.

General: I have the honor to present the following report of the opera-

tions of the corps of engineers for the fiscal year ending June 30, 1867: The number of officers in the corps of engineers at the end of the year was one hundred and seven on the active list and five on the retired. During the year eleven were added to the corps by promotion of graduates of the Military Academy. There have been retired from active service, four; resigned, one; one was promoted out of the corps, and one died. On the 30th of June, 1867, the officers were distributed as follows: On duty at the headquarters of the corps...... 5 On duty with board of engineers for fortifications..... 5 On duty with engineer battalion..... 13 On duty on construction of defences and on river and harbor improvements 39 On duty on river and harbor improvements...... 12 On duty on survey of the lakes..... 5 3 On detached duty....... 17. 1 Retired off duty...... 1 Graduates Military Academy..... 11 112 The officers detached were on duty as follows: Members of the Light-house Board, Brigadier General Richard Delafield, brevet major general United States army, and Colonel Hartman Bache, brevet brigadier general United States army...... Engineer secretary to Light-house Board, Major O. M. Poe, brevet brigadier general United States army..... 1 United States commissioner for Pacific railway, Colonel James H. Simpson, brevet brigadier general United States army..... 1 Aides-de-camp of the General of the army, Major C. B. Comstock, brevet brigadier general United States army, and Major O. E. Babcock, brevet brigadier general United States army..... Chief astronomer and surveyor, northwest boundary commission, Major J. G. Parke, brevet major general United States army...... 1 On staff of Lieutenant General, commanding military division of the Missouri, Major W. E. Merrill, brevet colonel United States army 1 On staff of major general commanding department of the Gulf, Captain G. L Gillespie, brevet lieutenant colonel United States army 1

On staff of brevet major general commanding department of Dakota, Captain W. J. Twining, brevet major United States army........

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1

On staff of major general commanding department of the Missouri, Lieutenant M. R. Brown	1
On staff of brevet major general commanding department of the Platte,	•
Lieutenant R. W. Petrikin. On duty at Military Academy, Major H. M. Robert; Captain P. S. Michie,	1
brevet lieutenant colonel United States army; Captain W. H. H. Ben- yaurd, brevet major United States army, and Lieutenant M. B. Adams	4
Member of special commission to reimburse the State of Ohio for aid in suppressing rebellion, Lieutenant Colonel Lorenzo Sitgreaves	;
- ·	1

SEA-COAST AND LAKE FRONTIER DEFENCES.

Such progress as was consistent with the state of the important questions now in course of investigation respecting the increased power of artillery, and the best method of covering scarps, guns, and men from its action, and as was practicable with the reduced appropriations made by Congress for the service, has been made, during the year, upon the several maritime defensive works now in process of construction, repair, or melioration. The efforts of the year have been directed for the most part to the advancement of those portions of the works, the character of which is least affected by these questions; such as piers, quarters, store-rooms, earthworks, platforms, and magazines.

Restoration of forts injured during the war has been continued, repairs of damages caused by the elements effected, and such precautionary works exe-

cuted as future security required.

A board of engineers for fortifications, consisting of the following officers: Colonel J. G. Barnard, brevet major general United States army; Colonel George W. Cullum, brevet major general United States army; Lieutenant Colonel Z. B. Tower, brevet major general United States army; Lieutenant Colonel H. G. Wright, brevet major general United States army, and Major C. B. Reese, brevet brigadier general United States army, secretary, has been constituted, to which is intrusted the duty of making examinations, inquiries, and experiments requisite to determine the force of the largest artillery, the means of using it in the forts, and of protecting our structures, guns, and gunners against its power. As soon as the deliberations of this board are completed and its conclusions matured and verified, the labors of construction should be greatly enlarged. This will require (especially in view of the great appreciation of cost of materials and workmanship) corresponding appropriations of money to enable work to be actively resumed on the defences, and the application of materials of increased cost to be made upon them.

In several cases it will be necessary, also, to apply to Congress for grants for the commencement of new works, which are required to keep an enemy at a greater distance from important points than has heretofore been the case.

Some special boards of officers have been constituted during the year, particularly one for the Pacific coast, presided over by Lieutenant Colonel B. S. Alexander, brevet brigadier general United States army, and composed of the senior constructing officers on that coast, the distance and importance of which have required this to be done.

The estimates presented for the service of the fiscal year ending June 30, 1869, provide only for the continuation, at moderate rates, of such parts of works in progress as are not likely to be affected by the questions above indicated. The solution of these questions will require special calls from this office for further appropriations, for the purpose of pressing forward other parts of these works, and for the commencement of new defences for important and exposed positions, maritime cities, naval establishments, &c.

FORTIFICATIONS.

Fort Wayne, Detroit, Michigan, in charge of Colonel T. J. Cram, brevet major general United States army.—The breast-height wall has been completed in the main work, with the exception of a space of twenty-five feet on the north face of east bastion. In the demilune it has been nearly finished. The doors of the casemates and magazines have been made, hung, and completed, except their fastenings. The road leading from the engineer dock to main entrance, and thence to the limit of government grounds at the county road, has been finished. A covered drain has been constructed in the ditch from the road to the main sewer. Another drain is in course of construction in the ditch, extending around the north bastion to the drain first referred to. The ramps, from the parade to the terreplein, have been paved. The gun platforms of the main work have been completed. In the demilune the platforms are now generally ready for their irons. The sodding and embanking for the parapet and banquettes have been nearly finished. The new demilune magazine is well on towards completion. Operations in contemplation for the fiscal year ending June 30, 1868: To complete the demilune magazine; the platform for 15-inch gun in its salient with its breast-height and parapet thereabout; to widen the ditch of the main work; to begin the new lateral batteries; to complete the drains in the ditch; to insert the pintles and set the traverse irons of the gun platforms; to place the fastenings upon all doors belonging to casemates and magazines; to construct two additional gates for sally ports; to complete drains, gate to entrance from county road, and prepare ground for raising sod for counterscarp.

Appropriation asked for the next fiscal year, \$50,000.

Fort Porter, Black Rock, near Buffalo, New York, in charge of Major John A. Tardy and Colonel T. J. Cram, brevet major general United States army.—
Operations at this work during the year were confined to the repair of two small temporary buildings for store rooms.

No appropriation asked for the next fiscal year.

Additional works for the defeace of Buffalo, New York.—Operations here are awaiting the result of experiments and the deliberations of the board of engineers upon the application of new materials to purposes of defence.

No appropriation asked for the next fiscal year.

Fort Niagara, mouth of Niagara river, New York, in charge of Major John A. Tardy and Colonel T. J. Cram, brevet major general United States army.— But little has been done at this work during the year—operations having been suspended in October, 1866. Labor, up to that period, was applied mainly in excavating earth, removing old timber of scarp and sallyport, making and laying concrete, building dry wall, constructing casements of southeast flank, and digging wells in rear of scarp.

It is proposed, during the present year, to complete the coping of the new scarp on the land front, to put in the filling back of the new scarp, and sod after completing the parapet, and to complete the new sallyport, which was left

in an untinished condition.

Appropriation asked for the next fiscal year, \$50,000.

Fort Ontario, Oswego, New York, in charge of Lieutenant Colonel C. E. Blan, brevet colonel United States army.—The principal operation of the year has been the raising of the scarp wall of the laud or entrance front to its final height along the curtain, flanks, and one-half of each face. The masonry of the gateway, postern arch, and casemates of left flank, including embrasures, has been completed. Part of the scarp of right face of bastion E has also been raised three feet on an average, to reference (13'). The stone used is quarried and cut on the public land. During the year ending June 30, 1868, it is proposed to complete the scarp wall, and to continue the construction of the flanking

arrangements. This work is now in such a condition that it is very desirable that the construction should progress continuously to completion.

Appropriation asked for the next fiscal year, \$60,000.

Fort Montgomery, outlet of Lake Champlain, New York, in charge of Major C. B. Reese, brevet brigadicr general United States army, and Lieutenant Colonel C. E. Blunt, brevet colonel United States army.—At this work the principal operations have been the following: Completion of seventeen barbette gun platforms; laying coping of parapet wall on curtains one, two, and four, and bastions A and B, (thus completing the scarp wall of the fort;) raising staircase of bastion C from foundation to level of gun casemates, (23.58';) laying flagging in lower story of bastion C, and concrete foundation for lower floor of bastion E; turning communication arches in lower story of bastion C; advancing interior finish of officers' quarters; embanking and laying stone facing on coverface; driving piles on south end of wet ditch with a view to a modification of the counterscarp. During the present year it is proposed to complete the staircase in bastion C, the parade wall of curtain three, and the barbette magazines on the land front, and to continue the interior finish of the officers' quarters, &c. The fort is now ready for a considerable portion of its armament.

Appropriation asked for the next fiscal year, \$60,000.

Fort Knox, narrows of Penobscot river, Maine, in charge of Major Godfrey Weitzel, brevet major general United States army, and Major Thomas Lincoln Casey, brevet colonel United States army.—During the past year the north covered way, northeast demi-bastion, and defensive gallery, together with the closure wall and single caponniere of the north ditch, have been completed. The south and west glacis were brought to grade for a distance of some forty feet from their crests. Some progress was made with store-rooms in the parade of the main work, and a large amount of the finish of the quarters in the west front put in; two 15-inch platforms were completed in batteries A and B, and seven platforms for front pintle guns in the north covered way. During the present year it is proposed to complete the store-rooms of the parade, except their mastic coverings, the banquettes of the north counterscarp gallery, the magazine of the northeast demi-bastion, and nine positions for guns requiring only their traverse irons; the postern doors of the main and out-works will also be hung.

Appropriation asked for the next fiscal year, \$50,000.

Fort Popham, Kennebec river, Maine, in charge of Major Godfrey Weitzel, brevet major general United States army, and Major Thomas Lincoln Casey, brevet colonel United States army.—During the past year the scarps of the gun fronts and bastions have been brought practically to the level of the cordon line; the second tier of casemate arches all along these fronts has been completed, with its accompanying parade wall, piers, and stairway towers, and the flooring of the second tier of casemates completed far enough to receive the traverse irons of the second tier of gun platforms. During the remainder of the present working season the roof surfaces will be covered with asphaltum over eleven arches of the gun fronts; the foundation of three of the magazine traverses of the barbette will be laid, and the remainder of the work covered with boards to protect it during the coming winter.

Appropriation asked for the next fiscal year, \$75,000.

Fort Gorges, on Hog Island ledge, Portland, Maine, in charge of Lieutenant Colonel George Thom, brevet brigadier general United States army.— The operations on this fort during the past year have been applied to completing the roof surfaces of the gorge, and filling the same with earth up to the level of the terreplein, in reference (47'); towards lining and finishing the quarters, which are partially floored, furred, and lathed, and have had all the window frames put in, and nearly all the sashes made, glazed, and fitted; to building four privy vaults and cutting outlets for same through the gorge scarp, and

completing the modifications of the drainage; to re-enforcing with granite arches the scarp walls of the gorge magazines, and lining the same; to building the stairway towers in the angles of fronts one, two, four, and five, the foundations of which have been laid, and the superstructure of one of them one-third laid; and to building foundation for bomb-proof traverse on front five; to the preparation of cut stone for the stairway towers, bomb-proof traverses, centre pintlegun platforms, re-enforcing magazines and other parts of the work. The gun casemates and embrasures have been all completed, in all their details, and are ready for their armament.

Appropriation asked for the next fiscal year, \$25,000.

New Fort Preble, Portland harbor, Maine, in charge of Lieutenant Colonel B. S. Alexander, brevet brigadier general United States army, Lieutenant Colonel George Thom, brevet brigadier general United States army, and Major Thomas Lincoln Casey, brevet colonel United States army.—During the past year all the grillage and capping for the foundations of the scarps was completed, and the foundations of all the scarps, excepting the north flank, brought to about reference (12'). Sufficient work was also done upon the superstructure of the scarps to complete eight more embrasures, and to bring to the level of the soles eight other embrasures. The rock excavation for the scarps of the old work was about one-half completed, and the masonry of fronts B and C commenced. During the present year it is proposed to complete all the additions to, and modifications of, the older portions of this fortification.

Appropriation asked for the next fiscal year, \$25,000.

Fort Scammell, Portland harbor, Maine, in charge of Lieutenant Colonel B. S. Alexander, brevet brigadier general United States army, and Major Thomas Lincoln Casey, brevet colonel United States army.—During the past year five embrasures were built in the scarps of the second tier of the west bastion, while in the east bastion four casemates for guns and one for flank howitzers were completed, and the two magazines in the gorge of this work nearly finished. During the coming year it is proposed to complete the four magazines in the gorges of the east and west bastions, and the magazine traverses A, B, and C in the main work.

Appropriation asked for the next fiscal year, \$60,000.

Fort Constitution, Portsmouth harbor, N. H., in charge of Lieutenant Colonel Z. B. Tower, brevet major general United States army, and Lieutenant Colonel J. G. Foster, brevet major general United States army.—The work performed during the year comprised the laying of one course of the scarp wall of the north front, the building of the scarp of the northeast front to the reference of (15'), the construction of two embrasures and commencement and almost completion of two others on that front, the laying of the flagging between four main piers upon the southeast front, and the construction of two embrasures on the south front.

No appropriation asked for the next fiscal year.

Fort McClary, Portsmouth harbor, Kittery Point, Me., in charge of Lieutenant Colonel Z. B. Tower, brevet major general United States army, and Lieutenant Colonel J. G. Foster, brevet major general United States army.— The work executed comprises the excavation for ditch, by blasting out the rock on the north and west fronts, which has been nearly completed; the preparations for laying the first courses of the scarp wall upon those fronts; the taking down of a portion of the scarp of the southwest front, and the commencement of its reconstruction.

It is proposed during the present year to complete the rock excavation of the ditch on the north and west fronts, to build the entire scarp wall of those fronts, including the two flanking caponnieres, with their entrances and the adjacent magazines, to complete the alteration of the west end of the scarp on the south

front, and to continue this wall to form the revetment of the foot of the ditch of the west front.

Appropriation asked for the next fiscal year, \$100,000.

Fort Warren, Boston harbor, Mass., in charge of Colonel H. W. Benham, brevet major general United States army.—The small amount of funds available was expended in completing fittings of casemates and relaying the traverse circles with longer radii on the parapet front number one. The fittings of the officers' quarters were arranged and a hospital completely prepared, the cutting of the main gateway stones executed, the ravelin magazine was embanked and sodded, and other parts of the slopes and embankments repaired and sodded,

It is expected that with the funds available for this year the main gateway will be completed; the two traverse magazines of the coverface, and the main one upon the ravelin, and one or two on the front number five, in rear of the ravelin, will be completed.

Appropriation asked for the next fiscal year, \$90,000.

Fort Winthrop and batteries, Boston harbor, Mass., in charge of Colonel H. W. Benham, brevet major general United States army.—During the year the earth covering and sodding of the west magazine of south battery was completed, the covering and sodding of the other magazines; and the parapet of the south battery was repaired and grassed, as well as those of the east and northwest batteries; the broken ground of the bluff below the south battery was regulated and dressed with mould and grass seed. The northeast and northwest bastions of the earth-work to surround the tower were commenced, the tunnel covered way to south battery was excavated, the concreted foundation of its walls laid and those walls commenced.

It is expected that the arched covered way to connect the ditch of the tower with the south battery will be completed and covered over this autumn, and that a great portion of the earthen embankment of the glacis at the tower, with part of its parapets, will, also, be in position in season to secure the consolidating action of the winter season upon it. On the opening of the next season this earth-work will be carried toward completion as far as the funds available will permit.

Appropriation asked for the next fiscal year, \$100,000.

Fort Independence, Boston harbor, Mass., in charge of Colonel H. W. Benham, brevet major general United States army.—No work was executed during the fiscal year for want of funds, but it is expected that the work of building the traverse magazines, and those in the northwest and southwest exterior batteries, with the modifications proposed for the parapet of the latter, with the double traverse magazine on the northeast bastion B will be completed during the present fiscal year. A sea wall, to connect with the present one in rear of the southeast exterior battery, is necessary to protect the shore at that point.

Appropriation asked for the next fiscal year, \$50,000.

Permanent forts at Provincetown harbor, Mass.—Commencement of operations here are waiting for the preparation of plans for the works.

No appropriation asked for the next fiscal year.

Fort at Clark's Point, New Bedford harbor, Mass., in charge of Major George H. Mendell, brevet colonel United States army, and Captain Jared A. Smith, brevet major United States army.—Three casemates have been made ready for guns, three casemates for quarters made ready for interior finish. Masonry of five magazines completed and three magazines made ready for powder. The scarp and parade walls, including stair towers, chimneys and flues, nearly completed. The arrangements for guns in barbette are far advanced. It is proposed during the present fiscal year to complete as nearly as possible the arrangements for guns "en barbette," and it is expected that not

less than three front pintle platforms, and two with centre pintles, will be ready for guns, and that the foundations for all the barbette guns will be ready to receive the platforms, and all the stone will be cut. The scarp and parade walls will be completed, and the breast height wall, parapet and terreplein well advanced. All the brick furring of quarters will be completed, and it is expected considerable progress will be made in construction of ditch and counter-scarp.

Appropriation asked for the next fiscal year, \$50,000.

Fort Adams, Newport harbor, R. I., in charge of Major D. C. Houston, brecet colonel United States army.—The work at this fort, on the main work. has consisted in relaying the traverse circles in the casemates of the west front and repairs of the officers' quarters, substituting cast iron hoppers for the open sinks in the southeast bastion for the use of the soldiers, repairs of brick terreplein, repairs of parapet, and various other repairs essential to the preservation of the work. The alteration of the coverface has been completed. Four additional platforms have been laid, two for fifteen-inch guns and two for ten-inch guns; also, two service magazines have been constructed. Work on the exterior batteries, to adapt them to an armament of fifteen-inch and heavy rifled guns, was commenced in September last. The work proposed for the year ending June 30, 1868, so far as can be foreseen at present, will consist in the prosecution of the work on the exterior batteries, repairs of permanent wharf, building a guard-house, repairs of brick embrasures of the fort, alterations of present magazines and construction of service magazines, covering the arches of the main work west front to prevent leakage, &c.

No appropriation asked for the next fiscal year.

Defences on Dutch island, western entrance, Narragansett bay, R. I., in charge of Major D. C. Houston, brevet colonel United States army.—The work at this point during the year, which was not commenced until the latter part of March, 1867, has consisted mainly in altering an earthen battery built during the war, and known as the "upper battery," to adapt it to receive an armament of heavy guns, centre pintle carriages. Since that time the old platforms have been removed, the embrasures filled up, and six pintle blocks set; concrete laid for the foundations of the gun platforms and magazines. In addition, repairs have been made to the various buildings and wharf necessary to their preservation, and roads constructed and repaired. The operations for the year ending June 30, 1868, will, it is expected, consist in completing the alterations of the "upper battery," building a permanent wharf, moving and altering buildings for office and overseer's quarters, making roads and preparing site for work on the summit of the island, and prosecuting the work under the plans already prepared. The "lower battery" has been provided with two service magazines.

No appropriation asked for the next fiscal year.

Fort Trumbull, New London harbor, Conn., in charge of Captain Samuel M. Mansfield, brevet lieutenant colonel United States army, and Major D. C. Houston, brevet colonel United States army.—The work at this fort during the year has consisted only in a few repairs necessary to its preservation in good order.

No appropriation asked for the next fiscal year.

Fort Hale, New Haven harbor, Conn., in charge of Captain Samuel M. Mansfield, brevet lieutenant colonel United States army, and Major D. C. Houston, brevet colonel United States Army.—In February of this year the work was completed, the public property, tools, &c., sold at auction, and the fort placed in charge of a fort keeper. The magazines of this work are in good condition.

No appropriation asked for the next fiscal year.

Fort Schuyler, eastern entrance to New York harbor, in charge of Major H. L. Abbot, brevet brigadier general United States army.—The operations during

the past year have been confined to completing two service magazines, beginning and partly constructing two other service magazines, preparing to lay two fifteen-inch gun platforms, with necessary removal of old platforms, a stone parapet and breast height wall, all upon the coverface, and to making necessary repairs upon the main work and engineer buildings. The operations proposed for the fiscal year ending 30th June, 1868, are the following: completing two magazines and two fifteen-inch gun platforms, &c., and repairs to wharf, which may be done with the balance now available; constructing magazine in place of arms, and extending sea wall.

Appropriation asked for the next fiscal year, \$45,000.

Fort at Willett's Point, eastern entrance to New York harbor, in charge of Licutenant Colonel James C. Duane, brevet brigadier general United States army.—Scarp walls have been built to within two courses of their tull height; piers have been built to spring lines of casemate arches, second tier; filling room in channel bastion fluished; magazine in channel bastion nearly finished; casemate floors, second tier, finished, excepting four; northwest drain gallery finished; northeast drain gallery commenced; new guard house nearly finished.

Appropriation asked for the next fiscal year, \$100,000.

Fort Columbus, Governor's island, New York harbor, in charge of Major Necolas Bowen, brevet colonel United States army.—The operations of the year at this work consisted, in part, in flagging the walks and curbs of the parade, and the area of the magazine, removing old cobble stone covering of postern and ramps and replacing it with the Belgian pavement, cleaning and pointing, where necessary, the masonry of the main gateway, postern, breast heights, magazines and sortie passages, cleaning and renewing drains and gutters, grading and repairing roads and walks on the island and improving their drainage, repairing drawbridge, and constructing a vault cellar, grassing bare spots on glacis and other slopes, renewing fences and removing hot-shot furnaces.

It is proposed during the present fiscal year to alter the slope of the cunettes and renovate the system of drainage for the work, to continue unfinished repairs, point the scarp walls and other masonry, repair the coping and top of main

gateway, and effect such other repairs as may be necessary.

No appropriation asked for the next fiscal year.

Casile Williams, Governor's island, New York harbor, in charge of Major Nicolas Bowen, brevet colonel United States army.—The tower steps have been finished, the roof of one tower has been entirely renewed and the other repaired and painted. All the masonry has been pointed. The three galleries of communication of the casemates and the one along the gorge wall have been almost entirely renewed, painted, and braced with iron. The doors of the magazine, and the gates, the window sashes, leaders and gutters, have been repaired and painted, the mastic covering of the barbette tier has been repaired and the leakage through it stopped. The shot furnace removed.

The present fiscal year it is proposed to make such repairs as may be required

for the preservation of the work.

No appropriation asked for the next fiscal year.

South Battery, Governor's island, New York harbor, in charge of Major Nicolas Bowen, brevet colonel United States army.—Slight repairs have been made to the scarp walls. Such other repairs as may be required for the preservation of the work will be made this year.

No appropriation asked for the next fiscal year.

Fort Wood, Bedioe's island, New York harbor, in charge of Major Nicolas Bowen, brevet colonel United States army.—During the year the drains have been cleaned and repaired. The appearance of cholera, and a resulting quarantine of the island, interrupted further work. For the operations of the present year it is proposed to point the masonry, paint the two magazines, improve the system of drainage, repair the drawbridge and gates, sow grass seed where

required, repave parade and posterns, and make such other repairs as a proper preservation of the work may call for.

No appropriation asked for the next fiscal year.

New Battery, near Fort Hamilton, New York harbor, in charge of Lieutenant Colonel John Newton, brevet major general United States army.—Operations during the year.—Construction of the south magazine, taking up platforms to make room for traverse magazines, and commencing the same. At seven platforms, revetment and platform flags have been set, and portions of breast-height wall not heretofore finished have been laid. At platform No. 22 the traverse stones have been rebedded and lowered one foot; five magazine doors made; embanking parapet of battery, sodding, grading and soiling slopes of parapet.

Present condition of work.—North magazine ready for service; south magazine arches turned and mastic covering applied, not entirely complete; three traverse magazines complete, one in process of being lengthened; five traverse magazines commenced and in process of construction. The battery is essentially ready for

service.

No appropriation asked for the next fiscal year.

Fort Hamilton, New York harbor, in charge of Lieutenant Colonel John Newton, brevet major general United States army.—Operations during the past year.—A trifling amount has been expended in cutting down some embrasures of the north front, to allow egress from the casemates, and in removing shot furnaces of the old pattern from the interior of the work.

Condition of the work.—The work needs occasional and slight repairs in

pointing, &c.

No appropriation asked for the next fiscal year.

Fort Wadsworth, New York harbor, in charge of Colonel J. G. Barnard, brevet major general United States army, and Mojor Q. A. Gillmore, brevet major general United States army.—On the interior of this work the guardhouse and the two cisterns adjacent thereto have been finished, and some necessary pointing of masonry done. On the exterior the sand and earth, which has for years been accumulating in the ditch from the washings of the slopes and other unfinished earthwork near the fort, has been excavated to within a few inches of the proper depth, and two sluice-ways from the ditch to tide-water completed.

No appropriation asked for the next fiscal year.

Fort on site of Fort Tompkins, New York harbor, in charge of Colonel J. G. Barnard, brevet major general United States army, and Major Q. A. Gillmore, brevet major general United States army.—During the year five additional casemates (second tier) have been furred, twenty-one casemates (eleven first tier and teu second tier) floored, and nine casemates (six in the south flank and three in the southwest face) fitted up for soldiers' quarters. All the doorway openings in the fort have been provided with suitable wrought-iron doors, so that very little except glazing remains to be done to render all the casemates in the four land fronts ready for occupancy. On the exterior the road leading down to North Cliff battery has been cut and formed, and the slopes on either side brought into shape and partially finished. Extensive damages by rain-storms to the main slopes on the channel front have likewise been repaired, a twelve-inch drain from south soldiers' latrine to foot of slope put in place, and the battery southeast of Fort Tompkins finished and put in readiness for its guns. The platforms for the guns on the four land fronts are finished. The operations of the current fiscal year will be limited to work upon the latrines on the slope in front of the work, to a moderate extent, and providing for the overflow of the cisterns within the work.

Appropriation asked for the next fiscal year, \$25,000.

Battery Hudson, New York harbor, in charge of Colonel J. G. Barnard, brevet major general United States army, and Major Q. A. Gillmore, brevet

major general United States army.—Nothing has been done at this battery during the year just closed beyond building a catch basin and trap, cleaning out the main drain thence near the light-house, and keeping the work in repair. The only work contemplated during the present fiscal year comprises the removal of the old shot furnaces from its terreplein, and some necessary grading and drainage.

No appropriation asked for the next fiscal year.

New casemated battery on Staten island, New York harbor, in charge of Colonel J. G. Barnard, brevet major general United States army, and Major Q. A. Gillmore, brevet major general United States army.—The construction of the work proper has not been begun, the preliminary preparations of previous years having been limited to the completion of a wharf for landing materials, the construction of a coffer-dam on the channel side of the site, to sundry borings in order to determine the nature and character of the substrata, to the purchase of stone, and cutting of rock-face ashlar, the preparation of stone for concrete to be used in foundations and piers of the work. The cutting of stone has been moderately continued through the year just closed.

No appropriation asked for the next fiscal year.

North Cliff battery, Staten island, New York harbor, in charge of Colonel J. G. Barnard, brevet major general United States army, and Major Q. A. Gillmore, brevet major general United States army.—At this work the operations in progress July 1, 1866, looked to the prompt completion of the battery and the mounting of its armament. Five platforms and the breast-height wall in front of them have been completed. The earth covering of the north traverse magazine has been completed and sodded, and the traverse itself finished. The settlement of the new earth-work at the south end of the battery has been made up by heavy filling, and the parapet roughly formed throughout the entire length of the battery. Excavations for the large magazine in the north end of the battery were commenced in April, and work thereon continued through the year. Two catch basins, with suitable drains through the parapet, were made, and a portion of the terreplein gutters paved. A sea-wall of dry rubble, six to seven feet in height, was built along the north half front, to prevent the washing of the exterior slope. The operations of the current year will be to construct the principal magazine in the north end of the battery, to form and finish the main slopes in rear thereof, and to complete the filling and prepare the foundations of the magazine and bomb-proof in the parados at the south end of the battery.

No appropriation asked for the next fiscal year.

South Cliff battery, Staten island, New York harbor, in charge of Colonel J. G. Barnard, brevet major general United States army, and Major Q. A. Gillmore, brevet major general United States army.—The completion of the main slopes in rear of this battery at the close of the last year left only the construction of the new magazine, which was in hand at that time, and the finishing up of the slope at the north end of the battery, to be carried on during the year just closed. The magazine was completed, and about one-third of the necessary earth covering placed over it, by the close of the last working season. This earth-work was resumed last spring, the slope at north end of battery formed and sodded, and the main slope repaired and top-dressed. The masonry work of the year was applied exclusively to the magazine. The operations of the present year will be limited to the interior fitting up of the magazine, the care and preservation of the slopes, and providing some necessary drainage.

No appropriation asked for the next fiscal year.

Fort at Sandy Hook, New Jersey, in charge of Lieutenant Colonel John Newton, brevet major general United States army.—Raising scarp of northeast bastion; north curtain casemate arches and communication arches in northeast bastion and north curtain constructed; piers of northwest front constructed;

additions made to walls of service magazines, &c., in northwest and southeast bastions. Upon the land front, taking up old work; excavating for foundations of scarp and piers of the new trace; laying foundations and building scarp wall and piers; embanking with sand to level of scarp.

It is proposed during the present season to complete the foundations of the scarp and piers of the land front, not including the caponniere, and raise the

scarp uniformly four courses.

Jettees 2, 3, and 4 were lengthened respectively one hundred and eight, twenty-three, and eighty-nine feet. A wing thirty-three feet long and another twenty-seven feet long were added respectively to jettees 3 and 2, to protect these jettees from the action of eddy currents and northerly storms. A new jettee, No. 5, between 3 and 4, was built out sixty-five feet, and connected by a plank fence fifty-three feet long with the highest water mark.

A new jettee is necessary between Nos. 1 and 2, to protect the bulkhead constructed across the hollows between sand ridges from being undermined, but

will not be undertaken this season unless indispensably necessary.

Appropriation asked for the next fiscal year, \$150,000.

Fort Mifflin, Delaware river, Penn., in charge of Lieutenant Colonel C. S. Siewart.—The new main magazine has been completed; the old traverse stones have been removed, and new foundations and broad low traverse stones put in place for six barbette platforms; new traverse stones have been cut for barbette platforms to be modified; four thousand four hundred cubic yards of deposit in main ditch have been removed, wharf and river embankments repaired, &c. Similar operations of repair and melioration are contemplated for the next year.

Appropriation asked for the next fiscal year, \$30,000.

Fort Delaware, Delaware river, Delaware, in charge of Licutenant Colonel C. S. Stroart.—The southern dock wall has been extended one hundred and fifty-eight feet. The coping of parade wall of all the fronts has been pointed. The exterior slopes of parapets have been repaired and resodded throughout. The glacis has been regraded on all the fronts; the pond filled in; the exterior embankment of the island repaired and brought to its proper level for a length of two thousand three hundred and eighteen feet. Extensive repairs have been made to quarters, &c.

Appropriation asked for the next fiscal year, \$60,000.

Battery opposite Fort Delaware, in charge of Lieutenant Colonel C. S. Stewart.—The platforms (temporary) of the fifteen-inch guns have been slightly repaired, and the traverse circles made level, so as to allow the guns to be rapidly manœuvred. Slight repairs, as needed, have been made to slopes and scarp.

No appropriation asked for the next fiscal year.

New fort opposite Fort Delaware.—Commencement of operations is deferred until the project for the work is revised.

No appropriation asked for the next fiscal year.

New fort near Delaware breakwater.—This work will be begun as soon as the best methods of applying iron to such constructions are determined on.

No appropriation asked for the next fiscal year.

Fort McHenry, Baltimore harbor, Maryland, in charge of Major William P. Craighill, brevet lieutenant colonel United States army.—The labor of the year has been applied entirely to the water battery. Re-enforcing of pintle centres for four front pintle platforms was completed, as also the substitution of the low for the high traverse circles for the same guns, which made the battery ready for its armament. The magazines and bomb-proofs and other traverses have been essentially completed. The thickening of the parapet and the rectification of its slopes, as well as the regrading of the terreplein, have been carried nearly to completion. The repairs of the water battery will be completed during the year 1867. Nothing is necessary for the main work.

No appropriation asked for the next fiscal year.

Fort Carroll, Baltimore harbor, Maryland, in charge of Major William P. Craighill, brevet licutenant colonel United States army.—Operations have been very limited in extent, being confined almost entirely to keeping the fort in proper police, and protecting it in its unfinished state from the weather. A portion of the flagging for the floors of second tier has been received. Except on front 6 and the adjacent parts of 1 and 5, the scarp wall is at the height of the top of the twenty-third course of masonry, the floor arches of the second tier of casemates completed, and the piers built somewhat above the springing lines of floor arches of third tier of casemates, as originally designed. Front 6, on account of settlement, has been kept down to a level a little below the floors of first tier of casemates.

No appropriation asked for the next fiscal year.

Obstructions of the Potomac.—The preparation of matériel for this important object is in hand.

No appropriation asked for the next fiscal year.

Fort Washington, Maryland, in charge of Major John A. Tardy.—The modifications of this work should not be undertaken until the best manner of using iron in our defences is determined on.

No appropriation asked for the next fiscal year.

Fort Monroe, Old Point Comfort, Virginia, in charge of Colonel Henry Brewerton, brevet brigadier general United States army.—Operations for the year have been chiefly confined to necessary repairs. The traverse stones of barbette platform No. 93 have been taken up, the concrete foundation cut down, and the stones relaid one foot below the original level. Barbette platform No. 121 has been removed and replaced by a new one. The bridge at the north postern (front 6) has been repaired, and a portion of the stone pavement at the interior entrance of postern of front 4 taken up and relaid, so as to form a more gentle ascent. At the foot of this slope a drain has been constructed to carry off the surface water. The terreplein and ramps of the work have been regraded, gravelled, and thoroughly repaired.

Water Battery.—The brick masonry of the embrasures of six casemates has been repaired; the traverse circles in casemate 33 have been removed. Eight-

een projectile platforms have been constructed for this battery.

It is proposed this year to widen the terreplein and increase the thickness of the parapets of the channel fronts of the main work, lay down new barbette platforms, and construct traverses and service magazines.

Appropriation asked for the next fiscal year, \$100,000.

Fort Wool, Hampton Roads, Virginia, in charge of Colonel Henry Brewerton, brevet brigadier general United States army.—Operations have been very limited the past year, and have been confined for more than half the working season to preparations for building the magazines of the first tier at the capital at the east and west ends. The tenth course of scarp has been nearly completed from casemate 31 to 43 inclusive. The tablet course of the second tier of casemates has been laid at pier 40, and the second course of pier stones adjoining the scarp has been laid at the piers from 31 to 38. Portions of the wall in rear of casemates have been removed, preparatory to the construction of the magazines of the first tier. The sites of these magazines at the capital and at the east and west ends have been loaded with additional stones, the extension of these structures requiring this addition. The magazines of the first tier having been commenced, will be pushed forward to completion as rapidly as possible. It is proposed to expend the amount available in the construction of the magazines of the first tier at the capital and at the east and west ends, and on such other portions of the work as may hereafter be decided upon.

No appropriation asked for the next fiscal year.

Fort Clinch, Amelia island, Florida, in charge of Captain J. W. Barlow,

brevet lieutenant colonel United States army.—The operations of the season have been mainly directed towards the construction of barbette gun platforms. those upon the sea fronts receiving the earliest attention. Four platforms, two upon the northeast and northwest curtains, respectively, have been completed, and are ready for their armament. Four other platforms upon the northeast and six upon the northwest curtains are also in readiness for their carriages, with the exception of setting traverse irons. These, with the centre pintle platform at the east angle, which is also completed, place the fort in a condition to receive the larger portion of its barbette armament. The foundations of breast height wall upon the above three fronts have been laid, and the concrete filling of the parapet, in conjunction therewith, raised to an equal height. The terreplein has been formed, graded, and planted with Bermuda grass. The exterior wall of parados has been built from the northwest angle along the northwest and part of the northeast curtains to the height of the springing line of the arch. This affords a permanent revetment for the rampart, and provides against endangering the security of the platforms by further excavation when the conatruction of the parados shall be recommenced. Considerable progress has been made upon other portions of the work. All the masonry and metal parts of the drawbridge below the reference of the bridge have been put in position, and the other material necessary for its completion purchased. About one-half of main sewer has been constructed. A large portion of the labor incident to the roofing of soldiers' barracks and store-house accomplished. It is deemed desirable to complete the remaining barbette platforms and gun parapets, to finish the superior slopes of bastionettes, construct the officers' quarters, and continue filling the glacis during the next season.

Appropriation asked for the next fiscal year, \$50,000.

Fort Taylor, Key West, Florida, in charge of Major Walter McFarland.—The operations of the year have consisted in the removal of a part of the wreck left by the hurricane of 1865, the completion of the permanent sea-wall of the southern end of the coverface up to the high-water course, the construction of the outer breakwater at the northern end of the coverface and the commencement of its permanent sea-wall, the filling in of one-half of the enclosure at the southern end, the partial construction of the glacis of tower one, and the completion of the railroad to the coverface. The operations contemplated for the present fiscal year are the completion of the enclosure and filling of the coverface, and the continuation of the embankment of the glacis of tower No. 1.

Appropriation asked for the next fiscal year, \$150,000.

Fort Jefferson, Garden Key, Tortugas, Florida, in charge of Mojor Walter McFarland.—The remainder of the appropriation for the past year was applied exclusively towards the completion of the officers' quarters; nearly the entire range was covered with galvanized iron, the iron stairways set up, and one section of the quarters completed except as to its hallways and piazzas. Sixteen platforms (barbette) were furnished with the new pattern pintle.

Appropriation asked for the next fiscal year, \$100,000.

New fort at Tortugas, Florida.—This work will be commenced as soon as the best combination of materials for its uncovered scarps is determined on.

No appropriation asked for the next fiscal year.

Fort Pickens, Pensacola harbor. Florida, in charge of Major M. D. McAlester, brevet brigadier general United States army.—The general condition of this work is good. The magazines and gun platforms generally are in good condition. No operations have been carried on during the past year.

It is contemplated to construct at this work during the current fiscal year three barbette platforms for fifteen-inch guns, and to execute such repairs as

may become necessary.

No appropriation asked for the next fiscal year.

Fort Morgan, eastern entrance to Mobile bay, Alabama, in charge of Major

M. D. Mc Alester, brevet brigadier general United States army.—This work is generally in good condition. As portions of the walls of the old defensive barrack in the parade, destroyed by the bombardment of August, 1864, are still standing, it is proposed to let them remain until the material composing them may be required for use. The counterscarp wall, southwest lunette, scarp and parapet of the land front, main gateway, parade walls and terrepleins of all the fronts, and leaden roof surfaces of casemates, have all been thoroughly repaired, and an extensive temporary breakwater to protect the site towards the west from encroachment of the sea has been constructed during the past year. No operations beyond ordinary repairs are contemplated during the current year.

No appropriation asked for the next fiscal year.

Fort on Ship island, coast of Mississippi, in charge of Major M. D. McAlester, brevet brigadier general United States army.—General condition is good, the work having during the year been completed as far as contemplated, and made ready for its armament. The magazines are in good order.

No appropriation asked for the next fiscal year.

Fort Jackson, Mississippi river, Louisiana, in charge of Major M. D. McAlester, brevet brigadier general United States army.—The general condition of the work is good. The magazines and platforms are, for the most part, serviceable. During the year twenty-two new barbette platforms for heavy guns, with the necessary modifications of breast height walls, parapets, terreplein, &c., have been completed, the casemates have been prepared for the reception of heavy guns, a new oven has been constructed, the new levee on the government reservation repaired, sixteen shot platforms finished, and two platforms for thirteeninch mortars commenced. During the present year it is contemplated to erect a depot magazine capable of keeping powder perfectly secure against the great and sudden rises of water which occur at long intervals, due to freshets in the river, combined with overflows from the Gulf, caused by the violent storms incident to this region.

Appropriation asked for the next fiscal year, \$75,000.

Fort St. Philip, Mississippi river, Louisiana, in charge of Major M. D. McAlester, brevet brigadier general United States army.—The general condition of this work is good. The gun platforms (excepting a few old ones) are in good order. The magazine in the lower battery is in excellent condition; the other two serviceable except during extraordinary storms. During the past year the magazine in the lower battery, fifteen new gun platforms, two mortar platforms, with the necessary modifications of parapets, breast-height walls, and terrepleins have been completed, and the bridge across the ditch and two gun platforms repaired. The principal operations contemplated for the current year are remodelling and repairing the magazines of the main work and upper battery.

Appropriation asked for the next fiscal year, \$30,000.

Fort at Fort Point, entrance of San Francisco harbor, California, in charge of Major George H. Elliot.—The cleaning and painting of the embrasures of the main work have been completed. The main drain has been finished. The western sca-wall has been carried to the reference (14'.) The excavation for western casemated battery has been carried into the hill far enough to receive retaining wall of covered way. The excavation for the sea-wall along the eastern shore has been commenced. A heavy coffer-dam, necessary to the construction of the eastern fifteen-inch gun barbette battery and its sea-wall, has been carried three hundred and fifty feet along the shore. A railroad has been laid from the wharf head to the main work to transport materials to the sites of the new batteries. The workshops and storehouses have been removed to new sites to give place to the new barbette battery. The wharf has been extended and repaired. The roads have been repaired and protected from the encroachments of the sea. A large amount of granite for the eastern sea-wall has been obtained. The eastern fifteen-inch gun battery and the sea-wall in its front will be constructed

during the present fiscal year; and, if possible, the casemate and barbette batteries on the western shore will be commenced.

Appropriation asked for the next fiscal year, \$200,000.

Fort at Lime Point, San Francisco harbor, in charge of Major George H. Mendell, brevet colonel United States army.—Title to this site having been acquired by the United States, instructions to commence the preliminary operations of road and wharf making, preparation of temporary buildings for workmen, quarters, shops, and storehouses have been sent forward, and the work of excavating and blasting out a site for the defences will be entered upon without delay.

Appropriation asked for the next fiscal year, \$100,000.

Fort at Alcatraz island, San Francisco harbor, California, in charge of Major George H. Elliot and Major G. H. Mendell, brevet colonel United States army.—Owing to want of means, work on Alcatraz was suspended in February, and the barrack placed in condition to lie over, under charge of a keeper. Previous to that time the masonry of the first tier of the barrack was completed, and that of the rear rooms of the second tier was more than half done, while the long piers of the casemate arches of the second tier and the parade walls were carried to the height of the springing lines. The scarp is about one foot above the crown of the arches of the second tier. No work has been done on the arches since December. A large amount of rock excavation has been made, mainly in the extension of battery Rosecrans, which is now ready for the masonry. A careful resurvey of the island is in progress.

Appropriation asked for the next fiscal year, \$50,000.

Survey at Point Lobos, California, in charge of Major George H. Elliot.— The survey of this position, as a site for defensive works, was commenced in May, 1866, and completed, including the maps and report, in April, 1867. The area included is somewhat over a square mile. All the particulars required have been obtained and are presented on the maps.

No appropriation asked for the next fiscal year.

Survey for land definces at San Francisco, in charge of Major George H. Elhot.—The winter season being rainy and unsuitable for this work in the climate of California, the survey was not commenced until the middle of March. It has been in progress since then, with the aid of a party of the Coast Survey which had already covered a part of the ground with their operations. Preliminary tracings showing the progress of the work have been received, and it is expected to be completed during the present fall.

No appropriation asked for the next fiscal year.

Military Academy.—By the law of July 13, 1866, it was enacted, "That the superintendent of the United States Military Academy may hereafter be selected, and the officers on duty at that institution detailed from any arm of the service; and the supervision and charge of the academy shall be in the War Department, under such officer or officers as the Secretary of War may assign to that duty;" and by General Order No. 54, Adjutant General's office, of July 30, 1866, the Chief of Engineers was relieved from duty as inspector of the academy, and directed to turn over all the books, records and papers relating thereto to the Adjutant General of the army.

This order has been complied with, and the Military Academy is separated from the corps of engineers. It had been a part of that corps, by law, for more

than sixty-four years.

Engineer battalion, commanded by Major Henry L. Abbot, brevet brigadier general United States army.—The strength of the five companies of engineer troops, on the 30th June, 1867, was six hundred and sixteen enlisted men and fourteen officers; one hundred and thirty-six recruits were needed to complete the organization.

By the law of July 28, 1866, section 20, the five companies were constituted a battalion of engineers, to be officered by officers detailed from the corps

of engineers.

The battalion should have a commissary sergeant and two principal musicians. Its strength is seven hundred and fifty-two men, about the same as a regiment of infantry. By the operation of the 7th section of the act of July 13, 1866, the enlisted men of the engineers are denied the allowance of the per diem paid to other soldiers when engaged in continuous labor. This discrimination should be removed; it operates to restrain superior men from enlisting in the battalion.

A small appropriation is needed to procure materials required to instruct the

men in their duties as engineer soldiers.

These several points will be made the subject of a special communication in

season for legislative action.

Three companies of these troops are stationed at Willett's Point, New York; one at the engineer depot, Jefferson Barracks, Missouri, commanded by Captain William Ludlow, brevet lieutenant colonel United States army, and one commanded by Captain Samuel M. Mansfield, brevet lieutenant colonel United States army, is en route for San Francisco, California. A detachment is at West Point, New York, for the purpose of affording instruction in practical engineering to the cadets of the Military Academy.

Engineer depots at Willett's Point, New York, and Jefferson Barracks, Missouri, in charge respectively of Major H. L. Abbot, brevet brigadier general United States army, and Captain William Ludlow, brevet lieutenant colonel United States army.—A large amount of engineer matériel is collected at these two depots. It is guarded and cared for by the companies above mentioned, and affords them the means of drilling at, and acquiring a knowledge of, their

peculiar functions.

For the construction of barracks for the officers and men at Willett's Point and Jefferson Barracks, appropriations were asked last year of twenty-five thousand dollars for the former, and twenty thousand dollars for the latter place. Congress granted these sums, but directed the use of half of each only, during the present fiscal year, withholding the remainder until further determination by them on the subject. The appropriation of these balances for the next fiscal year, viz., twelve thousand five hundred dollars for Willett's Point, and ten thousand dollars for Jefferson Barracks, is asked in the annual estimate of this office.

RIVER AND HARBOR IMPROVEMENT.

The last annual report alluded to the measures that had been taken to carry into effect the act of Congress approved June 23, 1866, making appropriations for repair, &c., of works of improvement which had been heretofore authorized, and requiring the examination and survey of other localities. The assignment of officers to the charge of these works and the progress of the operations, so far

as reports had been received, was also presented.

Subsequent thereto, in accordance with the requirements of the act referred to, "at the earliest practicable time to report to Congress the result of any survey or re-survey, with the plan adopted and the items of expenditure under said plan, and * * * of all action taken under the provisions of this act, * * make such a report at the commencement of every session of Congress until the works herein provided for shall all be completed," a special report was made to the Secretary of Waron the 26th of January, 1867, which was submitted to Congress and is found in printed Executive Document 56, House of Representatives, thirty-ninth Congress, second session. It embraces reports upon sixty-three works of improvement, both of rivers and harbors.

A special report upon the survey, examination, and improvement of the Upper Mississippi river and its tributaries, covering a length of river course of more than one thousand miles, was submitted on the 29th of January, 1867, was printed, and is found in Executive Document 58, House of Representatives,

thirty-ninth Congress, second session.

The following reports were also submitted, namely:

Upon the harbors of Pere Marquette and Pentwater, on Lake Michigan, on the 2d of February, 1867, printed Executive Document 70, House of Representatives, thirty-ninth Congress, second session.

Upon the harbor of Ontonagon, Lake Superior, on the 5th of February, 1867; printed Executive Document 80, House of Representatives, thirty-ninth Con-

gress, second session.

On the survey and improvement of the Des Moines and Rock Island rapids of the Mississippi river, on the 5th of February, 1867, printed Executive Document 79, House of Representatives, thirty-ninth Congress, second session.

On the survey and improvement at Hell Gate, New York, on the 9th of February, 1867, printed Executive Document 90, House of Representatives,

thirty-ninth Congress, second session.

On the survey and improvement of the Kennebec and Penobscot rivers, Maine, printed Executive Document 91, House of Representatives, thirty-ninth

Congress, second session.

On the survey and improvement of Plattsburg harbor, New York, on 13th of February, 1867, printed Executive Document 89, House of Representatives, thirty-ninth Congress, second session.

On the encroachments upon the harbor of New York, on the 15th of February, 1867, printed Executive Document 28, Senate, thirty-ninth Congress,

second session.

On the survey and improvement of Rock river, Illinois and Wisconsin, on the 11th of April, 1867, printed Executive Document 15, House of Representatives, fortieth Congress, first session.

On the survey and improvement of Illinois river, Illinois, on the 13th of May, 1867, printed Executive Document 16, House of Representatives, fortieth Con-

gress, first session.

In addition to the foregoing, special reports were made, but not printed, upon the survey and improvement of the following localities, in obedience to resolutions of the Senate or House of Representatives, or on the call of the committees, namely:

On the harbor at Point Sal, California, on the 1st of February, 1867.

On the practicability of steamboat navigation from Chesapeake Bay to Lake Ontario, and estimate for a survey of the route, on the 7th of February, 1867.

On the works necessary for the preservation and improvement of Boston

harbor, on the 11th of February, 1867.

On the improvement of Kalamazoo river, Michigan, on the 13th of February, 1867.

On the improvement of Willamette river, Oregon, on the 14th of February,

On the survey of Galveston harbor, Texas, on the 16th of February, 1867. On the increased depths to be given to the harbors on the lakes, to admit vessels drawing fourteen feet water, on the 19th of February, 1867.

On the changes in the channel of Potomac river, from Georgetown to Green-

leaf's Point, on the 21st of February, 1867.

On the selection of a position for a breakwater or harbor of refuge at Block

island, Rhode Island, on the 23d of February, 1867.

The information furnished in the above reports resulted in the appropriation of funds for the improvement of the harbors or rivers which formed the subject of those communications, or in the provision for surveys or examination of other localities, with a view to their improvement.

Measures were taken immediately upon the passage of the act of Congress, approved March 2, 1867, for the execution of its provisions, and the results of these measures as well as the progress of the works appropriated for in June, 1866, are embraced in these reports, so far as the information has been obtained. Special communications will be made upon the surveys in progress, but not completed, as soon as the reports are received.

The surveys are:

 For a ship canal connecting Lakes Erie and Ontario, under the charge of Brevet Colonel C. E. Blunt.

Of the Ohio river, in connection with its improvement, in charge of Mr.
 W. Milnor Roberts, United States civil engineer.

3. For a ship canal around the falls of the Ohio, in charge of Brevet Major General G. Weitzel.

4. Of the Tennessee river, in charge of Brevet Major General G. Weitzel.

5. Of the Illinois river, with a view to its improvement and to its connection by a ship canal with Lake Michigan, in charge of Lieutenant Colonel and Brevet Major General J. H. Wilson.

6. Of the Mississippi river, above Rock Island rapids, with a view to its improvement, and between St. Paul and St. Louis, with a view to the location of bridges, and above the falls of St. Anthony, &c., in charge of Brevet Major General G. K. Warren.

7. Of the harbor of Port Clinton, Ohio, in charge of Brevet Major General

8. Of the Connecticut river, between Hartford and its mouth, in charge of Brevet Colonel D. C. Houston.

9. At Block island, in the State of Rhode Island, with a view to a harbor of refuge, in charge of Brevet Colonel D. C. Houston.

10. At Reedy island and Listen Tree, in Delaware river, in charge of Lieutenant Colonel Charles S. Stewart.

11. Of the Potomac river, in the District of Columbia, in charge of Brevet Brigadier General N. Michler.

12. Of the bay and harbor of Galveston, Texas, in charge of Brevet Brigadier General M. D. McAlester.

Further legislation is very desirable to carry into effect more advantageously the provisions of the recent river and harbor bill. Serious difficulties, delays, and embarrassments attend the execution of some of the provisions of the acts of June, 1866, and March, 1867, for those objects.

It is suggested that the acts be amended so as to authorize the Secretary of War to apply the sums appropriated therein by contract or otherwise than by contract, when in his judgment it may best subserve the interests of the government to do so, and that the same provision be made for the application of any sums that may be appropriated by Congress at its approaching session.

As an illustration of the embarrassments and delays incident to the execution of the law, it is to be observed that the acts require each class of material, as timber, iron, stone, and brush, and each class of labor on such material, as carpentry, smithery, masonry, and ordinary labor, to be contracted for separately. With every effort to simplify the work and reduce the number of classes, especially those of labor, it has been found in some instances impracticable to bring each class together at proper times.

It is to be further observed that there are cases where, from the destructive effects of storms, or from wrecks in the channels, the delay in procuring material and labor for the object by means of contracts, after soliciting proposals by advertisements, leads to serious injuries to the works.

With the exception of some of the greater works of improvement, there has been but little competition manifested by bidders.

The objections to the exclusive use of the contract system in carrying on these improvements were presented in some detail in the last annual report, and particularly in a report to the Secretary of War of January 24th, 1867, on a letter from the chairman of the Committee on Commerce of the Senate.

It will be found, by reference to the reports appended hereto, that further experience does not diminish the difficulties.

The number of copies of the reports emanating from these headquarters upon subjects purely of engineering nature, and ordered to be printed, allotted by law for its use and for distribution to officers and agents, is too small for the intended purposes. It is therefore recommended that the Superintendent of Congressional Printing be directed by resolution to increase the number from one hundred copies, (the number now allowed by law,) to two hundred copies.

There is herewith submitted a summary of the operations at each of the localities where surveys and improvements are in progress, showing the progress of the work since the last annual or special report was rendered, up to the close of the fiscal year, and containing recommendations for further appropriations for

carrying into effect the improvements authorized by Congress.

The reports of the engineers in charge of the improvements, made in accordance with the following circular, and with the requirements of the law, are also appended. Attention is respectfully invited to them for full details of the work during the last fiscal year:

[Circular No. 11.]

Engineer Department, Washington, June 10, 1867.

Your annual report of progress in all works of river and harbor improvements and surveys in your charge, must be transmitted so as to reach this department by the 15th of September next, and should be accompanied by a brief or synopsis of its contents, to be embodied in the report of the Chief of Engineers.

Your attention is also invited, at the same time, to sections 2 and 3 of the act making appropriations for the repair, preservation, and completion of certain public works, &c., &c., approved June 23, 1866, and to section 2 of the act (for the same purpose) of March 2, 1867, with the view of reporting to Congress on the following points, to be embraced in your annual reports, namely:

1. Result of survey or resurvey, with plan adopted, and items of expenditure

under that plan.

2. The amount that is required for the entire and permanent completion of each work under your charge.

3. The amount that can be profitably expended upon each work during the next fiscal year.

4. The collection district in which each work is located.

- 5. At or near what port of entry, light-house, or fort, each work is located.
- 6. What amount of revenue was collected at the nearest port of entry to each work for the last fiscal year.
- 7. As far as practicable, what amount of commerce and navigation would bebenefited by the completion of each particular work.
 - 8. Abstract of proposals for each work, with names of bidders.
 - 9. Abstract of contracts for each work, with names of contractors.

10. Abstract of contracts for each class of materials or labor for each work.

Where the nature of the work is such as not to admit of permanent completion, a plan and estimate should be submitted as to the perpetual annual expenditure required to maintain the harbors and rivers in the condition contemplated in the plan of improvement; or, if the maintenance of the improved condition does not require an annual expenditure, then the probable periods at which the preservation of the works and maintenance of depth of water will require expenditures of money, and the amount of such expenditure.

A. A. HUMPHREYS, Chief of Engineers, Major General.

LAKE HARBORS.

HARBORS ON LAKES SUPERIOR AND MICHIGAN.

Officer in charge Brevet Colonel J. B. Wheeler, major corps of engineers, who has assigned the officers under his orders to the following duties:

Captain A. MacKenzie, corps of engineers, special superintendent for the harbors of Black Lake, Grand Haven, Muskegon, White River, Pentwater, Pere Marquette, Manistee, and Aux Becs Scies.

Captain D. P. Heap, corps of engineers, special superintendent for the harbors of Milwaukee, Racine, Kenosha, Chicago, Michigan City, New Buffalo, St. Joseph, and South Haven.

Lieutenant J.B. Quinn, corps of engineers, special superintendent for the harbors of Marquette, mouth of Fox river, Green Bay, Manistee, Sheboygan.

Assistant Henry Bacon, special superintendent for the harbors of Superior

City, Ontonagon, and Eagle Harbor.

Assistants W. T. Casgrain and W. H. Hearding, engaged in surveys and the preparation of maps.

I .- Lake Superior.

1. SUPERIOR CITY HARBOR, WISCONSIN.

The original plan contemplates piers composed of two rows of piles, fifteen feet apart, capped with timber, forming a crib superstructure, and filled with stone; an estimate for which was submitted in the report from this office of January 26, 1867, amounting to \$178,000.

After some delay, arising from the failure of bidders to comply with their bids,

contracts were finally entered into, and the work is now in progress.

2. ONTONAGON HARBOR, LAKE SUPERIOR.

The plan recommended for this harbor is to extend two parallel piers, composed of cribs ballasted with stone, from the mouth of the river outward into the lake, a distance of 2,160 feet for the west pier, and 2,340 for the east pier. The direction of the piers to be the same as that of the present west pier.

▲	-
The total estimated cost was	\$292,801 50
And there was appropriated in 1867	97,600 00
Amount required to complete the work	195, 201 50
Amount required for next fiscal year	97, 600 00
Contracts were entered into in the latter part of June, 1867.	

(See Appendix A 2.)

3. BAGLE HARBOR, LAKE SUPERIOR.

The plan proposed is to remove the rock that forms a bar in the entrance, and to build breakwaters from the eastern and western points, in order to narrow the entrance to the harbor.

There were 1,803 cubic yards of trap rock to be removed in order to attain the depth of 14 feet of water, and 1,290 running feet of pier work to be built in order to improve the harbor.

Advertisements were inserted in the newspapers calling for proposals to do

the work.

The proposals were opened on the 20th of June, but the price in the lowest

bid for removing the rock being unreasonably high, all bids were rejected, and proposals were again invited, and this time for removing the rock only. These proposals were opened on the 31st of July, and the contract was awarded, the work to be commenced without delay and prosecuted during the winter.

The estimate for doing this was	\$ 191, 189 8 6
There was appropriated	65,000 00
Amount required to complete the work	126, 189 86
Amount required for next fiscal year	66, 000 00

4. MARQUETTE HARBOR, LAKE SUPERIOR.

The contractors have commenced work upon the breakwater, and fair progress may be expected during this season.

The estimated cost of the improvement is	\$ 385, 129	58
The sum appropriated is	85,000	
The sum that can be profitably expended during the next fiscal		
vear is	100,000	00

(See Appendix A 3.)

II .- Lake Michigan.

5. HARBOR OF GREEN BAY, WISCONSIN.

During the present season more than 30,000 cubic yards of earth and sand have been removed, giving a cut 12 feet deep across Grass island. It is expected that by the close of the working season a channel of this depth, 100 feet wide, will be opened, as well as one of the same depth from 12 feet water on the south to 12 feet water on the north side of the island.

The revetment of the cut will be postponed until the action of the ordinary

current and the effect of freshets upon the new cut are ascertained.

The engineer in charge recommends that instead of sheet-piling, as originally proposed, the cut across the island and in shoal water be revetted with close piling, as more durable work; and in deep water on the north side and at both ends of the cut, that pier work composed of cribs ballasted with stone be used.

A decision upon this modification is reserved until further information is

obtained.

The estimated cost of the present improvement was	\$155, 416 1	17
Amount already appropriated	75, 500 (
Amount required for next fiscal year	80,000 (00
(See Appendix A 4.)		

6. HARBOR OF MANITOWOC, WISCONSIN.

The plan adopted for the improvement of this harbor is to extend two parallel piers outward into Lake Michigan until a depth of 12 feet is obtained, and to dredge the waterway between the piers to the same depth. The piers to be composed of cribs ballasted with stone.

This work is under contract and progressing well.

At the date of Colonel Wheeler's report there had been expended \$40,324 72 for labor and materials, and 576 feet of piers had been constructed. 960 feet in all will be completed during the present season, provided the weather is favorable.

The estimated cost of the work was	\$141,747	82
Amount already appropriated	97, 000	
Amount necessary to complete, and required for next fiscal year	45,000	00
(See Appendix A 5.)		

7. HARBOR OF SHEBOYGAN, WISCONSIN.

The plan adopted for this harbor was to extend the north pier 120 feet, the south pier 320 feet, and to dredge the channel between the piers.

For this purpose there was appropriated, in 1866, \$47,598 91, and in 1867

an additional appropriation was made of \$8,000.

The greater part of this sum has been expended in completing the piers as

proposed, and the work will be finished this season.

The engineer in charge recommends, as a further improvement of this harbor, that the piers be extended into the lake across the present outer bar, opposite the entrance of the river. This will require 416 running feet of additional pier work and some dredging, at an estimated cost of \$49,000, which can be profitably expended during the next fiscal year. This recommendation is approved.

(Sce Appendix A 6.)

8. HARBOR OF MILWAUKER, WISCONSIN.

The only work done at this harbor during the present year was the filling up of the cribs with stone. The stone having settled considerably through the grillage bottoms, or having been taken away, it was considered necessary to replace it.

The general condition of the work, upon close examination, has been found to be good, but there are evidences of decay—a necessary consequence of using timber above the water surface. The portion above the water must be replaced

in the course of a few years.

A bar from the northward is encroaching upon the entrance of this harbor

and threatening to injure it seriously.

An extension of both piers 300 feet would postpone for many years the injurious results now threatened. This extension is therefore recommended by the engineer in charge, at a cost of \$65,872 80. Deducting present balance of appropriation on hand, \$38,354 53, would leave, say \$28,000, which could be profitably expended during the next fiscal year. The recommendation is approved.

(See Appendix A 7.)

9. HARBOR OF RACINE, WISCONSIN.

The plan for this harbor is to extend both piers, composed of cribs ballasted with stone, until a depth of 15 feet of water is reached, and to dredge between the piers until 12 feet is obtained throughout.

Due notice having been given, the bids were opened, and contracts entered

into for prolonging the north pier the required distance.

The engineer in charge recommends dredging between the piers to a depth of 14 feet. The estimated cost of this improvement was \$84,172 48; the amount appropriated was \$45,000; amount required to complete the work, \$39,172 48; add for additional dredging, \$5,000; amount which can be profitably expended during the next fiscal year, \$45,000. The recommendation is approved.

(See Appendix A 8.)

10. HARBOR OF KENOSHA, WISCONSIN.

During the present season the contractors have extended the south pier 352

feet, and will complete the extension of the north pier 192 feet.

A depth of 12 feet has been obtained throughout the greater part of the water-way, between the piers, by dredging. The old piers are in bad condition, and require rebuilding from the water surface. The basin inside is very shallow. The engineer in charge does not consider it necessary to extend the piers

further at the present time, but thinks it proper to repair the old work and dredge the basin to the depth of 10 feet; for which he estimates—

He asks an additional appropriation for the next fiscal year of \$40,000. (See Appendix A 9.)

11. HARBOR OF CHICAGO, ILLINOIS.

In 1865 the plan recommended by Colonel Cram was to extend the north pier 600 feet, and rebuilding the south pier to extend it 610 feet. Under the expectation that 110 feet of the northern extension would be done by the city authorities, his estimates were made for adding 490 feet only to the north pier. The estimated cost for the entire work was \$88,704, which sum was appropriated. But, as heretofore reported, this amount was found to be inadequate to the whole work.

The extension of the north pier was contracted for in October, 1866. Before work was commenced this season, the Chicago Canal and Dock Company submitted for approval their project for an entrance to their basin at the end of the present north pier. This application was favorably recommended to the Secretary of War from this office, and the authority asked for was granted. This company have guaranteed to close in the basin extending to the north and east of the present pier before the end of the working season. It is expected that the north pier will be extended 300 feet before the season closes.

The engineer in charge recommends that this pier be extended only 300 feet, and that the balance of the appropriation be expended upon the south pier, the extension of which, as proposed, will require an additional appropriation of \$48,000. This amount can be profitably expended in the next fiscal year. The recommendation is approved.

(See Appendix A 10 and A 11.)

12. HARBOR OF MICHIGAN CITY, INDIANA.

It having been satisfactorily shown that the Michigan City Harbor Company had expended \$100,000 upon this harbor, as required by Congress before the appropriation of \$75,000 could be available, the work was let on the 7th of August, and the contractors are under bonds to furnish materials and build not less than 128 running feet of pier work this year.

The improvement proposed for this harbor is to extend the northeastern pier 288 feet on a line with its present direction, extend the western pier 320 feet on a line with direction of present end crib, and dredge the channel between the piers to a depth of twelve feet. The dredging is done by the steam-dredge belonging to the harbor company, the United States paying all the running expenses.

The materials for the piers, and labor, have been contracted for.

(See Appendix A 12, A 13, A 14.)

13. HARBOR OF NEW BUFFALO, MICHIGAN.

The sum of \$60,000 was appropriated for the improvement of this harbor. The plan of improvement is that recommended by the late Colonel Graham, United States corps of engineers, the estimated cost of which was \$384,020.

Contracts have been entered into, and the contractors are at work excavating the cut. They will remove this year not less than 50,000 cubic yards of sand.

From a recent inspection of this harbor, the engineer in charge reports unfavorably upon the practicability of keeping it open, without a large annual expenditure, after the costly harbor works proposed have been completed. The project will be referred to a board of engineers before the resumption of operations in the spring.

Should the work be continued, the sum required for the next fiscal year will

be \$110,000.

(See Appendix A 15.)

14. THE HARBOR OF ST. JOSEPH, MICHIGAN.

During the present year the old piers have been repaired, and the south pier extended. The extension was 200 feet, using a pile foundation with rib super-structure, and filling the pier with brush and stone. Greater strength has been

given to this pier than was originally intended.

The engineer in charge recommends that the south pier be extended 700 feet. The cost of this extension, using the pile structure, would be \$49,000; if cribs be used, the cost will be \$77,000. This extension is necessary for the preservation of the channel. The project is approved, and it is recommended that there be appropriated, to be expended during the next fiscal year, in extending the south pier, the sum of \$77,000.

(See Appendix A 16.)

15. SOUTH HAVEN HARBOR, MICHIGAN.

Contracts have been entered into for material and labor for the construction of two piers, extending into the lake a total distance of 640 feet, and require that not less than 200 feet shall be completed on or before the 31st of October, 1867.

16. BLACK LAKE HARBOR, MICHIGAN.

The plan of improvement is the extension of the piers, and dredging the channel between the piers. This work has been contracted for, and will be completed in 1868.

(See Appendix A 18.)

17. GRAND HAVEN HARBOR, MICHIGAN.

During the present year the south pier has been extended 192 feet into the lake, and about 500 feet of close piling has been constructed. The present appropriation will complete the close piling and the present pier.

The engineer in charge recommends that the building of the north pier should

not be deferred. This recommendation is approved.

18. MUSKEGON HARBOR, LAKE MICHIGAN.

The plan proposed was to extend the present piers to 17 feet water. The estimated cost was \$58,450. This amount was appropriated, and will build the piers.

The engineer in charge recommends that the upper portion of the present slab piers be replaced by a crib superstructure, ballasted with stone, as the slab pier is liable to be burnt. The cost of this improvement is estimated at \$64,000. There appears to be no immediate necessity for this expenditure. An appropriation for the next fiscal year is not, therefore, recommended.

(See Appendix A 20.)

19. WHITE RIVER HARBOR, LAKE MICHIGAN.

The sum of \$57,000 was appropriated in 1867 for the improvement of this harbor. The plan adopted is to cut a new channel across the neck of land separating the two lakes, at a distance of 1,200 yards from the present outlet of White river. The contractors are at work, and unless some unforeseen difficulties arise, the cut of the proper width and depth will be made before the season closes.

20. PENTWATER HARBOR, LAKE MICHIGAN.

The plan adopted for the improvement of this harbor was to increase the width of the present outlet from 76 feet to 150 feet, and to dredge to a depth of 12 feet throughout its entire length, which involves the removal of 127,028 cubic yards of earth and sand, and the construction of 2,120 running feet of close piling and 2,560 running feet of pier work, at an estimated cost of \$327,713 40.

Contracts for material, dredging, and building a pier of 768 feet in length have

been entered into.

21. PERE MARQUETTE HARBOR, LAKE MICHIGAN.

Two localities were proposed for the channel to the inner lake. One the present entrance, the other distant from it nearly a mile.

The estimated cost was as follows, viz: Improvement at first or present entrance, \$270,682 16; improvement at second or new cut, \$269,136 49.

As the two estimates were equal in amount, and as the sum appropriated could be expended to greater advantage at the present entrance, that locality was adopted. The contractors have pushed the work with vigor, and have placed 14 cribs in position (nearly 450 feet of pier work) this season, and expect to finish as much more before the season closes.

22. MANISTEE HARBOR, MICHIGAN.

The plan of improvement adopted at this harbor was to extend the south pier in its present direction 960 feet; to commence the extension of the north pier, at a point opposite the outer end of the old south pier, and extend it in a direction parallel to the south pier a like distance of 960 feet, to 12 feet water; to cut down all slab work to the water surface, and build crib work on the old foundation.

Since the survey of September, 1866, 100 feet of the old north pier has been

washed away, causing a slight change in plan, the north pier being thrown 20 feet further to the north, and the work upon it commenced at end of old pier.

Contracts have been entered into for building and sinking 24 cribs, (768 feet linear, in extension of the north pier,) and not less than 8 cribs to be sunk the present season.

23. HARBOR OF AUX BECS SCIES, MICHIGAN.

A resurvey recently made of this harbor shows that to improve it as contemplated, by extending the piers to a depth of 12 feet, an additional length of 384 feet of crib work and 150 feet of close piling will be necessary, which, with the increase of the cost of dredging over that estimated for, will require a further appropriation of \$48,000.

Contracts were entered into, and the work has progressed very satisfactorily. It is expected by the end of the season that the cut will be completed, the channel close piled, and 480 feet of cribs constructed.

24. SURVEY OF MICHIGAN CITY HARBOR, INDIANA.

This harbor was surveyed in April, 1867, by Captain A. MacKenzie, United States engineers, and a tracing of the map of the harbor, with his report upon the condition of the piers and other works, has been received.

25. SURVEY OF RACINE REEFS, HARBOR OF RACINE, WISCONSIN.

The object of this survey was to ascertain the practicability and necessity of constructing a light-house and breakwater on the reefs.

The survey was attended with difficulty and expense. The reef is nearly two miles from the entrance to the harbor, and is exposed to the open waters of the lake. It is a dangerous obstacle to navigation; a season rarely passes without vessels running upon it. These accidents happen mostly to vessels not bound to the harbor of Racine, but coasting near the shore.

The construction of a light-house with breakwater would be costly. Assuming as a guide the light-houses on Waugoshance shoal and Skilligallee rock, these points being similar in exposure to Racine reef, the cost of building a suitable light-house and breakwater around the house is estimated at about \$175,000.

Considering the large amount of this first outlay, and the subsequent annual expenditure to keep it in order, &c., the engineer in charge considers it questionable whether the case would justify the expenditure, and suggests as a substitute a large can-buoy, firmly anchored on the reef, to warn off vessels during the day, and a combination of range lights with the present light at the entrance to the harbor, and one on Wind Point, to warn them at night. Some such plan he therefore recommends.

(See Appendix A 26.)

26. SURVEY OF SAUGATUCK HARBOR, MOUTH OF KALAMAZOO RIVER, MICHIGAN.

A report upon the capacity of this harbor and plan of improvement are given in appendix.

The engineer in charge states that it will be seen by reference to this report

and map that the residents of that locality have expended their means freely in improving the entrance. The plan adopted by the residents is to confine the waters of the river by parallel piers built of slabs, having a direction S. 83 W.

The engineer in charge recommends the continuance of this improvement, so as to make this a harbor of refuge. To do this these piers should be extended until a depth of 12 feet of water is reached, and the waterway between the piers should be dredged to the same depth. The piers should be built of cribs, ballasted with stone, and the tops of the present piers should be removed to the surface of the water, and a crib superstructure, filled with stone, placed thereon.

The estimated cost is \$202,295 80; required for the next fiscal year

\$67,500. The project is approved.

(See Appendix A 27.)

27. SURVEY OF THE MOUTH OF THE MENOMONEE RIVER, GREEN BAY.

The survey of this harbor was completed in August, and a plan submitted

for its improvement.

In consequence of the shifting sand bar across the mouth of the present channel, its improvement was considered impracticable, and it was therefore proposed to cut through the neck of land between Green bay and the Menomonee river, and build two parallel piers in a due east direction, 1,375 feet in length on the north, and 1,275 feet on the south side, respectively, and to dredge a basin inside the river to a depth of 12 feet, at an estimated cost of \$252,571.

In the opinion of the engineer in charge commerce generally would not be

benefited by the improvement of this harbor.

(See Appendix A 28.)

III .- Hurbors on Lakes Huron and Erie, and improvement of the St. Mary's river and the St. Clair flats.

Officer in charge, Brevet Major General T. J. Cram, colonel of engineers, having under his orders Brevet Lieutenant Colonel F. Harwood, captain corps of engineers, and Captain G. J. Lydecker, corps of engineers.

1. IMPROVEMENT OF THE ST. MARY'S RIVER, MICHIGAN.

The contract for dredging the channel of this river not having been concluded until October of last year, it was not deemed practicable to commence operations at so late a period, and therefore the work was not entered upon until the opening of navigation the present year, in June. Since that time the work has progressed very satisfactorily.

The amount already appropriated is.....\$100,000 00

(See Appendix B.)

2. AU SABLE RIVER, LAKE HURON, MICHIGAN.

Proposals were advertised for this improvement, but no contract concluded up to June 30, 1867.

Amount therefore available July 1, \$50,000.

A closer estimate has been made for completing this improvement, based upon the bids already received, from which the officer in charge, Brevet Major General Cram, concludes the whole cost to be \$69,367. Hence, additional appropriation will be required of \$19,367.

(See Appendix B, B 1, and B 2.)

3. IMPROVEMENT OF THE MOUTH OF THE SAGINAW RIVER, MICHIGAN.

A contract for dredging was made in October, 1866, but owing to the lateness of the season no work was done until the following May, when, owing to the

dredge-boat having been driven ashore in a gale, the work was further delayed until June 15. From that time to June 30, 3,426 cubic yards of very hard material were raised, composed chiefly of hard conglomerate clay and gravel. The engineer in charge reports that notwithstanding the hardness of the material, the work is progressing very satisfactorily. There is no doubt that during the present working season a channel will be opened to admit vessels drawing 10 feet water. Eventually, the portion to be dredged will have a depth of 12 feet.

No additional appropriation is required. (See Appendix B)

4. LAKE ST. CLAIR-SAINT CLAIR FLATS.

The act of Congress of March 2, 1867, directed an appropriation of \$150,000 to be expended upon a straight cut or canal 300 feet wide and 13 feet deep, from deep water at the entrance of the South Pass, to sufficiently deep water in Lake St. Clair, a distance of about one and a half mile.

Froposals were advertised for, but no contracts made up to the 30th of June, 1867. Since that time contracts have been entered into under new proposals, the work to be commenced in September.

5. MONROB, MICHIGAN.

Contracts were entered into October, 1866. The delivery of materials, and the work of repair, were carried on during the winter. All the exterior underwater work upon the south pier was securely put in before the spring.

At the end of the fiscal year there had been expended \$6,760 06, leaving available, July 1, 1867, \$24,255 21. At the present contract prices the cost of the repairs and new work will be \$12,484 30; leaving a balance to be applied hereafter, as circumstances may develop, of \$12,770 65.

No additional appropriation required.

(See Appendix B.)

6. HARBOR OF TOLEDO, MAUMBE BAY, OHIO.

A contract for deepening the existing channel to a depth of 12 feet, with a width of 200 feet, was entered into October 12, 1866. Up to the end of the fiscal year, June 30, 16,236 cubic yards were excavated. Amount expended to June 30, 1867, \$4,564 62; leaving available for fiscal year commencing July 1, \$35,435 38.

In view of the commercial importance of this harbor, it is one of those deemed necessary to be enlarged to an increased depth of 15 feet, and a width of 300 feet. The officer in charge estimates for such improvement an additional cost of \$470,000. Additional appropriation required for the next fiscal year, to carry out this plan, \$150,000.

(See Appendix B.)

7. SURVEY AND IMPROVEMENT OF SANDUSKY RIVER, OHIO.

This survey has been made, and a report submitted for improving the navigation between Frémont and Sandusky bay, by dredging.

To make a channel from 160 to 200 feet wide, with a depth of 12 feet, will require the excavation of 185,075 cubic yards of sandy earth.

A contract was entered into, upon favorable terms, for the work, to the extent of the present appropriation of \$20,000. At the prices of the contract, there will be required, to complete the dredging of this channel, an additional appropriation (which should be appropriated for the next fiscal year) of \$35,000.

(See Appendix B, and B 19.)

8. SANDUSKY CITY HARBOR, OHIO.

A contract was entered into in October, 1866, for dredging upon the outer bar. The engineer in charge reports that the weather has not allowed dredging in that exposed place up to the expiration of the fiscal year, June 30. No further appropriation required until the effects of the dredging are known.

(See Appendix B.)

9. VERMILLION HARBOR, OHIO.

Contracts for materials and labor were entered into in October, 1866. Amount expended to end of fiscal year, June 30, 1867, \$11,987 16; leaving a balance

available July 1, 1867, of only \$3,328 58.

A more critical examination than that made in the winter, upon which the estimates were based, disclosed that the old under-water work of the west pier was more damaged than it was at first supposed to be, and a cutting away of the shore which has taken place since a breach of 400 feet has been stopped, requires much work to secure the pier at that point from destruction. These items will add \$6,511 to the cost of repair, which can be taken from the general fund for "repairs and preservation of lake harbors." No additional appropriation will therefore be required.

(See Appendix B, and B 20.)

10. HURON HARBOR, OHIO.

Contracts were entered into October 3, 1866, for materials and labor for the repair of both piers, and for rebuilding the cribs at the extremity of the east pier. The contractor having failed to furnish all the timber required, new proposals were advertised for, and a new contract made June 22, 1867. The percentage retained from the first contractor was more than sufficient to make good all loss to the work by his failure.

Amount expended to end of fiscal year June 30, 1867, \$10,960 24; leaving available July 1, 1867, \$28,039 76; less amount of repairs, as per contract, \$25,070 15; leaving a balance of \$13,929 85. No additional appropriation is

therefore required.

(See Appendix B.)

11. BLACK RIVER HARBOR, OHIO.

The rebuilding of the outer extremity of the east pier has much improved the depth over the outer bar. In a year or two hence an expenditure of a few thousand dollars will probably be required for stopping such holes as may develop themselves in the old under-water work of the west pier.

The appropriation of \$10,000, made in 1866, is still available for repairs. No

additional appropriation required.

(See Appendix B.)

12. CLEVELAND HARBOR, OHIO.

The principal part of the east pier is still occupied by a railroad company. Nothing will be done towards its repair until vacated by them. The officer in charge reports that, in consequence of the use and abuse of this pier, it is fast going to pieces.

Contracts for the extension of both piers into the lake were made in October,

1866, both for material and labor.

During the fiscal year ending June 30, 1867, there was expended from the appropriation of 1866, principally for materials, \$8,215. Amount available July 1, 1867, \$51,590 45. Amount required to complete the present plan, \$63,497. Additional amount required, \$3,691. This harbor is one of those selected for an increased depth, that will admit vessels of 14 feet draught. The estimated cost of this improvement is \$39,000. Additional appropriation required for the next fiscal year, \$43,000.

(See Appendix B.)

13. GRAND RIVER HARBOR, OHIO.

The officer in charge was instructed to make an accurate survey of this harbor, in order that the question of improvement might be more fully discussed. In his report, transmitting the result of this survey, he recommended that the extension of the east pier should be made in a direction parallel to the flare of the west pier. This was not approved, and instructions were given for the

extension of the pier in its present direction.

The work of extension was put under contract (for materials and labor) March 20, 1867. From this time to June 30, 1867, there has been expended, \$3,143 02; leaving available July 1, 1867, \$53,310 22. The cost of extending the east pier 320 feet, according to contract, with ten per cent. for contingencies, will be \$21,631; leaving for dredging and further improvements, \$33,040. No additional appropriation will be required for the next fiscal year.

(See Appendix B, B 21, and B 22.)

14. ASHTABULA HARBOR, OHIO.

Contracts were entered into for material and labor for the repair of both the old piers October 1, 1866.

Proposals were invited in May for dredging, but owing to the terms of the only bid made it was not accepted. Some doubts as to the character of the material to be dredged led to the making of borings similar to those at Conneaut harbor. No contracts have been entered into; no additional appropriation required.

(See Appendix B.)

15. CONNEAUT HARBOR, OHIO.

Proposals were invited for dredging May 17, resulting in a very high bid, owing to doubts entertained by contractors as to the character of the bottom. Borings were therefore made to remove all doubt as to the nature of the bottom. These have shown the rock to be everywhere more than twelve feet below the low-water stage, and that the material to be dredged is sand. The above bid being unreasonably high for this material, new bids will be invited, at the same time advertising for materials and work for the extension of the west pier, as far as the amount available will admit.

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Amount appropriated for this harbor in 1866 and 1867 for dredging and for repairing old works	\$30,513	00
Amount expended during the fiscal year ending June 30	3, 134	81
Additional appropriation required for the next fiscal year to extend		
the west pier into twelve feet water, and to complete the work		
to the extent and capacity planned	11,000	00
(See Appendix B.)	,	

16. ERIE HARBOR.

The amount appropriated for the harbor in 1866 and 1867 was	\$61,961	00
Considerable time was necessarily consumed in perfecting contracts		
before the work could be begun. Contracts were entered into on		
the 20th March, and from that time to 30th June there was ex-		
pended	6, 214	91
The cost of extending the north pier, (new work) including 10 per		
cent. for contingencies, is estimated at	34, 339	00
Leaving available for dredging and other purposes	27,622	00
The dredging upon the outer bar was begun on the 16th April	, and dur	ing
the month 1.878 cubic yards were removed. But from that time t		

no more dredging was done, in order not to be in advance of the extension of the north pier. The work is progressing under the contracts.

A thorough examination of the old breakwater, from the south pier to shore. built many years ago, and of the old north and south piers, has resulted in an estimate from the officer in charge, for the complete repair of all the old harbor works, of \$33,739, in addition to the amount already appropriated for the new improvement.

This harbor is one of those selected for an increased depth that will admit

vessels of fourteen feet draught.

The estimated cost of this improvement is	\$ 35, 000	00
Total additional appropriation required for the next fiscal year to	-	
repair old piers and increase the depth of the channel for vessels		
of fourteen feet draught	69, 000	00
(See Appendix B.)		

17. HARBOR OF DUNKIRK, NEW YORK.

Contracts for the work on the west pier have been entered into and the work commenced.

Owing to the length of time required to collect materials not more than three hundred feet can be built this season.

Amount available for the harbor June 30...... \$100,000 00 Additional appropriation required for the next fiscal year...... 50,000 00 (See Appendices B, B 23, and B 24.)

18. BUFFALO HARBOR.

Colonel and Brevet Major General Cram has submitted a report and plan of improvement for this harbor in which he proposes:

1. To repair and protect the existing north and south piers. 2. To extend the south pier three hundred to six hundred feet.

3. To remove two hundred to four hundred feet of south end of Erie basin

4. To construct a new breakwater in twenty-five feet water, about four thousand feet long and eight feet high, to shelter the harbor from prevailing winds, and to secure a larger space for refuge.

5. To ascertain, upon careful examination, the practicability of opening a

ship channel from the lake at South Cut directly to Buffalo creek.

In order to obtain sufficient data to perfect this plan, numerous measurements and examinations into the condition of the existing piers were made, as well as lines of soundings and borings into the bed of the lake, to ascertain the depth of water and nature of the bottom upon which the proposed structures are to rest. These examinations have occupied Captain and Brevet Lieutenant Colonel Harwood, under the direction of General Cram, up to the end of the fiscal year.

These plans will be submitted to a board of engineers, and in the mean time the repairs and protection of the present north and south piers will be made, and the south pier will be extended three hundred feet, or as far as the present appropriation will admit.

The amount of funds available for this work on the 30th of June, 1867, was \$200,000. No part of this appropriation has been expended up to this time.

(See Appendices B and B 25.)

19. SKA WALL AT BUFFALO.

Under the appropriations of 1864 and 1866 the late Major Tardy, corps of engineers, commenced operations upon the sea wall in October, 1864, and continued the construction until the close of the working season of 1866. The balance of appropriations remaining unexpended is \$23,751 39. The further extension of this wall does not appear to be required at present.

(See Appendix B.)

IV .- Harbors on Lakes Ontario and Champlain.

Officer in charge, Brevet Colonel C. E. Blunt, lieutenant colonel corps of engineers, having under his orders Brevet Major C. J. Allen, captain corps of engineers.

1. IMPROVEMENT OF OLCOTT HARBOR, (EIGHTBEN-MILE CRREK,) NEW YORK.

Contracts for this object have been entered into. The plans proposed and approved, and which will be partially carried out with the amount now available, contemplate the construction of two parallel piers, 200 feet apart, and 1,000 feet each in length, and dredging to give ten feet of water.

The engineer in charge states in his report that the commencement of operations at this place, as well as at all other harbors on Lakes Ontario and Champlain, has been seriously delayed by the difficulties and obstacles arising from the provisions of the law, which, instead of facilitating the speedy and economical execution of the works for which appropriations have been made, have had precisely the opposite effect.

Amount appropriated at last session of Congress	\$60,000 00
Amount required for completion of work	118,000 00
Amount required for next fiscal year	58,000 00
(See Appendix C.)	

2. IMPROVEMENT OF OAK ORCHARD HARBOR, NEW YORK.

Within the entrance of Oak Orchard creek there is a depth of eighteen feet, and piers constructed some years ago made a good harbor of refuge for vessels. These piers, being much decayed, require repair, and some dredging is needed of a bar which has formed since the injury to the piers occurred. Contracts for these objects have been entered into.

Amount appropriated at last session of Congress, \$87,000. No further

appropriation is deemed necessary.

(See Appendix C.)

3. IMPROVEMENT OF HARBOR AT CHARLOTTE, (MOUTH OF GENESEE RIVER,) NEW YORK.

The rebuilding of the west pier was continued during the fall of 1866, under partial contracts already reported. The completion of this reconstruction and of that of the east pier have been provided for by contracts made late in the present working season, by the close of which the work on the west pier will be finished, and, it is hoped, some progress made upon the east pier.

The last appropriation (in 1866) was supposed to be sufficient for all necessary improvements at this point; but the data furnished by the work already executed, and the terms of the contracts for the remaining work, render it probable that a small additional sum will be required.

4. SURVEY AND EXAMINATION AT PUTNEYVILLE, NEW YORK.

This was made in June, 1867. To make a harbor here (there being now none worthy of the name) will require the construction of two piers, 850 and 900 feet long, and 32,000 cubic yards of dredging, the cost of which is estimated at \$87,000.

(See Appendix C.)

5. IMPROVEMENT OF BIG SODUS HARBOR, NEW YORK.

The work of rebuilding the west pier and dredging the channel, under contracts made and reported last year, was commenced late this working season, by the end of which it is expected that six hundred feet of pier will be rebuilt, and a channel eighty feet wide dredged to twelve feet at low water. A contract for dredging under the last appropriation has been made, and some work will be done under it this year. Contracts for the reconstruction of the remainder of the piers will be made in time for next season's work. It is thought that the sum now available will be sufficient to complete the necessary improvement.

Amount appropriated at last session of Congress, \$80,000. No further appropriation is required.

(See Appendix C.)

6. IMPROVEMENT OF LITTLE SODUS HARBOR, NRW YORK.

The dredging of the channel and extension of the west pier, for which contracts were made and reported last year, have progressed during the season of 1867, at the end of which it is expected that four hundred and fifty feet of new pier will be finished, and a cut eighty feet wide, with twelve feet water, made in the channel. A contract has been made for dredging under the new appropriation, and work has been commenced. Contracts for the further extension of the pier will be made in time to resume the work next spring. A short east pier (about two hundred and fifty feet) is deemed advisable to define the channel; and the closure (by a cheap crib work) of the opening between the present pier and the west shore of the entrance is considered necessary.

A moderate appropriation is needed for these objects, which, when accomplished, will complete all that is necessary at this harbor until the decay of the

part of the wooden piers above water requires their repair.

7. IMPROVEMENT OF OSWEGO HARBOR, NEW YORK.

The dredging of the harbor and repairs of the United States pier have been continued during the year, under contracts heretofore reported. The dredging so far executed has rendered available for vessels a part of the pier and west portion of the harbor. The revenue cutter will now be enabled to winter here, which has not been possible for some years. The dredging will be continued

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and can be nearly completed under a new contract already made. The pier will probably require an annual expenditure to maintain it in serviceable condition. For this repair and the completion of the dredging, an additional appropriation is asked.

8. IMPROVEMENT OF OGDENSBURG HARBOR, NEW YORK.

The special board of engineers, of which Colonel and Brevet Brigadier General H. Bache was president, convened in June, 1867, for the purpose of considering the plans proposed for the improvement of this harbor, after full examination reported in favor of expending the whole appropriation in dredging. Their report has been approved, and a contract entered into under which some work is expected to be done this season.

The amount appropriated at the last session of Congress (\$40,000) is sufficient

for the work at this point, and no further sum is asked for.

(See Appendix C and C 1.)

9. IMPROVEMENT OF PLATTSBURG HARBOR, NEW YORK.

The repair of the breakwater above low water, decided upon last year, has been contracted for, and will probably be completed in 1867. The reconstructed work will be somewhat narrower than the old, and the cost will be rather less than the original estimate. The shoal, reported to exist as long ago as 1864, has been surveyed. A special report relating to it has been made to this office. This shoal should be removed, as it is a serious injury to the harbor. For this purpose an appropriation of \$20,000 is required for the next fiscal year.

(See Appendix C and C 2.)

Improvement of Burlington harbor, Vermont-Extension of breakwater.

A special board of engineers, of which Colonel and Brevet Brigadier General H. Bache was president, recommended (in June, 1867) the extension of the

present breakwater northward in a line parallel to the shore.

The extension (by crib work, ballasted with stone) should be 1,500 feet in length. Contracts for this work, as far as the available amount will permit, have been entered into, (in August,) and it is hoped something may be done this season.

Increased depth of lake harbors.

In answer to an inquiry by the Committee on Commerce of the Senate, in relation to the increase of depth of harbors on the lakes to admit vessels drawing fourteen feet, a report was submitted to the Secretary of War on the 19th day of February, 1867, recommending certain harbors for such increased depth, and the estimated cost for the improvement. A copy of the report, accompanied by reports of each of the engineers in charge of the harbor works is herewith appended.

(See Appendix C 4.)

Surveys and improvement of the upper Mississippi, Minnesota, Wisconsin, and Fox rivers, &c.

Officer in charge, Major and Brevet Major General G K. Warren, corps of engineers, assisted by Captain and Brevet Major C. R. Suter, corps of engineers.

In May last, the engineer in charge, having completed his duties as a member of the board of engineers for considering the plan for the improvement of the Des Moines rapids of the Mississippi, was directed to proceed to St. Louis and Cincinnati for the purpose of examining steamboats with a view to procuring a suitable one for snagging and dredging on the upper Mississippi and Wisconsin rivers. Upon his return to St. Paul, in June, both of the above rivers were found to be too high for carrying on advantageously any portion of the surveys, &c., other than gauging, and this was immediately commenced.

Proposals were invited in June, first, for the sale of steamboats to the United States, to be used in dredging the sand-bars; second, for wrecking the steamer Northern Light; and, third, for removing snags and boulders from the Minnesota river, it having become obvious that nothing could be accomplished this

season with boats to be constructed.

No results having been obtained from the invitation referred to, and other efforts proving unavailing, steamboats were subsequently purchased, and immediately put in preparation by attaching the requisite scrapers, &c., for the work designed for them.

A favorable contract was made for removing the obstructions from the Minnesota river; and the wreck of the Northern Light was removed by the wrecker belonging to the western river improvements, which was directed to perform the duty, having been sent to the upper Mississippi as a sanitary measure.

The surveys of the upper Mississippi and Wisconsin rivers have progressed

satisfactorily, and the latter will be completed this season.

All of the works are being urged forward as rapidly as is consistent with.

accuracy and a due regard for economy.

The engineer in charge asks an appropriation of \$50,000 for continuing the survey of the upper Mississippi, which has been included in the estimate for examination and surveys on western and northwestern rivers.

His estimate for continuing to operate two snag and scraper boats on the Mis-

sissippi river during the next fiscal year is \$36,000.

He further recommends the construction on the Mississippi river of the dams at Prescott, at a cost of \$5,000, and at Waconta chute, at a cost of \$5,000; and on the Minnesota river of the dam and lock at Little Falls, at a cost of \$60,000, as without this work the benefit to be derived from the removal of the snags and boulders, which is now going on, will be imperfectly realized.

The construction of the dam and lock at Meeker's island, on the Mississippi, at a cost of \$235,665, is also recommended by him. The items for these constructions have not gone into the annual estimates from this office, since the works

have not yet been approved by Congress.

(See Appendix D.)

Improvement of the Des Moines rapids, and of the Rock Island rapids of the Mississippi river—Survey of the Illinois river, and survey of the Rock river in Wisconsin and Illinois.

Officer in charge, Brevet Major General J. H. Wilson, lieutenant colonel thirty-fifth infantry, having under his orders Brevet Lieutenant Colonel P. C. Hains, captain corps of engineers.

IMPROVEMENT OF THE DES MOINES RAPIDS OF THE MISSISSIPPI RIVER.

A careful survey of these rapids was made during the autumn of 1866, the results of which, together with the details of the plan of improvement proposed by the

officer in charge, were submitted from this office and transmitted to Congress February 6, 1867, and printed in House document No. 79, second session 30th

Congress.

In this report the officer in charge recommended the construction of a lateral canal, extending along the Iowa shore, from Keokuk to the village of Nashville, a distance of seven and six-tenths miles, and that the improvement should be completed to Montrose, by making a through cut two hundred feet wide, and five or six feet deep, along the natural channel of the "upper chain." That the dimensions of the canal should be as follows: length seven and sixth-tenths miles; width on water surface, three hundred feet; depth at lowest stage, six feet, with two lift locks and one guard lock, each three hundred and fifty feet long, and eighty feet wide at top; and that the embankment should be made twenty feet wide on top, be carried up to four feet above the highest known flood, and be covered inside and outside and on the top with rip-rap of broken stone.

The appropriation of \$500,000, by act of March 2, 1867, required that this amount should be expended according to such plan as the Secretary of War should, on the report of a board of engineers, approve. Accordingly, a board of engineers, composed of the following officers: Colonel J. N. Macomb, corps of engineers, president; Lieutenant Colonel and Brevet Major General J. H. Wilson, thirty-fifth infantry; Major and Brevet Major General G. K. Warren, corps of engineers; W. Milnor Roberts, esq., United States civil engineer, and Captain and Brevet Lieutenant Colonel P. C. Hains, corps of engineers, recorder, was convened at Keckuk, Iowa, April 16, 1867, which, after a careful consideration of the subject, recommended certain changes in the details of the above-mentioned plan, viz: that the embankment be reduced to ten feet in width ou the top, with a riprap covering two feet thick, to be carried two feet above extreme high water instead of four feet; the prism of the canal to be three hundred feet wide in embankment, but reduced to two hundred and fifty feet in excavation; the minimum depth of water to be five feet, maximum depth eight feet.

These recommendations of the board were concurred in at this office, and approved by the Secretary of War, and the officer in charge directed to proceed at once to carry out the plan adopted. Accordingly a contract has been entered into after due advertisement for proposals for excavating the prism, and building the embankment of the canal. This part of the improvement was selected as the first to be commenced, for the reason that the embankment being, through most of its length, in the river, will require a more uncertain length of time for its completion, and be more liable to be delayed. After partial completion it will afford protection to the lock work from damage by high water or ice; and, also, for the reason that the \$700,000 already appropriated will nearly complete the embankment, leaving the locks and the channel improvement at the "upper chain" to be pro-

vided for by future appropriations.

The officer in charge estimates that \$1,500,000 will be required for the remainder of the work, and reports that the entire improvement can be economically completed by the 1st of November, 1869, provided the total sum required be appropriated at the next session of Congress.

(See Appendix E, E 1, E 2, E 3, and E 4.)

2. IMPROVEMENT OF THE ROCK ISLAND RAPIDS.

The act of March 2, 1867, appropriated \$200,000, which, in addition to \$100,000 already appropriated and still unexpended, gives \$300,000 for the prosecution of the improvement.

The plan adopted is to excavate and straighten the natural channel, which

will require about 58,000 cubic yards of rock excavation.

A contract has been entered into, the contractors being required to remove at least 5,000 cubic yards of rock per month. They are to use coffer-dams, chis-

els, or sub-aqueous blasting, as they may think best; but all tools, implements, machinery, boats, and materials of whatever character must be furnished at their

own expense, the government paying only for the work done.

As yet but little has been accomplished, in consequence of continued high water. During the summer the chisels have been put to work on one of the upper reefs of Duck Creek "chain," and the contractors are building a cofferdam, immediately below the point at which the chisels are working, for the purpose of cutting through the principal "chain" at that place.

The officer in charge estimates that \$813,602, including the \$300,000 already appropriated, will be required, under the plan adopted, for the entire and permanent completion of this improvement, and that \$513,602 can be profitably expended during the next fiscal year. He asks that this sum be appropriated as soon as practicable, in order that advantage may be taken of every opportunity presented by low water in the river for doing the work, and he reports that the entire improvement can be completed by the 1st of November, 1869, if the necessary funds are provided.

(See Appendix E, E 5, E 6, and E 7.)

3. SURVEY OF THE ILLINOIS RIVER.

The act of June 23, 1866, authorized the survey of Illinois river from La Salle to its mouth. This was made during the autumn of 1866, and a report thereon, submitted from this office, transmitted to Congress and printed in Ex. Doc. No. 16, House of Representatives, fortieth Congress, first session.

As the operations of this survey were confined to that part of the country lying below La Salle, it was thought necessary to extend the survey to Lake Michigan before absolutely fixing upon the plans of improvement. Accordingly a detailed and exhaustive survey of the country between La Salle and Chicago, as well as a low-water survey of the river from La Salle to its mouth, have been undertaken, and are now satisfactorily progressing.

William Gooding, esq., civil engineer, and late chief engineer of the Illinois canal, was appointed (under the resolution of Congress in relation to surveys of western rivers) to co-operate with Brevet Major General Wilson in the execu-

tion of these surveys and examinations.

The plans will be submitted as soon as these surveys are finished.

4. SURVEY OF ROCK BIVER IN WISCONSIN AND ILLINOIS.

This survey, provided for by act of June 23, 1866, was made during the autumn of 1866, and a report thereon, with plans for a system of canal and slack-water navigation, submitted from this office, transmitted to Congress April 11, 1867, and printed in Ex. Doc. No. 15, House of Representatives, fortieth Congress, first session.

Improvement of the mouth of the Mississippi river; survey of Galveston harbor, Texas; and survey of Bayou Manchac and Amite river, Louisiana.

Officer in charge, Brevet Brigadier General M. D. McAlester, major corps of engineers, having under his orders First Lieutenants D. W. Payne and W. S. Stanton, corps of engineers, and First Lieutenant John K. Heslep, corps of engineers, since deceased.

1. IMPROVEMENT OF THE MOUTH OF THE MISSISSIPPI RIVER.

The contractor, who, according to contract, was to have formed a channel across the Southwest Pass, of eighteen feet depth and two hundred feet in width, by the 23d January, 1867, and maintain the same three months, failed to complete his dredge-boat before the latter part of March, 1867, when he commenced

work. The time for completing the formation of the channel was, on his application, repeatedly extended till the latter part of May, 1867, when it appearing that owing to the inadequate and imperfect character of his boat and machinery

he was likely to accomplish no result, his contract was annulled.

Surveys of the Southwest Pass and Pass à Loutre were made during the spring months of the year, with the assistance of two parties of the United States Coast Survey, kindly detailed for the purpose by the superintendent. The examinations consisted mainly in ascertaining the soundings, the extent of the bars along the mid-channel line, and the character of the surfaces and sub-strata of the bars.

The officer in charge of this improvement has perfected models, drawings, and specifications of a dredge-boat authorized by the joint resolution of Congress approved March 29, 1867, and has advertised for proposals for constructing and delivering the same.

Pass à Loutre has been selected for improvement under the appropriation for the current fiscal year, which, added to the unexpended balance on hand, amounts

to about \$273,000.

The plan adopted is that of excavating and stirring up the minute alluvial material by means of double-ender dredge-boats fitted with an excavating screw and an auxiliary scraper. The light material thus again brought into a floating condition will be gradually carried off to deep soundings by the current of the river and tide of the Gulf. Owing to the condition under which the bars are formed and maintained, this work is not susceptible of "entire and permanent completion." After the completion of the two dredge-boats required for the work, an annual expenditure of \$100,000 will be required for the constant maintenance of a twenty-feet channel.

During the next fiscal year \$375,000 can be profitably expended upon the work, and is absolutely essential to its success, of which \$100,000 is estimated for running expenses, repairs, &c., and \$275,000 for construction and delivery of the second of the two required dredge-boats.

(See Appendix F, F 1, and F 2)

2. SURVEY OF GALVESTON HARBOR WITH A VIEW TO ITS PRESERVATION AND IMPROVEMENT.

The Superintendent of the Coast Survey kindly ordered one of its parties for the execution of the survey under the direction of the engineer officer in charge. The survey was begun in June, and assiduously continued until the breaking out of sickness, when the party was discharged.

The survey will be resumed as soon as the subsidence of the yellow fever will

permit.

3. SURVEY OF PASS AND BAYOU MANCHAC, AND OF AMITE RIVER, TO ASCERTAIN THE PRACTICABILITY OF STEAMBOAT NAVIGATION BETWEEN THE MISSISSIPPI RIVER AND LAKE PONTCHARTRAIN.

The field-work of the survey was begun and completed by First Lieutenant J. K. Heslep, corps of engineers, since deceased.

The reports and accompanying maps will be submitted at an early day. (See Appendix F.)

1. Survey for a ship canal around the falls of the Ohio.

Brevet Major General G. Weitzel, major corps of engineers, was placed in charge of this improvement May 11, 1867, with instructions to make a special report upon the same at the earliest day practicable.

The survey has been diligently prosecuted, and is still in progress. It will probably be completed, and the report submitted by the 1st of February next.

2. Survey of Tennessee river.

This survey is also progressing satisfactorily. As soon as completed the results will be made the subject of a special report.

Improvement of the western rivers, excepting the Ohio river.

Officer in charge, Colonel J. N. Macomb, corps of engineers, brevet colonel United States army, having under his orders Brevet Major C. W. Howell, cap-

tain corps of engineers.

The work upon these improvements has thus far been principally of a preparatory character. Three double-hulled snag-boats have been contracted for, and only await a rise of water to be launched. They embrace several important improvements upon the plan of those formerly in use, and hopes are entertained of their being very efficient. Should they meet these expectations and show the superiority of the machinery provided for them, an additional number will probably be built.

Amount of funds made subject to the requisition of the engineer in charge on commencing this work, (under appropriation of 23d of June, 1866)	\$400,000	
Leaving available on the 1st July, 1868	a construct	ion

Examination and surveys on western and northwestern rivers.

An examination was made of the Arkansas river in the winter of and one is now in progress on the Missouri river.	of 1866–'6	57,
Amount of funds under this head (from the appropriation of 23d June, 1866) made subject to the requisitions of the engineer in charge of the western river improvements, as above	\$20,000 11,875	00 00
Leaving as available on 1st July, 1868	8, 125	00
Amount probably required for the fiscal year ending on the 30th June, 1869		
Leaving amount to be appropriated to fill the estimate under head of "examinations and surveys of western and northwestern rivers," for the fiscal year ending 30th June, 1869 to be allotted to the engineer in charge of western river improvement	21, 875	

Improving the Mississippi, Missouri, and Arkansas rivers.

Under this head a wrecking boat has been purchased and fitted for the service of blowing up wrecks encumbering the channels, and has thus far been employed upon the Mississippi in working at wrecks near St. Louis and near Memphis; but the water proving to be too high, it was found necessary to defer the finishing work upon those wrecks until the recurrence of a low stage of water.

Whilst employed as above the crew of the wrecking steamer were instrumental

in saving life from the wreck of the sinking steamer Gov. Sharkey.

The wreck of the steamer Northern Light, in Coon slough, of the upper Mississippi, is reported as removed, so as to leave nothing of it remaining within thirteen feet of the water surface at low water.

Contracts have been made with parties to work at the removal of snags and other such obstructions to the navigation of the upper waters of the Arkaness and Missouri rivers, and one of the large snag-boats will probably be worked in the lower waters of each of those rivers as soon as possible.

Amount made subject to the requisitions of the engineer in charge. Total probable expenditures under this head of appropriation to		00
end of fiscal year ending on 30th June, 1868	255, 110	00
Leaving available on 1st July, 1868	111, 556	00
Estimate for fiscal year ending on the 30th of June, 1869 Deduct amount that will probably remain on hand as above	\$396, 000 111, 556	00 00
Showing that there will be required to be appropriated under this head of "Improvement of the Mississippi, Missouri, and Arkansas rivers," for the fiscal year ending on the 30th of June, 1869, the sum of.	284, 444	
		=

(See Appendix H, H 1, and H 2.)

Ohio river improvement.

Engineer in charge, W. Milnor Roberts, United States civil engineer, who was also a member of the board of engineers on the improvement of the Des

Moines rapids of the Mississippi river.

Two surveying parties are engaged upon the river, one in surveying between Gallipolis and Cincinnati, a distance of about 200 miles; another between Cincinnati and Louisville, about 145 miles. Upon the completion of these surveys the parties, should the season permit, will proceed with the surveys between Louisville and Cairo. The object of these surveys is to gather the necessary detailed information respecting the pools, ripples, shoals, islands, &c., and to prepare reliable maps of the same for use in the execution of the projects of improvement.

Estimated cost of the necessary examinations and surveys of the Ohio river to the end of the fiscal year June 30, 1868 Of which there was expended to June 30, 1867	\$22,000 6,677	00 18
Leaving to be expended during the fiscal year ending June 30, '68. Additional amount required for next fiscal year ending June 30,	15, 322	00
1869	10, 000	00

Contracts have been entered into for the delivery and putting in place of stone at the following riprap dams, viz: White's ripple, Logtown bar, Twin island, Captina island, Fish creek, Petticoat bar, Muskingum island, Blennerhasset's island, and Buffington island. It is expected that these dams will be completed before the 1st of December, 1867. Some of the dams are advanced far enough to cause a marked improvement in the depth of water in the channels.

A contract has been made and the work commenced for removing river obstructions, such as snags, wrecks, &c., at a fixed price per day, the contractor furnishing vessels, tools, men, &c., at his expense. This plan, which was to some extent experimental, has, on the upper Ohio, so far proved successful and advantageous to the government.

The engineer in charge estimates the cost of one steamer, &c., for a full season

of seven months' work at \$17,017; and estimates the total cost of removing obstructions from the whole length of the river, as follows:

Total from the beginning of 1867...... 74, 775 00

The known obstructions in the river, as reported in October, 1866, consisted of snags and snaggy places, 90; logs and loggy places, 66; wrecks, 46; sunken boats, barges, &c., 83; making a total of 285.

It is considered important that adequate appropriations should be made for continuing the present general system of improvement, which is, in brief, the construction of proper wing-dams, the excavation of portions of certain bars, the removal of rocks and numerous other obstructions, a detailed statement respecting which will be found in the report of the superintending engineer—(Appendix I.) He states that although accurate detailed estimates as to the necessary expenditure at particular points cannot as yet be made, approximate estimates may be made of the probable cost of perfecting such works as are likely to be necessary; that above Louisville, and especially above Cincinnati, a nearer approach can be made to accuracy than at points below, owing to the difference in the regimen of the river; that below Louisville, the question is complicated by the river being much wider, the fall and current much less, and by the formation of extensive moving sand shoals, the perfect control of which has not yet been attained.

836,000 00

And states that, in his opinion, should the season prove favorable, the whole work estimated for can be finished by the end of the season of 1868, provided the appropriation be made in time to admit of his entering into contracts by the 1st of April.

In view of the fact that no plans have yet been presented by the engineer in charge for the works below Louisville, excepting at one or two points, and that the improvement of that portion of the river is a very difficult problem, and, further, that some of the works above Louisville are contingent in their character, the whole amount of his estimate for the Ohio river cannot at this time be approved. It is recommended that there be appropriated for the next fiscal year the sum of \$500,000.

(See Appendix I.)

Improvement of the Patapsco below Fort McHenry, and of the Susquehanna below Havre de Grace.

Officer in charge, Brevet Lieutenant Colonel W. P. Craighill, major corps of engineers.

1. IMPROVEMENT OF THE PATAPSCO RIVER BELOW FORT M'HENRY.

A resurvey was made of the Brewerton channel and of the space east and south of Seven-feet knoll to the Belvidere shoal.

This survey was made by a Coast Survey party, the expense being borne by the appropriation for "examinations and surveys on the Atlantic coast."

From the close of the last fiscal year to the close of the working season of 1866, two dredges were worked in the Brewerton channel below North Point,

where it was found to have filled up to some extent.

It has been decided to dredge a channel two hundred feet wide to a depth of twenty-two feet at mean low water, adhering to the direction of the Brewerton channel until a point is reached nearly opposite the Seven-feet knoll, and then adopting a direction almost due south to the deep water of the bay.

The dredges and scows belonging to the government required very extensive repairs. These were nearly completed June 30. Since that time the dredging

has been successfully prosecuted.

Amount expended during the year ending June 30, 1867	\$34,682 88
Amount available July 1, 1867	55, 7 93 27
To be expended up to June 30, 1868	55, 793 27
Appropriation required for year ending June 30, 1869	125,000 00
(See Appendix K.)	•

2. IMPROVEMENT OF THE NAVIGATION OF THE SUSQUEHANNA RIVER BELOW HAVES DE GRACE.

A resurvey has been made of the portion of this river between Spesutie island and the railroad bridge near Havre de Grace. The survey was made by a Coast Survey party, the expense being borne by the appropriation for "examinations

and surveys on the Atlantic coast."

It has been decided to dredge a channel of eight feet in depth and one hundred feet in width nearly in the direction of the channel dredged some years ago. This channel had filled up to a great extent, and would doubtless fill up again if no other method of improvement than dredging were used. It has been determined, however, to construct a floating deflector, with the view of throwing an increased volume of water through the channel, and thereby obtaining a greater depth of water and greater permanence of condition in the channel.

Should the effect of this floating jettee prove to be as advantageous as it is anticipated, it should be replaced by a permanent structure of stone. The depth

of the channel should also be increased by dredging to ten feet.

The dredging and construction are going on.

Amount available July 1, 1867	\$26, 400 00
To be expended during the year ending June 30, 1868	26,400 00
For dredging to ten feet depth and making the deflector perma-	
nent an appropriation will be required for the year ending June	
30, 1869, of	50,000 00
(See Appendix K 1.)	

Construction of Delaware breakwater, improvement of harbor of Marcus Hook and Chester harbor, and surveys of Reedy island and Liston Tree Point, in Delaware bay and river.

Officer in charge, Lieutenant Colonel C. S. Stewart, corps of engineers.

DELAWARE BREAKWATER.

Contracts have been entered into and work begun for the completion of the superstructure. About 1,200 tons of stone have been put in position, and the superstructure raised thereby to its full height for a length of 115 feet, leaving about 560 feet in length of the breakwater proper to be completed.

No work has been done on the ice-breaker.

The existing appropriation will probably be sufficient to complete the works on their present basis.

(See Appendix L and L 1.)

2. HARBOR OF MARCUS HOOK, PENNSYLVANIA.

Contracts were entered into during the year for the repair of the old piers, wharves, and landings, and others have been made since for the construction of new piers for the improvement of the harbor. No work had been done at the close of the fiscal year, June, 1867, but it is now in progress.

The present appropriation will probably be sufficient to complete the work.

(See Appendix L and L 2.)

3. IMPROVEMENT OF CHESTER HARBOR, PENNSYLVANIA.

Proposals were invited and received, and contracts were in course of preparation for making the necessary repairs as soon as practicable after the 30th of June, 1867. The work is now in progress.

The present appropriation will probably suffice for the completion of the work.

(See Appendix L and L 3.)

4. THE SURVEY OF REEDY ISLAND

Has been completed. The report has not yet been received. (See Appendix L 4.)

5. THE SURVEY AT LISTON TREE POINT.

Will be completed in November, and the report will be transmitted as soon as received.

(See Appendix L 5.)

Hudson river improvement.—Repairs of United States dikes above and below Albany.—Commencement of resurvey of the river, with tidal observations and velocities of currents.

Officer in charge, Lieutenant Colonel and Brevet Major General John Newton, corps of engineers, assisted by Brevet Lieutenant Colonel John M. Wilson,

major of engineers.

The United States dikes above and below Albany having been found upon examination to require extensive repairs, it was decided to expend the appropriation of 1866, together with the balance remaining from the allotment of 1864, amounting in all to \$83,000, upon these repairs; and notwithstanding the lateness of the season at which work was commenced, and the unusual number of freshets which postponed active operations, considerable progress has been made.

A further appropriation of \$305,108 was made by act of Congress, approved March 2, 1867, and before deciding upon the parts of the plan of improvement upon which it should be expended and the order of their execution, a board of engineers, Colonel and Brevet Brigadier General Hartman Bache president, was convened in Albany, in May last, to consider the project proposed by Brevet Major General John Newton, lieutenant colonel of engineers, the officer in charge, which was described in the report of the Chief of Engineers of January 26, 1867.

The board concurred in the views of General Newton, and recommended that the money should be distributed throughout the entire distance from New Baltimore to Troy, in such a manner as to meet the most pressing needs of navi-

gation, beginning at the New Baltimore section.

The report of the board was approved, and the engineer in charge was

directed to carry out its views.

A survey of the river near New Baltimore, and also at Cuyler's island above Albany, was made in May and June, for the object of locating correctly the works of improvement at those localities.

Tidal and current observations have also been taken. A survey of all that portion of the river which is to be improved is in progress.

Amount required for next fiscal year, \$152,000.

(See Appendix M and M 1.)

Examinations at Hell Gate.

Examinations were made of Hell Gate, and a report and estimates for improving the navigation transmitted to Congress February 12, 1867, and printed in Ex. Doc. No. 91, House of Representatives, 39th Congress, second session.

Improvement of Westport harbor, Connecticut, Thames, Providence, and Pautucket rivers, removal of Middle rock, New Haven, and survey of Rock island.

Officer in charge, Major and Brevet Colonel D. C. Houston, corps of engineers.

1. WESTPORT HARBOR, CONNECTICUT.

The original appropriation for this harbor, \$2,500, is entirely inadequate to making the necessary improvement, which should consist of the repairs of the breakwater, Cedar Point, the repair of the walls of the canal leading from the habor to the sound, and the excavation of the bed of the canal, together with the removal of the obstructions in the channel of the river.

With the funds at present available the only work that can be done to advantage is the repair of the breakwater and of the walls of the canal, which will be

prosecuted this fall as far as the appropriation will permit.

The amount required for the improvement of the harbor, in addition to the sum already appropriated, and which is required for the next fiscal year, is \$10,000.

(See Appendix N.)

2. REMOVAL OF MIDDLE ROCK, NEW HAVEN, CONNECTICUT.

The work of removing this rock, which was commenced on the 13th of July, 1867, is progressing rapidly and satisfactorily. Some attempts had previously been made to blow off the top of the rock by exploding powder placed on it. This, however, failed, probably for the reason that there was not sufficient depth of water. Since then, holes have been drilled, cartridges of powder introduced, and exploded by means of a battery, with great success. On commencing operations this year the depth on the rock at low water was only nine feet. By the 1st of September five feet of the rock had been removed, giving fourteen feet at low water.

To secure a depth of seventeen feet at low water on this rock, as originally proposed, an appropriation will be required for the next fiscal year of \$30,000. (See Appendix N.)

3. IMPROVEMENT OF CONNECTICUT RIVER.

A survey of this river with a view to its improvement is now in progress, having been commenced on the 10th of August. Sufficient progress in the examination has not yet been made to enable a determination as to the precise nature of the works that will be necessary. When the survey is completed, and the plans and estimates prepared, the report will be submitted.

(See Appendix N.)

4. IMPROVEMENT OF THAMES RIVER, CONNECTICUT.

The work of deepening the channel of this river for a distance of about three miles below the city of Norwich, to obtain a depth of fourteen feet at high water,

necessitated a preliminary survey, which was completed late in 1866. Since June, 1867, dredging has been prosecuted as rapidly as possible, and up to the present month, September, 14,820 cubic yards of material, mostly sand, have been excavated and removed from the channel and deposited in localities carefully selected, that it may not find its way back again to the channel. This river is subject to great freshets in the early spring, at which time quantities of ice come down from the inland water-courses of which the Thames is the outlet to Long Island sound. For this reason it is deemed best not to attempt to complete the work this year, in order that the effect of the freshets may be observed and the work so directed as to prevent the channel being injured in the future. The work is now progressing satisfactorily.

It is believed that the amount appropriated for the improvement of this river

will suffice.

(See Appendix N.)

5. IMPROVEMENT OF PROVIDENCE RIVER, RHODE ISLAND; PAWTUCKET BAR, AND AT THE "CROOK."

The work during the year, in this river, consisted in dredging the channel at the "Crook," and raising and removing from the channel the wreck of a sunken schooner. This latter was successfully done, and the wreck deposited in seven feet of water at low water, and entirely out of the way of navigation.

The dredging consisted in deepening and widening the channel at the "Crook." Up to the 1st of September, 42,219 cubic yards of material, chiefly mud, had been removed, which, it is believed, will be all that is required to render free

navigation practicable.

The disbursements made under the appropriations for this work up to the 1st of September, 1867, amount to \$23,012 94. Amount available September 1, 1867. \$1.987 06.

It is believed that no farther work at this point is necessary.

(See Appendix N.)

6. IMPROVEMENT OF PAWTUCKET RIVER, RHODE ISLAND.

The work during the year in this river consisted in dredging the channel to obtain a depth of six feet at low water. The material excavated so far, 12,430 cubic yards, is fine sand, and has been deposited on flats on either side of the channel. After the channel is once thoroughly opened, the constant passage of vessels in tow of steam tugs will tend to keep it open, and the improvement may thus prove to be permanent.

The disbursements made under the appropriation for this work up to the 1st day of September, 1867, amount to \$4,409; amount available September 1, 1867,

\$12,591.

It is believed the amount already appropriated for this river will suffice.

(See Appendix N.)

7. SURVEY OF BLOCK ISLAND, RHODE ISLAND.

A survey of this island, with a view to the construction of a breakwater to form an artificial harbor, was commenced on the 19th of July, and has just been

completed. The report will be submitted as soon as prepared.

Colonel Houston states that there appears to be no question that a harbor of refuge should be provided at this point, not only for the benefit of the immense fleet of mercantile marine that traverse Long Island sound and this part of our Atlantic coast, but also for our navy at any time, in peace or war.

(See Appendix N.)

Plymouth beach, and examination of Duxbury beach, Massachusetts.

Officer in charge, Captain and Brevet Major J. A. Smith, corps of engineers

1. PLYMOUTH BEACH, MASSACHUSETTS.

One thousand three hundred lineal feet of breakwater have been built. It consists of triangular frames placed at intervals of four feet, covered with two-inch plank, set in a trench twelve feet wide and three feet deep, the trench and frame being filled to the level of the ground. It is situated on the crest of a ridge of sand of about three miles in extent, and of an average width of 800 feet. At intervals along this ridge there are depressions through which the sea breaks with great violence. It is in these intervals that the breakwater is placed.

The engineer in charge believes that further improvements will be needed, principally cross jettees, to arrest the sand in its motion along the beach, but has

not submitted a plan or estimate for this.

2. EXAMINATION OF DUXBURY BRACH.

The engineer in charge reports as the result of his examination of this locality that several depressions occur in the beach, of an aggregate length of 1,500 feet. But 500 feet, however, require repairs.

He submits an estimate for these repairs of \$4,000, but is of the opinion that neither the value of the harbor nor the fear of immediate injury to the beach are sufficient to justify a recommendation of the appropriation.

(See Appendix O.)

Provincetown harbor and sea-walls at Great Brewster, Deer, and Lovell's islands, Boston harbor.

Officer in charge, Colonel and Brevet Major General H. W. Benham, corps of engineers. Brevet Major George Burroughs, captain corps of engineers, assistant.

1. PROVINCETOWN HARBOR, MASSACHUSETTS.

A new plank-fence bulkhead, about 700 feet long, with the necessary jettees, has been constructed for the protection of the shore and battery, about one-third of a mile south west from Long Point, the extremity of the cape. During the autumn of 1866 the old bulkhead adjacent to the light house was repaired. A contract has been made for the restoration of the bulkhead to the extent of 600 or 700 feet, and for a rough stone breakwater, to be completed during the present season

At the outer sea beach, east of East harbor, the necessary catch-sands and about ten acres of beach grass have been planted, giving all the protection which

thus far appears to be necessary.

At Beach Point peninsula a plank-fence bulkhead was constructed in the autumn of 1866, one third of a mile from the bridge, about 1,250 feet long, with the necessary jettees. This, though seriously injured by the storms of winter, has for the most part remained in position.

During the past year the inlet of East harbor, at the extremity of Beach Point, has filled up to a great extent by natural causes, and it appears to be probable that little if any expenditure will be required for closing it by artificial means.

No further appropriation required for the present.

(See Appendix P.)

2. SEA WALLS OF GREAT BREWSTER ISLAND, BOSTON HARBOR.

As soon as the appropriation for the past fiscal year was available the work was resumed, and was continued during the present season. It is expected that at its close all the main sea-wall as originally plauned will be completed, which, in the judgment of the engineer in charge, will give ample protection to all the bluffs of the island.

The continued wearing action of the sea upon the beach between the two main walls renders it necessary to connect them, at an expense, as estimated by the engineer in charge, of \$34,000, which appropriation is asked for the next fiscal year.

(See Appendix P 1.)

3. DEER AND LOVELL'S ISLANDS, BOSTON HARBOR.

The rebuilding of the wall of the Middle bluff in Deer island was commenced in August, 1866, and by the close of the fiscal year, June 30, 1867, 340 lineal feet of the dry wall were securely rebuilt with mortar joints and concrete backing, with a paving of heavy flat stones 12 to 15 feet in rear of coping. The wall is about 17½ feet high, with an average thickness of 8 feet.

It is expected that the remaining portions of the wall requiring relaying in

Middle Head and South Head will be rebuilt during the present season.

At Lovell's island nothing has been done but the perfecting of contracts for facing stone for a wall at Southeast Head and for jettee stone for the old wall. It is expected that all the necessary stone will be delivered this autumn, and the work will be commenced as soon as the season permits in 1868.

Preservation and improvement of Boston harbor, Boston, Massachusetts.

Officer in charge, Lieutenant Colonel and Brevet Major General J. G. Foster,

corps of engineers.

The short time before the close of the fiscal year was occupied in making preliminary surveys, in advertising for proposals for dredging a channel across the Upper Middle bar, the removal by dredging of the southwest extremity of Lovell's island, and the extremity of Great Brewster Spit, and for blasting and removing the rocks in the Narrows (Tower Rock and Corwin Rock) lying between Fort Warren and the Narrows light, upon the extremity of Great Brewster Spit.

It is proposed during the present fiscal year to remove by dredging forty thousand cubic yards of material from the Upper Middle bar, so as to render the channel across it twenty-three feet deep at mean low water, with a width of about three hundred feet; to remove by dredging one hundred and forty-five thousand cubic yards from the southwestern point of Lovell's island and the extremity of Great Brewster Spit, so as to widen the main ship channel at that point from three hundred and sixty-five feet, its present width, to about five hundred feet, with a depth of twenty-three feet at mean low water; to remove entirely Tower Rock, by blasting, to twenty-three feet at mean low water, and to remove by blasting as much of Corwin Rock as the portion of the appropriation assigned to that object will admit; to commence the construction of the sea-walls at Long Island Head and on the north side of Gallup's island, and to commence the construction of the sea-wall to protect Point Allerton.

The work of dredging and blasting is now being successfully prosecuted

Tower Rock has been removed.

Amount of	appropriation	required	for the next	fiscal	vear.	viz:
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Additional for the removal of Corwin Rock	\$24,000 00
Dredging Great Brewster and Lovell's Islands Spits	60,000 00
Dredging Upper Middle bar	50,000 00
Preservation of North Head of Long island	75,000 00
Preservation of Gallup's island	54,000 00
Preservation of Point Allerton	21,000 00
Total amount required	284, 000 00

(See Appendix Q and Q 1.)

Improvement of rivers and harbors in the State of Maine.

Officer in charge Brevet Brigadier General George Thom, lieutena... . . . corps of engineers.

1. IMPROVEMENT OF SACO RIVER.

A board of engineers, of which Colonel and Brevet Brigadier General Hartman Bache was president, was directed to report upon the proper position and extent of a breakwater at the mouth of the Saco river. The board recommended the plan of Brevet Brigadier General Alexander, to the extent and in the position he proposed. This was approved by the department, and contracts have been entered into for rough stone for the breakwater, and for the removal of some sunken rocks.

Estimated cost of the breakwater, with an additional coping on the exterior face, and other improvements in Saco river already		
reported upon	\$270,000	00
Total amount appropriated in 1866 and 1867	80,000	00
Amount still required	150,000	00
Of which there can be profitably expended durig the next fiscal		
year	75,000	00
(See Appendix R and R 1.)		

2. SURVEY OF RICHMOND ISLAND, CAPE ELIZABETH.

A survey of this locality has been made with a view to forming an estimate of the probable cost of a breakwater to connect the island with the main land. Such a breakwater would form a good harbor of refuge, affording safe anchorage and good holding-ground with the wind from any point between north and southwest, affording refuge to vessels prevented by northeast storms from entering Portland or adjacent harbors.

The breakwater, to be permanent, should be built of rubble stone, of which the engineer in charge estimates there will be required 68,000 tons. This, when placed in the structure, would cost \$93,000. Amount which can be profitably

expended during the next fiscal year, \$50,000.

(See Appendix R 2.)

3. PORTLAND BREAKWATER.

A board of engineers, of which Colonel and Brevet Brigadier General Hartman Bache was president, was convened to investigate the question of the proper direction and length for the extension of the breakwater. The board recommended the adoption of the plan proposed by Brevet Brigadier General Alexander, corps of engineers.

Final action upon this project has not yet been taken. In the meantime, the

repairs upon the present breakwater, and the completion of its unfinished portions, are in progress.

Contracts have been entered into, and it is probable that these will be com-

pleted during the next fiscal year.

The amount appropriated is deemed sufficient.

(See Appendix R 3 and R 4.)

4. SURVEY OF THE KENNESEC RIVER ABOVE GARDINER.

The survey between Augusta and Shepard's Point was completed, and plans and estimates for the improvement of the river were submitted to Congress, and an appropriation for the purpose was made in the act of March 2, 1867.

Since that time the examination has been extended below Shepard's Point as far as Gardiner, which shows that some small amount of dredging is required on two shoals known as Hinckley's and Brown's Island shoals, and that a sunken rock in the channel must be removed. For these purposes an appropriation will be required of \$3,000.

(See Appendix R 5.)

5. IMPROVEMENT OF THE KENNEBEC RIVER BETWEEN SHEPARD'S FOINT AND AUGUSTA.

This work consists in straightening and deepening, by dredging, the channel of the river through several shoals.

The plan adopted will secure a channel through these shoals of a width of seventy-five feet at bottom, and a depth of eight feet at lowest water up to Hallowell, and seven feet thence to Augusta.

A contract for dredging has been entered into, and it is believed that the work will be finished during the fiscal year terminating June 30, 1868. The amount already appropriated is deemed sufficient.

(See Appendix R 6, R 7, and R 8.)

6. SURVBY OF THE "GUT" OPPOSITE THE CITY OF BATH.

Owing to the contraction of the channel of Back river at the upper Hell Gate, the tidal current runs through this gate with such violence as to endanger the navigation at any other time than at high and low water, except for steamers. The difficulties are still further increased by a large rock known as Boiler rock, which lies in mid-channel some seventy-five yards below the gate.

The engineer in charge examined this rock with the aid of a submarine party. It lies in from three to four fathoms of water at low water, its highest point being only about three feet below the surface at mean low water, and ten feet

below at high water.

For the improvement of the navigation at this place, the engineer in charge

1. Boiler Rock to be removed to a depth of 12 feet, requiring 70 cubic yards of blasting, which, at \$50 per cubic yard, would cost. \$3,500 00

2. The point of ledge contracting the channel at upper Hell Gate to be blasted off, requiring about 1,500 cubic yards, at \$4.... 6,000 00

3. Deepening the bar about midway between upper Hell Gate and Arrowsic bridge, so as to afford a channel 100 feet wide and 10 feet deep at mean low water, requiring 11,000 cubic yards dredging which at 50 cents now which word would cont

Total required for the proposed improvement...... 16,500 00

All of which could be profitably expended during the next fiscal year.

(See Appendix R 9.)

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7. SURVEY OF THE PENOBSCOT RIVER ABOVE HAMPDEN.

This survey is progressing and will be completed during the present fall. No report in relation to it has as yet been received from the officer in charge.

8. SURVEY OF UNION RIVER.

A careful examination of this river from its mouth to Ellsworth has been made. The engineer in charge estimates that for the improvement of the navigation between these points by clearing it of slabs, edgings, and saw-dust, removing boulders and sunken rocks, and erecting five stone beacons, there will be required an appropriation of \$40,000, which, in view of the large lumber trade, he recommends to be made.

Some legislation appears to be necessary, in addition to the local laws of Maine, for the protection of this and other navigable waters from obstructions

caused by deposits of the refuse matter from these saw-mills.

(See Appendix R 10.)

9. IMPROVEMENT OF THE NAVIGATION OF THE ST. CROIX ABOVE THE LEDGE.

An examination of this river from the ledge to head of navigation, above Calais, a distance of about four miles, shows that the improvement of its navigation requires its channel to be deepened by the removal of slabs, edgings, and saw-dust, which for thirty years or more have been accumulating in large quantities from the numerous saw-mills above and near Calais.

The act of Congress making appropriation for this improvement requires the co-operation of the province of New Brunswick. The subject was brought to the notice of the British minister to the United States through the Department

of State.

(See Appendix R 11.)

Rivers and harbors on the Pacific coast.

Officer in charge, Brevet Lieutenant Colonel R. S. Williamson, major corps of engineers, who has under his orders Lieutenant W. H. Heuer, corps of engineers.

IMPROVEMENT OF WILLAMETTE RIVER BELOW THE CITY OF PORTLAND, OREGON.

It was not practicable to do any work upon this river during the winter and

The city of Portland has, during the two past years, done some dredging with the view of deepening the channel of the river, and has provided suitable

dredging and other apparatus.

When proposals were invited for doing this work by contract, the city was the only bidder for removing Swan Island bar, at a price exceeding the amount of appropriation available, and as the act of Congress requires the work to be done by contract if possible, proposals were again invited, but no bidders presented themselves. It became necessary, therefore, to do the work with hired labor, and an arrangement was made with the city authorities of Portland to use their machinery free of charge, provided the United States kept it repaired.

At the date of the last report from the engineer in charge, August 5th, he was about to commence work, the state of the river not having admitted it

previous to that time.

There is no data to estimate with certainty the amount required for the com-

pletion of this improvement, as there had been no work actually done at the date of the last report. The engineer in charge, however, estimates it roughly at \$25,000, in addition to the amount now on hand.

Amount appropriated by act of March 2, 1867..... **\$**30,000 00 871 49 Amount expended during the fiscal year Additional appropriation required for the next fiscal year 25,000 00 (See Appendix S.)

2. SURVEYS AND EXAMINATIONS ON THE PACIFIC COAST.

The acts of June 23, 1866, and March 2, 1867, directed the following surveys to be made:

- 1. Survey of Blossom and Rincon Rocks in the harbor of San Francisco. with the view to their removal.
 - 2. Survey of the upper Columbia river, Oregon.
- 3. Survey of Crescent City harbor.

 I. Survey of Blossom and Rincon Rocks.—A report upon the removal of Rincon Rock was included in the report from this office of January last. Experiments have been made upon Blossom Rock in order to estimate the cost of its removal. From the report of Lieutenant Heuer, corps of engineers, who had immediate charge of these experiments, the amount expended on them was \$3,148 05, and the quantity of stone removed, 69 cubic yards. This expenditure would have been much greater but for the assistance of the United States Coast Survey in furnishing a vessel, officers and crew. Lieutenant Heuer estimates for the probable cost of removing Blossom Rock, and which should be appropriated for the next fiscal year, \$60,060.

(See Appendix S.)

II. Survey of the Upper Columbia. On the upper Columbia, no work has been done up to July 1, owing to the state of the river. A preliminary examination of the river has been made, as well as all preparations to prosecute the work with vigor after its commencement. A letter from the President of the Oregon Steam Navigation Company, giving much information upon the commerce and navigation of the upper Columbia, will be found in the Appendix.

(See Appendix S, S 1, and S 2.)

III. Surrey of Crescent City harbor.—Colonel Williamson reports upon this work as follows: "No survey of Crescent City harbor was deemed necessary, as the chart of this locality by the United States Coast Survey is sufficiently in detail to determine the position and extent of the proposed breakwater." It should be 3,760 feet long, with a height varying between 28 and 32 feet, and a cross section as strong as that of the Delaware breakwater. This would afford a well protected harbor, ample in area and depth. The amount of stone required for such a work would be about 400,000 cubic yards, but the cost of putting this in place is so indefinite as to make it impossible to estimate the total cost, except very roughly. It may be set down at \$2,000,000, but might exceed largely that amount. It is probable that constant dredging would be required to maintain a proper depth in the harbor.

The argument in favor of a work of this kind is, that Crescent City is the only position between San Francisco and Cape Flattery, where a harbor of reluge can be made. But it has only one or two hundred inhabitants, and the country in rear is sparsely settled or in dense forest. In view of these facts and the large expenditure for the construction of a breakwater, an appropriation for the commencement, at the present time, of such a work at Crescent

City is not recommended by the engineer in charge.

(See Appendix S and S 3.)

Public buildings, grounds, and works in the District of Columbia.

Officer in charge, Major and Brevet Brigadier General N. Michler, corps of

engineers.

Upon the transfer of public buildings, grounds and works to the direction of the Chief of Engineers, Major and Brevet Brigadier General N. Michler, corps of engineers, was assigned to their charge, including the Washington aqueduct. The report of that officer is appended hereto. Having been previously charged with surveys and plans for a national park, and the selection of a site for a presidential mansion, his report in relation to those subjects is likewise appended.

In the first named report the condition of the public works in the city of Washington and District of Columbia is stated in detail, and suggestions are offered as to such further improvements that are necessary for the convenience of the public service, the health of the inhabitants and the proper arrangement

of the grounds and avenues.

The bridges across the Potomac and Eastern Branch have been placed in passable condition. Some of them require additional repairs.

The fence around the botanical garden and the culvert through it have been

completed as far as the appropriations will admit.

General Michler recommends that Tiber creek be arched over as far as the northern limits of the city, and that certain improvements relating to the Washington canal be made.

With few exceptions the public squares have been placed in excellent condition. Authority is asked to enclose and improve several others, more especially

the mall.

Suggestions are made in reference to the removal of the public market-houses

and the erection of suitable buildings.

The avenues have been placed in as good repair as the means provided would admit, the greater amount of labor having been devoted to Pennsylvania and Virginia avenues; on others, nuisances growing out of insufficient drainage have been abated and improvements made. General Michler recommends that Pennsylvania avenue be repaved with some one of the more recent and improved pavements, either of stone or wood.

The estimated cost of carrying out these recommendations, which are ap-

proved, is \$713,931 88.

(See Appendix T, T 1, and T 2.)

WASHINGTON AQUEDUCT.

A detailed report upon the different works completed along the line of the Washington aqueduct is submitted by General Michler, with suggestions and recommendations for the completion of the unfinished parts of the aqueduct, at an estimated cost of \$712,838.

The recommendations are approved.

(See Appendix T and T 3.)

Survey of the North and Northwest lakes.

Officer in charge, Lieutenant Colonel and Brevet Brigadier General W. F. Raynolds, corps of engineers, assisted by Captain and Brevet Lieutenant Colonel F. U. Farquhar and Lieutenants M. R. Brown, J. F. Gregory, J. Mercur, and B. D. Greene, corps of engineers. One steamer and two shore parties were engaged during the summer of 1866 on Lake Michigan, carrying the surveys of both shores to the southward, and in executing the primary triangulation, off-shore sounding, &c.

The work was carried on the west side to a point near Two Rivers, Wisconsin,

and on the east side of the lake to Little Point au Sable, giving a connected survey from these points to the foot of Lake Huron and head of Green bay.

Two steamers and two shore parties were engaged in Lake Superior in connecting previous surveys in Keweenaw bay and at Marquette, in making reconnoissance for primary triangulation over the entire lake, and in making a hydro-

graphical survey from Keweenaw Point to Grand island.

Three astronomical parties were engaged during the first of the season in Lake Superior, and subsequently in Lake Michigan, in determining the latitudes of points by the aid of the differential zenith telescopes, and differences of longitude by means of instantaneous signals, and also in reading the angles of primary triangles at some of the points occupied. Seven (7) points in Lake Superior and eight (8) in Lake Michigan were occupied.

In the months of April and May, 1867, parties on board three steamers were engaged in the survey of the St. Clair river, and the survey was completed

from Port Huron to include the greater portion of the delta.

The above work on Lakes Michigan and Superior was plotted during the past winter, and comprised twenty-six sheets of antiquarian paper, embracing 2,7254 square inches of topography and 2,290% square inches of hydrography. The St. Clair work has not yet been projected, the parties having gone into other

fields immediately upon its completion.

Five thousand four hundred and sixty-four lake survey charts were distributed during the year, showing an increase of 1,829 over the distribution of the previous year. One detail chart of the south end of Green bay, on a scale of one-one hundred and twenty thousandth, (1720000,) and one of the south end of Lake Michigan, including the Straits of Mackinac and Green bay, on a scale of one-four hundred thousandth, (2000000) have been completed for publication.

The amount required for the next fiscal year, including the amount withheld

from the estimate of last year by the terms of the act of appropriation approved

March 2, 1867, is \$242,000.

(See Appendix U.)

Reconnoissances and explorations.

An officer of engineers has been on duty in each of the military divisions of the Missouri and the Pacific, and in each of the military departments of the Missouri, of the Platte, and of Dakota, on the staff or subject to the orders of the division or department commanders. The chief duties of these officers have been reconnoissances and surveys, and the preparation of sketches and maps, and their distribution within the commands to which they were attached. No special reports of operations have been received from these officers, with the exception of Major and Brevet Lieutenant Colonel R. S. Williamson, who reports, in relation to the surveys and maps in the division of the Pacific, that a military reconnoissance was made of the country lying between Fort Churchill, Nevada, and Ruby City, Idaho, with the view of furnishing a more direct wagon route connecting these points.

From the notes of this reconnoissance, and a similar one of the year previous, a map has been made of that comparatively unknown region. A reduced copy of this has been published in a cheap form, by authority of the major general commanding the division of the Pacific. A map of the whole of California, Nevada, Oregon, and a portion of Idaho, in one sheet, and also a map of Ari-20na, are in progress, and are nearly completed. These maps will represent all

the known portions of these regions in much detail.

A topographical assistant has been sent by Colonel Williamson to Arizona, with instructions to accompany scouting parties and trains and gather such topographical information as can be obtained in that manner. Two other assistants have been sent with the party of the State geologist of California through the portion of the State of Nevada lying between the 37th and 38th parallels of latitude. From these very interesting results are expected, the

country being nearly unknown.

Colonel Williamson has been instructed to report upon the subject of organizing an exploring party for the Colorado river, commencing at the point where the former exploration terminated. A report in relation to such an exploration was made to the Secretary of War in May last, a copy of which is hereto

appended. (See Appendix V.)

In accordance with the third section of the act of Congress, approved March 2, 1867, authorizing the Secretary of War to direct a geological and topographical exploration to be made of the territory between the Rocky mountains and the Sierra Nevada mountains, including the route or routes of the Pacific railroad, instructions were prepared at these headquarters for the government of the geologist appointed to take charge of the explorations, (Mr. Clarence King.) which have in view the examination of the mountain ranges, rock formations, detrital plains, soils, saline and alkaline deposits, mines, coal deposits, minerals, ores, &c., and the collection of all data for detailed topographical maps, as well as maps of the mining districts, &c. The party was partly organized in New York, proceeded to California, where the outfit was completed, and then started for the field of operations in Nevada, where, at the date of the latest report, it was actively engaged in prosecuting its labors. The party will pass the winter in the field of exploration, and will resume its operations as early in the spring as the season will admit.

(See Appendix V 1.)

Much progress has been made in the recompilation of the map of the territory between the Mississippi river and the Pacific ocean. Valuable results of the labors of the northwest boundary commission were kindly furnished by the commissioner, Mr. Archibald Campbell, which have been used in the compilation; also, the surveys at the head-waters of the Yellowstone and Missouri rivers by Brevet Brigadier General W. F. Raynolds, corps of engineers, and additional information furnished by Brevet Major General G. K. Warren, corps of engineers, and astronomical positions by Brevet Brigadier General Comstock,

corps of engineers.

The engraving of the map has progressed with the compilation, and from time to time editions have been issued for the use of the troops occupying the country. A map of Kansas, Texas, and the Indian territory has within the last year been compiled and engraved, for the use of troops operating therein. The general map of the country west of the Mississippi river, exhibiting the military departments and posts, has been extensively distributed in answer to the demand for it. This map is based upon the map first referred to above. The positions occupied by troops and those abandoned have been verified in a great measure by Brevet Brigadier General Comstock and Brevet Colonel Merrill, corps of engineers.

The demand for maps has required the constant application of the employés of the office in their preparation, which have been multiplied either by photography or by engraving; of the former there have been printed 1,335 sheets,

and of the latter 8,708 sheets.

Maps of campaigns and battle-fields.

Two general maps—one illustrating the campaigns of Lieutenant General Sherman, the other the campaigns of Major General Thomas—are in the hands of the engravers, and are nearly finished. Much progress has been made in the preparation of the detailed maps intended to illustrate the principal campaigns of the latter part of the war. They should be published, not only for the explanation of the official reports upon the great operations of the war, but for the valuable information they afford for military and civil uses.

Major and Brevet Brigadier General N. Michler has charge of the preparation of these, fifty in number, connected with the operations in Virginia. These maps are substantially finished. Colonel Edward Ruger, late of the volunteers, has charge of the preparation of the maps exhibiting the operations of the western armies.

An appropriation of fifty thousand dollars for engraving and printing the most important of these maps is recommended to be made. The amount on hand of the appropriation for surveys for military defences, &c., is sufficient for the present and next fiscal year, a considerable balance having been left on hand at the close of the war.

In the labors of this office during the fiscal year I have been assisted by the following officers, charged with the direction of the four divisions among which

its duties are distributed:

Fortifications—Brevet Major General Q. A. Gillmore, Brevet Major General H. G. Wright, Brevet Colonel J. D. Kurtz.

Armament, personnel, orders, &c.—Brevet Colonel J. D. Kurtz.

River and harbor improvements and surveys, survey of the lakes, military and geographical surveys and explorations—Brevet Brigadier General I. C. Woodruff.

Finances, accounts, estimates, lands—Brevet Lieutenant Colonel W. P. Craighill.

On special duty—Brevet Major W. R. King.

Very respectfully, your obedient servant,

A. A. HUMPHREYS,

Brigadier General of Engineers, Commanding.

General U. S. GRANT.

Commanding Armies of the United States, Secretary of War ad interim.

APPENDIX A.

Extracts from the annual report of Brevet Colonel J. B. Wheeler, major of engineers, upon the lake harbor improvements under his direction.

The assistants in charge of the harbors are required to visit those under their charge at least once during each month, and oftener if necessary. They are required to make inspection of work and materials offered by contractors, and see that the plan proposed for the improvement of the harbor is strictly complied with. To assist them in these duties a foreman is employed at each harbor, who keeps a daily record and reports weekly to each inspector the quantity of materials received, accepted, rejected, and the amount of labor performed. These reports are duly entered upon books of record, and kept on file in this office.

Payments are made from time to time, generally once a month, to contractors for the materials furnished or work done, provided satisfactory progress is shown. Each voucher before payment passes through the hands of the assistant, who makes a critical examination as to the quantities, and compares them with his record book. All vouchers are paid by myself, as the regulations require the

senior engineer officer to be the disbursing officer.

In concluding this my annual report for 1867, I would say a few words concerning the general plan of improvement adopted and the character of the estimates made. The plan of improvement consists in a system of jettees or piers, extending into deep water, confining the volume of river water emptying into the lake. This plan meets with the approbation of all persons. Various opinions,

however, are expressed as to the composition, construction, and direction of the piers. Again, discussions arise as to the length of the piers—which should be

longer of the two, or whether they both be made of equal length.

My opinions are decided on these points, as far as the general class of improvements are concerned. The plan of piers composed of cribs with grillage bottoms, conforming to the general principles laid down by the board of engineers, is the one adopted by me as fulfilling all the purposes required, and possessing superior excellence over the other plans proposed. The direction of the piers and their respective lengths admit of discussion. I am in favor of the piers being parallel and equal in length. Their direction should be a prolongation of the last course of the river before meeting the obstacles thrown up by the lake, avoiding particularly all angles or deflections of the course as the stream enters the lake. The general use of steam power renders this more practicable than in former days, when sailing vessels only were used, and great stress was laid upon wide entrances and weather piers.

The estimates offered are generally based upon the prices paid for materials and labor at the nearest harbor where improvements are made, and are as close as could be made. Probably materials could be furnished more cheaply and labor would cost less at some of the points named. I have thought it best to be safe in the estimates, by comparing the cost at other harbors where the actual experiment had been made of letting the work by contract, and basing the esti-

mates on these prices.

Accompanying this report are the following maps and tracings, made up in a separate package, viz:

1. Map of Racine reef, on scale of 505.

Tracing of Racine reef, on scale of \$\frac{1}{50\cdot\text{\$\sigma}}\$\text{\$\text{\$\sigma}\$}\$.
 Tracing of Racine reef, on scale of \$\frac{1}{36\cdot\text{\$\sigma}}\$\text{\$\text{\$\sigma}\$}\$.

4. Tracing of entrance to harbor of Racine on scale of 3600.

5. Tracing of harbor of Saugatuck, Kalamazoo river, on scale of 4800.
 6. Tracing of mouth of Menomenee river, Green bay, on scale of 4800.

7. Tracing of entrance to harbor of Milwaukee, Wisconsin, on scale of 3 600.

8. Plan of crib 32' × 20' × 17'.

The army regulations require that "an engineer superintending a work or operation shall disburse the money for the same," &c. This regulation compels me to disburse money from twenty-four (24) different appropriations, and to render monthly twenty-four (24) sets of accounts. Under the present law and regulations the number of papers required are many, and the greater part of my time is occupied in seeing these papers properly prepared and signing them. Can there be no amelioration made in the law or regulations that would reduce this labor? A return to the quarterly system of rendering accounts, as practiced before 1861, would reduce the labor and the expense very materially. If this be not practicable, a division of the labor among the assistants would lighten the weight of the onerous burden now resting upon me.

Attached, marked A, is an abstract of all contracts for the harbors under my

charge, with names of contractors and prices paid.

Also, marked B, is an abstract of the harbors, giving their locality, amounts

required for completion, amounts for next fiscal year, &c.

It is well to state that the word "completion" is relative, meaning, in this case, the completion of the present plan. From the very nature of things, and

this kind of work, a permanent completion cannot be expected.

I wish to return thanks to the headquarters corps of engineers

I wish to return thanks to the headquarters corps of engineers for the promptness that has ever been shown in replying to all my requisitions and requests. I wish to acknowledge the kindness that Brevet Brigadier General W. F. Raynolds, corps of engineers, superintendent of lake survey, has shown me in furnishing maps, tracings, and information upon my application. My assistants, as named on page three of this report, have been prompt, active, and efficient, and I am much indebted to them for their services.

APPENDIX A.

Abstract of contracts entered into by Major and Brevet Colonel J. B. Wheeler, corps of engineers, for the several works of harbor improvement in his charge.

Harbor.	Name of contractor.	Material or labor contracted for.	Price.
Superior City, Wis	Peter White	Iron boltsper lb	\$0 05
D∞	do	Spikeper lb	05
Do	Coburn & Ray	Framing per lin. ft. Twelve-inch pine timber per lin. ft.	20
Do		Pilesper lin. ft	20 15
		Stoneper cord	11 00
Do	do	Brushper cord	3 00
ntonagon, Mich	8. S. Vaughn	Stone per cub. vd	3 75
Do	ldo	Stone	6 00
Do	Peter White	Iron boltsper lb	05
Do	do	Spikes	05
<u>D</u> o	Gelley & Weston	Twelve-inch pine timber per lin. ft	15 7-10
Do	do	Plank	18 00
Do	do	Pilesper lin. ft.	08 16 7–10
Do	David Quinn	Framing	58 00
Larquette, Mich	William Ferguson	Twelve-inch pine timber per cut yu.	14
Do		Stoneper cub. yd	1 25
	do	Brushper cub. yd	1 25
Do	do	Plank ner M (h m)	16 00
Do	Peter White	Iron boltsper lb	04∰
Do	do	Iron bolts. per lb. Spikes. per lb. Framing. per lin. ft.	04€
Do	Gelley & Weston	Framingper lin. ft	10 7-10
reen Bay, Wis	William S. Smith	Dredgingper cub. yd	50 15
Canitowoc, Wis	David Smokedo	Twelve-inch pine timber per lin. ft Twelve-inch bemlock timber per lin. ft	14
Do		Stoneper cord	9 50
Do	do	Brush per cord.	2 00
Do	do	Framingper lin. ft	15
Do	do	Driving pileseach	3 00
Do	do	Driving pileseach Placing and sinking cribs	90 00
Do	Hoes & Packard	Oak plankper M, (b. m.)	25 00
	. do	Piue plankper M, (b. m.)	18 00
Do	do	Seantling per M, (b. m.)	18 00
Do	John Schutte	Dredging per cub. yd	20
Do		Iron bolts	071 12
heboygan, Wis	Dillingham & Co	Twelve-inch oak timber per lin. ft	27
Do	do	Twelve-inch pine timberper lin. ft	24
Do	. do	Plank and scantlingper M, (b. m.)	25 00
Do	do	Piles, white oak	16
Do		Stoneper cord	10 00
Do	Sanger, Ledlie & Corse.	Brush per cord. Iron bolts per lb	6 00
Do	dodododo	Iron boltsper lb	10
Do	do	Spikesper lb	10
Do	do	Framing	19 49 00
Do	do	Dredgingper cub. yd	49
Do		Driving niles ner lin ft	19
Gwankee, Wis	J. Kuhlman	Driving piles per lin. ft. Stone in cribs per cub. yd.	2 70
	1866.	,	
Lacine, Wis	Sanger, Ledlie & Corse.	Piles, white oakper lin. ft	19
Ďo	do	Plank, three-inch per M. (b. m.)	30 00
Do	do	Seantling three-inch	30 00
Do	do	Twelve-inch timber	24
Do	do	Iron bolts and spikesper lb	10
Do		Brushper cord	6 00
Do	dodo	Stoneper cord	13 89
Do	do	Placing cribseach.	39 00
De	do	Driving pilesper lin. ft	19 16
	do	Framingper lin. ft Dredgingper cub. yd	49
		21ougus	
CONTRACT OF 186	7REPAIRS TO BE PAID	FOR AT RATES FIXED BY ENGINEER OFFICER IN	CHARGE.
Racine, Wis	P. M Danaher	Twelve-inch timber per M, (b. m.).	\$14.97
Do	do	Three-inch plank per M. (b. m.)	14 97
Do	. do.,	Three-inch plank	134
Do		. Stoneper cub. vd	2 69
Do	do	Brush per cord	5 40
Do	. R. Nelson Gere	. Iron boltsper lb	05
Do	.ldo	. Spikesper lb	07

Abstract of contracts—Continued.

CONTRACT OF 1867, ETC.—Continued.

Harbor.	Name of contractor.	Material or labor contracted for.	Price.
Cenosha, Wis	Sanger, Ledlie & Corse	Pilesper lin. ft	\$0 19
Do	do	Three-inch plank, oakper M, (b. m.). Three-inch plank, pineper M, (b. m.).	35 00
Do	ldo	Three-inch plank, pineper M. (b. m.)	30 00
Do	do	Three inch scantling ner M (h m)	30 00
Do	dodododododo	Three-inch scantling per M, (b. m.). Twelve-inch timber, pine per lin. ft. Twelve-inch timber, oak per lin. ft.	29
Do	do	Twelve inch timber oak ner lin ft	39
Do	do	Your below and anilose	10
Do	do	Iron bolts and spikesper lb	5 00
100	. qo	Brush in cribs per cord. Stene per cord.	
ро	do	Steneper cord	13 87
	. do	Driving pilesper lin. ft	13
	do	Framingper lin. ft	16
Do	do	Placing cribseach	39 00
Do	Caleb H. Parker	For dredging between the piers and for founda- tions for cribs	37
Ъо	do	Dredging on bars or points outside of proposed extension per cub. yd.	74
hicago, Ill	John M. Corse	Twelve-inch timberper lin. ft	28
The Transfer of Tr		Twelve-lock dinber	31 75
50	do	Three-inch plankper M, (b. m.)	
		Brushper cord	5 74
₽•	do	Stoneper cord	8 74
Do	do	Stoneper cord Delivering and driving white oak pileseach	11 98
Do	do	Framing and sinking cribsper lin. ft	111
Do	do	Tuon holds non-th	l nni
Do.	do	Snikes now th	14
Do	do	Drudging inside of pier ner sub ad	44
Do		Desdring antide of ples	74
ichigan City	F. A. Slater	The clark timber	17
acmigan City		weive-inch umberper lin. R.	14
Do	do	Bylkes per lb. Dredging inside of pier per cub. yd. Dredging outside of pier per cub. yd. Twelve-inch timber per lin. ft. Three-inch plank per lin. ft.	
<u>D</u> o	do	Framing per lin. ft. Iron bolts per lb. Spikes per lb. Stone per cub. yd.	. 10
Do	Chapin & Wells	Iron boltsper lb	047
Do	do	Spikesper lb	06
Do	J. D. Dolan	Stoneper cub. vd	2 624
Do	do	Rrush ner-enh vd	75
ew Buffalo	Carkin & Kimball	Twelve-inch timber per M (b. m.)	15 00
Do	do	Twelve-inch timber per M. (b. m.). Three-inch plank per M. (b. m.).	16 00
Do	do	Pilesper lin. ft .	08
Do	do	Stone per cord	
		Stoneper cord	15 75
<u>n</u> o	do	Brushper cord	2 90
Do	do	Framingper lin. ft	061
Do	do	Driving pilesper lin. ft	061
Do	do	Dredg'g and excav'g in ord'ry mat'al. per cub. yd	34
Do	do	Dredg'g and excav'g in ord'ry mat'al. per cub. yd Dredg'g and excav'g in hard mat'al. per cub. yd	70
Do	R. Nelson Gere	Iron boltsper lb	04.9
Do	do	Iron bolts per lb. Iron spikes per lb.	04.9
. Joseph	Hasbrouck & Copro	Oak piles each Twelve-inch timber per lin, ft.	16 00
Do	do	Twolve-inch timber ner lin fi	35
Do	do	Brushper cord	3 00
Do	do	Other state of the	
ъ	ao	Stoneper cord	15 00
Do	do	Iron holtsper lb	10
Do	do	Iron bolts with screw and nutper lb	18
outh Haven	Galen Eastman	Twelve-inch timberper M, (b. m.)	13 25
Do	do	mb tbb	15 00
Do	do	Piles per lin. ft.	08
Do	do	Stone	14 37
Do	R. Nelson Gere	Piles	2 00
Do	R. Nelson Gara	Iron boltsper lb	04.9
Do	do	Snikes ner ih	04 9
	George Hannahs	Wasning II. A.	141
Do	George namans	Framing per lin. ft. Twelve-inch timber per lin. ft. Three-inch plank per M, (b. m.)	
ack Lake	John Roost	I weive-inen timberper lin. ft	20
<u>D</u> o	do	Inree-inch plankper M, (b. m.)	25 00
Do	S. N. Kimball	Sink pieces, placed per sq. yd. Stone per cord. Brush per cord. Iron bolts 14-inch square per ib. Iron spikes, 6-inch per ib.	3 70
Do	8. N. Kimball	Stoneper cord	13 40
Do	do	Brush per cord	3 00
Do	James H. Ledlie	Iron bolts 14-inch square	06 9
Do	do	Iron spikes 6-inch	15
Do	James E. Miller	Framingper lin. ft. of timber	09
Do	James E. Millerdo	William seibs with stone	80
		Filling cribs with stoneper cord	341
Do	do	Dredging in ordinary materialper cub. yd	
Do	do	Dredging in hard pan or slip clayper cub. yd	69
rand Haven	James H. Ledlle	Twelve-inch timberper lin. ft	29
Do	do	Three-inch plankper M, (b. m.)	25 00
	do	Pine piles	08
	do	Twelve-inch timber per lin. ft. Three-inch plank per M. (b. m.) Pine piles per lin. ft. Oak piles per lin. ft.	17
	do	Nione per cord i	16 94
	do	Remah	2 20
Do	do	Brush	~ 09
		ALVE VVIIS	
Do	do	lean anilyse	14

Abstract of contracts—Continued.

CONTRACT OF 1867, ETC.—Continued.

		1	
rand Haven	James H. Ledlie	Driving pilesper lin. ft	\$0 074
Do	R. A. Conolly	Twelve-inch timberper lin. ft. of timber	24 00
	do	Pine pilesper lin. ft	10
	do	Stoneper cord.	18 75
	dodo		
		Brushper cord	1 75
Do	do	Iron bolts and spikesper lb	09
	do	Driving pileseach	1 75
	do	Framingper lin. ft. of timber	09
uskegon	Galen Eastman	Twelve-inch timberper M, (b. m.)	12 83
	do	Three-inch plankper M, (b. m.)	14 00
	do	Piles per ft.	08
	Ledlie & Corse	Stone per cord	13 88
Do		Brushper cord	
			2 00
	R. Nelson Gere	Iron bolts and spikesper lb	049-
Do	Heber Squier	Framing per lin. ft. of timber	12
/hite River	Thomas L. White	Twelve-inch timber per lin. ft. of timber	14 1-1
Do	do	Three-inch plank per M, (b. m.)	14 50
Do	do	Pilesper lin. ft	07
	Carkin & Kimball	Framing per lin. ft.	07
	do	Driving pilesper lin. ft	054
	do	Driving puesper im. it	
		Stoneper cord	14 90
Do	do	Brushper cord	1 42
Do	R. Nelson Gere	Iron bolts and spikesper lb	04 9-
Do	Fox & Howard	Dredgingper cub. yd	28
entwater	P. M. Danaher	Twelve-inch timberper lin. ft	12
	do	Three-inch plankper M, (b. m.)	12 00
	do	Piles per lin. ft.	08
D	F. D. Van Wagener		
ъ	F. D. Van Wagener	Stoneper cord	16 00
ро	do	Brushper cord	6 00
	R. Nelson Gere	Iron bolts and spikesper lb	04 9-1
Do	Hasbrouck & Conro	Framingper lin. ft.	14
Do	do	Dredgingper cub. yd	36
	P. M. Danaber	Twelve-inch timberper lin. ft	124
	do	Three-inch plankper M, (b. m)	18 00
	do	Diles	
Do		Pilesper lin. ft	10 08
		Stoneper cord	13 98
Do	R. Nelson Gere	Brushper cord	1 49
Do	R. Nelson Gere	Iron bolts and spikesper lb	04 9-1
Do	Hasbrouck & Conro	Framingper lin. ft	14
Do	do	Dredgingper cub. yd	36
anistee	Gellev & Weston	Twelve-inch timberper M.	12 90
Do	dency at wearon	Three-inch plankper M.	14 00
Do	do		
		Pilesper lin. ft	08
Do	Galen Eastman	Stoneper cord.	14 88
Do		Brushper cord	2 00
Do	R. Nelson Gere	Iron bolts and spikesper lb	04 9-1
Do	H. Starke	Framingper lin. ft. of timber	15
ux Bec Scies	Whitwood & Hubbell	Twelve-inch timber per lin, ft	18
	do	Three-inch plank, pineper M, (b. m.)	20 00
	do	Diles (20 feet lengt) selv	5 75
10		Piles, (30 feet long,) oakeach	
D0	do	Piles, (30 feet long,) elmeach	2 75
no	do	Stoneper cord	17 00
Do	do	Brushper cord	4 50
Do	do	Slabsper cord	2 50
Do	do	Iron bolts, 14-inchper lb	06
I)o	do	Iron spikes, 6-inchper lb	071
Do	do		6 00
Du		Driving pileseach	
ñο	do		168 00
Do	do	Placing, sinking, and fillingeuch	30 00
Do	do	Dredging in common earth, sand, or soft	
		clay per cub, yd	35
Do	do	Dredging in hard clay and hard pan. per cub. yd	70

A 1.

Ontonagon, Michigan, August 31, 1867.

SIR: In obedience to your instructions, embraced in the circular of July 20, 1867, I present a general report of the harbor improvements under my charge, which include Superior City, Ontonagon, and Eagle harbor.

I am not prepared to report on Eagle harbor. The first notice of its assignment to my care was received at Milwaukee July 31, and my time has been fully occupied at the other points since my arrival on Lake Superior. I will at

an early day examine that harbor, and present a separate report.

We will first refer to the harbor of Superior City. This will be to some extent a repetition of my communication or report of July 26, 1867, on the harbor of Superior City. This harbor is at the western extremity of Lake Superior, and, in natural advantages of location, it is to Lake Superior and the country south and west and northwest what the harbor of Chicago is to Lake Michigan and the regions in the same directions. As a depot for the reception and forwarding of wheat from places of production, the bay of Superior may eventually become the greatest in the world. The distance from Superior City by lake and canal navigation to tide water is about the same as from Chicago. The commerce of Superior City is very small at present, as its communications have never been opened with the country from the south to the west and northwest, which will become tributary.

Five different railways, all having valuable land grants, are expected to terminate at Superior City or on the bay of Superior, viz: The Northern Pacific, the Mississippi and Lake Superior, (from St. Paul's,) the Lake Superior branch of the St. Paul's and Pacific, the St. Croix and Lake Superior, and the Portage and Superior. Hence the harbor of Superior seems by far to be the most important on the lake, and one of the most important in all the great chain of lakes.

The improvement should be begun and carried forward in the most substantial The bay of Superior is the second widening of the St Louis river. the first being the St. Louis bay above. Both of these bays seem to have been, at periods not very remote, the proper termini of the lake, the points of land which separate the two bays having first been formed by the united action of the lake and St. Louis river, and afterwards the Minnesota Point was formed by the same agencies, beginning at the Minnesota bluffs and extending gradually to its present length, which has been increased within the last six years. similar process has been going forward in the formation of Wisconsin Point and bay of Allowes, in the connection with the Nemadjii river and other streams emptying into the bay of Allowes, all concentrated and flowing through a circuitous channel from the bay of Superior into Lake Superior, between Minnesota and Wisconsin Points. The St. Louis river probably discharges at least five times the quantity of water as the Nemadjii, but as it spreads over the large bay of St. Louis and Superior, the proportionate amount of sediment carried into the lake by the Nemadjii is very greatly increased, especially in times of freshets such as came under my observation from the 18th to 24th of July, 1867. plan which has been proposed for improving the harbor is to extend a pier of cribs from Wisconsin Point straight into the lake, in a nearly northeast direction, skirting the easterly side of the channel for some distance, then across the bar to sixteen feet depth of water in the lake, a distance of about two thousand feet. On the other side of the channel it is proposed to place loose rock along the inside of the Minnesota Point, (or rather the extension of the same, which is rapidly forming,) for a distance of about one thousand feet, and then begin the crib-work and gradually contract the channel to three hundred feet in width. This plan will require about seven hundred cords of loose rock or filling, and about forty-three hundred feet length of cribs, and about forty-two thousand yards dredging. The estimates of this work are annexed.

The plan which I have recommended in a former communication is to restore a former channel of the St. Louis river by cutting across Minnesota Point about one and one-fourth mile northwest of the light-house, thus avoiding the circuitous channel of the present natural entrance and the sediments of the Nemadjii river and a portion of the St. Louis, and the accumulations of sediment which extend nearly half a mile into the lake.

At the proposed point the land is narrow and low, being about two hundred feet from the lake to the bay shore, and eighteen feet of water within six hundred feet of lake shore, and eleven feet within the same distance from the shore of the bay. I would recommend an artificial channel not exceeding two hundred feet in width, which should not be opened for the passage of water till the crib-work is as nearly as possible completed. The sides of the artificial channel should be cribbed to a depth of at least twelve feet below the surface of the water; and when finished a part of the ordinary current of the St. Louis river would pass through it, on the principle that water will seek its level on the shortest distance, and thus keep the channel clear till the further extension of the cribs into the lake should be required. The effect of this might be to diminish the depth of water in the present channel, as it would divide the current with it, but it would not close up, as it would still remain the most natural outlet of the Nemadjii, and the proposed cut would be insufficient for the St. Louis in time of freshets. The amount of work required for the full improvement of the harbor would be less on the artificial channel, and the facility of doing it would be much greater than at the present entrance. There would be no current to contend with, and more than half the work could be done in the protection of the bay. For these reasons the same amount of work at the cut can be done at much less actual cost. The effect of projecting piers into the lake at either place may be to cause an abrasion on the lake shore northwest of the pier, which may in such case be remedied by small jettees into the lake at those points. difference of level of the water of the bay and lake at the light-house on the 22d of July last, two days after the freshet began to subside, was seventy-six onehundredths of a foot. This caused a rapid current through the channel.

By reference to the chart the advantages of the artificial channel are so obvious that the subject needs no further comment. A good harbor can be made by improving the present entrance; but a better one can be secured by the cut, at a less original cost and less for future maintenance; but it is possible that private interest might be so effected or invaded as to make it most expedient to maintain the natural channel. I do not fully understand the law and equity of this subject. It has been intimated that the owners of property on the bay near the Nemadjii river claim to foresee damage to their property by changing the natural outlet of the St. Louis river; but if the public advantages are very great and obvious, it seems as though such private claims could hardly be maintained either in law or equity. The work already done in the improvement of the harbor and materials delivered are as follows:

3742 cords of stone deposited along the inside the Minnesota Point,

at \$11 per cord	\$4,119 50 1,010 00
Total	5, 129 50

It is expected that the full amount of the present appropriation can be profitably expended during the present fiscal year. During the next fiscal year one hundred and twenty thousand dollars (\$120,000) can be profitably expended.

The following are the estimates of the cost of improving the harbor on each of the different plans, and annexed thereto are the abstracts of contracts and tables of commerce, &c. Superior City is in the collection district of Michili-

mackinac. The nearest port of entry is Bayfield; the nearest light-house is at Raspberry island, which is a flash light. There were no import duties collected at Bayfield during the last year. There were thirty-four arrivals and thirty-three departures of vessels to and from Superior City during the year ending June 30, 1867.

Abstract of contracts for improvement of the harbor of Superior City.

Names of contractors.	1st class, timber.			2d class, iron.		3d class, filling.		4th class.	5th class.
	Pine per lineal foot.	Plank per M., b. m.	Piles per lineal fcot.	Bolts per lb.	Spikes per lb.	Stone per cord.	Brush p'r cord.	Framing per lineal foot.	Dredging per cubic yard.
R. G. Coburn	Cents. 21	\$16	Cents.	Cents.	Cents.	\$11	\$3	Cents. 20	Cents.

Cost of one crib 32'×20'×17': 2, 368 feet, (lineal,) 12×12 timber in the work, at 44 cents 288 feet b. in. plank in the work, at \$25. 56 cords stone filling, at \$11 7 cords brush filling, at \$3 3 piles driven in work, say 3, 560 pounds iron bolts and spikes, at 5 cents	\$1,041 92 7 20 616 00 21 00 23 88 178 00
288 feet b. in. plank in the work, at \$25	7 20 616 00 21 00 23 88
288 feet b. in. plank in the work, at \$25	7 20 616 00 21 00 23 88
56 cords stone filling, at \$11 7 cords brush filling, at \$3	21 00 23 88
3 piles driven in work, say	23 88
3, 560 pounds iron bolts and spikes, at 5 cents	
_	178 00
. ————————————————————————————————————	
Total	1,888 00
Cost per lineal foot, \$59.	
4, 300 lineal feet of crib piers, at \$59, averaging 17 feet in depth	.co 200 00
and 20 feet wide	253, 700 00
42, 000 cubic yards dredging, at 48 cents	20, 160 00 7, 700 00
	28, 156 00
Add ten per cent. for superintendent contingencies	20, 100 00
Total 36	09,716 00

exclusive of land damages.

3,000 lineal feet of crib piers, averaging 17 feet in depth and		
20 feet wide, at \$59 per foot		00
110,000 cubic yards dredging and excavating, at 48 cents		00
Add ten per cent. for superintendent contingencies	22, 980	00

There were no foreign exports or imports at Superior during the year ending June 30, 1867.

DOMESTIC EXPOR	rs.	DOMESTIC IMPORTS.	
Lumber, feet, b. m. Shingles, feet Laths Fish, half barrels Cattle Furs, bales Merchandise, tons	255, 000 500, 000 1, 478 170 13	Flour, barrels Pork, barrels Corn meal, barrels Salt, barrels Corn, bushels Oats, bushels Merchandise, tons	194 273 352 2, 762 5, 352

Respectfully submitted:

HENRY BACON, Assistant.

Colonel J. B. WHEELER,

Major of Engineers, U. S. Army.

A 2.

The harbor of Ontonagon is situated at about an equal distance between the natural harbors of the Apostle islands and Copper Harbor, it being about one hundred and sixty miles from Bayfield to Copper Harbor, with no other intermediate harbor excepting Eagle Harbor, fourteen miles from Copper Harbor, which, till improved, is very difficult of access. There are no other intermediate places where artificial harbors can be made to advantage. If there were no trade or commerce, present or prospective, at Ontonagon, it would be very necessary to improve the harbor as a harbor of refuge.

Lake Superior, like the other great lakes, is subject, at certain seasons, to sudden and violent storms and gales of wind, which make harbors of refuge

not only desirable but essential to the safety of navigation.

It is very apparent that one is a necessity between Copper Harbor and the Apostle islands. The Ontonagon river, from its mouth about one mile, has a minimum depth of ten (10) feet, and eleven (11) feet to the dock. It has a minimum depth of six (6) feet for about six miles further up.

Ontonagon is the commercial centre of the best agricultural region of Lake Superior, though undeveloped, and also of one of the best mining regions,

which, also, is partially developed.

The modern deposits of the Ontonagon river have extended about half a mile into the lake. Through this bar there is, at present, a channel somewhat circuitous, having about eleven feet minimum depth of water. This channel is variable, and has been, for several years before 1866, closed, so that vessels drawing more than seven feet of water could not enter during the years 1863, 1864, and 1865

The plan for improvement adopted is to extend two parallel piers of cribs in a direction nearly northwest, two hundred and fifty feet apart, and each

about two thousand five hundred (2,500) feet into the lake.

The east pier will cross the present channel and cut across the bar for a distance of about one thousand feet, to eighteen feet of water into the lake. The east pier should be extended to the channel in the first place, and then the work should begin at the west pier; and it may be a question whether it will be best to extend the west pier to its full length, and then extend the east pier, leaving open a space for the channel to be closed last, after a channel is dredged through the bar between the cribs, leaving the spring and summer freshets to finish the channel. This position of piers involves more expense

than a more northerly direction, as it requires more length and more dredging; but it is undoubtedly a more proper direction for permanence, as it carries the works beyond the extreme point of the bar formed by the river deposits, and is the best protection against the most prevailing winds, which are from the northeast.

The Ontonagon river, for several miles from its mouth, is a sluggish stream; but it is subject to heavy freshets in spring and summer, which bring down large amounts of drift-wood and deposits of sediment, both of which have helped to obstruct the entrance; and the same causes may, after a few years, make it necessary to extend the piers still further into the lake. If the work is completed according to the plan, in a proper manner, it will make Ontonagon a safe and acceptable harbor at all times during the navigable seasons.

A small amount of dredging needs to be done inside of the shore line to the docks, which properly belongs to private or local enterprise. There is no work

done, in place, on the improvement of the harbor.

The materials delivered are as follows:	
20,200 lbs. of iron bolts, at 5 cents	\$1,010 00
1,500 cubic yards of stone for filling, at \$3 75	5 , 625 00
1,200 lineal feet of timber, at 15 7 cents	188 40
Total	6, 823 40

It is expected that work to the amount of the present appropriation can be done during the present fiscal year.

During the next fiscal year, one hundred and twenty-five thousand dollars

(\$125,000) can be profitably expended.

The revenue collected at the port of Ontonagon during the last fiscal year was \$232 68; of which only \$5 50 was from duties. The number of arrivals of vessels at the port was 224; departures, 225.

With these remarks I submit the following estimates, with abstracts of the

contracts and tables of domestic exports and imports.

The estimates for Ontonagon and Superior are based upon the prices of the present contracts, and are for a thorough execution of the plans, which will make our work secure and permanent; requiring few repairs excepting the natural decay of the timber above the surface of the water.

Very respectfully, your obedient servant,

HENRY BACON, Assistant.

Abstract of contracts for the improvement of the harbor of Ontonagon.

		lst class	-Timber.		2d class	—Iron.	3d class-	-Filling.	ram g	rdg'g.
Names of contract's.	Pine, per lineal foot.	Oak, per M. (b. m.)	Plank, per M. (b. m.)	Piles, per lines foot.	Bolts, per pound.	Spikes, per pound.	Stone, per cubic yard.	Brush, per cord.	4th class—Fr Per lineal f	5th class Dr Per cubic y
Gelly & Weston	Cents.		\$ 18 0 0	Cents.	Cents.	Cents.			Cents.	Cents.
S. S. Vaughn Peter White			•••••		05		\$ 3 75	\$ 6 00		40
Peter waite			••••		L 05	05		' !		

ESTIMATE OF THE COST OF IMPROVING THE HARBOR OF ONTONAGON.

Determined to the cool of the koving the himbon of outfills	adom.	
Cost of one crib, 32'×20'×17': 2,368 lineal feet of timber frames done in work, at 32 ⁴ / ₁₀ 288 feet b. m. plank, at \$25 265½ yards filling stone, at \$3 75 7 cords filling brush, at \$6 3 piles and driving 3,560 pounds iron and spikes, at 5 cents Total	995 42 24 178	20 62 00 00 00
Or \$62 94 per lineal foot.		
ESTIMATE OF TOTAL COST.		
Add 10 per cent. for superintendence and contingencies 3	14, 700 16, 000 33, 070 53, 770	00 00
DOMESTIC EXPORTS.		=
From the port of Ontovagon during the year ending June 30, 1867	7 : Tor	
Copper, 1,500 tons. Hay, 100 tons. Fish, 1,000 half barrels. Leather, 50 tons. Potatoes, 2,000 bushels. Lumber, 125 M feet. Sundries, 75 tons. Potash, 20 tons. Total tons.	1, 5	500 75 50 60 75 20
DOMESTIC IMPORTS.		
To the port of Ontonagon during the year ending June 30, 1867:		ns.
Flour, 4,790 barrels Lime, 420 barrels Lard, 190 barrels Molasses, 44 barrels Oil, 123 barrels Pork, 278 barrels Sugar, 600 barrels Sait, 600 barrels	•••	526 53 30 11 19 42 84 90
Vinegar, 100 barrels Fuze, 60 barrels Hams, 384 casks Nails, 160 kegs Powder, 8,200 kegs	••	17 8 53 8 23

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	Toes.
Candles, 2,000 boxes	70
Corn, 2,500 bushels	70
Oats, 17,000 bushels	272
Corn-meal	
Sundries	
Total	3, 051
=	

A 3.

MILWAUKEE, WISCONSIN, August 31, 1867.

COLONEL: I have the honor to submit the subjoined report of Marquette har-

bor, Michigan.

This harbor is situated about 170 miles from the south, at that point where the line of the south shore of Lake Superior changes its general direction from northwest and southeast to east by north and west by south. The harbor is formed by an indentation in the shore-line of about a mile to the westward making a natural harbor secure from all winds except those from the east around to the northeast, and vessels at anchor in it are, in consequence, obliged to sail upon the slightest indication of a storm from these unsheltered directions. This harbor, therefore, needs the agency of art to render it perfectly secure.

The almost fabulous mineral richness of peninsular Michigan early attracted the attention of capitalists, who, in opening the mines, unearthed the talisman that dotted the shores with cities, filled the valleys with the busy hum of life, whitened the lake with the sail of vessels bearing to a market the products of the mines, caused prosperity to smile from the sterile rock, and spread the man-

tle of enterprise over a country wild and uninviting.

Marquette is a production of this genius of progress. In sailing up Lake Superior, upon emerging from the wild loneliness of the region below, one is filled with surprise and wonder at the first sight of so fine a town. The town is beautifully situated on a fine elevation that descends by gentle slopes to the shores of the bay. It has a population of about three thousand, which is rapidly increasing, and appearances indicate that about half of the buildings have been erected within two years, while still others are in process of construction.

The streets throughout the entire day resound with the hum of business, and in the evening, when the iron furnaces pour out their industrious swarm, they

have a most lively appearance.

Through this port pass the productions of the iron furnaces, mines, and exchange commodities, and, therefore, anything that may be done at this place for the protection of shipping, during the tempestuous weather, will exert a most beneficial influence upon not only the tributary country and attendant commerce alone, but upon every vessel that encounters the fierce storms upon Lake Superior, the largest of the American lakes.

Marquette is in railroad communication with Escanaba, and another road is in course of construction between Marquette and Ontonagon, being completed as far as Lake Michigan. The several mines upon the lines of these roads transport most of their products by them to Marquette, where they are transferred

to vessels and shipped for reduction and manufacture to points east.

In consideration of their vast extent, the iron mines of this country comprise, no doubt, the most wonderful mineral deposit in the world. Within the last fifteen years, millions of tons of ore have been excavated, representing fabulous wealth; yet scarcely any perceptible impression has been made upon the mass. The ore is very rich, yielding about 66 per cent. of pure iron. A number of furnaces are in operation in the vicinity of Marquette, which yield considerable

pig iron; others of still greater capacity are being built, and mills for its further

manipulation and manufacture are springing up on every side.

Perhaps a better idea of the mineral wealth of this country may be formed by an acquaintance with the quantity and valuation of iron and ore shipped up to the 10th of August, 1867, and estimated quantity to the close of navigation.

Receipts as above, tons ore, 166,497; pig iron, 15,198. Estimated receipts from August 10 to close of navigation, tons ore, 105,000; pig iron, 6,000. Total from Marquette for 1867, 171,497 tons ore, and pig iron, 21,198.

VALUATION.	
171,497 tons ore, at \$5	
21,198 tons charcoal pig iron, at \$40	847, 920
Total	1, 705, 405

To this must be added the value of iron and ore shipped on estimated quantity for 1867, which is \$1,165,000, or a grand total for the county of Marquette of \$3,370,405.

Appreciating the necessity of a good harbor at this point, Colonel W. F. Raynolds, United States engineers, made an examination of this harbor with a view to its improvement, and upon his estimates, \$85,000 were appropriated by the thirty-ninth Congress for this purpose.

The plan of improvement proposed is to build a breakwater some two hundred feet in length in front of the town, this breakwater to be composed of cribs filled with ballast stone, and built upon either of the lines indicated in the charts

accompanying Colonel Raynolds's report.

The charge of this harbor was not transferred to this department until late in the spring of this year, which left but little hope of accomplishing much this The work was advertised, however, and the bids opened and work

awarded on the 20th of June. (For an abstract of bids see appendix.)

The persons to whom the award was made, and the nature of their contracts. I have also appended. The point nearest the light-house of the two proposed was selected and work commenced as soon after the letting as possible, and has progressed satisfactorily, there being three cribs placed up to the present time, and without doubt, before the close of the season, one hundred and sixty lineal feet of this breakwater will be completed.

During the ensuing season I apprehend that five hundred and ten running feet may be added to this; this will cost, at present contract prices, \$43,150 32. It is proposed by the contractors for framing to continue the work during the winter as the timber is delivered; if this course is pursued, one hundred feet

more, costing \$22,490 16, may be added to the above.

That this amount can be most advantageously expended in the further extension of the work already begun, I think is without question, since the location is such that any expenditures for this purpose will be of incalculable benefit to Lake Superior commerce and navigation.

Very respectfully, your obedient servant,

JAMES B. QUINN, Lieutenant U.S. Engineers.

Major and Brevet Colonel J. B. WHEELER, Corps of Engineers.



Abstract of bids received and opened June 20, 1867, for improving harbor at Marquette, Michigan.

-		:	lst class.	lst class.—Timber.	ı.	2d class.—Iron.	-Iron.	3d class.—Filling.	-Filling.		,
No.	Name of bidder.	Kealdenoe.	Pine.	Plank.	Piles.	Bolts. Spikes.	Spikes.	Stone,	Brush.	fth cla	Remarks.
				Per M.	Cents.		Per 16.			Park f.	
- 8.	R. Nelson Gere William Smith and William Nichols		2 0 25	00 818	12	8 & \$7 \$	ည် (၁၈ ၀	99 00 per cord.	\$1 00 per c. y'd.		\$4 for each pile driven.
4 10	James Gayner Ledlie & Corfe	Milwan	22.22	19 90	61	= 8	, 1, 1	2 50 per c. y'd. 2 49	2 00 per c. y'd.	ಜಕ	No guarantes.
9 ~ 0	Wm. Ferguson. R. P. Hineman.	Marque Green	144 15 90 per M	988 988	18 88	42	~~	31. 31.	1 25 per c. y'd. 3 00 per cord.	28	
2000	A. K. Harlon Peter White	Marque do	1 61	8 8		#	#				,
3=2	Data & Digiow David Smoke Tarache & Harly	368	ន	3				88	3 00 per cord.	1 82	Trademod
122	George & James Freeman	do			Q .	7	a			;	Cumguett
122	S. S. Vangha. Gelley & Weston	Bayfield	17 9-10 28	25 88	10	.	. z	11 70 per cord. 8 00 per cord.	8 00 per co	rd. 10 7-10	
14	R. A. Connelly	Chicago	\$	80 22		Or	11	4 00 per c. y'd.	8	55	

Abstract of contracts for improving Marquette harbor, Michigan.

Contractors.	Nature of contracts.	Price.
William Ferguson	12-inch square pine timber per lineal foot Stone per cubic yard	\$0 14\frac{1}{4} 1 25 1 25 16 00 0 04\frac{7}{4} 0 10\frac{7}{4}

Abstract of materials received and used, labor performed, and amount paid to contractor up to September 1, 1867, at harbor of Marquette, Michigan.

Contractor: Peter White. Material received: 20,200 pounds of iron. Materials used: 4,296 feet pine timber; 17 cubic yards stone; 500 pounds iron. Labor performed: 4,296 feet framing. Amount paid to Peter White, contractor, for iron: \$984 75.

MILWAUKBE, WISCONSIN, August 31, 1867.

SIR: In obedience to your instructions, embodied in circular of 20th ultimo, I

have the honor to submit the following report upon Green bay, Wisconsiu:

This harbor is situated at the mouth of the Fox river, and takes its name from the bay into which the river empties. The towns of Green Bay and Fort Howard are situated upon it, and are rapidly increasing in wealth and population; being situated at the head of Green bay, and in direct communication with the interior of the State, both by railroad and water, the trade of the northern and northwestern portions of Wisconsin naturally accumulates at these points. harbor is deep and spacious, and protected from all violent winds and storms; and it is only necessary, therefore, to render this harbor accessible to all vessels from Green bay, in order to confer a great benefit upon the industry of the country and commerce of the lakes. With this object in view, the present course of improvement was proposed and adopted.

An appropriation for the improvement of this harbor was made in 1866, of \$30,500, and during the second session of the thirty-ninth Congress a further appropriation of \$15,500. Of this there has been expended up to the present time (see abstract appended) \$12,447 25 in dredging out the channel between Grass island and the mouth of Fox river, and in opening cut across Grass island. The work of improving this entrance was begun late in October, 1866, but owing to the inclemency of the weather and accidents to machinery of the dredge, very little was accomplished, the whole quantity of earth removed being only 1,298.5 cubic yards; work was renewed in June, 1867, on the south side of Grass island in seven feet of water, distant from the island about two hundred yards, and has progressed favorably, there having been removed, up to the present time, thirty-two thousand two hundred and ninety-four yards of earth, and it is probable that by the close of the working season a channel twelve feet deep and one hundred feet wide will be made, extending from the twelve feet water on the south side of Grass island, to the twelve feet channel on the north. Already a strong current begins to flow through the cut, and I apprehend considerable earth may be removed from the channel by the freshets

of the ensuing fall and spring. In order to observe the abrasive effect of these freshets, and the storms of winter, no protection for the sides of the channel has been provided. During the ensuing summer, however, the dredging will be sufficiently advanced to require this protection to be commenced, if not completed.

In my last report upon the improvement of this harbor (see Appendix to Chief of Engineers' report for the year 1866) I recommended a system of sheet piling for the protection of the sides of the channel, constructed in accordance with plans proposed by Colonel J. D. Graham, and costing, when completed, \$55,416 77. Upon more mature consideration I am convinced that although this plan possesses the properties of stringent economy in first construction, it does not possess sufficient stability to withstand the violence of tempests, the rush of water during the period of freshets, or the pressure of sand from the exterior for any length of time; and it is, therefore, a question whether this, from its frailty, must certainly be continually undergoing expensive repairs, would prove as economical, in fact, as a structure which, though costing more to build, possesses the quality of permanency. In view of the very great importance which exists of giving to the work already begun as great a degree of durability as possible, with the least possible expenditure, I am persuaded to recommend the substitution of a system of close piling for seven thousand two hundred and thirty-six feet of the sheet piling, and cribs, twenty feet wide, filled with stone, for the remaining three thousand six hundred and sixty-four feet; one crib to be placed at the end of each pier on the southern extremity, and the remainder to be placed upon the northern extremity. The close piling to be constructed by driving piles in juxtaposition next to the water-way; at a distance of twelve feet behind this, a second row, at distances apart of twelve feet; the two rows to be properly capped, bolted, clamped by string pieces, and connected by ties every twelve feet. This will cost (see estimate marked A) \$130,947 87; the cribs to be of the plan followed in the harbor improvements of Lake Michigan. The cost of the ninety-two cribs of this description used will be (see estimate B) \$184,579 60. Total for the entire protection, \$215,527 47.

During the ensuing year the whole of this close piling and four cribs at least may be built, and, provided no unforeseen circumstances occur to prevent, fifty thousand yards of earth be removed; therefore, \$161,000 may be profitably expended during the year 1868. Although this is a large sum, we cannot consider it as exorbitant in comparison with the immense benefit that must result

from the completion of this work.

In the present condition of the entrance a vessel can get out of Chicago harbor loaded with one hundred tons more freight than it can from this; and by reason of its tortuousness, it is entirely inaccessible by night, and dangerous at all times.

Notwithstanding the many disadvantages of difficult entrance to harbor and the almost total failure of the harvest during the past three years, the trade of Green Bay has steadily increased. Perhaps some idea may be obtained of this increase by comparing the number and tonnage of vessels arriving and departing last year, and those of this:

This year's arrivals, 677 vessels; tonnage	139,608
Last year's arrivals, 557 vessels; tounage	108, 608
This year's departures, 673 vessels; tonnage	140, 250
Last year's departures, 573 vessels; tonnage	107, 200

There are some fifty vessels sailing from this port this year, the tonnage of which I am unable to ascertain. Ten steamers ply between this port and points on Green bay, whose tonnage aggregates 3,468 tons; and a number engaged in the navigation of the Fox river, the tonnage of which amounts to upwards of 1,000 tons.

I append an abstract of some of the most important imports and exports, a camparison of which with that of last year shows a considerable increase.

Green bay is in the collection district of Milwaukee, and is the port of entry for this harbor. The amount of revenue collected during the past year was not ascertained. The nearest light-house is called Long-tail Point light. The

light is a fixed white light of the fourth order.

In contemplation of the easier access to Green bay that the present improvement will allow, when completed, vessel owners are calculating upon receiving the freight of Minnesota and northwestern Wisconsin entirely at this port, instead of receiving a large portion of it at Milwaukee and Chicago, as at present. In consequence of the crooked channel, and the difficulty and expense of navigating it, it costs from three cents to four cents per bushel more to ship wheat from this port to the same ports eastward.

Improvements are in progress upon the Green Bay and Mississippi canal that will soon place this port in excellent water communication with the Mississippi river; when this is completed Green Bay will become the principal shipping point for the surplus millions of bushels of wheat of Minnesota, Dakota, and northern Wisconsin; and, in return, of the machinery and tools of agriculture, goods, furniture, iron, nails, salt, and all the articles of merchandise needed in the country drained by the Fox and Wisconsin rivers, and that of the upper Mississippi, since freights from these places to Buffalo can be shipped by this route cheaper than by any route in operation or projected.

I have herein endeavored to present a few of the leading inducements for the continuation of the work of improvement already begun. If anything further concerning the position and character of the work is required, I respectfully

refer to my preceding report of November 29, 1866.

I am, sir, very respectfully, your obedient servant,

JAMES B. QUINN. Lieutenant United States Engineers.

Brevet Colonel J. B. WHERLER. Major Corps of Engineers.

A.—Estimate of cost of close piling necessary to protect the sides of the cut across Grass island.

For 235,350 lineal feet of white oak piles, at 18 cents per foot	\$4 2, 363	00
For 14,472 lineal feet of 12-inch square timber, for caps, at 20 cents per lineal foot.	2, 894	40
For 21,708 lineal feet of 6×12-inch timber, for stringers, at 10 cents per lineal foot	2, 170	80
For 7,891 lineal feet of 12-inch square timber, for ties, at 20 cents per lineal foot	1, 578	90
For 101,017 lbs. of wrought iron bolts, at 6 cents per lb	6,061	
For 19,666 cubic yards of brush or slabs, for filling, at \$1 25 per		
yard	24, 583	00
For 6,432 cubic yards of stone, for ballast, at \$3 per yard	19, 296	00
For framing 44,071 lineal feet of timber, at 10 cents per foot	4, 407	10
For driving 7,845 piles, at \$2 per pile	15, 690	
	119, 043	
Contingencies, at 10 per cent	11,904	35
Estimated cost of close piling	130, 947	87

B.—Estimated cost of one crib 20' × 32' × 17', constructed according to plan adopted by Major J. B. Wheeler, U. S. Engineers.

adopted by Major J. B. Wheeter, U.S. Engineers.		
For 2, 368 lineal feet of 12-inch square timber, at 20 cents per lineal		
foot	\$47 3	60
For 3,554 lbs. wrought iron bolts, at 6 cents per lb	213,	24
For $265\frac{1}{2}$ cubic yards of stone, at \$3	796	50
For 32 cubic yards of brush or slabs, at \$1 25 per yard	40	00
For 288' B. M. 3-inch planks, at \$20 per M	5	76
For 6 lbs. 6-inch wrought iron spikes, at 10 cents		60
For framing 2,368 lineal feet of timber, at 20 cents per lineal foot.	473	60
	2, 006	30
Contingencies, at 10 per cent	200	63
Estimated cost of one crib	2, 206	93
Estimated cost of 92 cribs	184, 579	60
Cost of close piling		
Estimated cost of protection	215, 527	47
-		

Abstract of imports and exports of Green Bay, Wisconsin, for the year ending July 31, 1867.

IMPORTS.

Articles.	Quantity.	Articles.	Quantity.
Tea, chests Tobacco, packages Fruit, packages Crackers, pounds Salt, pounds Barley malt, bushels Stoves Horse-rakes Nails, kegs	400 600 1, 045 505 11, 505 1, 500 2, 835 264 4, 064	Oil, pounds	2, 307 1, 876 700 400 300 500 50 7, 500

EXPORTS.

Flour, barrels	114, 500 303, 313 65, 000 365, 915	Hides	753 3,740
Beef and pork, barrels Potatoes, bushels Beans, barrels Rye, bushels Butter, firkins Eggs, barrels	4,600 7,132 11,627 8,000 12,182 335	Pelts Hay, bales Pearlash, casks Railroad cars Shingles	2,000 4,576 300
Meal and feed, bags	200, 000 93 15, 227	Lumber, feetLathsStaves and headings, sets	40, 640, 000 5, 000, 000 10, 384 5, 485

Abstract of labor performed and amount paid to contractor up to September 1, 1867, at harbor of Green Bay, Wisconsin.

Labor, 32, 294 yards of dredging. Paid William S. Smith, contractor, \$12,447 25.

A 5.

MILWAUKEE, WISCONSIN, August 31, 1867.

Siz: The subjoined report upon the present and prospective condition of the harbor of Manitowoc, Wisconsin, is respectfully submitted in compliance with your instructions of July 20, 1867. Manitowoc, Wisconsin, is situated at the mouth of the Manitowoc river, which empties into a beautiful bay on Lake Michigan, of some six miles in length, and two and a half miles in width; excellent anchorage is found in this bay, and partial protection from the fierceness of northeast and southeastern gales. During the spring freshets, a large volume of water is projected into the lake by the Manitowoc river, sufficient to maintain an adequate depth of water at its mouth, were it confined to a straight and uninterrupted course to the deep water of the lake.

As a refuge, the existence of a good harbor at this point is patent; it is the most northern harbor upon the west shore of Lake Michigan upon which any improvement by the United States has been made. It is twenty-five miles north of Sheboygan, and eighty-five miles south of Manitou islands, and is the only harbor intermediate, in which vessels can seek refuge in tempestuous weather. The safety, therefore, of the lake navigation calls loudly for the completion of

the works already begun.

Congress, during the thirty-ninth session, appropriated \$45,000, in addition to the \$52,000 existing for the improvement of this harbor. The plan of improvement proposed was to construct piers on each side of the river's mouth, extending in a parallel direction to the deep water of the lake; this required the construction of 2,150 feet of pier work, and, in order to secure twelve feet depth of

water between them, the removal of 57,677 cubic yards of earth.

The work of improvement was begun early in June of this year, and has progressed satisfactorily up to the present time. Mr. David Smoke, the contractor for furnishing material, and framing, placing, and sinking cribs, has constructed and placed in position sixteen cribs on the north side; the superstructure has been already commenced, and, if hindrances do not occur, will shortly be completed as far as the cribs extend; further additions will be made this fall. The harbor commissioners (the contractors for dredging) have removed up to the present time 13,955 cubic yards of earth, and the probabilities are, that from 8,000 to 10,000 cubic yards may be removed between this and the close of the working season.

There may be, according to the present contracted prices for timber, iron, labor, and dredging, \$55,000 profitably expended in the extension of these cribs and

dredging between them during the ensuing year.

The present depth of water is such that vessels drawing over five and a half feet of water cannot enter; when once across the bar that obstructs the mouth of the river, ample depth of water to meet the usual requirements of vessels

sailing upon Lake Michigan is obtained.

Notwithstanding the shallowness and tortuousness of the channel from the river to the lake, during the past year the exports and imports of this place have steadily increased; and the commercial lumbering, ship-building, and agricultural interest of that portion of Wisconsin which finds an egress at this point have rapidly increased in wealth and importance.

Perhaps some idea of this advancement may be obtained by an inspection of the number and tonnage of versels sailing to and from this harbor during the past year: Arrivals, 685; tonnage, 149,894; departures, 689; tonnage, 151,590; tonnage on stocks at present, 1,300. A tabular statement of some of the most

important exports I append.

A heavy expense is incurred in the loading of vessels with the more bulky of the articles exported, it being necessary to employ lighters for the purpose; whereas, were the entrance so improved as to admit vessels, lumber could be loaded directly from the mills, and staple produce of the tributary country from docks, instead of being transferred to lighters, or hauled to piers, where they are subjected to heavy percentage for pierage before they can be shipped.

This harbor is in the collection district of Milwaukee, the latter place being the port of entry. A light-house stands upon the bluff near the entrance; the

light is a fixed white light of the fifth order.

Manitowoc and the surrounding country is still in its infancy; and a retrespective glance at its past history leads to a brilliant hope for the future; and we feel convinced that, if the present plan of improvement is pursued, inestimable benefit must result to not only the local interests of the place, but to the entire commerce and navigation of Lake Michigan.

Very respectfully, your obedient servant,

JAMES B. QUINN, Lieutenant United States Engineers.

Brevet Colonel J. B. WHRELER,

Major Corps of Engineers.

Abstract of materials received and used, labor performed, and amount paid to contractors up to August 31, 1867, at harbor of Manitowoc, Wisconsin.

.						:	Materials	s re	ceive	i.					
Contract	ors.	Pin timl		Other		nk.	Piles.	1	[ron.	Ston	в.	Ren	arks.		
David Smol Joseph Vila Hero & Pac John Schea	as kard.	68,			17,	100		6	ounds. 0, 536	68	1	,200 lbs	s. of spikes.		
	M	ateria	ls us	ed.					La	bor pe	rfor	med.			
Timber.	Pile	 :8.	St	one. Iron.			Dredgin	ng.	Fran	Framing.		Pilès riven.	Cribs placed.		
Feet. 48, 380		et. 710	Cub	. y'ds. 685	Pounds. 45, 414				13, 9	55	48	, 60 0		Feet. 1,710	Feet. 16
	Co	ntrac	tors.				For	wh	at pa	id.		Amo	unt paid.		
David Smoke					P	raming placing slankredging.	and	sinki	ng cri	bs		\$32, 555 62 356 60 2, 726 40 4, 685 70			
												1	40, 324 72		

Abstract of some of the principal articles exported from Manitowoc, Wisconsin, during past year.

Articles.	Quantity.	Articles.	Quantity.
Lumber feet Shingles Laths Broom-handles Posts Staves Pickets Wood cords	59, 328, 000 1, 544, 000 67, 000 46, 400 64, 000	Wheat bushels. Peas bushels. Flour barrels. Leather rolls. Cattle pounds. Sundries packages. Fish half barrels.	78, 492 2, 170 2, 160 1, 608 748 12, 700 14, 407 1, 887

A 6.

MILWAUKER, WISCONSIN, August 31, 1867.

Siz: I have the honor to submit the subjoined report upon the harbor improvements, present and prospective, of Sheboygan, Wisconsin, in conformity with your instructions contained in circular of July 20, 1867.

This harbor is situated in the collection district of Mi waukee; the latter place being the nearest port of entry. Like the majority of the harbors upon Lake Michigan, this harbor consists of a portion of the mouth of the river of the same

name, improved by artificial means.

The method of improving this harbor is the ordinary one pursued in similar places upon Lake Michigan—that of building piers on each side of the river, extending from the shore to the deep water of the lake, and dredging out between these piers in such a manner as to secure sufficient depth of water to permit

vessels to pass into the river.

As to the vitality of the towns situated at, and the country tributary to, these commercial portals depends upon their efficiency, the maintenance of this egress in a serviceable condition is to them of the first importance; were, however, local interests alone benefited by the improvement of these harbor entrances, we might stop here and insist that private enterprise should make the necessary improvements; but when we turn our attention to the vast amount of shipping engaged in the commerce of Lake Michigan, and the severity of the gales upon this freshwater sea, it becomes of the gravest importance that points should exist upon its shores in which vessels can find refuge during unfavorable weather.

Owing to the absence of any suitable bays on the coasts of this lake, the government is forced to improve the mouths of the largest rivers emptying into it; for when the bars generally formed at the mouths of these rivers are once passed sufficient depth and extent of water is found to answer all the requirements of a

good harbor of refuge.

As there is at present no harbor existing between Milwaukee and the Manitou islands to which vessels can flee in stormy weather, the interests of the community at large, as well as local interests, called attention to the necessity of improvements being made at this point; and the government, following the policy that the safety and welfare of the people must be provided for, caused an examination and survey to be made, and appropriated in 1865 \$47,000, and in 1866 \$8,000 more, to perfect the improvements already commenced at the harbor of Shebovgan.

The plan of improvement proposed and approved was to extend the north pier one hundred and twenty feet, the south pier three hundred and twenty feet, and to dredge between the piers to a depth of twelve feet. Work was begun upon the harbor early in June of this year, and has progressed satisfactorily,

under the circumstances up to the present time, and I apprehend, if no further accidents occur, the improvement will be completed by the close of the working season.

The existence of a bed of quicksand, upon which the cribs were necessarily placed, has occasioned considerable unforeseen expense and difficulty to retain the cribs in place and keep them ballasted properly since they were sunk. I think \$2,000 will repair all deficiencies, however.

I caused a few lines of soundings to be taken in the prolongation of the new pier, and find that some few changes have occurred since the last survey, though not unexpected, there merely having been some slight accretion upon the bar off

the entrance.

As the original plan was, I believe, to extend the piers to the curve of twelve feet water, it will be necessary to do this to extend the pier across this bar lying in front of the entrance; this will require an additional four hundred and sixteen feet of piering, as the pier will project some distance into the lake. I think it advisable to increase the width of the cribs, of which it would be composed, to twenty-five feet, as such cribs are more stable, on account of their extended base and increased weight.

To give a water way of twelve feet in depth between these piers, it will be necessary to remove, by dredging, fifteen hundred cubic yards of earth; this, added to the cost of piering and incidental expenses, gives \$49,026 38 as the

sum required to complete his extension. (See estimates)

The completion of this extension will secure a safe and commodious harbor,

with flattering prospects of remaining so for a number of years.

I am informed that vessels passing through the straits on their way to Chicago and Milwaukee, after leaving the Manitou islands, steer direct for the Sheboygan light-house; the probabilities are, therefore, that vessels passing to and fro between points south and the Manitou islands would naturally sail close to this point, and, in the event of stormy weather, would seek shelter in it.

During the past year 7,496 vessels passed the light-house. This light is situated on Sheboygan north-point, about one mile north of the harbor; it is a fixed white light of the fifth order. This light being situated north of the harbor, no account is kept of the number of vessels sailing between this port and points south, nor of the vessels passing in the night; so the number reported is rather incomplete.

During the past year there cleared from this port 1,114 vessels, the tonnage

of which I have been unable to ascertain.

I append an abstract of some of the most important imports and exports, a comparison of which with that of preceding years will show a decided increase. These tables are interesting from the fact that, as the commerce of a place is the exponent of the thrift thereof, they will indicate with considerable accuracy the importance of this place, and, by comparison, the rapidly increasing producing qualities of the tributary country, which needs but the stimulus of a good harbor to ship from to make much more rapid strides in wealth and importance, and I therefore have no apprehension that any money that might be expended in the improvement of this harbor would not result in incalculable advantage to general as well as local interests.

I am, sir, very respectfully, your obedient servant,

JAMES B. QUINN,

Lieutenant United States Engineers.
Brevet Colonel J. B. Wheeler,

Major Corps of Engineers.

Abstract of some of the most important imports and exports at the harbor of Sheboygan, Wisconsin, for the year ending June 30, 1867.

IMPORTS.

Articles.	Quantity.	Articles.	Quantity.
Merchandise packages Lumber feet Green hides kegs Coal tons Oats bushels	36, 872 413, 000 3, 819 1, 758 306 7, 722	Apples and sundriespackagesBarkcordsIronbundlesSaltbarrelsCornbushels	4, 806 197 5, 557 4, 227 9, 212

EXPORTS.

			
Wood cords Butter tubs Flour barrels Wheat bushels Wool pounds Eggs barrels Wagon stuff packages Packing barrels Pish barrels Cattle and sheep head Peas bushels	3,370 24,223 173,769 110,281 488 24,187 10,769 861 1,374	Ashes casks Laths Chair stuff packages Wooden shoes pairs Bricks Beer barrels Lime barrels Hay tons Cheese pounds Hogs head Sundries packages	1, 065, 000 445 2, 000 119, 000 541 205

Abstract of materials received and used, labor performed, and amount paid to contractors, up to September 1, 1867, at harbor of Sheboygan, Wisconsin.

	MATERIALS RECEIVED.						
Contractors.	Timber.		Plank.	Piles.	Stone	Brush.	Tron
	Pine.	Other.	I Ioua.	A 110b.	Stone.	Di tibil.	11011.
Sanger, Ledlie & Corse	Feet.	Feet.	Feet.	Feet.	Cords.		Lbs. 47, 011
Dillingham & Co. Locklin & Jenkins	11,428	17,021	3,716	1,830	5264		

MATERIALS USED.			LABOR PERFORMED.				
Timber.	Piles.	Stone.	Iron.	Dredging.	Framing.	Piles driven.	Cribs placed.
Feet. 29, 917	Feet. 840	Cords. 452	Lbs. 31, 340	Cub. yard. 18, 096	Feet. 29, 917	28	13

Contractors.	For what paid.	Amount paid.
Sanger, Ledlie & Corse Dillingham & Co Locklin & Jenkins	Brush, iron, dredging, framing, and placing cribs and driving piles Timber, plank scantling, and piles Stone	\$14, 480 53 6, 820 37 4, 520 00

COST OF ONE CRIB $32' \times 25' \times 20'$

Estimate.

Materials.	Quantity.	Rates paid.	Amount.
4 oak piles	120 running feet	16 cts. per r. foot	\$ 19 2 0
12-inch square timber, oak	2,697 running feet		728 19
12-inch square timber, pine.	710 running feet	24 cts. per r. foot	170 40
3-inch plank	288 feet, board measure	\$25 per M	7 20
Iron bolts	3,880.97 pounds	10 cts. per lb	3 85 09
Iron spikes	6 pounds		60
Stone ballasting			89 () (ii)
Brush for intervals	10 cords	\$6 per cord	60 (11)
Driving piles			22 🖽
Framing			661 40
Cost of material for one crib			2,966 88
Cost of material for thirteen	cribs		38, 569 44
Dredging 1,500 cubic yards,	at 40 cents per cubic yard.		6,000 00
			44, 569 44
Contingencies of 10 per cent			4, 456 94
Total cost of improve	ment		49 026 38

A 7.

MILWAUKEE, WISCONSIN, September 1, 1867.

COLONEL: I have the honor to submit the following report on Milwaukee harbor:

The harbor of Milwaukee, Wisconsin, is one of the best, and next to Chicago harbor, the most important on the west shore of Lake Michigan. From the formation of the shore it is well protected from some of the severest winds, and is not liable to be soon obstructed by the formation of a bar at its mouth. The freshets in the spring are also of service in helping to keep open the mouth of the harbor. The harbor was sounded in April of this year immediately before, and in June immediately after, the freshets, and the soundings show that the bar was moved considerably out into the lake.

The harbor is now in a good condition, the only work at present being done in the filling of the old cribs with stone; this work will be finished about the 15th of September.

The harbor can be further improved by extending each pier three hundred and twenty feet; this will probably be the only improvement necessary for many years, except the repairs needed on the old pier, and perhaps occasionally a little dredging.

This extension is necessary; for the bar, though forming slowly, will in course of time obstruct the entrance if nothing be done to prevent it, and if the work

be delayed it will cost more when it is done, because the bar will have to be

dredged away, thus incurring an expense not necessary now.

As the extension will in many places be in fourteen or fifteen feet of water, the cribs composing it will have to be thirty-two feet long, twenty-five feet wide, and twenty feet high.

The following is an estimate of the cost of one of such cribs, the prices taken

are the same as the present contract prices at Racine, Wisconsin:

3,378 running feet of 12-inch square timber, at 24 cts	810 8	72 64
3,880 pounds of iron bolts, at 10 cts	388	00
6 pounds of iron spikes, at 10 cts		60
90 cords of stone, at \$12 80	1, 152	00
10 cords of brush, at \$6	60	00
3,378 feet of framing, at 17 cts	574	26
Add 10 per cent. for contingencies	2, 994 299	
And to bet cent tot commisencies	299	42
	3, 293	64
		_

As the extension will consist of twenty cribs, the whole cost will be \$65,872 80.

The following information was obtained from Mr. C. L. Sholes, collector at this port:

Customs collected during the year ending June 30, 1867, \$83,815 19.

All customs for this district are paid at Milwaukee.

Amount of exports during same time	\$750,901
Amount of imports during same time	75, 990
Number of vessels arrived	3, 761
Number of vessels cleared	3,687

The business of this, as well as of all other ports in the district, is rapidly and uniformly on the increase.

The amount collected in duties is no particular indication of the importance of a place or port, as many places of much less importance generally than Milwaukee collect much more duties.

The light-house is located at the north point of Milwaukee bay, in latitude, north, 43° 02′ 24″, longitude, west, 87° 54′ 08.″ The light is fixed, varied by flashes at intervals of two minutes, and is visible fourteen nautical miles. The color of the tower is yellow, and its height is twenty-eight feet; height of light above sea level, one hundred and two feet; the lens is of the fourth order; the light-house was built in 1855.

North cut beacon is situated at the extreme end of North harbor pier, in latitude, north, 43° 01′ 37″, longitude, west, 87° 53′ 59″. The fog signal is a horn; the light is a fixed red one, and is visible eight nautical miles; the height of the tower is thirty-six feet, and height of light above sea level, forty-eight feet; the lens is of the sixth order.

Annexed is a tabular statement of work done and material received; also, abstract of contracts and abstract of bids.

Very respectfully,

D. B. HEAP, Captain of Engineers.

Major and Brevet Col. J. B. WHEBLER, Corps of Engineers.

Abstract of contract for improving Milwaukee harbor, Wisconsin.

Contractor.	Nature of contract.	Price.
Valentine Kuhlman	Stone, per cubic yard	\$2 70

Abstract of materials received and used, labor performed, and amount paid to contractors, up to the 1st of September, 1867, at harbor of Milwaukee, Wisconsin.

Contractors.	Stone received.	For what paid.	Amount paid.	Remarks.
V. Kuhlman	Cubic yds. 3, 327	Stone	\$8,601 74	All the stone has been put in the cribs that has been received.

The following is an abstract of the bids opened on June 27, 1867, for furnishing the stone at Milwaukee:

- 1. Thos. Duffy, to furnish the stone and fill the cribs at \$2 80 per cubic yard.
- 2. Hasbrouck & Conro, to furnish the stone and fill the cribs at \$13 per cord.
- 3. Hunt & Rassiter, to furnish the stone and fill the cribs at \$2 85 per cubic yard.
- 4. J. D. Dolan, to furnish the stone and fill the cribs at \$2 73 per cubic yard.
- 5. V. Kuhlman, to furnish the stone and fill the cribs at \$2 70 per cubic yard

A 8.

MILWAUKEE, WISCONSIN, September 1, 1867.

COLONEL: I have the honor to submit the following report of the harbor of Racine, Wisconsin:

This harbor, situated at the mouth of Root river, is of considerable importance. Racine being next in importance and wealth to Milwaukee, and a considerable amount of trade passing through it, makes it necessary to keep the harbor in a state of thorough repair.

In August, 1866, there was a contract made to dredge out the harbor to a depth of twelve feet; this has been nearly completed; at the same time there was a contract made to construct two hundred and fifty-six feet of pier work, which was placed on the end of the south pier, and is now nearly completed, all the cribs being sunk, and the superstructure only to be finished.

On June 27, 1867, a contract was let to construct four hundred and sixteen feet of pier work. It is the intention to add this to the north pier. If this be done, the south pier should be extended an equal length, then both piers will reach water twenty feet deep, and the harbor will not probably need any additional improvement of magnitude for years.

The cribs composing this extension should be thirty-two feet long, twenty-five feet wide, and average twenty feet high.

Such a crib will cost as follows:

D 4011 W 1111 VOLT 410 10110 W 1		
3, 378 running feet of 12-inch square timber, at 24 cts	\$810	72
3,378 running feet of framing, at 17 cts	574	26
288 feet of 3-inch plank, at \$30 per M	8	64
3,880 pounds of iron bolts, at 10 cts. per pound	388	00

6 pends of iron spikes, at 10 cts. per pound	1, 152 60	60 00 00
Add per cent. for contingencies	2, 994 299	
·	3, 293	64

Thirteen cribs will then cost \$42,817 32.

There is a sunken reef about three miles from shore, on which it would be advisable to build a light-house as a protection to shipping.

Annexed is a report of the exports and imports; abstracts of the contracts of August, 1866, and June, 1867; abstract of bids in June, 1867; and tabular statement of work done and paid for, and material received, up to 1st September.

Very respectfully, your obedient servant,

D. B. HEAP, Captain of Engineers.

Major and Brevet Col. J. B. WHEELER, Corps of Engineers.

Racine is situated in district of Milwaukee, and is itself a port of entry. There is a fixed white light in the light-house; the lens is of the fifth order, and shows at an elevation of forty-seven and a half feet above the lake, (less lake rise since 1866.) The following are the imports and exports in the year 1867:

IMPORTS.

Lumber, feet B. M	25, 805, 600	Tons of coal	11, 452
Laths, number	2, 090, 000	Tons of pig iron	530
Shingles, number	6, 236, 000	Barrels of salt	28, 572
Cedar posts, number	66, 050	Barrels of water lime	1,400
Railroad ties, number	22, 713	Staves	5, 525
Cords of wood	1, 520	Kegs of powder	200
Cords of bark	2, 262	Boxes peaches	950
Cords of bolts	423	Barrels apples	388
Piles	3, 600	Telegraph poles	400
Cords cut stone	200		11, 156
Cords rough stone	40	Hoops	14,000

Five hundred and thirty-two sailing vessels arrived at, and five hundred and sixty cleared from, Racine. Forty-two steam vessels arrived and cleared. Two and three steamboats per day stop at Racine during eight months of the year.

The revenue collected at Racine is for entrance and clearance dues. It amounted to \$584 75 during 1866; and from 1st January, 1867, to 1st July, 1867, to \$176.

The exports are as follows:

Tons hay 694 Barrels 5, 107 Tens mill-feed 147 Bricks 135, 500	Bushels corn Bushels oats. Barrels flour Barrels beef	555, 981 560, 739 17, 310 155	Tons scrap iron	2, 300 287 150 185
	Barrels beef	694	Barrels	5, 107

The following is the abstract of bids for the work at Racine, which was let on 27th June, 1867:

1. Hunt & Rassiter, Milwaukee.—Stone, \$2 72 per cubic yard; brush, \$1 24

per cubic yard; framing, 17 cents per lineal foot.

2. Ledlie & Corse.—12-inch square timber, 211 cents per lineal foot; pine plank, \$20 per M.; iron 5 4-10 cents per pound; spikes, 12 cents per pound; stone, \$2 69 per cubic yard; brush, \$5 40 per cord; framing, 10 cents per foot; sinking and filling cribs, \$110 each crib; delivering piles, 19 cents per lineal foot; driving piles, 17 cents per lineal foot.

3. Patrick M. Danaher, of Ludington, Michigan.—12-inch square timber, \$14 97 per M.; plank, \$14 97 per M.; stone, \$14 90 per cord; iron bolts, 5k

cents per pound; framing, 13\frac{3}{2} cents per lineal foot.

4. R. Nelson Gere, of Syracuse, New York.—Iron bolts, 5 cents per pound;

iron spikes, 7 cents per pound.
5. M. V. Thompson, of Geddes, Onondaga county, New York.—Iron bolts, 47 cents per pound; iron spikes, 7 cents per pound.

Remarks.—Mr. Thompson's bid was rejected as informal, he furnishing no

guarantee.

Abstract of contracts for improving the harbor at Racine, Wisconsin.

CONTRACTS OF AUGUST, 1866.

Contractors.	Nature of contract.	Price.	
James V. Sanger James H. Ledlie John M. Corse	White oak piles, per lineal foot Plank per thousand, board measure, 3-inch. Scantling per thousand, board measure. Square timber, 12-inches, per lineal foot Iron bolts and spikes, per lb. Brush in cribs, per cord. Stone in cribs, per cord. Placing cribs each. Driving piles, per lineal foot Framing, per lineal foot.	\$0 19 30 00 30 00 2 10 6 0 13 8 39 0 19	

Repairs to be paid for at rates fixed by engineer officer in charge.

CONTRACTS OF JUNE, 1867.

	Nature of contracts.		
Ledlie & Corse Stor	nch square timber per M, b. m	2 69 5 40	

Abstract of materials received and used, labor performed, and amount paid to contractors up to September 1, 1867, at harbor of Racine, Wisconsin.

	Material received.				
Contractors.	Pine timber.	Piles.	Stone.	Brush.	Iron.
Sanger, Ledlie & Corse	Feet. 20, 857	No. 11	Cords. 629	Cords. 139	Pounds. 27, 316

Materials used.			La	bor perform	ed.	
Pine timber.	Piles.	!Stone.	Iron.	Dredging.	Framing.	Cribs placed.
Feet. 20,857	No. 11 == 330 feet.	Cords. 240	Pounds. 18,000	Cub. yards. 5, 962	Lineal fact. 13, 129	8

Contractors.	For what paid.	Amount paid.
James V. Sanger	Iron, timber, stone, brush, dredging, and framing	\$20,617 98

A 9.

MILWAUKER, WISCONSIN, September 1, 1867.

COLONEL: I have the honor to submit the following report on Kenosha har-bor, Wisconsin.

This harbor, situated on the west shore of Lake Michigan, about twelve miles south of Racine, is of considerable importance, as will be seen by referring to the annexed table of imports and exports.

The present improvement consists in dredging out the channel to a depth of twelve feet, and in extending the north pier one hundred and ninety-two feet, and the sonth pier three hundred and fifty-two feet, which carrries the end of each pier into twelve feet water.

I would recommend that each pier be further extended two hundred and twenty-four feet, carrying the ends of the piers into about twenty feet water. This extension will take fourteen cribs; eight of these cribs should be thirty-two feet long, twenty feet wide, and average twenty feet high; the remaining six should be thirty-two feet long, twenty-five feet wide, and average twenty-five feet high. 'The cost for one of each of these cribs is as follows:

Crib $32' \times 20' \times 20'$:

2.760 feet of 12-inch square timber, at 29 cents,	\$802	40
258 feet plank, at \$30 per thousand	8	64
4,074 pounds of bolts, at 10 cents	407	40
6 pounds of spikes, at 10 cents		60

\$856 80

42 00

124 50

222 57 347 07

315 cubic yards of stone, at \$2 72.....

40 cubic yards of brush, at \$1 05.....

2,760 running feet of framing, at 17 cents	469 20
Add 10 per cent. for contingencies	2, 587 04 258 70
	2, 845 74
Cost of eight cribs	\$ 22, 765 92
Crib 32' × 25' × 25':	
4,159 running feet of timber, at 24 cents	\$998 16 8 64
4,762 pounds of iron bolts, at 10 cents	476 20 60
112 cords of stone, at \$13 87	1, 553 44
13 cords of brush, at \$5	65 00 707 03
Add 10 per cent. for contingencies	3, 809 07 380 90
Add to per cent for contingencies	
	4, 189 97
Cost of six cribs	\$25, 139 82 22, 765 92
Whole cost	47, 905 74
It is not probable that any dredging will be necessary next year south pier is very much in need of repair, and it will cost about \$3 it in good order; it is 1,200 feet long. The following is a copy of a letter from the deputy collector at 1	20,000 to put
Kenosha is in the district of Milwaukee. The nearest port of entry is Racine. One hundred and fourteen vessels arrived during the year 1866; cleared during the year 1866; 125 vessels arrived during the y July 1; 121 vessels cleared during the year 1867, to July 1.	; 115 vessels ear 1867, to
The fees collected for year 1866 were	

The imports for 1866 were 4,358,000 feet lumber, 160,000 laths, 7,400 posts, 1,429,000 shingles, 760 cords of wood, 9,500 bushels barley, 278 cords bark, 705 tons of coal, 100 cords stone.

The fees collected for year 1867, to July 1, were.....

Tonnage duties for the year 1867, to July 1, were.....

The exports for 1866 were 16 barrels lime, 3 barrels beans, 500 dozen eggs.

1,000 pounds cheese, 500 pounds butter, 100 barrels beef.

The imports for six months ending July 1, 1867, were 7,299 bushels barley, 2,712 cords wood, 11,500 posts, 204,000 laths, 37,000 staves, 3,210,000 feet lumber, 12 barrels flour, 200 barrels maple sugar, 153 cords tan bark, 365 cords stone, 5 tons hay, 218,000 feet timber.

Exports for six months ending July 1, 1867, were 81,000 feet lumber, 267½ tons hay, 49 wagons, 1 cutter, 2 tons coal, 1 dozen brooms, 14 barrels eggs, 2,077 pounds butter, 897 bushels potatoes, 3 barrels pork, 46 sacks feed, 37 bags oats, 10 tons castings, 1,780 barrels flour, 300 bushels oats, 10 pounds cheese.

The goods received and discharged by steamer at the pier were not included in the above report.

E. SIMMONS, Deputy Collector.

Amexed there is a tabular statement of work done and material received up to 31st August; also an abstract of contract for improving the harbor.

Very respectfully your obedient servant,

D. P. HEAP, Captain of Engineers.

Major and Brevet Col. J. B. WHEELER, Corps of Engineers.

Abstract of contract for improving the harbor of Kenosha, Wisconsin.

Contractor. Nature of contract.		Price.
James V. Sanger	Piles, per lineal foot	\$ 0 19
James H. Ledlie		25 00
John M Corse		30 00
Do	Scantling, per M, (b. m.)	30 00
Do		29
Do	12-inch square timber, oak, per lineal foot	39
Do		10
Do		5 00
Do	Stone in cribs, per cord	13 87
Do	Driving piles, per lineal foot	13
Do		10
Do		39 00
20	Repairs, to be paid for at rates fixed by engineer officer in charge.	<i>55</i> 00
Caleb H. Parker	For dredging between piers, and for foundation for cribs, per cubic yard	37
Do	Dredging on bars or points outside of proposed ex-	
	tension	74

Abstract of material received and used, labor performed, and amount paid to contractors up to September 1, 1867, at harbor of Kenosha, Wisconsin.

-		Materials received.			
Contractors.	Timber.	Stone.	Brush.	Iron.	
James W. Sanger James H. Ledlie	Feet. 27,678	Cords. 910	Cords.	Pounds. 53, 827	

Materials used.		Materials used. Labor performed.			L.
Timber.	Stone.	Iron.	Dredging.	Framing.	Cribs placed.
Feet.	· Cords.	Pounds.	Cubic yards.	Feet.	
15, 46 0	330	26,000	40,050	15, 460	11

Cont. actors.	For what paid.	Am't paid.
Caleb H. Parker	Dredging Timber, framing, brush, stone, and iron	\$13, 199 75 22, 772 44
Total		35, 972 19

A 10.

MILWAUKEB, WISCONSIN, September 1, 1867.

COLONEL: I have the honor to respectfully submit the following report on Chicago harbor.

The government has made no recent surveys of this harbor.

In August, 1866, a contract was made to extend the north pier six hundred feet. The work was not commenced until this year.

While Lieutenant Colonel J. D. Graham was in charge of the harbor improvements of this lake, the Chicago Dock and Canal Company had the privilege granted them from the War Department "to make an opening through the United States north pier, to communicate from this harbor with ship basins and canals, which the said company propose to construct within certain grounds owned by it, immediately north of and adjacent to said pier."

Since then this company has greatly enlarged its plans, and intend constructing a much larger basin. To save the expense of cutting through the old pier, they applied for and obtained permission to have an opening of three hundred

feet left when the new government extension was built.

This new extension will now commence at a point three l

This new extension will now commence at a point three hundred feet distant from the old pier and on a line with it, and then will run due east for a distance of six hundred feet.

The Chicago Dock and Canal Company are required to enclose their basin sufficiently this year to protect this opening from the action of the wind and waves, and there was also an agreement entered into with them by which they assume any extra expense that the contractors may incur, by commencing the extension at the point referred to instead of at the end of the old pier.

This arrangement is advantageous to the government, for it virtually extends the pier nine hundred instead of six hundred feet; the crib-work enclosing the basin effectually preventing any wash through the opening from the north, the direction of all the severe winds, and where the drifting sand and gravel come from.

This extension reaches water twenty-four feet deep, a depth at which the bottom is not appreciably affected by the action of the waves. I do not believe that any bar will be formed at the end of this pier for many years.

There is no necessity for building a south pier. In the first place the entrance

to the harbor is none too large now, and a south pier would cramp it still more; and in the second place it would be useless, as the citizens of Chicago have induced Lake Michigan to run into the Chicago river; consequently the river current will not be of the slightest use in removing obstructions at the entrance.

I append the report of the collector, an abstract of the contracts, and a tabular statement of material used and work done up to date.

Very respectfully,

D. P. HEAP, Corps of Engineers.

Major and Brevet Col. J. B. WHEELER,

Corps of Engineers.

The following is information concerning Chicago, obtained from Mr. Walker B. Scates, the collector at that port:

Chicago light-house.—Official number of light-houses on the northern and northwestern lakes, No. 68; 41° 53′ 25″ latitude north, 87° 36′ 56″ longitude west. Has one fixed light, visible fifteen nautical miles. Height of light above sea-level eighty-three feet. Order of lens, third order lens apparatus. Tower built 1859, of iron, painted black.

Total number of vessels arrived during 1866, 11,084; seamen, 93,554; tonnage, 2,258,527. Total number of vessels cleared during 1866, 11,115;

seamen, 94,464; tonnage, 2,361,529.

Number of vessels arrived during 1867, up to and including July 30, 1867, 5,556; seamen, 48,451; tonnage, 1,039,197. Number of vessels cleared during 1867, up to and including July 30, 1867, 5,819; seamen, 49,720; tonnage, 1,018,198.

Amount of duties collected on imported merchandise from January 1, 1866, to July 1, 1867, \$676,876 92, specified as follows:

676, 876 92

Summary statement of receipts by lake at the port of Chicago, Illinois, during the year 1866.

Articles.	Amount.	Articles.	Amount.	
Lumber feet.	676, 236, 000	Eggsbarrels.	867	
Shingles thousand.	197, 169	Peasbushels.	12, 182	
Lathdo	118, 405	Beansdo	651	
Timber, squarefeet.	11,615,000	Wheatdodo	236, 882	
Heading mille.	11,644	Corndo	2,210	
Stavesdo	10,789	Ontsdo	4,041	
Cedar postsnumber.	762, 105	Barleydo	5, 546	
Railroad tiesdo	434, 452	Ryedo	412	
Shingle and stave bolts cords.	12, 845	Green hidesnumber.	1,750	
Barkdo	21,868	Dry hidesdo	4, 428	
Fire wood do	140, 921	Leather rolls	12,507	
Slabsdo	1,095		105	
Picketsnumber.	2,651,000		446	
Telegraph polesdo	12, 852		1, 374	
Hoops do	2, 287, 000	Hay bales	438	
Sparsdo	2,207,000	Hams	295	

Summary statement of rescipts by lake, &c.—Continued.

Articles.	Amount.	Articles.	Amount.
Woodenwaresets.	113, 121	Porkdo	2, 223
Furniturepackages.	66, 513	Beefdo	224
Wagon stuffsets.	38, 913	Cattlehead.	181
Household goodspackages.	11,933	Sheepdo	203
Pig irontons.	22, 447 8, 830	Hogsdo	10 337
Railroad irondo Dobar s.	69, 864	Horsesdo Wagonsnumber.	1,079
Ironbundles.	55, 399	Cementbarrels	7, 478
Iron oretons.	100	Limedo	28,247
scrap irondo	74	Vinegardo	3,795
Dobarrels	12	Plasterdo	23, 129
rinboxes.	23, 553	Woolbales.	325
Zinccasks.	1,881	Hairbales and barrels.	3, 228 69
Lead pigs.	6, 111	Hempbales.	2, 421
Dotons.	160	Pitch barrels	2,774
Dobarrels.	354	Tar do	1,416
Dosheets.	171	Oildo	10, 206
Doingots.	587	Cordagecoils.	12, 615
Coaltons.	378, 713	Herringsboxes.	17,54
Hardwarepackages.	196, 693	Agricultural implements . No.	3, 806
Nailskegs.	30, 642	Reapers, mowers, and thresh-	616
Stovesnumber	55, 203 99, 041	Page hales	1, 849
Castingspackages. Coppermasses.	9	Bagsbales. Empty barrelsnumber.	20, 95
Sugarhogsheads.	11,840	Ship kneesdo	86
Dobarrels.	100, 046	Sleighsdo	40
Molassesdo	16, 180	Iron safesdo	190
Baltdo	493, 407	Machinerypackages.	8,54
Dobags.	2, 381	Do tons.	83 16 07
Dotons.	2,915 101,206	Marblepieces. Dotons.	16, 973 88
Liquorspackages.	19, 123	Granitedo	15
lass and glasswaredo	46, 801	Stonecords.	24
Earthenwarecrates.	11, 139	Fire-worksboxes.	1, 96
Drugs packages.	43, 321	Tree boxesnumber.	74
Fruitdo	66, 458	Boots and shoescases.	16, 92
Froceriesdo	318, 212	Boiler plates number.	10
Paintsdo	28,871	Enginesdo	1:
Ory goodsboxes and bales.	21,937 20,667	Boilersdo do do	23
Butterkegs or tubs.	5, 285	Grindstonesdo	2, 56
otatoesbushels.	49, 400	Dotons.	23
lourbarrels.	47,752	Pianosnumber.	310
Melodeonsnumber.	38	Marble dustbarrels.	43:
Organsdo	22	Burr blocksnumber.	2, 44
lothingboxes.	1,706	Dotons	40
Fire-bricknumber.	2, 193, 000	Maltbags.	595 537
Dotons. Docasks.	10 731	Hopsbales. Clockscases.	3, 75
Fire clay barrels.	695	Powder kegs.	4, 019
Dotons	50	Broom cornbales.	1,800
Drain tileboxes.	146	Powdertons.	61
Retortsdo	221	Buffalo robes bales.	25
calesdo	5, 095	Row boatsnumber.	
Blatepackages.	58	Powder magazinestons.	•
Dotons.	131	i I	

Summary statement of shipments by lake from the port of Chicago, Illinois, during the year 1866.

Articles.	Amount.	Articles.	Amount.
Theatbushels.	5, 827, 846	Iron bundles.	8,48
orndo	32, 457, 855	Dotons.	98
arleydo	988, 240	Lumber feet.	236, 00
atsdo	7, 395, 113	Coaltons.	72
ye do	1,029,629	Castingsdo	3
lourbarrels.	481,491	Dopackages.	1, 17 1, 20
orn mealdo	10, 143 51, 937	Machinerydo	1, 20
eedbags. orn mealdo	10, 143	Provisions barrels.	1, 40
æddo	21,881	Doors, sash, &cdozen.	1,06
room cornbales.	3,079	Earthenware crates.	1,05
aydo	9,523	Glassware packages.	1,68
eefbarrela.	12, 923	Stoneware pieces.	4, 09
orkdo	26, 66 1	Woodenware sets.	1,55
urddo	2, 334	Leatherrolls.	1 99
allowdo	1,817	Stovesnumber.	55
reasedo	453	Boots and shoescases.	55
amsdo	2,306	Horsesnumber.	49
acondo	556 5.587	Buggiesdo	2 10
utter	5, 587 63, 839	Wagonsdododo	10
ry hidesdo	31,819	Cattlehead.	1, 40
igh winesbarrels.	2, 410	Hogdo	12
Cakedo	22, 120	Sheepdo	25
oolbales.	1,583	Tobaccopackages.	1, 19
adpigs.	762	Pitchbarrels.	10
Dotons.	890	Oakumbales.	16
y goodsboxes and bales.	13,508	Ship-storespackages.	34
urdwarepackages.	30, 284	Stonecords.	1, 43
oceriesdo	108, 941	Paperbundles.	1,36
pusebold goodsdo	8, 205 2, 683	Felting	1, 19
intsdo gsbarrels.	2,317	Beltingdo Potatoesbushels.	3,84
s do	2, 901	Cabbageheads.	11,50
ltdo	5,914	Vegetables barrels.	6 4
rniturepieces.	4, 134	Pianos number.	1
quor peckages.	4, 246	Melodeonsdo	1
uit barrels.	6, 595	Organsdo	-
medo	4,695	Sewing machinesdo]
ngs packages.	3, 580	Enginesdo	494 00
oilersnumberdo	35 7	Hoopsnumber.	424, 00
indstones do	166	Shinglesthousand. Stearinebarrels.	ž
naceboxes.	82	Hempbales.	ė
apers and mowers number .	193	Marblepieces.	ě
ricultural implementsdo	309	Scrap-ironbarrels.	15
messdo	87	Plasterdo	_{{{\{\!\!\!\ }}}
alesboxes.	82	Slate-roofing tons.	3
mentbarrels.	565	Billiard tablenumber.	
ocksboxes.	49	Life-boatdo	
urbales.	783	Gunscases.	27
astedo	18	Mill-stonesnumber.	40
gsdo	215 107	Lightning-rodsdo	19
nny bagsdo npty barrelsnumber.	107 6, 156	Mattingbarrels.	4
ickdo	138,700	Hopsdo	
ain-tiledo	1,529	Marble dust barrels.	
opecoils.	291	Stavesthousand.	
eswaxbarrels.	17	Feathersbales.	٠,
eep peltsbales.	727	Lead ore pounds	2,00
one-dust barrels.	271	Buffalo robesbales.	•
owderkegs.	54	Fishbarrels.	93
altbags.	111	j	

Abstract of contract for improving Chicago harbor, Illinois.

Contractor.	Nature of contract.	Price.
Do Do Do Do Do Do	12" timber, per linear foot. 3" plank, per thousand feet (board measure) Brush, per cord. Stone, per cord. Delivering and driving white oak piles, each. Framing and sinking cribs, per linear foot Iron bolts, per pound. Iron spikes, per pound Dredging inside of piers, per cubic yard. Dredging outside of piers, per cubic yard.	\$0 28 31 75 5 74 8 74 11 98 11 9 14 44

Abstract of materials received and used, labor performed, and amount paid we contractor up to September 1, 1867, at harbor of Chicago, Illinois.

Contractor, John M. Corse: timber, 33,282 feet; brush, 210 cords; iron, 133,147 pounds Materials used and labor performed: timber, 28,910 feet; brush, 210 cords; iron, 17,770 pounds; framing, 11,628 feet.

Contractor.	For what paid.	Amount paid.
Do	IronBrush	\$12,649 96 1,205 40 8,074 80 21,950 16

A 11.

ENGINEER DEPARTMENT,

Washington, July 10, 1867.

SIR: I respectfully transmit herewith a letter from Brevet Colonel J. B. Wheeler, corps of engineers, enclosing letters from W. B. Ogden and R. A. Conolly, asking the privilege for the Chicago Canal and Dock Company of having an opening at the extremity of the present north pier of Chicago harbor, for the purpose of improving the same.

The company referred to was incorporated by act of the legislature of Illinois, and has authority from the War Department, as well as from the State of Illinois, to construct a ship basin and canal immediately north of and adjacent to the north pier of Chicago harbor; and also the privilege of making an opening through the United States pier. This authorized opening would be at some point west of the light-house.

The United States has contracted for the extension of the north pier 600 feet, and as the contractor has not as yet sunk a crib, the company referred to wishes him to commence his work at a point distant 300 feet from the end of the present pier, instead of at the end, as required by the contract.

Colonel Wheeler expresses the opinion that the plan of improvement contemplated will not in any way injure the harbor, and recommends that the privilege asked for be granted. This department concurs in his recommendation, and accordingly submits the question for your decision.

Very respectfully, your obedient servant,

A. A. HUMPHREYS, Chief of Engineers, Major General.

Hon. E. M. STANTON, Secretary of War. MILWAUKEE, WISCONSIN, July 2, 1867.

GENERAL: I have the honor to transmit herewith a letter of Mr. W. B. Ogden, of Chicago, and a letter of Mr. R. A. Conolly, with a tracing, forwarded me by them, concerning the projected improvement proposed by the Chicago Dock and

Canal Company at the harbor of Chicago, Illinois.

This company was incorporated by an act of the legislature of the State of Illinois, and has permission and authority from the War Department, as well as from the State of Illinois, to construct a ship basin and canal immediately north of and adjacent to the north pier of Chicago harbor, and the privilege of making an opening through the United States pier. (See report of Colonel J. D. Graham for the year 1857.) This opening through the pier, under this permission, would be at some point west of the light-house.

They now ask the privilege of having the opening at the extremity of the

present north pier, as shown in the accompanying tracing.

By having the opening at this point they are saved great expense in dredging

out a basin, and the cost of cutting the opening in the pier.

The United States have contracted with John M. Corse to build six hundred (600) feet of pier-work, completing not less than three hundred and twenty (320) feet this season; and as the contractor has not yet sunk a crib, they wish to have him commence building this extension, not from the end of the present pier, as his contract calls for, but to commence at a point distant three hundred (300) feet. The question is, will it interfere with the proposed plan of improvement, or injure the harbor?

No, is the answer. It will do neither. The only interference will be an inconvenience to the contractor, and probably some additional expense to him.

I would therefore recommend that permission be granted the Chicago Dock and Canal Company to have an opening of three hundred (300) feet left at the extremity of the present pier, provided they guarantee to close in the basin by building the piers marked F E and E D during the present season. The piers to be built in a workmanlike manner, and upon a plan satisfactory to the engineer officer in charge; and that they also guarantee to settle all claims that the contractor may bring against the United States for increase of compensation for change of place, &c. The time of completion of contract, in this case, will have to be extended, and I would recommend (in case the privilege be granted) that he be allowed one year longer to build the six hundred (600) feet contracted for.

Any plan of improvement that will increase the dock facilities, or enlarge the harbor of Chicago, meets with my approval, and I earnestly recommend this one to your favorable consideration.

As the season is well advanced, I would ask for an early answer to this letter.

I am, general, very respectfully, your obedient servant,

J. B. WHEELER,
Major of Engineers and Brevet Colonel.

Major General A. A. HUMPHREYS.

Chief of Engineers U.S. Army, Washington, D.C.

CHICAGO, June 29, 1867.

DEAR SIR: With this you will receive the Chicago Dock and Canal Company's plans for enlarging and improving the Chicago harbor, in connection with the proposed extension of the north harbor pier by government, as prepared and submitted by R. A. Conolly, chief engineer of said Chicago Dock and Canal Company.

The present facilities and accommodations of the Chicago harbor are, as you are no doubt aware, very cramped and illiberal, and promise soon to be wholly

madequate to its rapidly increasing commerce.

The river is so narrow, and already so over occupied, that enlarged harbor facilities near its mouth would clearly afford the most efficient, ready, and important relief. This relief the plans and improvements of the Chicago Dock and Canal Company (a corporate body, and of which I am the largest proprietor) propose, in connection with the extension by the government of the north pier, to furnish now and hereafter, as shown on the plans of Mr. Conolly herewith submitted.

The changes effected by the plan submitted are, an extension of the north pier fifty per cent. further into the lake than as now proposed by government, furnishing thereby a more extended shelter for vessels and a protection of the harbor from drifting sands for a period of time at least fifty per cent. longer; second, an opening of three hundred feet in width east of and between the present end of the north pier and the beginning of the westerly end of the proposed six hundred feet extension of said north pier, through which opening vessels would have ready entrance and access to the basin and canals of the Chicago Dock and Canal Company, as shown on the map, which basin and canals will be extended and completed as commercial uses shall call for them, and will in the end serve to increase the facilities of the Chicago harbor to nearly twice its present capacity, and at the most convenient point possible for doing its business.

The Dock and Canal Company is ready to proceed at once and complete, as early this season as possible, the pier from the shore at F to the northeast corner at E, and from E to the point of junction with the proposed government extension of the north pier at D, the same to be done in a manner satisfactory to, and to be approved by, you. The company will also commence and proceed at once with their canal and interior piers or bulkhead, south and west of the

pier above described.

Should the plans herewith submitted meet with the proper official approval, the details and particulars, and the proper adjustment and accommodation of the matter with Messrs. Ledlie and Corse, government contractors for the extension of the north pier, we would proceed at once with you and them to satisfactorily arrange.

With great respect, I remain, very truly yours,

W. B. OGDEN.

Colonel J. B. Whebler, Engineer Corps U. S. Army, Milwaukee.

CHICAGO, July 1, 1867.

SIR: I submit for your inspection a plan of the proposed harbor improvement at Chicago by the Chicago Dock and Canal Company, and with it a letter from the Hon. William B. Ogden, (one of the largest proprietors,) describing the proposed improvement.

The work the company contemplates this year is the construction of the pier from F to E and from E to D, as shown on the plan. The dotted blue lines show the proposed work by the company from year to year as business may demand

its completion.

Very truly yours,

R. A. CONOLLY,

Chief Engineer Chicago Dock & Canal Company.

Major J. B. WHEBLER,

United States Engineers and Colonel U. S. Army.

A 12.

MILWAUKEE, WISCONSIN, September 1, 1867.

COLONBL: I have the honor to submit the following report on Michigan City harber:

This harbor was surveyed in April, 1867, by Captain A. Mackenzie, corps of engineers.

Congress had appropriated \$75,000 for its improvement, to be applied when the city should show that \$100,000 had already been expended in improving it.

The city having complied with its portion of the contract, the work was advertised and let on the 7th of August, 1867. The contemplated improvement consists in adding 288 feet of pier work to the eastern pier, the cribs composing the pier work to be each twenty-five feet wide, thirty-two feet long, and about twenty-five feet high and in adding 320 feet to the western pier, the cribs to be twenty feet wide, thirty-two feet long, and about seventeen feet high.

As the cost of a dredge and two mud-scows was included in the expenditure of the \$100,000 by Michigan City, the government claims the right to use the dredge and scows without incurring any additional expense except the running expenses of the dredge. The government is now dredging out the channel and

creek.

From the peculiar position of this harbor, at the head of Lake Michigan, exposed to all the severest winds, I hardly think it would be profitable to extend the piers further than the present contract calls for; and no matter how long they may be, they will not prevent the accumulation of sand at the mouth of the harbor, and the only remedy for this is periodical dredging. As the bed of the lake is a hard clay, the present dredging is quite a difficult work, but hereafter it will be quite easy, there being nothing but the accumulating sand to remove.

The importance of this harbor as a port of refuge can hardly be overestimated, as it is the only port south of Chicago where a vessel can take shelter. When finished, I also believe that it will be of great advantage to Indiana, affording the State an easy and cheap means of transporting her products and receiving those of other States. Now she has to depend on her railroads, always an expensive means of transportation.

Annexed there is the report of the collector, a tabular statement of material received and work done to 31st of August, and an abstract of the contracts for improving the harbor; also estimates for the cost of improving it.

Very respectfully, your obedient servant,

D. P. HEAP, Captain of Engineers.

Major and Brevet Col. J. B. WHEELER, Corps of Engineers.

The port of Michigan City is included in the collection district of Chicago. The nearest port of entry is Chicago. Michigan City was made a port of delivery by act of Congress, under date of February 23, 1865. Order of light fifth; fixed white light. No revenue collected. Imports on cargo, 158,000 feet lumber during 1866. No exports. Vessels sailing to Michigan City since July 1, 1866, 94; vessels sailing from Michigan City since July 1, 1866, 94.

Abstract of bids for improving the harbor at Michigan City, Indiana.

1. Francis A. Slater, Chicago.—12-inch square timber, at 14 cents per lineal foot; iron bolts, at 7½ cents per pound; stone, at \$2 66½ per cubic yard; framing, at 13 cents per lineal foot.

2. J. D. Dolan, Milwaukee.—12 inch square timber, at 15 cents per lineal foot; 3-inch plank, at 16 cents per lineal foot; iron bolts, at 6 cents per pound; iron spikes, at 8 cents per pound; stone, at \$2 62} per cubic yard; brush, at

75 cents per cubic yard; framing, at 17 cents per lineal foot.

3. GALBN EASTMAN, Grand Haven.—12-inch square timber, at 17.9 cents per lineal foot; 3-inch plank, at \$15 75 per thousand feet; iron bolts, at 5.9 cents per pound; iron spikes, at 7.9 cents per pound; stone, at \$2 90 per cord; brush, at 60 cents per cubic yard; framing, at 24 cents per lineal foot.

4. George Hannahs, South Haven,—12-inch square timber, at 16% cents per

lineal foot; framing, at 142 cents per lineal foot.

5. M. G. Sherman, Michigan City.—12-inch square timber, at 19 cents per lineal foot, pine; 12-inch square timber, at 17 cents per lineal foot, other; 3-inch plank, at \$16 per thousand feet; iron bolts, at 5 cents per pound; iron spikes, at 81 cents per pound; stone, at \$3 18 per cubic yard; brush, at \$1 25 per cubic yard; framing, at 17 cents per lineal foot.

6. CHAPIN & WELLS, Milwaukee.—Iron bolts, at 47 cents per pound; iron

spikes, at 6½ cents per pound.

7. HASBROUCK & CONRO, Milwaukee.—12-inch square timber, at 20 cents per lineal foot; iron bolts, at 7 cents per pound; stone, at \$16 per cord; brush, at

\$6 per cord; framing, at 15 cents per lineal foot.

8. JAMES H. MALLORY, Elgin, Kane county, Illinois.—12-inch square timber, at 21½ cents per lineal foot; 3-inch plank, at \$24 per thousand feet; iron bolts, at $5\frac{3}{4}$ cents per pound; iron spikes, at 13 cents per pound; stone, at \$15 90 per cord; brush, at \$6 per cord; framing, at 13d cents per lineal foot.

9. SILVER & Bowes, Michigan City.—12-inch square timber, at 19 cents per lineal foot; iron bolts, at 5½ cents per pound; stone, at \$15 75 per cord; fram-

ing, at 19½ cents per lineal foot.

10. BARKER & ROBERTS, Michigan City.—12-inch square timber, at 18 cents per lineal foot; 3-inch plank, at \$16 per thousand feet; iron bolts, at 49 cents per pound; iron spikes, at 8 cents per pound; stone, at \$15 50 per cord; brush, at \$3 per cord; framing, at 18 cents per lineal foot.

Abstract of contract for improving the harbor of Michigan City, Indiana.

Contractor.	Nature of contract.	Price.
F. A. Slater	Stone, per cubic yard	\$2 62 1 75 14
Chapin & Wells	cribs, per lineal foot. For bolts, per pound For spike, per pound.	13 4 6

Abstract of materials received and used, labor performed, and amount paid to cuntractors up to September 1, 1867, at harbor of Michigan City, Indiana.

Labor performed, 6,065 cubic yards of dredging. The contractors have not been paid for any work done or material delivered up to 1st September, 1867.

At the present contract prices the cost of extending these piers as proposed will be as follows:

CRIBS $32' \times 25' \times 25'$.

4,159 running feet of timber, at 14 cents	\$582 26
288 feet of plank, at \$14 per M	4 03

BELOBI OF THE SECRETARI OF WAR.		00
4,762 pounds of iron bolts, at 47 cents	\$ 232	15 39
112 cords of stone, at \$12 44	1, 393	28
13 cords of brush, at \$3 55	46	15
4,159 running feet of framing, at 13 cents	540	67
Cost of one crib		
•		
288 feet of pier-work will take nine cribs, which at this price will cost		
Add ten per cent. for contingencies	2, 519	03
Making the sum total for extending the eastern pier	27, 709	40
· •		
The cost of extending the western pier will be as follows:		
CRIB $32' \times 20' \times 17'$.		
3,378 running feet of timber, at 14 cents	\$ 472	92
288 feet of 3-inch plank, at \$14 per M	4	03
3,880 pounds of iron bolts, at $4\frac{7}{8}$ cents	189	15
6 pounds of iron spikes, at 6½ cents		39
90 cords of stone, at \$12 44	1, 119	
10 cords of brush, at \$3 55	35	
3,318 running feet of framing, at 13 cents	439	14
Cost of one crib	2, 260	
320 feet of pier-work will take ten cribs, which at this price will cost		
Add ten per cent. for contingencies		
	2 260	
to por outs to outsing outside the contract of the contra	2, 260 ———	
	2, 260 24, 868	
. Add cost of eastern pier	24, 868	00

Subtracting this sum from \$75,000, the amount of the appropriation, leaves \$22,422 60. a sum sufficiently large to pay for all the dredging required, estimating the cost of dredging at 20 cents per cubic yard. This sum will pay for 112,113 cubic yards, and I do not think it likely that a greater amount of earth will have to be removed.

The bottom of the channel is almost entirely composed of remarkably hard and tenacious clay. So difficult is it to be removed that in places it is necessary for the dredge to break it up into lumps before attempting to take it out. This dredge is extra large, and there is probably not a stronger one on the lake. I do not think that any reliable contractor could be found who would be willing to undertake the dredging for much less than 50 cents per cubic yard. There will be therefore a large saving on account of having the use of this dredge for nothing.

MILWAUKEE, WISCONSIN, September 12, 1867.

COLONEL: In addition to my report on Michigan City, I have the honor to respectfully submit the following estimate of the cost of dredging at that place. I only received the data to-day on which to base the estimate.

By roll of men engaged in working the dredge and hauling and

1,079 39

The pay-roll includes the pay of the foreman at six dollars per day.

The earth was dumped by laborers, who hauled the scows out into the lake by hand—a tedious, slow, and laborious process. During the greater portion of this month there will be a tug used for this purpose, and six of the laborers discharged. In this way I think there will be a much larger amount of earth excavated, and that it will cost no more per cubic yard.

Very respectfully, your obedient servant,

D. P. HEAP, Captain of Engineers.

Major and Brevet Col. J. B. WHEBLER, Corps of Engineers.

A 13.

MILWAUKEE, WISCONSIN, June 5, 1867.

General: In compliance with your instructions of March 9, 1867, I ordered Captain A. Mackenzie, United States engineers, on the 2d of April, 1867, to proceed to Michigan City, Indiana, and make a full and accurate survey of that harbor, a careful examination of the piers and other works, and to report as to the safety and convenience of the harbor for the purposes of commerce. A copy of his report, marked A, and a tracing of the map of the harbor, is herewith appended.

As the law requires that the Michigan City Harbor Company shall show an expenditure of the sum of one hundred thousand (100,000) dollars in the construction of a safe and convenient harbor at that place before the appropriation was available, the engineer and secretary of the company, Mr. Daniel Kennedy, was requested to inform me when the expenditures would reach that amount. I received a letter from him, dated May 21, 1867, stating that, including the pay-rolls for the month of May, the amount of expenditures of the company in improving the harbor would be slightly in excess of \$100,000.

I proceeded with Captain Mackenzie to Michigan City on the 31st of May, and on that day and the 1st of June made an examination of the harbor, the piers, dredges, scows, and vouchers of disbursement of the company. I find that they have expended, in the construction of the harbor at that place, a sum slightly in excess of \$100,000. An abstract of these expenditures is herewith appended, marked B.

The piers are composed of cribs filled with stone, and conform in the main features to the plans adopted by the engineer department. They were built by contract and appear to have been executed in a workmanlike manner. For dimensions, details, &c., see report of Captain Mackenzie.

The company contracted for dredging, and this was sub-let to Fox & Howard,

of Chicago, Illinois. Disputes between the contractors and the directors of the company resulted in an abandonment of the dredging by the contractors. Hence there was but little dredging done, and that of no use as far as improvement of the harbor was concerned.

The directors decided to build a dredge and do the work themselves. They now have a fine dredge with dump scows, the whole costing about \$17,000. The dredge was at work when I was there, and proved itself an excellent machine

capable of doing the required work.

The question is, have they constructed a safe and convenient harbor at Michigan City? My answer is, they have not, but have done much towards it By a liberal construction of the law, I think they have complied with its require-

ments, and are entitled to the benefit of the present appropriation.

I would recommend that this amount appropriated be applied to extending the northeastern pier a distance of two hundred and eighty-eight (288) feet on a line with its present direction, an extension of the western pier three hundred and twenty (320) feet on a line with the direction of the end crib, and dredge the channel between the piers and in the creek to a depth of twelve feet. These extensions to be composed of cribs built in accordance with plans approved by the engineer department. This will carry the piers into a depth greater than twelve feet of water, will widen the entrance to one hundred and fifty feet, and will render it a safe and convenient harbor for those vessels sailing to Michigan City, or compelled to seek refuge there during severe gales from the north and northeast, which are the worst that prevail in that section.

I would recommend, in preference to letting the work of dredging by contract, to use the dredge now on hand, paying the running expenses. And I take advantage of this opportunity to call the attention of the department to the fact that this company, consulting its best interests, have abandoned the system of contracting for dredging and do the work themselves, while we have abandoned the system of doing the work, and gone into a worse one of doing the work by contract. A latitude of judgment should be given in these matters to the officer in charge, and power given him to stop any contract, and do the work himself if

the contractors fail, or do not progress satisfactorily in their work.

I am, general, very respectfully, your obedient servant,

J. B. WHEELER,

Major of Engineers and Brevet Colonel.

Major General A. A. HUMPHRBYS,

Corps of Engineers, Chaef of Engineers U. S. Army,

Washington, D. C.

[A.].

MILWAUERE, WISCONSIN, May, 1867.

Colonel: In accordance with your instructions of April 2, 1367, I proceeded to Michigan City, Indiana, and made a complete survey of the proposed harbor. I have the honor to submit the following report, attached to which will be found a map of harbor, abstracts of work done by the Michigan City Harbor Company, money and material expended by them, and the material still on hand. These abstracts were prepared by the secretary of the company in my presence, and taken from the vouchers properly signed. I have added a table showing the shipments from Michigan City for the year 1847; and one showing the rise and fall of Lake Michigan during the time I was making the survey.

The harbor at Michigan City is in so poor a condition that it can hardly be called a harbor. The government commenced work here in 1836, and have expended in all about \$135,000. The different appropriations were small and

made at long intervals; large amounts were paid for salaries, dredges, scows, &c. The material was collected on the beach, and rotted before money could be had to put it in the piers. The work consequently went to decay and was abandoned. A few cribs were put in place; these have been cut down and made the foundations for new piers. The old plan of improvement contemplated a breakwater in front of the entrance, and one crib was put in place, but it was torn from its bottom timbers by the waves, and washed ashore in a few hours.

A few years since, the people of Michigan City and vicinity organized the Michigan City Harbor Company, with a capital stock of \$300,000. They raised by subscription about \$206,000. Authority was granted by Congress to use the old government piers as foundations, and contracts were let to repair the old work, extend the piers into 12 feet of water and dredge about 50,000 cubic yards. The contractors, failed to complete their work, only sinking 16 cribs and removing 10,000 cubic yards. Very little of the material dredged was carried out, but dumped into the channel between the piers; during the winter it washed back into its old bed.

The east pier is 1,135 feet in length, and the west pier 994 feet; 250 feet will carry the east pier over the bar. The cribs are substantially built, and their plan is similar to the one you have adopted. The channel between the piers is 100 feet wide, and the depth of water will only allow the passage of small fish-

ing boats.

Trail creek has an average depth of 3 feet, and a width of 60 or 70 feet; its bed is hard clay covered with sand, washed from the sand hills through which the creek passes. About three-quarters of a mile above the mouth the creek widens into a marshy swamp of more than fifty acres, which the company propose to use as a basin. It is to be dredged together with the creek, and the shores from the piers up to the railroad bridge docked. Above the Michigan Central railroad bridge is a still more extensive swamp. Nearly all the sand in the channel is washed from the hills below the bridge; very little comes down from above. Most of the wash will be stopped when the banks are docked, though some sand will still find its way into the channel and require occasional dredging. The harbor company have almost completed a dredge; it is built in the best possible manner, and will be in operation by the 1st of June, 1867. They propose to extend both piers this year.

A break-water at the entrance to Michigan City would be of great advantage to the commerce of the lake, as it would afford vessels a safe anchorage in hard mortherly blows; but to be of service, it should be at least half a mile from shore, and more extensive than the one formerly proposed. It would require a

very strong structure and cost an immense sum.

A good harbor is needed at Michigan City, not only on account of its being at the foot of the lake, where vessels can run when they cannot make side ports, but on account of the fine country back of it. For the year ending September, 1847, the shipments of grain, &c., were very large. A table of shipments, &c., is added. Most of the vessels were loaded at wharves running into the lake; others came up the creek and loaded at the warehouses shown on the map. The old wharves have disappeared. Keeping the harbor open will always be attended with more or less difficulty, but the people have gone to work with the determination of making a harbor, and I think they will succeed. They have already expended nearly \$100,000, and expect by the middle of May, 1867, to fully reach that sum.

I am, very respectfully, your obedient servant,

A. MACKENZIE,

Captain United States Engineers.

Brevet Colonel J. B. WHEBLER,

Major of Engineers United States Army,

Superintending Engineer of Harbor Improvements, Milwaukee, Wis.

Shipments of grain, &c., at M	lichiga n (Sity for the year ending Scpt	ember, 1847.
Wheat, corn, and other grain	s. bushel	3	. 617, 134
		rels	
Wool, sacks			. 175
Merchandise received, tons.	· · · · · · · ·		. 2,946
Number of arrivals of vessel	ls, includi	ng propellers	- 196
Number of times steamboats	s touched	at this point with passenger	rs
and freight	• • • • • • •	• • • • • • • • • • • • • • • • • • • •	- 144
Amount expended by the Mi		ity Harbor Company in but May, 1867.	ilding piers,
Organization collecting subs	crintions	&c	\$3, 964 81 .
Salaries	or ipuons,	••••••	2, 798 50
Office expenses, rent, furnitu	re. &c	• • • • • • • • • • • • • • • • • • • •	735 64
		• • • • • • • • • • • • • • • • • • • •	62, 417 10
		cribs)	6, 874 56
		, &c	5, 374 55
Dredge, four scows, pile drive	er, ropes,	chains, and tools	15,028 03
		• • • • • • • • • • • • • • • • • • • •	1,400 00
-			
		·	98, 593 19
•	36-4	2 2	
Timber onbig foot		ls on hand.	19 990
Plank M fact based masses	· · · · · · · · · · · · · · · · · · ·	• • • • • • • • • • • • • • • • • • • •	. 13, 332 . 55
Piles lineal feet	C		. 6,965
J			. 2002
, Materials used	in constr	uction and repair of piers.	
Timber, cubic feet			81, 118
Plank, M feet, board measur	·e		12
Iron, pounds		••••••	56, 241
Stone, cords	• • • • • • •		
• •	Amount o	f work done.	
	BAS	r Pier.	-
371 feet, 4 feet high, 20	0 feet wid	e .	
200 do. 5 do.	do.	-	
34 do. 7 do.	do.		
	· do.		
30 do. 9 do.	do.		
30 do. 10 do.	do.	crib sunk.	
50 do. 11½ do.	do.	do.	•
50 do. 13 do.	do.	do.	
30 do. 16 do.	do.	do.	
60 do. 20 do.	do.	do.	•
60 do. 23 do.	do.	do.	
60 do. 24 do.	do.	do.	•
60 do. 25 do. 60 do. 26 do.	do.	do.	•
60 do. 26 do.	do.	do.	

^{1,135} do.

WEST PIER.

385	feet,	, 4	feet high,	20 feet wide.	
	do.		do.	do.	
40	do.	6	do.	do.	crib sunk.
60	do.	8	do.	do.	do.
50	do.	103	do.	do.	do.
50	do.	13	do.	do.	do.
60	do.	16	do.	do.	do.
60	do.	22	do.	do.	do.

995 feet.

Record of rise and fall of Lake Michigan from April 12 to April 20, 1867.

Date.	7 a. m.	12 m.	6 p. m.	Remarks.
Friday, April 12	0 1.5 4.75 3.75 4.25 8 4.5 5 6.5	1. 25 3. 75 3. 5 5 8. 5 4. 75 4. 75	" 0 4.75 3.25 4.5 9 2.75 4	Southwest wind, light. Southwest wind, light. Southwest wind, light; rain. North wind, heavy; hard rain. North wind, very heavy; foggy; rain. Southwest wind, light. Southwest wind, light.

NOTE.—The numbers in the above table are relative. There has been no record kept for several years, and no marks could be found to which the present could be referred. The inhabitants think the lake is about two feet lower than at the time Mr. Bowes made his survey in 1852.

Statement of expenditures of Michigan City Harbor Company to date, June 5, 1867.

In organizing and collecting stock subscriptions	
Salaries Office expenses, rent, furniture, fuel, stationery, &c	735 64
Repairs and construction of piers	
Materials on hand, timber, plank, iron, piles and wood	
Dredge and four scows, pile driver, and other tools	17, 753 65
	100, 526 03

A 14.

MILWAUKEB, WISCONSIN, June 24, 1867.

GENERAL: I have the honor to transmit herewith the statement of expenditures made by the Michigan City Harbor Company in improving the harbor at Michigan City, duly authenticated by the engineer and secretary of the company, Mr. Kennedy, as requested by you in your letter of June 14, 1867.

I think in reference to the expenditure of \$17,753 65 for a dredge, scows, &c.,

that this amount can fairly be allowed them.

It is true that the sum is large, and that the amount of dredging required to be done by the United States would hardly seem to justify so large an expenditure of money.

But a close examination of the amount and kind of material to be dredged

changes in a slight degree this opinion.

It is estimated, by Captain A. Mackenzie, United States engineers, to require the removal of 45.000 cubic yards of material in order to deepen the present channel between the piers, from the extremity of the present eastern pier to the extremity of the inside government work, to a uniform depth of twelve feet. A portion, about one-half of the material to be removed will be a very hard and tough clay.

Contractors are paid by the cubic yard, measured in scows, and their prices at this place would average for the ordinary or soft material thirty-five cents per cubic yard, and for the hard clay about seventy (70) cents per cubic yard.

The 45,000 cubic yards, obtained by calculation from the cross-sections, would

measure from 60,000 to 75,000 cubic yards in scows.

Taking the smaller of these amounts, and estimating the cost of one-half at thirty-five cents and the other at seventy cents per cubic yard, we would find the cost to be not less than \$31,500. The cost of the dredging required to be done by the United States will not be less than this if done by contract.

And I firmly believe that they have shown true economy in building a dredge, and they can fairly claim the amount expended as coming under the head of money expended in the construction of a safe and convenient harbor at that

place.

The item of "organizing and obtaining stock," amounting to \$3,964 81, is hardly a legitimate expenditure under the law; but as this amount is less than five (5) per centum expended, I think it might fairly come under the head of contingent expenses and be allowed.

I am, general, very respectfully, your obedient servant,

J. B. WHEELER,

Major of Engineers and Brevet Colonel.

Major General A. A. HUMPHREYS,

Corps of Engineers, Chief of Engineers U. S. A., Washington, D. C.

Statement of expenditures of the Michigan City Harbor Company to June 5, 1867.

In organizing and collecting stock subscription	\$3,964 81 3,111 20
In office expenses, (rent, furniture, fuel, stationery, &c.)	735 64
In repairs and construction of piers	62, 478 25
In excavation, (dredging channel and for placing cribs)	7, 107 93
In material on hand, (timber, plank, piles, wood, and iron)	5, 374 55
In building dredge, four scows, pile-driver, and other tools	17, 753 65
	100, 526 03

DAN. KENNEDY,

Engineer and Secretary Michigan City Harbor Company.

STATE OF INDIANA, Laporte County, 88:

Be it known that on the nineteenth day of June, in the year 1867, before me, William Schoenemann, a notary public in and for said county, personally

appeared Dan. Kennedy, and made oath in due form of law that the annexed statement of the expenditures of the Michigan City Halbor Company is a true and faithful statement and exhibit of the amount of money expended on the harbor at Michigan City, under his direction as engineer of said harbor company.

Sworn and subscribed to before me the day and year aforesaid.

WM. SCHOENEMANN,
Notary Public.

I hereby certify that Dan. Kennedy, whose name is subscribed to the foregoing statement, is now, and has been since our organization as the Michigan City Harbor Company, engineer and secretary of the same; and that the annexed statement is a true and faithful exhibit of the amount of money expended by said company in the improvement of the harbor at Michigan City since its organization, October 12, 1864, under his superintendence.

D. J. BALDWIN,

Director and Treasurer Michigan City Harbor Company.

A 15.

MILWAUKEE, WISCONSIN, September 1, 1867.

COLONEL: I have the honor to submit the following report on the harbor at

New Buffalo, Michigan:

This harbor, as will be seen by the annexed table of exports and imports, is of no value commercially. As a harbor of refuge, however, it is of considerable importance, as it is to the leeward of the most prevalent and violent storms of Lake Michigan.

The present improvement consists in cutting through the embankment between Lake Pottawatomie and Lake Michigan, and in sheath-piling the sides of the cut. This cut is to be two hundred feet wide, and dredged to a depth of twelve feet. Such pier work is to be done as the appropriation will warrant.

I would recommend that next year two piers be built on the extension of the cut, and their end carried into twelve feet of water. They will each have to be six hundred feet long. The channel will also have to be dredged to a depth of twelve feet, necessitating the removal of 28,000 cubic yards of material.

The cost of this improvement will be as follows:

		•
One crib, $32' \times 20' \times 17'$.		
2, 368 running feet of timber, at 24 cents	\$ 568	32
3, 554 pounds of iron bolts, at 10 cents	355	
6 pounds of iron spikes, at 10 cents		60
226 cubic yards of stone, at \$2 70	718	20
32 cubic yards of brush, at \$3	96	00
2, 368 running feet of fencing, at 17 cents	402	50
288 feet of 3" plank, at \$30 per thousand		64
	2, 149	72
Add ten per cent. for contingencies	, 214	97
	2, 364	69
Forty cribs will then cost	\$94, 587	60
28,000 cabic yards of dredging, at 50 cents	14,000	
·	108, 587	60

Collection district of New Buffalo, Chicago. Nearest port of entry, Chicago. No revenue collected. No exports; no imports. Nearest light-house, at Michigan City. No vessels sailing to or from the harbor.

Abnexed is an abstract of contracts; also an abstract of work performed and

paid for up to date.

Very respectfully,

D. P. HEAP, Captain of Engineers.

Major and Brevet Col. J. B. WHEELER, Corps of Engineers.

Abstract of materials received and used, labor performed, and amount paid to contractors up to September 1, 1867, at harbor of New Buffalo, Michigan.

Materials received, none.

Materials used, none.

Labor performed, 11,250 cubic yards dredging.

Paid to Carkin & Kimball for dredging, \$2,135 20.

Abstract of contracts for improving New Buffalo harbor, Michigan.

Contractors.	Nature of contract.	Pri	ce.
Do		\$15 16	00
Do Do	Piles, per lineal foot. Stone, per cord of 128 cubic feet. Brush, per cord of 128 cubic feet. Framing, per lineal foot of timber		8 75 90 81
Do	Driving piles, per lineal foot of timber		6 ₁
Do	Dredging and excavating in hard material		70 4. 4.

A 16.

MILWAUKEE, WISCONSIN, September 1, 1867.

COLONBL: I have the honor to submit the following report on the harbor of

St. Joseph, Michigan.

This harbor, when developed, will make one of the finest harbors of the lake. St. Joseph's river, before entering Lake Michigan, spreads itself into a basin 600 feet wide, with an opening into Lake Michigan 225 feet wide. The town is situated on the south of this basin, and a good depth of water is obtained for a distance of 2,000 feet from the outlet. The channel averages 150 feet in width.

The north pier is about 1,100 feet long. This spring a contract was made to extend the south pier 200 feet. 700 feet more will be necessary to make this

pier equal in length to the north pier.

The current of the river is quite rapid, and as soon as it passes the end of the south pier it turns to the south and runs along the shore. By extending the routh pier, it will confine the current, and probably clear out a portion of the interior basin. I would therefore recommend that the south pier be made equal in length to the north.

The extension of the 200 feet previously spoken of costs at the rate of \$70

per foot of pier. It consists of a pile pier, with a crib-work superstructure. The additional 700 feet will cost at about the same rate, making a total of \$49,000. To build this extension of cribs will cost \$110 per foot of pier, making a total cost of \$77,000.

I would recommend that the extension be built of piles, on the score of ecosomy. Though not as lasting as crib-work, still they will make a very good, ser-

viceable pier, and will last for a long time.

The repairs on the old pier were let at the same time with the south pier excession. No additional appropriation should be necessary next year.

The following is an extract from a letter written on August 3, 1867, by Mr.

S. C. B. Carpenter, the government agent at St. Joseph's:

"The following information I have gathered from captains and others since I have been here:

"That the harbor is an easy one to make in a blow from the northerd, but

lacks sufficient depth of water on the bar for large vessels.

"It is the opinion of all captains sailing here that I have talked with that if the south pier were extended equal to the north pier, the current of the river would be sufficient to cut the bar, give a straight channel, and at all times sixteen feet water.

"There is a large fleet of vessels trading at the different piers between here and Michigan City and Saugatuck that make this a place of refuge during storms

"There are now four propellers making tri-weekly trips (two each night) between here and Chicago; another now being built (the Hippocampus) will run between here and Milwaukee.

"Mr. Aldrich, who took some pains last winter to obtain the amount of business done here, states that for the trade of this town about \$600,000 worth of merchandise were brought to St. Joseph's in 1866. Warehouse men state that about one-half that much is received for merchants back in the country.

"Shipments from the port for 1866 were, in part, 14.000,000 feet of lumber,

3,000 cords of wood, and fish to the amount of \$50,000.

"The lowest estimate I have heard for the crop of black and whortleberries not yet gathered is 1,500 bushels, and 700,000 baskets of peaches; 50,000 baskets of pears; 30,000 barrels of apples, and 100,000 boxes of grapes.

"Exports for the months of June and July, 1867, from the deputy collector

of the port of St. Joseph's:

Lumber, 2,634,000 feet.

Wood, 1,446 cords.
Railroad ties, 37,590.
Potatoes, 3,128 bushels.
Berries, 11,700 bushels.
Fish, 655 boxes of 200 pounds each.

Fish, 411 half barrels. Flour, 127 barrels. Leather, 128 rolls. Rags, 92 bales. Brick, 15,000. Tallow, 20 barrels.

"Whole number of clearances within this period, 178; aggregate tonuage, 29,577 tons; revenue obtained from clearances is about \$800 per year."

Annexed is a tabular statement of material required, and work done up to date, also an abstract of the contracts.

Very respectfully, your obedient servant,

D. P. HEAP, Captain of Engineers.

Major and Brevet Col. J. B. WHEBLER,

Corps of Engineers.

Note.—The light-house is at the mouth of the river, south side; the light is fixed, varied by flashes; intervals of flash, one minute and thirty seconds; distance visible, fifteen nautical miles; light of fourth order. The light is on the keeper's wooden dwelling.

D. P. H.

Abstract of contracts for improving the harbor at St. Joseph's, Michigan.

Contractors.	Nature of contract.	Price.
120	For oak piles, each For pine timber, per cubic foot For brush, per cord For stone, per cord For iron bolts, per pound For iron bolts with nut and screw, per pound	าย กก

Abstract of materials received and used, labor performed, and amount paid to contractors up to September 1, 1867, at harbor of St. Joseph, Michigan.

Materials received of Hasbrouck & Conro, 20,913 feet timber, 171 piles, 551 cords stone, 30 cords brush, 26,378 pounds iron.

Materials used, 16,101 feet timber, 145 piles, 250 cords stone.

Labor performed, 16,101 feet framing.

Paid to Hasbrouck & Conro for timber, piles, stone, and iron, \$20,708 79.

A.17.

MILWAUKEE, WISCONSIN, September 1, 1867.

COLONEL: I have the honor to submit the following report on the harbor of South Haven, Michigan:

This harbor is situated about sixty miles south of Grand Haven, it is important as a harbor of refuge and as an outlet to the lumber district of the counties of Allegan and Van Buren.

The proposed plan is to extend two piers 120 feet apart until they reach a depth of twelve feet, and to widen the river between the old piers; the only modification that I would suggest is to make the distance between the piers 200 feet.

I would respectfully refer to Assistant U. T. Casgrain's report of last December for estimates and a more detailed account of the place.

I wrote last July to obtain information concerning the exports and imports of

the place, but as yet have received no reply.

Annexed is a tabular statement of work done and material received, also abstract of contracts.

Very respectfully,

D. P. HEAP, Captain of Engineers.

Major and Brevet Col. J. B. WHEBLER,

Corps of Engineers.

Abstract of contracts for improving the harbor of South Haven.

Contractors.	Nature of contract.	Price.
Do	12-inch timber, per M, board measure 3-inch plank, per M, board measure. Piles, per lineal foot. Stone, per cord of 128 cubic feet. Brush, per cord of 128 cubic feet. Iron bolts, per pound. Iron spikes, per pound. Framing, per lineal toot of timber.	14 37 1 2 00 4.9

Abstract of materials received and used, labor performed, and amount paid to contractors up to September 1, 1867, at harbor of South Haven, Michigan.

•		Materials received.			
Contractors.	Pine timber.	Other timber.	Stone,	Iron.	
Galen Eastman R. Nelson Gere	Feet. 2, 974	Feet. 20, 166	Cords.	Pounds. 29. 074	

Materials used, 15,908 feet timber, 387 cords stone and 26.655 pounds iron. Labor performed, 15,908 feet framing, and nine cribs placed.

Contractors.	For what paid.	Am't p'd.
Galen Eastman	Stone and timber	\$9, 323 8 2, 127 4

A. 18.

BLACK LAKE, MICHIGAN.

This harbor was surveyed in October, 1866. A full report, showing the condition of the harbor and proposing plans for its improvement, was made at that time, and will be found in your last annual report. The plans there proposed were to improve the present outlet, rather than open a new channel. To do this the north pier was to be extended 250 feet, and the south pier 275 feet. At the entrance into Black lake the channel was to be close piled, 1,125 feet on the north side and 425 feet on the south side. The whole channel was to be dredged to a depth of twelve feet. These plans were approved and contracts let for all the work except the close piling. It was thought the appropriation would not be sufficient for the latter work.

The dredging was commenced August 6, and to the present time about 15,000 cubic yards have been removed. When the weather is pleasant the dredge works outside, preparing a foundation for sink pieces, and when rough it works inside between piers. It was so late when the dredging was commenced, that the framers have not had time to complete any work. Before the close of navigation they will place twelve cribs, six on each pier, and dredge the channel to twelve feet. Some of the cribs in place at the time of survey rested on sink pieces, and others on the sand. While those on the sand had settled unevenly and tipped, the cribs resting on the sink pieces were level and firm; it was therefore determined to use sink pieces as foundations for the new cribs. The sink pieces are made of brush done up in bundles, and formed into a float thirty feet wide and long enough to extend under two or more cribs, breaking joints with them. The bottom is dredged level and the sink pieces loaded evenly with stone until they sink.

The cost of entire work, including close piling, at present contract prices will be about \$98,000. The appropriations of 1866 and 1867 amount to \$106.615 31, and will be sufficient to complete the whole work in a permanent manner.

Black lake is in the district of Michigan, twenty wo miles south of Grand Haven, the nearest port of entry. The nearest light-house is at the mouth of the Kalamazoo river, eight miles distant; it is a fixed light of the sixth order.

Holland, at the head of the lake, six miles from the mouth, is the nearest settlement.

There are fifty-seven small vessels sailing to and from this harbor. The number of arrivals and departures during the past year has been about seven hundred, and the amount of revenue collected \$1,000.

The exports have been as follows:

Sawed timber, feet board measure, 1,066,808	\$ 53, 340	00
Fire-wood, cords, 10,489	41,956	00
Staves, number, 2,733,462	54,669	24
Railroad ties, number, 29.985	8, 995	50
Hemlock bark, cords, 2,420	12, 100	00
Shingles, thousand, 379	1, 516	00
Sawed heading, pieces, 1,088,262	6, 529	57
Staves and shingles bolts, cords, 87	870	00
Leather	112,000	00
Miscellaneous articlesflour, butter, &c	65, 214	
	357, 191	

The imports, consisting of merchandise, machinery, hides, household furniture, castings, manufactures, emigrants' effects, &c., will amount in value to not less than \$450,000.

The benefits to commerce and navigation from the completion of this harbor are many. As a harbor of refuge it can hardly be excelled; it is easy of access, and has a capacity for any number of vessels, and vessels of any size. It will open to commerce a large tract of country, settled principally by Hollanders, and will have a tendency to increase the emigration of a class of thrifty and industrious European laborers. It opens to cultivation a tract of land valuable for fruit culture; and it is estimated that within a few years the shipments of fruit alone will exceed present exports.

I am, very respectfully, your obedient servant,

A. MACKENZIE, Captain United States Engineers.

Abstract of material received and used, labor performed, and amount paid to contractors up to September 1, 1867, at harbor of Black Lake, Michigan.

Materials received.				
Timber.	Plank.	Stone.	Brush.	Iron.
Feet.	Feet. 4.608	Cords.	Cords.	Pounds.
		354	80	56,012
	Timber. Feet. 44,808	Timber. Plank. Feet. Feet. 44,808 4,608	Timber. Plank. Stone. Feet. Feet. 44,808 4,608	Timber. Plank. Stone. Brush. Feet. Feet. Cords. Cords. 44,808

Materials used, 30 cords stone. Labor performed, 13,447 yards dredging one sink piece in place.

Contractors.	For what paid.	Am't paid.	Remarks.
James H. Ledlie,	Timber and plank	8, 169 12	Amount with 10 per cent., \$18,314 e5.
Am't expended, less 10 per cent.		16, 483 38	

Abstract of contracts for improving Black Lake harbor, Michigan.

Contractors.	Nature of contract.	
	12-inch timber, per lineal foot	\$ 0 20
Do	3-inch plank, per thousand, board measure	25 00
L. N. Kimball	Sink pieces placed, per square yard Stone, per cord of 125 cubic feet	3 70 13 40
Do	Brush, per cord of 128 cubic feet	3 (0
J. H. Ledlie	Iron bolts, per pound	6.9
Do	Iron spikes, per pound	15 9
J. E. Miller	Framing, per lineal foot of timber	
Do	Filling with stone, per cord	80
Do	Dredging in ordinary material, per cubic yard	344
Do	Dredging in hard pan or stiff clay, per cubic yard	69

A 19.

GRAND HAVEN, MICHIGAN.

The last survey of this harbor was made in 1865 by Colonel Raynolds. A tracing, on which was marked the proposed improvements, was forwarded last year. These improvements were, to extend south pier 600 feet; to build a north pier, commencing at shore opposite inner end of south pier and running parallel to south pier, to twelve-feet curve. The width of the channel will be 200 feet, and the direction of the piers will be that of present south pier. The bend in river on south side was to be close-piled.

The present appropriation, \$105,000, will be sufficient to complete south pier and repairs. For the construction of a north pier \$200,000 will be required,

\$75,000 of which can be used during the next fiscal year.

The contractors for the pier work commenced operations in June, 1867. They have placed six cribs in extension of south pier, and do not propose putting in any more this year. The superstructure of this work will be put on before the close of navigation. The cribs have been very poorly placed—they are uneven and out of line. To complete the superstructure piles must be driven to complete the line where cribs are displaced. The piles are necessary to prevent forming a shoulder, upon which the waves and ice would have great effect, and probably remove the superstructure. The trouble in sinking the cribs has arisen from not levelling the bottom to receive them. The close-piling was commenced this month, and will be completed this year. It has been run straight across the bend, instead of following the fifteen-feet curve, as was first intended. The latter plan gave a curved line, which would turn the current more gradually and avoid all danger of the piles being undermined; but they are now free from all danger, being driven firmly in very deep water.

The 309 feet of south pier repaired last year has not been completed. The stone for new work was so piled on this portion, the fender piles could not be

driven. It was concluded to omit driving the piles at present, as they were not considered necessary for the safety of the work. The piles at the end on which the superstructure was built were some distance apart, and no intermediate piles could be driven on account of stones, which had gone through and covered the bottom. The end of this work should be filled with slabs and stone, which will

now be kept in by a crib, which abuts against old work.

Grand Haven is the port of entry for the district of Michigan, which embraces all harbors from Aux Bec Scies south to St. Joseph. It is an important point, being one terminus of the Detroit and Milwaukee railroad, and on an extensive line of communication between the east and west. It is also a lumber market of some importance. It has a light-house, with light of —— order, a pier light, range lights, and a fog bell. The harbor has been this season in very good condition, there being fifteen feet of water on the bar between the red and black buoys, which mark the channel. When the close piling now under way is completed, all the bends in shore, from which sand formerly washed, will be protected. With a north pier the current, which is very strong at times, will be confined, and must keep the channel open.

It is estimated that, on an average, thirty vessels are lost annually, in consequence of not being able to enter the harbor during storms. A good and safe harbor, easy of access, south of Big and Little Points au Sable, would lead vessels to make it, in heavy weather, from the north, instead of attempting to weather these points, and running great risks of being driven upon the shore.

The number of vessels entering and leaving this harbor during the past year is 4,000, and the amount of revenue collected is \$5,000.

I am, very respectfully, your obedient servant,

A. MACKENZIE,
Captain United States Engineers.

Abstract of contract for improving Grand Haven harbor, Michigan.—Piering.

Contractor.	Nature of contract	Price.
	12-inch timber, per lineal foot	\$0 22
Do Do	3-inch plank, per thousand, board measure Piles, pine, per lineal foot Piles, oak, per lineal foot	25 00 8 17
Do	Stone, per cord of 128 cubic feet	16 94 2 20
Do	Iron bolts, per pound	9
Do	Framing, per lineal foot of timber Driving piles, per lineal foot	11

Abstract of contracts for improving Grand Haven harbor, Michigan.—Close piling.

Contractor.	Nature of contract.	Price.
Do	12-inch timber, per lineal foot	18 75
Do	Brush, per cord of 128 cubic feet	9 1 75

Abstract of materials received, labor performed, materials used, and amounts paid to contractors at harbor of Grand Haven, to reptember 1, 1867.

	Materials received.				
Contractors.	Pine timber.	Other timber.	Piles.	Iron.	Stone.
James H. Ledlie	Feet. 14,760 7,788	Feet. 5, 168	Feet. 58, 638	Pounds. 99, 894 30, 800	Cords. 627

Materials used, 15,533 feet timber, 820 piles. Labor performed, 15,533 feet framing, 826 piles driven, 6 cribs.

Contractors to whom paid	For what paid.	Amount.	Remarks.
J. H. Ledlie Do	Timber Stone	\$3,834 08 9,559 25 3,537 48	Amount with 10 per cent., \$3,230 90.
Do	Framing Timber Piles Iron Framing	1,228 40 7,671 84 5,276 42 2,494 80 174 96	
Do		1,300 94	

A 20.

MUSKEGON, MICHIGAN.

The plans for improving this harbor, as determined upon last year, have never been changed. The plan is to extend both piers on a line parallel to the inner face of south pier, commencing at the present extremities of present piers, and extending them to seventeen feet of water. For this extension there is required 700 feet of pier work. Its estimated cost was \$58,450.

Fifty-nine thousand dollars (\$59 000) has been appropriated for the work, and will be sufficient to complete it. Contracts for extending the piers seventy feet (twenty-two feet) were made May 31, 1867. The delivery of stone was commenced immediately, and more than one thousand cords have been already received. A quantity of stone has been lost in the channel through the care-lessness of vessels carrying stone.

The contractor for framing has obtained permission to build his cribs at Grand Haven and tow them to Muskegon. He has not yet placed any, but has several, and ready to put in when the weather will permit.

No reports of exports or imports have been received. The shipments of lumber will probably amount to 200,000,000 feet. The number of vessels arriving and departing last year was about ,3,000, and the amount of revenue collected \$4,000.

Muskegon is in the collection district of Michigan, and the nearest port of entry is Grand Haven, twelve miles south. It has a small light-house, with lights of sixth order.

I am, very respectfully, your obedient servant,

A. MACKENZIE, Captain United States Engineers. Abstract of materials received and used, labor performed, and amount paid to contractors up to September 1, 1867, at harbor of Muskegon, Michigan.

Contractors.	Materials	received.
	Pine tim- ber.	Stone.
Ladia & Corea	Board meas	Cords.
Ledlie & Corse	174, 744	1, 185

Materials used, none; labor performed, none.

Contractors.	For what paid.	Amount paid.	Remarks.
Ledlie & Corse	Stone Timber	\$12,442 04 2,017 77	Amount with 10 per cent., \$16,066 44.
Amount expended, less 10 per ct.		14,459 81	,

Abstract of contracts for improving harbor at Muskegon, Michigan.

Contractors.	Nature of contract.	Price.
Do	12" timber, per M, board measure. 3" plank, per M, board measure. Piles, per lineal foot. Stone, per cord of 128 cubic feet. Brush, per cord of 128 cubic feet. Iron bolts, per lb. Iron spikes, per lb. Framing, per lineal foot of timber.	14 00 08 13 88 2 00 4.9

A 21.

WHITE RIVER, MICHIGAN.

From the results of the survey of this harbor in October, 1866, it was determined to make a new cut at the foot of White lake, and thus avoid the narrow, winding channel through which the lake now discharges itself into Lake lichigan.

It was estimated last year that the total cost of harbor would be \$170,530 80. The prices at which contracts were let are much below estimated prices, and the above amount will still be amply sufficient. During the next fiscal year \$75,000 can be judiciously expended. The contractors, Messrs. Fox and Howard, of Chicago, commenced dredging in July, 1867, and have already removed over 20,000 cubic yards. They expect to finish the cut before the close of navigation.

The contractors for material have commenced the delivery, and all the timber

and piles will probably be received this year.

The framing contractors are to close pile the cut, and build such pier-work as shall be necessary. They will be unable to commence this work until the cut is finished.

With balance of present appropriation, after dredging and close piling, ten or

twelve cribs can be placed.

White river is in the district of Michigan, and the nearest port of entry is Grand Haven. A small light is kept up by individual enterprise on north pier. The nearest light house is at Muskegon, twelve miles distant. The number of arrivals and departures for the last fiscal year have been about nine hundred.

There is only five feet of water in the channel, and the class of vessels run-

ning to and from the harbor, necessarily small.

The shipments for the past year have been as follows:

Lumber, feet, board measure	. 60,000,000
Shingle bolts, cords	. 1,000
Shingles	20,000,000
Square timber, feet	. 500,000
The imports consisted of dry goods, provisions, &c., and amount	int in value to

\$300,000; machinery of all kinds, \$100,000.

White lake is a beautiful body of water, six miles long, three miles wide, and sufficiently deep for vessels of any size. The principal settlement is Whitehall, formerly Mears, at the head of the lake.

I am, very respectfully, your obedient servant,

A. MACKENZIE. Captain United States Engineers.

Abstract of materials received and used, lahor performed, and amount paid to contractors up to September 1, 1867, at harbor of White Lake, Michigan.

Materials received, none.

Materials used, none.

Labor performed: Dredging, 21,335 feet.

Amount paid for dredging to Fox & Howard, \$3,669 19.

Abstract of contracts for improving White River harbor, Michigan.

Contractors.	Nature of contract.	Price.
Do		14 50
Carkin & Kimball	Piles, per lineal foot Stone, per cord of 128 cubic feet Brush, per cord of 128 cubic feet	14 90
R. Nelson Gere	Iron bolts, per pound	4.
Do	Framing, per lineal foot of timber	51

A 22.

PENTWATER, MICHIGAN.

The plans and estimates for this harbor were made last year from a map prepared by the lake survey. The plan adopted for its improvement was to throw aside entirely the old slab piers; they were so irregular, and ran in so many directions, that no use could be made of them. The width of the outlet was to be increased to one hundred and fifty feet, the sides of the cut close piled, and piers extended into Lake Michigan over the bar a distance of twelve hundred and eighty feet. The whole channel was then to be dredged to a depth of twelve feet. The direction of the proposed piers was to be north seventy degrees west, (magnetic.) This has been slightly modified. The south pier commences at end of present south pier. In extending this pier the first crib has been made to abut against the southern portion of old pier, leaving about thirty feet of old slab work to be removed.

The north pier will commence at some point of old pier and be parallel to

south pier.

The channel should be two hundred feet in width, but it will not be practicable to give it a greater width than one hundred and fifty feet.

The estimated cost of work last year was \$327,713 40. This amount will

be sufficient to complete the work in a permanent manner.

For the next fiscal year \$50,000 will be required.

Pentwater is in the collection district of Michigan, and the nearest port of

entry is Grand Haven, sixty miles distant.

The nearest light-house is at Muskegon, fifty miles south. There are from fifteen to twenty vessels arriving and departing weekly; the total number of arrivals and departures during the past year being about eight hundred.

The shipments for the past season have been: Lumber, about 20,000,000

The shipments for the past season have been: Lumber, about 20,000,000 feet, board measure; shingles, about 25,000,000, number; bolts, 100,000 cords.

The imports compare favorably with exports, and consist principally of mer-

chandise.

Pentwater is in the bay formed by Big and Little Points Au Sable, and when the harbor is completed will be easy of access in all kinds of weather.

As a benefit to commerce the effect of the appropriation has already been felt. The amount of business has almost doubled during the past season.

I am, very respectfully, your obedient servant,

A. MACKENZIE, Captain United States Engineers.

Abstract of materials received and used, labor performed, and amount paid to contractor up to September 1, 1867, at the harbor of Pentwater, Michigan.

Materials received: P. M. Danaher, timber, 41,666‡ feet, 7,000 plank; F. D. Van Wagener, stone, 395 cords. Materials used, none.

Materials used, none. Labor performed, none.

Contractors.	For what paid.	Amount paid.	Remarks.
P. M. Danaher F. D. Van Wagener	Timber	\$4 575 60 4, 320 00	Amount with 10 per cent., \$9,884.
Total		8,895 60	

Abstract of contracts for improving Pentwater harbor, Michigan.

Contractors.	Nature of contract.	Price.
Do	12-inch timber, per lineal foot. 3-inch plank, per M, board measure. Piles, per lineal foot. Stone, per cord of 128 cubic feet. Brush, per cord of 128 cubic feet Iron bolts, per pound Iron spikes, per pound Framing, per lineal foot of timber. Dredging, per cubic yard.	16 00 6 00 4. 4.

A 23.

PÈRE MARQUETTE.

This harbor was surveyed by Colonel Raynolds last year. The plan adopted for its improvement is to build a south pier, commencing at a point on shore twenty feet south of present slab pier, and extending to bar, a distance of about six hundred and forty feet; to extend the north pier to the bar, a distance of four hundred and fifty feet; to remove the old slab pier on south side of channel; to cut down slab pier on north side to surface of water and replace it by crib work; to close pile to south side and to dredge to a depth of twelve feet, if necessary.

The direction of the piers will be north three degrees west, and the channel two hundred feet wide. The estimate for the improvement of this harbor made last year was \$270,682 16, and will be sufficient to complete the work in a

permanent manner.

During the next fiscal year \$75,000 can be used. The contractor for building the piers, Mr. P. M. Danaher, is required to put in place eight cribs this year. He commenced the work in June, and has placed fourteen cribs up to this date. He expects to finish the entire work this year. The cribs already in position have not settled evenly, but the line is good, and there will be no difficulty in building the superstructure. Some cribs have settled into the sand six feet.

In accordance with your instructions I applied to parties at Père Marquette for information concerning the imports, exports, &c. I have been unable to learn anything. The only export of any consequence is lumber. The number of arrivals and departures of vessels for the past year was about five hundred.

Père Marquette is in the collection district of Michigan. The nearest port of entry is Grand Haven, and the nearest light-house at Muskegon. A light-house is being erected at Big Point Au Sable, eight miles north. The light, I understand, is to be of the first order.

I am, very respectfully, your obedient servant,

A. MACKENZIE,
Captain United States Engineers.

Abstract of contracts for improving Père Marquette harbor, Michigan.

Contractors.	Nature of contract.	Price.
P. M. Danaher Do Do Ledlie & Corse Do R. Nelson Gere Do Hasbrouck & Conro Do	3-inch plank, per M, board measure	13 98 1 49 4.9

Abstract of materials received and used, labor performed, and amount paid to contractors up to September 1, 1867, at harbor of Père Marquette, Michigan.

Materials received: P. M. Danaher, timber, 41,6661 feet; plank, 7,000 feet, board measure; Ledlie & Corse, stone, 1,378 cords.
Material used, none.

Labor performed: Framing, 24,000 feet; cribs placed, 14.

Contractors.	For what paid.	Amount paid.	Remarks.
Ledlie & Corse	Timber	\$17,338 00 4,800 89 3,024 00	Amount with 10 per cent., \$27,958 79.
Total		25, 162 89	

A 24

MANISTEE, MICHIGAN.

This harbor was surveyed in September, 1866, by W. T. Casgrain, assistant engineer, and party. Plans based upon the result of this survey were discussed in last yearly report. The plan of improvement proposed was to extend south pier in present direction nine hundred and sixty feet; to commence north pier at a point on present north pier opposite outer end of south pier and run it in a direction parallel to south pier to the twelve foot curve, a distance of nine hundred and sixty feet; to cut off a point of slab work inside south pier and widen the channel: to cut down all slab work to water surface and build crib work on old foundation; and to dredge the channel to a depth of twelve feet.

Since the survey one hundred feet of the north pier has been washed away. This has caused a slight change in the plans, the north pier being thrown twenty feet further to the north, and commenced at end of old pier. The estimates last year called for \$180,949 to complete the work; \$60,000 has since been appropriated, leaving \$120,949 still needed. Of this \$60,000 can be used

during the next fiscal year.

Contracts have been made with Mr. Starke, of Milwaukee, to build and sink twenty-four cribs (768 feet) in extension of north pier. He is to sink not less

than eight cribs this season; he has already placed three cribs.

There is at present eight and a half feet of water on the bar in front of entrance, from nine and a half to ten feet between piers, and from seven and a half to eight feet in river above the piers. To deepen the channel between Lake Michigan and Manistee lake to a depth of twelve feet will require the removal of four and a half feet of dirt for the whole length of Manistee river, a distance of one mile; the average width is about three hundred feet. This work has not been included in estimates. It has not been considered a portion of the work ordered. If it is to be done it will absorb the entire appropriation.

Manistee is in the collection district of Michigan, and the nearest port of entry is Grand Haven. The nearest light-house in operation is at Point Betsie, thirty-five miles north. A light-house is being built at Big Point Au Sable, twenty

miles south of Manistee.

There is at present not even a pier light at Manistee, but money has been appropriated, and a light will be put up at end of piers when they are completed.

The number of vessels arriving and departing the past year is as follows:

Sail vessels. 640; averaging thirty-five per week this season.

Steamers, 95; averaging five per week this season.

The exports for past year are:

Material.	Quantity.	Value
Lumber and timber (pine)	64,000,000 feet.	\$800,000
Spars	1,000	25,000
Railroad ties	5,000	1,000
Cords of wood	3,000	7,500
Barrels of fish	• • • • • • • • • • • • • • • • • • • •	15,000
		848,500
•		=
The imports are:		
General merchandise		\$700,000
Live stock		50,000
Machinery		60,000
Fruit trees		4,000
•		814,000

Manistee is the shore town of Manistee county; it lies in latitude 44° 20' north; it is about ninety miles north of Grand Haven, and one hundred and eighty-five by steamboat route from Milwaukee. Its present population is only three thousand, but its growth is very rapid, and the number of its inhabitants will soon be doubled.

The country back of Manistee is heavily wooded. The soil varies, but is principally a sandy loam. New saw-mills are being erected, and in one or two years Manistee will become probably the largest lumber market in the world. For the past six years navigation has been closed on an average eighty-seven days. There is very little float-ice off the mouth of the harbor, the current being sufficiently rapid to keep it clear. Vessels all pass within twelve miles of Manistee, and will find this a safe and convenient harbor of refuge when ice prevents them from making other ports. As a wooding point it will be an important aid to steam navigation. It will furnish an outlet for the products of a large tract of country five hundred square miles in extent, rapidly being opened and settled.

The Big Manistee river, which empties into Manistee lake, and from thence into Lake Michigan, is navigable for small boats for a distance of one hundred and fifty miles from its mouth, but navigation on it is almost entirely obstructed by logs being floated down to the mills.

I am very respectfully, your obedient servant,

A. MACKENZIE, Captain United States Engineers.

Abstract of contracts for improving Manistee harbor, Michigan.

Contractors.	Nature of contract.	Price.
Galen Eastman Do R. Nelson Gere	3-inch plank, per M, board measure Piles, per lineal foot Stone, per cord of 128 cubic feet	14 00 8 14 88 2 00 4.9

Abstract of materials received and used, labor performed, and amount paid to contractors up to September 1, 1867, at harbor of Manistee, Michigan.

Materials received: Gelley & Weston, timber, pine, 4,925 lineal feet; other timber, 15,163 lineal feet; Galen Eastman, stone, 493 cords; R. N. Gere, iron, 33,027 pounds.

Materials used: Timber, 6,460 feet.

Labor performed: Framing, 6,460 feet; cribs placed, 5.

Contractors.	For what paid.	Amount paid.	Remarks.
Gelley & Weston	Timber Stone	\$2,798 67 4,901 48	Amount with 10 per cent., \$8,555 72.
Total		7,700 15	

A 25.

AUX BEC SCIES, MICHIGAN.

This harbor was fully discussed in your last yearly report, and the plans for its improvement were then given. They were to make a new cut seven hundred and fifty feet south of present outlet, two hundred feet wide, to protect the sides of the cut by close piling, to extend piers into Lake Michigan eleven hundred and twenty feet due west, and to dredge the entire channel to a depth of twelve feet.

This work was surveyed the present month by Mr. Hearding, assistant engineer, and party. The results of the survey show that the only change in plan required will be to extend the piers further into Lake Michigan, and increase the estimate for close piling and dredging. The north pier will be six hundred and seventy-two feet (twenty-one cribs) in length, and the south pier eight hundred and thirty-two feet (twenty-six cribs) in length. There will be required five hundred and fifty feet of close piling, and eighty-five thousand cubic yards of dredging. The cost of this work at contract prices will be—

of dredging. The cost of this work at contract prices will be—	cubic yarus
For pier work, 47 cribs	\$103, 400 13, 000 30, 000
Total	146, 000 98, 541
Balance required	47, 859

The contract requires the work to be finished in 1868. The total amount,

\$47,859, will be required during the next fiscal year.

The contractors, Messrs. Whitewood and Hubbell, of Detroit, commenced the work last winter, collecting materials, &c. As soon after the opening of navigation as practicable they commenced the dredging, and up to date have removed 40,000 cubic yards. They expect before the close of navigation to finish the cut, close-pile the channel, and sink fifteen cribs, (480 feet.)

Frankford, or Aux Bec Scies, is thirty miles north of Manistee. Its nearest light-house is on Point Aux Bec Scies, or Point Betsie, as it is generally known. Betsie light is four and six-tenths miles north of the new channel; it is a fixed light of the fourth order, varied by flashes. Frankford is in the collection dis-

trict of Michigan, and its nearest port of entry is Grand Haven.

Its imports for last year amounted in value to \$4,000, and consisted of dry goods and lumber; its exports consisted of cord wood, and amounted in value to \$120. The importance of this harbor is as a harbor of refuge. It is an exposed point, and in the vicinity of one of the most dangerous parts of the lake. All vessels bound up or down pass within five miles of the harbor. It will without doubt afford a safe anchorge during storms for many vessels yearly, and will save more to commerce in a short time than has been expended in its construction. A large tract of land around Frankford, extending fifty miles north and south and seventy-five miles east, is being settled slowly. This country abounds in hard timber, and the soil is good.

Betsie river is unnavigable except for small flat-boats; logs cannot be floated down. There is but one small saw-mill on the little lake, and that seldom in operation. The great difficulty in obtaining logs in great quantities will prevent

this point from ever becoming a great lumber market.

Bentzonia, a small, religious village, ten miles inland on the Betsie river, is

the only settlement of any importance in the vicinity.

Crystal or Cass lake, two miles north of Frankford, is a large and beautiful sheet of water; it is only one-half mile from Lake Michigan, but many feet above it. It is surrounded by high hills, and has no visible inlet except the sides of the hills during rains. Near the middle it is several hundred feet in depth. It has an outlet into Betsie river, open at times, but generally closed. It is proposed by the inhabitants of the surrounding country to lower Crystal lake two feet to drain some valuable land now under water. Were this lake on a level with Lake Michigan, and a communication practicable, a magnificent harbor could be made.

I am, very respectfully, your obedient servant,

A. MACKENZIE, Captain Engineers.

Colonel J. B. WHEBLER,

Major Corps of Engineers U. S. Army, Superintendent of Harbor Works, Milwaukee, Wisconsin.

Abstract of contracts for improving Aux Bec Scies harbor, Michigan.

Contractors.	Nature of contract.	
Whitewood & Hubbell	3-inch plank, per M, board measure Pile*, 30 feet long, oak, each Piles, 30 feet long, elm, each Stone, per cord of 128 cubic feet Brush, per cord of 128 cubic feet Slabs, per cord of 128 cubic feet Wrought-iron bolts, 11 square, per pound Driving piles, per pile Framing and building cribs Placing, sinking, and filling	20 00 5 75 2 75 17 00

Abstract of materials received and used, labor performed, and amount paid to contractors up to September 1, 1867, at Aux Bec Scies, Michigan.

Materials received: Whitewood & Hubbell, timber, 30,885 feet; piles, 634; iron, 121,821 pounds.

Materials used: Piles, 23.

Labor performed: Dredging, 32,688 feet; piles driven, 23.

Amount paid: Whitewood & Hubbell, dredging, timber, &c., \$25,159 38; amount with ten per cent., \$27,954 86.

A 26.

OFFICE UNITED STATES ENGINEERS, Milwaukce, Wisconsin, September 2, 1867.

COLONEL: In compliance with article five of your circular, dated July 20, 1867, as follows, viz: "Assistant W. T. Casgrain is assigned to the special superintendence of the surveys being or to be made by this office," I have the honor to present you a general report upon the several surveys made since

August 10, 1866, at which time I reported to you for duty.

From that date to September 6, 1866, the time was occupied in making tracings of the following maps: Grand Haven, Muskegon, and St. Joseph's harbors, Michigan, Chicago harbor, Illinois, and Racine, Kenosha, and Sheboygan harbors, Wisconsin, which were surveyed under the direction of Colonel W. P. Raynolds, superintendent United States lake survey, in 1865; also, in making preparations, previous to taking the field, for making surveys of Manistee, White Lake, Muskegon, South Haven, and New Buffalo harbors, Michigan.

In obedience to your orders of September 6, 1866, I left Milwaukee for Manistee, Michigan, and made a survey of that place. The time occupied was from September 7 to October 2, 1866, the uniavorable state of the weather

causing unexpected delay.

The survey was carried one-half $(\frac{1}{2})$ mile on each side of the harbor and the whole length of the river, one and a half $(1\frac{1}{2})$ mile to Manistee lake; then extended about three-fourths $(\frac{3}{4})$ of a mile on each side of the entrance.

The details of the work accomplished by myself and party at that point are as follows, viz:

Number of triangulation stations built and occupied Number of sounding stations built	26 106 21 5, 881 353 50.40 392 178 680 1.50 1 62 7 30
Number of square lines of hydrography made: Number of miles run with stadia instruments for topography Number of shore line chained and sketched Number of observations to determine the true meridian	

The map containing this survey was plotted on a scale of one inch to two hundred feet, in December, 1866, on a sheet of paper thirty-six by sixty inches, and embraces three hundred and twelve square inches of minute hydrography,

and four hundred inches of topography.

A detail sheet of the mouth of the river, on a scale of one inch to one hundred feet, showing the pier work done by private enterprise at the mouth of the river, and about three hundred feet of shore line, on each side of them, was also plotted. Duplicate tracings of each of these were made, a copy sent to the engineer department, and the other furnished Colonel W. F. Raynolds, superintendent United States lake survey, Detroit, Michigan.

Your orders of October 2, 1866, directing me to proceed to Père Marquette and Pentwater, Michigan, to make a reconnoissance of each of these places, and then to White river, Michigan, my next field duty, was handed to me by Mr. James Southall on the following day. The survey of Manistee being completed, I immediately packed up all the instruments, surveying implements, &c., and left the same evening with my party for Pentwater, having been unable to procure a boat to land me at Père Marquette as desired in your instructions.

I landed at Pentwater, Michigan, at 11 p. m. October 3, and spent the next day in making a thorough reconnoissance of the entrance of the river and character of its bottom, as well as the general topography around the harbor

and lake.

At 7 a. m. October 5th I left with party for White river, where I landed at 2 p. m. The survey of White river was commenced October 6, and closed on the 19th of the same month. The shore line and soundings on Lake Michigan were carried one-half $(\frac{1}{2})$ mile north of the mouth of the river and one and a quarter $(1\frac{1}{4})$ of a mile south of it. The river was carefully measured and minute soundings taken from its mouth to its entrance into White lake, a distance of nearly one mile. The shore line of White lake was surveyed on each side of the river, three-fourths $(\frac{3}{4})$ of a mile, and soundings taken at the entrance of the river and lake. I thought it necessary to take the levels on the narrow belt of land between White lake and Lake Michigan, to show the character of the ground on it, and enable you to make estimates for a channel across it should it be found advantageous.

The amount of work done at this point is as follows:

Number of triangulation stations	29
Number of sounding stations.	83
Number of buoys placed out and located	18
Number of casts of the lead made	3,018
Number of lines of soundings run	226

Number of miles of soundings run	44.41
Number of theodolite horizontal angles measured	497
Number of vertical angles measured for topography	145
Number of theodolite readings made and recorded	576
Number of square miles of topography made	1.33
Number of square miles of hydrography made	
Number of miles run with the stadia instrument for topography	11.11
Number of miles of shore line chained and sketched	
Number of observations taken to determine the true meridian	

The work of plotting this map was done in the field, on a scale of one inch

to 300 feet, and a tracing from it sent to the engineer department.

During the month of January it was plotted on a scale of one inch to 200 feet, on a sheet of antiquarian paper, embracing 330 square inches of minute hydrography and 164 square inches of topography. A portion of the point between Lake Michigan and White lake was also plotted on a scale of one inch to 100 feet, to show the minute topography on the land and the soundings abreast of it, to the depth of 14 feet. Tracings of each of these were made and furnished Colonel Wm. F. Raynolds, superintendent United States lake survey, Detroit, Michigan.

Mr. E. Dumais, draughtsman, reported to me for duty at White river, and handed me your orders of October 17, 1866, directing me to proceed immediately upon the completion of the survey of White lake to Muskegon, my next field

duty.

I closed the work at White lake on the 19th, and on the 20th left with my party, instruments, and equipage for Muskegon, fifteen miles south of White river, with propeller Brittan, where after a two hours' ride I landed safely.

The work at this point was only to make a thorough examination of the mouth of the river, take the soundings outside of the pier to the depth of 24 feet, and notice whatever changes had occurred since the last survey, made under the direction of Colonel Wm. F. Raynolds in September, 1865.

I was delayed at that point from October 20th to November 4th on account of stormy weather. The fair weather was devoted to the field work, while the stormy days were occupied in making computations and plotting the maps of Manistee and White river. The length of the north pier I found to be 1,454 feet instead of 1.404 feet, as shown on the map of the survey of 1865.

Below will be found a statement of the amount of work performed at this point:

Number of triangulation stations erected and occupied	4
Number of sounding stations built	69
Number of buoys placed out and located	
Number of casts of the lead made	1,983
Number of lines of soundings run	
Number of miles of soundings run	15.33
Number of theodolite horizontal angles measured	52
Number of vertical angles measured for topography	15
Number of theodolite readings taken and recorded	86
Number of miles run with the stadia instrument for topography	1.03
Number of miles of shore line chained and sketched	1.25

A map of this survey was plotted in the office in March, 1867, on a scale of one inch to 200 feet, embracing 240 square inches of topography and ninety square inches of hydrography. A map on a scale of one inch to 100 feet, showing the piers, soundings and character of the bottom at the entrance of the river and the soundings outside of it to the depth of fourteen feet, was also made. Tracings of each of these maps were made and sent to the engineer department

in December, 1866, together with a report of exports and imports for the past

year and estimates also for improving the harbor.

My next field duty, by your orders of November 2, was South Haven, Michigan. On November 5th I left Muskegon for Grand Haven, where I found Captain Squier waiting for me, who had orders to take my party to South Haven. I at once proceeded to that place and landed on the morning of the 6th.

The survey was immediately begun and occupied nine days; i. e., until the 14th of the same mouth. The hydrography in Lake Michigan was limited to the depth of nineteen feet, and I carried the soundings up the river as far as the bridge, situated three-fourths of a mile from the mouth. The shore line was run with the theodolite, chained and sketched accurately a distance of half a mile on each side the mouth of the river. The topography surrounding the river was also sketched, and the height of the ground determined by the stadia instrument to show the contour of the hills. The amount of work done at this place is as follows, viz:

Number of triangulation stations erected and occupied	9
Number of sounding stations built	51
Number of buoys placed and located	12
Number of casts of lead made	1,981
Number of lines of soundings	200
Number of miles of soundings	21.26
Number of theodolite horizontal angles measured	185
Number of theodolite vertical angles measured	126
Number of theodolite readings taken and recorded	330
Number of square miles of topography sketched	0 50
Number of square miles of hydrography made	0.78
Number of miles run with the stadia instrument for topography	6.01
Number of miles of shore line chained and sketched	
Number of observations to determine the true meridian	3
Number of compass readings to ascertain the magnetic variations	24

The survey was plotted in pencil in the office during the month of December, 1866, and finished in April, 1867. It embraces 112 square inches of minute hydrography and 130 square inches of minute topography. The mouth of the river was plotted on a scale of one inch to one hundred feet, to show fully the nature and capacity of the harbor, as well as the position of the old pier work done by private enterprise, and the soundings outside to the depth of fourteen feet. A tracing of each of these was made from pencil notes in December, and sent to the bureau. Also a report of the amount of business transacted, together with the exports and imports for the past year. Estimates for the improvement of the harbor were likewise made and submitted to you.

The survey of South Haven was completed on the 14th of November, and on the 15th I left with my party for New Buffalo, where I arrived on the 17th

of the same month.

A copy of the survey of this harbor made, under the direction of Colonel J. D. Graham, corps of engineers United States army, in September, 1857, was forwarded to me in October, 1866, at Muskegon, with orders to make a thorough reconnoissance, and show the changes that had taken place since this last survey; the season being too far advanced to make a new one. I made a few measurements to determine the course of the river, and to show the change of the shore line. I found the outlet of the river removed seven hundred feet west of its original course, having cut into the adjoining hills on the south side about 120 feet. At a point half a mile east of the old channel the accretions or sand deposits were first noticed, gradually increasing in width as you advanced westward, until abreast of the old outlet, which was found to be two hundred feet in width and diminishing gradually towards its present course.

The depth of the river was from two to three feet, and scarcely any perceptible changes had occurred in Lake Pottswatomie.

I also visited the spot chosen by the late Colonel J. D. Graham, corps of engineers United States army, for a thorough cut across the point between Lakes Pottawatomie and Michigan, and the only changes noticed were the sand dones, which had been blown into different shapes from those indicated on the map.

Having completed the reconnoissance of New Buffalo on the 21st of November, I discharged my party and left with my assistant for Milwaukee, Wisconsin, to report at the office, where I arrived on the 22d, and at once started the

computation and projection of the field notes.

On the 17th of August, 1866, Captain Alexander Mackenzie made a reconnoissance of the harbor of Grand Haven, to ascertain the changes on the outer bar since the last survey of 1865, and took a few soundings at that point, which were plotted on the tracing of Grand river.

A survey of Black Lake harbor, Michigan, was made by Captain Alexander Mackenzie, corps of engineers United States army, and the following amount

of work performed between the 4th and 20th of October, 1866, viz:

Number of triangulation stations erected and occupied
Number of buoys placed out and located
Number of casts of the lead made
Number of lines of soundings run
Number of miles of soundings run
Number of theodolite horizontal angles measured
Number of vertical angles measured
Number of theodolite readings traced and recorded
Number of square miles of topography
Number of square miles of hydrogaphy 0.30
Number of miles of shore line chained and sketched 2.59
Number of observations to determine true meridian
Number of compass readings taken

The map of this survey was projected on a scale of one inch to two hundred feet, in November, 1866, by J. T. Baker, then draughtsman on the work, and embraces 118 square inches of hydrography and ninety square inches of topography. The channel, piers, and a portion of the shore line on each side of the entrance, as well as the soundings to the depth of fourteen feet, were plotted on a scale of one inch to one hundred feet. Duplicate tracings of this map were made and a copy sent to the engineer department in November, 1866.

A reconnoissance of each of the harbors of Kenosha and Manitowoc, Wisconsin, was made in October, 1866, by Lieutenant James B. Quinn, corps of engineers United States army, showing the changes which have taken place since last surveyed. Sketches of these reconnoissances were made by the same officer in November.

By permission of the engineer department it is contemplated to make a new survey of these harbors this season; the delay has been occasioned by the amount of office-work on hand previous to making the annual report. It is hoped, however, that the field-work will soon be resumed, and these surveys made before the close of the season.

On the 25th day of March, 1867. I was ordered to make an examination of the Milwaukee harbor, and take soundings to the depth of twenty-one feet, to accertain the position and character of a bar reported forming abreast of it. Preparations were at once made, stations erected, and measurements taken to forward the work, as it was desirable that soundings be taken before the spring freshets took place, to ascertain the changes the current would produce. Owing

to the unfavorable state of the weather the soundings could not be taken till the second of April, causing some eight days' delay. From June 24 to July 2, another examination was made to show the changes since the last one. A bar 10.8 feet for the shallowest depth of water was found to exist 300 feet east of the light-house crib, located at the outer end of the north pier, tending nearly south-southeast and extending as far as the line of the north pier, at which point is found thirteen feet of water. This bar is shown on the sketches which have been made in April and July, a copy of which accompanies this report.

From the 9th to the 19th of April, 1867, Captain Alexander Mackenzie made a survey of the harbor of Michigan City, and accomplished the following amount of work, viz:

Number of triangulation stations built and occupied	12
Number of sounding stations and stakes	
Number of casts of lead made	
Number of lines of soundings run	167
Number of miles of soundings run	
	72
Number of vertical angles for topography	92
Number of readings taken and recorded	144
Number of miles of shore line chained and sketched	4.86
Number of observations to determine the true meridian	2
Number of compass readings made	24

The computations and projections of this map were made in May last, by J. T. Baker, draughtsman, and embrace 154 square inches of topography and 178 inches of hydrography.

A portion of the harbor, including the piers built by the city, and showing the soundings, shore line, &c., was plotted on a general scale of one inch to one hundred feet. A tracing of this map has been made to accompany the annual

report.

On the fifth of May I received orders from you to organize a party and proceed to Racine, Wisconsin, to make a survey of the reef situated two miles eastsoutheast of the harbor, and left the office with my party, instruments, and equipage on the following day, and commenced operations forthwith. In consequence of the prevailing winds, frequently accompanied by rain, much delay was experienced in prosecuting this survey. Our boat—a borrowed one—ill calculated for the work, being small and crank, which often compelled me to seek refuge in the harbor when the wind at all freshened. It was obvious that, in order to push the work forward without delay, a steam-tug was indispensable, but the rate for services charged were so exorbitant that I was forced to dispense with them as much as possible. Another great delay was occasioned by the carrying away of a water station in a gale on Sunday, the 12th of that month, which had been placed on the reef in nine feet of water. It cousisted of three pieces of timber, eight inches square, thirty-eight feet in length, put together in the form of a tripod, and strongly bolted and braced. At twenty feet from the base a platform was fixed, upon which was placed about threefourths of a cord of stone to anchor it. Above the stone was a small platform, sufficiently large to enable one person to observe with an instrument, and locate the buoys placed out. Three of these stations were carried away during the time of the survey, which occupied one month.

The amount of work performed at this place is as follows, viz:

10
86
30
1,456
4,846
36
373
71.25
185.30
777
12
744
2
9.87
4.50
2

The computation of the triangulation was made immediately after returning from the field, on June 5th, the notes plotted in pencil, and the map placed in the draughtsman's hands on July 15th, and is nearly completed. It is plotted on a scale of one inch to eight hundred feet, or one ninety-six-hundredths, and comprises four hundred and twenty square inches of topography, and six hundred square inches of hydrography. A copy of this map will also be made to accompany your annual report.

On May 18, 1867, a surveying party, under the direction of J. W. Judson, was organized and sent to Menomonee river, Green Bay, to make a survey of its mouth, with a view of making estimates for improving the same. On account of unavoidable delay from bad weather, and difficulties encountered by loss of buoys, stations, &c., which were carried away by lumber, rafts, and scows constantly passing to and fro, the work progressed slowly, and it was not until the 5th of August that Mr. Judson completed the survey, when he was ordered to report to the office, where he arrived on the 8th of the same month.

The work performed by Mr. Judson was not found sufficiently accurate to warrant satisfactory results, and his services, in consequence, were no longer required.

The amount of work performed by Mr. Judson on the survey of Menomonee is as follows, viz:

Number of sounding stations built	26
Number of buoys placed out and located	17
Number of casts of the lead made	2,250
Number of lines of soundings run	193
Number of miles of soundings run	32
Number of theodolite horizontal angles measured	320
Number of theodolite readings recorded	395
Number of square miles of topography	0.41
Number of square miles of hydrography	1.10
Number of miles of shore line run	3.75
Number of observations to determine true meridian	1

Assistant W. H. Hearding, formerly attached to the United States lake survey, reported here for duty about July 1, and has been detailed to project

the map of Menomonee from the notes taken by Mr. Judson, and a copy of it

will be ready to accompany your report.

In compliance with your orders of July 3, I proceeded with a leadsman and chainmen to the mouth of Kalamazoo river to make a survey of that place for the purpose of making estimates for its improvement. We reached that place on the evening of the 6th, and commenced the survey the following morning, completing it on the 13th of July, and left immediately for Milwaukee, Wisconsin, reporting at this office on the 14th, the following day.

The amount of work performed during these six days is as follows. viz:

	- •
Number of triangulation stations built and occupied	19
Number of sounding stations built	46
Number of buoys placed out and located	20
Number of casts of lead made	2,858
Number of lines of soundings made	217
Number of miles of soundings made	25.5
Number of theodolite horizontal angles measured	367
Number of vertical angles measured for topography	51
Number of theodolite readings taken and recorded	512
Number of miles measured with the stadia instrument	9.25
Number of miles of shore line run and sketched	3.75
Number of observations made to determine true meridian	2
Number of compass readings taken to ascertain the variations	24
The work of computing and pletting the nates of this surrow was done	at the

The work of computing and plotting the notes of this survey was done at the office in July, and the map has been projected in pencil on a scale of one inch to two hundred feet; a tracing of it has also been made to accompany your

annual report.

On the 29th of July, 1867, Assistant W. H. Hearding received orders from you to proceed to Aux Becs Scies harbor and make a new survey of that place, so as to facilitate the improvements now being made, and verify the accuracy of the previous surveys made by Lieutenant O. M. Poe, corps of engineers United States army, in 1859, and the one furnished you by Colonel Cram, corps of engineers, which had been made in 1865. A great many discrepancies were found to exist between the survey of Lieutenant Poe and that furnished by Colonel Cram, and involved the necessity of making an accurate one. He left on the evening of the 29th, and arrived at Aux Becs Scies on the 2d day of August, commencing the survey on that day and closing it on the 8th day of the same month.

He reports the following amount of work performed during that time, as follows, viz:

10110 110, 112 1	
Number of triangulations built and occupied	25
Number of sounding stations built	28
Number of buoys placed out and located	20
Number of casts of the lead made	1,975
Number of lines of soundings run	175
Number of miles of soundings run	31.00
Number of theodolite horizontal angles measured	454
Number of vertical angles for topography measured	54
Number of theodolite readings taken and recorded	541
Number of square miles of topography sketched	0.33
Number of square miles of hydrography made	0.75
Number of miles run with the stadia for topography	3.01
Number of miles of shore line run and sketched	2.02
Number of observations to determine the true meridian	1

Assistant Hearding returned from the field on the 13th of August, since which time he has made the computations of the field-notes and projected them on a scale of one inch to 200 feet, and are now plotted in pencil.

An abstract of the work done at each of the harbors surveyed since August, 1866, is attached to this report, and shows the total amount performed during the past year, under the head of Appendix A.

OFFICE-WORK.

The months of December, 1866, January, February, March, and April, 1867, were devoted to office-work. During that time the computations and projection of all the season's field-work were made, and the notes taken of each harbor plotted on a general scale of one inch to 200 feet. Wherever improvements were contemplated detail drawings of one inch to 100 feet have been made to accompany the report and estimates of last year.

The following are the number of surveys which have been projected and

drawn, viz :

One map of Black lake, on a scale of one twenty-four-hundredth, the details on one twelve-hundredth; also, duplicate tracings of each made.

One map of Manistee, on a scale of one twenty-four-hundredth, the details on

one twelve-hundredth; also duplicate tracings of each made.

One map of White river, on a scale of one twenty-four-hundredth, the details on one twelve-hundredth, and a tracing made also.

One map of Muskegon, on a scale of one twenty-four-hundredth, the details

on one twelve-hundredth, and a tracing made.

One map of South Haven, on a scale of one twenty-four-hundredth, the de-

tails one twelve-hundredth, and a tracing made.

One tracing of New Buffalo harbor, from survey made in 1857, under the direction of Colonel J. D. Graham, corps of engineers United States army, to which were added the changes from the reconnoissance made in November.

One tracing of Pentwater harbor, as surveyed under the direction of Colonel W. F. Raynolds, corps of engineers United States army, and superintendent United States lake survey in October, 1866, to which was added the topography as sketched by myself in October, 1866.

One tracing of Pere Marquette, as surveyed under the direction of Colonel W.

F. Raynolds, superintendent United States lake survey in October, 1866.

A report and estimates for the improvement of each of these harbors was made by Captain Alexander Mackenzie and Lieutenant James B. Quinn, corps of engineers United States army, and myself, and submitted to you in December last.

Besides the above, maps, plans, and specifications of crib-work and close

piling were also made as follows, viz:

One map in water colors, containing drawings of crib-work for piers on Lake Michigan, as described and adopted by Brevet Colonel J. B. Wheeler, major corps of engineers United States army, was drawn by J. T. Baker, draughtsman. Six tracings of this map were also made for distribution to parties awarded contracts.

One map in water colors, containing drawings of crib-work for Chicago harbor, as designed and adopted by Brevet Colonel J. B. Wheeler, major of engineers United States army, was also drawn by J. T. Baker, draughtsman, and

duplicate tracings of this map made.

One map in water colors, containing a plan of pier work as designed and adopted by Brevet Colonel J. B. Wheeler, major corps of engineers United States army, for the improvement of the harbor of St. Joseph, Michigan, was

drawn by J. T. Baker, and duplicate tracings of it made.

The notes and soundings of the survey of Michigan City were plotted on a scale of one inch to two hundred feet, and the map drawn partly by J. T. Bater, draughtsman, who was discharged on July 6 for neglect of duty. The map was then completed by Mr. Dumais.

A map of the survey of Racine reef has been drawn on a scale one inch to

eight hundred feet, and will be completed in a few days. A copy of it will be

ready for your annual report.

The field notes of the survey made in July of Kalamazoo river have all been plotted on a scale of one inch to two hundred feet, and a tracing of this map made to accompany your report.

Aux Bec Scies survey, made in August by Assistant W. H. Hearding, has been projected in pencil on a scale of one inch to two hundred feet, and will be

completed as soon as the season's field-work is over.

Assistant Hearding has been engaged since his return from the field on the map of Menomonee river, which he has projected on a scale of one inch to two hundred feet. He is now engaged in making a tracing of this map, which will be completed for your report.

Before closing this report I wish to tender my sincere thanks to Assistant Hearding for the efficient manner in which he has accomplished his work, and

my indebtedness to him in assisting me to make this report.

Attached to this will be found, in Appendix B, report and estimates for the improvement of Kalamazoo and Menomonee rivers, Michigan.

I am, colonel, very respectfully, your obedient servant,

WM. T. CASGRAIN,
Assistant and Superintendent of Surveys.

A 27.

Report and estimates on Kalamazoo river.

This river takes its rise in the southern part of Michigan, and after a circuitous course of about three hundred miles, empties into Lake Michigan about midway between Grand Haven and St. Joseph. During its course several falls of considerable extent occur, furnishing a number of water-powers of respectable capacity, and thus inducing a large amount of manufacturing, which will largely increase as the country grows older. The last and largest of these falls is at Allegan, a town of nearly three thousand inhabitants, twenty-five miles by land and fifty miles by river from Lake Michigan. Previous to settlement, the entire country traversed by it was densely covered with forests of pine and hard wood. The first settlement made in the country, which bears the same name as the town, was about 1830. The town was commenced a few years later—about 1835. The first business entered upon was the manufacture of lumber, and for several years those engaged in it were blessed with prosperity. The financial crisis of 1837 swept over them, and fair prospects received a sudden and discouraging blow, resulting in the suppression of operations by nearly all the mills.

About 1843 business began to revive, and Allegan has since had a comparatively prosperous career. Its chief growth, however, has been within the last five years. The volume of water in the river is quite large, and at Allegan takes a fall of nine feet, giving a very valuable power. This has already been improved, and each year adds to the number of manufacturing establishments. The manufacture of pine lumber is being somewhat superseded by that of hard wood, which finds a ready market at home and in Chicago.

The town of Allegan has a post office, two churches, two hotels, a large school-house, nineteen dry goods, hardware, groceries, drug, and crockery stores, one banking house, and three flouring mills, with a capacity of 1,200 bushels per day. These mills find an ample supply of wheat in the surrounding country without importing, and their flour, except custom work, is shipped to Chi-

cago; about 300 barrels are forwarded weekly.

The presence of large tracts of the finest hard-wood timber of various kinds in Allegan county has induced parties to give considerable attention to shipbuilding; since 1862 they have built two propellers, two river steamers, two tugs, two schooners, and seven lumber barges, at a total cost of \$177,500.

The connection from Allegan to the mouth of the river is made by a daily line of river boats; leaving Allegan in the morning, a ride of six or seven hours brings you to the thriving villages of Saugatuck and Douglas, situated on either side of Kalamazoo lake, a beautiful inland sheet of water, six miles in length and one and a half mile in width, and about three miles from the outlet of the river in Lake Michigan. The larger of these is Saugatuck, situated on the north side of the river, and contains a population of over one thousand inhabitants. The chief business is the manufacture of lumber in various ways, the supply of logs being derived from the pineries along the river between this place and Allegan, and the country back. Saugatuck has a post office, two churches, two hotels, sixteen stores, eight saw-mills, one tannery, and a large school-house, just completed at a cost of \$10,000; it occupies a beautiful situation overlooking Kalamazoo lake, and is a credit to the young village.

Douglas is situated on the south side of the lake; its population is about six or seven hundred inhabitants; it has two steam saw mills and a large tannery,

which turns out about 10,000 hides annually.

Singapore, a small village of two or three hundred inhabitants, is situated about one mile from the mouth of the river, on the north bank; has two steam saw-mills, two stores, and a few dwellings.

Besides the various branches of business enumerated above, there are numerous manufactories of furniture, wagons, &c., which find at the extensive water-power on the river a congenial field for operations, and when means of transportation

are permanently insured, they will largely increase.

One of the great sources of wealth for the country bordering on the lake in this part of Michigan will be, and in fact now is, the fruit crop. soil and lake breezes seem particularly fitted for nourishing fruit trees, more especially the peach; already large orchards abound, and fruit-men are every year investing capital and enlarging the business.

The amount of business done on the river has, through the kindness of Mr. F. B. Stockbridge, been furnished me. It contains a full statement of the amount of lumber, shingles, &c., manufactured at all the points on the river and thipped to Chicago, Milwaukee, Racine, Kenosha, and various other ports during the season of 1867, and will serve to show what other development might arise, should the navigation be made safe and certain.

	Value.
Lumber made by 14 water mills and 19 steam mills, 70,000,000	
feet	\$840,000 00
Shingles made by 23 shingle mills, 85,000,000	300, 000 00
Sides of leather, 60,000, at \$6	360,000 00
Flour, 70,000 barrels, at \$10	700,000 00
Potatoes, 4,000 barrels, at \$2 25	10,000 00
Apples, 10,000 barrels, at \$3	30,600 00
Peaches, 25,000 packages, at \$1	25,000 00
Hemlock bark, 5,000 cords, at \$5	25,000 00
Wood, beach and maple, 10,000 cords, at \$3	30,000 00
Railroad ties, 40,000 cords, at 50 cents	20,000 00
Hewed timber, 500,000 feet, at \$10	5,000 00
Turned stuff for bedsteads, chairs, &c	15,000 00

2, 360, 000 00

AMOUNT AND VALUE OF IMPORTS.

250 tons of mixed merchandise, valued at, probably	\$500,000 00 240,000 00
	740 000 00

740,000 00

IMPROVEMENTS AT THE MOUTH OF THE RIVER.

The existence of two large bars across the mouth of the Kalamazoo river has heretofore greatly impeded the growth of the villages and towns on the river, and of the entire country. Its natural market is Chicago, and although a railway is now in course of construction from the town of Kalamazoo, situated on the line of the Michigan Central railroad, which will do a large business, yet the lake must eventually furnish a means for the exit of the chief heavy freight. The realization of this fact led to active measures for the removal of the bar. A company with a subscribed capital of \$30,000 was formed, principally of the manufacturers and shippers, and nearly the whole amount spent in improvements. They have built two piers or jettees, consisting of 500 feet on the north side and 1,575 feet on the south side, of slab-work, placed together firmly, and are the best of the kind I have ever seen. These piers have confined the current. which has kept a channel of seven feet of water open this season. One result of the present work has been to allow the entrance of a fair class of lake steamers, and the enterprise of several parties has already established the propeller line with Chicago by the new and staunch propeller Ira Chaffee, which makes triweekly trips. She was built at Allegan and commenced her trips in May last.

The amount of business done at this point, together with the number of vessels which frequent that thoroughfare, (it being a central point for those running to Chicago and Milwaukee on the west shore, and Grand Haven and St Joseph on the east shore,) necessitates the improvement of the harbor, of which there are few on Lake Michigan which could afford, with comparatively a small expensiture, better shelter during the prevailing autumnal storms. To make transportation safe and certain is to increase the value and results of human industry, and the attention of the government should be brought to bear on that fact.

PROPOSED IMPROVEMENTS.

Near the village of Singapore, where the river makes a bend, and on the line A B shown on the map accompanying this report, is a low piece of ground showing the probable outlet of the river at some remote period. This sort of gulley is about 400 to 500 feet in width, and the ground on either side of it is rolling, and ten to fifteen feet higher. The distance across this neck of land is 990 feet, and the average height above the lake surface is about seven feet. It might be true economy, for a permanent work, to cut across this neck of land and make the improvement at that point; but on account of the improvements already made at the present entrance of the river which are available, estimates for opening this channel were not made, the expenditure being deemed too great.

The estimates made out and submitted consist in improving the present entrance, the south pier, 416 feet, to the depth of twelve feet, and the north pier, 1,632 feet, also to a depth of twelve feet; these piers to be parallel to each other and 200 feet apart. Dredge a channel between them to the depth of twelve feet; also cut a channel in the bay 200 feet wide and twelve feet deep, to connect with the deep water near the first bend; all of which is shown by red lines on the accompanying man.

By making such improvements a good harbor of refuge can be made, which will be of great benefit to the large fleet of vessels frequenting the head of Lake Michigan, as well as improving the local trade.

A.—Estimated cost of one crib, $32' \times 25' \times 20'$.	
3,353 feet square timber, at 20 cents per lineal foot	\$ 670 60
288 feet 3 inch plank, board measure, at \$15 per M	4 32
3,938 pounds iron bolts, at 10 cents per pound	393 80
6 pounds iron bolts, at 10 cents per pound	60
89 cords of stone, at \$16 per cord	1,424 00
10 cords of brush or slabs, at \$2 50 per cord	25 00
Labor of framing and placing 3,353 feet, at 20 cents	670 60
	3,188 92
Add ten per cent. for contingencies	318 89
Total cost	3,507 81
B.—Estimated cost of one crib, $32' \times 20' \times 17'$.	
2,368 feet square timber, at 20 cents per lineal foot	\$473 60
288 feet 3-inch plank, board measure, at \$15 per M	4 32
3,554 pounds iron bolts, at 10 cents per pound	355 40
6 pounds iron spikes, at 10 cents per pound	60
56 cords of stone, at \$16 per cord	896 00
63 cords of brush, at \$2 50	16 88
foot	473 60
	2,220 40
Add 10 per cent. for contingencies	222 04
Total cost of crib	2,442 44
Estimated cost of building 2,048 feet of crib work, or sixty-four of proving Kalamazoo harbor.	cribs, for im-
50 cribs, at \$2,442 44, for north pier	\$122,122 00
12 cribs, at \$2,442 44, for south pier	29,309 28
2 cribs, at \$2,770 15, for outer ends of piers	7,015 62
	158,446 90 ————
C.—Estimated cost of dredging the harbor of Kalamazoo to place cribs and the channel between them to the depth of twelve	e the proposed feet.
To excavate 69,329.34 cubic yards outside to place cribs and open a channel 200 feet wide, at 40 cents	\$ 27,731 73
To excavate 32,600 cubic yards inside the river to deepen the channel to 12 feet and 200 feet wide, at 40 cents per yard	13,040 00
Add 10 non-cent for contingencies	40,771 73
Add 10 per cent. for contingencies	4,077 17
	44,848 90

Recapitulation of estimates and costs.

To build 2,048 feet of crib work	\$158,446 90 44,848 90
Total cost	203,295 80

The town of Saugatuck is in the collection district of Grand Haven. It is not a port of entry. There is a light-house of the sixth order on the north bank of the river, situated on a rise of 18 feet. Its focal plane is 53 feet above the lake surface, and visible a distance of ten miles. It was rebuilt in 1859, having been washed away twice prior to that time.

I am, colonel, very respectfully, your obedient servant,

WM. T. CASGRAIN,
Assistant and Superintendent of Surveys.

A 28.

MENOMONEE RIVER.

This river takes its source in the "Lac Vieux Desert," situated in the northern part of Michigan, and is the boundary line between the States of Michigan and Wisconsin for a distance of nearly 400 miles by the trend of the river. It has many tributaries, all of which are well adapted for rafting purposes, and is considered the best lumbering district of Michigan and Wisconsin. This river empties into Green bay, about half way between Green Bay City and Esconaba, situated in little bay Noquette, which is the terminus of the Esconaba and Marquette railroad. There is a daily line of steamers from Green Bay City to Esconaba, also from Green Bay City to Menomonee. The steamers plying between the last places are of a very light draught and adapted to the navigation of the Menomonee river, where a depth of six feet is found on the outer bar. The west shore of Green bay is frequented by a large fleet of vessels engaged in carrying lumber to Chicago and Milwaukee. There are a number of points on the bay where large shipments of lumber are made; these are Big and Little Suamico, Pensaukee, Oconto, Peshtigo, Menomonee, Cedar and Ford rivers, also Chambers's island. Below is a table showing the amount of lumber shipped from each of the above points.

	Feet.	Value.
Little Suamico		\$720, 000
Big Suamico		936,000
Pensaukee	75 millions	90 0, 000
Oconto	100 millions	1,200,000
Peshtigo	85 millions	1,020,000
Menomonee		1,080,000
Cedar river.	40 millions	480,000
Ford river.	22 millions	264, 000
Chambers's island	15 millions	180, 000
Total	565 millions	6,780,00

The large fleet of vessels that is required to carry this lumber need a sheltering place at some point on the bay to run to during the severe storms, most of which are from the northeast. Vessels at anchor close to the shore often find it a difficult matter to make an offing on account of the shoal water and the banks of Peshtigo and Oconto, which they have to keep clear of. It may be thought that they can make a lee under Chamber and Green islands, but with wind from the northeast it is impossible.

On the east shore of Green bay, good natural harbors can be found at Little and Big Sturgeon bays, Egg harbor, Fish creek, and Horseshoe island, for pro-

tection during the northeasterly gales.

The Peshtigo Company have expended large sums of money to construct a harbor at the mouth of the Peshtigo river, but it fails to meet the wants of the

shipping interests in that locality during the severe autumnal storms.

The bar, about one mile north of the river, extends out into the bay about seven miles before nine-feet water is reached, and thus cuts off vessels from making that harbor during northeasterly gales. It will be seen by a glance at the map that a vessel desiring to make the harbor during a heavy blow from northeast or east-northeast would be compelled to run down below the point of the Peshtigo bar, and then haul close to windward to make the harbor at all; a feat very difficult for the best navigators to perform. There are 120 miles on the west shore of the bay without any harbor whatever, and Menomonee is about half-way between Green Bay and Esconaba, and a central point for a harbor.

Through the kindness of Mr. E. S. Ingalls, I have been furnished a table showing the amount of business done at the mouth of this river, which I give

in this report.

Statement of lumber, &c., shipped from Menomonee for year 1867.

Article.	Quantity.	Value.
Lumber Shingles Lath Pickets Square timber, hewn Railroad ties Fish Amount exported	25, 000, 000 feet. 12, 000, 000 feet. 1, 000, 000 feet. 2, 000, 000 feet. 50, 000 feet. 1, 500 bbls.	\$1,080,000 1,000,000 48,000 12,000 300,000 25,000 15,000

Two hundred tons mixed merchandise, valued at about \$400,000.

The number of arrivals of large vessels for 1867 were 400; steamers, 175; transient vessels, 25.

There are also three tugs plying on the river to supply the wants of vessels

and towing rafts, &c.

The population and business near the mouth of the river is contained in three villages: Menekaunee and Marinette, on the south or Wisconsin side, and Menomonee, on the north or Michigan side. Menekaunee is in the town of Marinette, at the mouth of the river, directly opposite Menomonee. Marinette is one mile up the river. Menomonee is on the neck of land lying between Green bay and the river; is the county seat of Menomonee county, which was organized in 1863; has a post office, two hotels, and a printing office, publishing the Menomonee Herald, and ten steam saw-mills. The entire population of the three villages is estimated at about 3,000 inhabitants.

The depth of the river at the entrance is only six feet, and but few vessels can enter, and those of very light draught. The principal business done on the river is lumber, all of which is shipped to Chicago and Milwaukee. One foundry

and a door and sash factory are now in progress of erection.

To improve the present channel of the river is impracticable, on account of

the shifting sand-bar across it. It is, therefore, proposed to cut across the neck of land between Green bay and Menomonee river, and run two parallel piers on a due east course a distance of 1,375 feet on the north side, and 1,275 feet on the south side, and dredge a basin inside the river to the depth of twelve feet, as shown on the map accompanying your annual report.

The benefit derived from such improvement would be the general commerce

The benefit derived from such improvement would be the general commerce on Green bay and Lake Michigan, and the local trade of Menomonee, besides developing the resources of the country in that section of the State, and making

a good harbor of refuge.

Estimates for the improvements, as above described, are herewith respectfully submitted.

A.—Estimated cost of one crib, $32' \times 25' \times 20'$		
3,353 feet square timber, 12 inches, at 24 cents per lineal foot 288 feet 3-inch plank, at \$15 per thousand	_	32
3,938 pounds iron bolts, at 10 cents per pound	3 93	
6 pounds iron spikes, at 10 cents per pound	890	60
89 cords stone, at \$10 per cord	25	
10 cords brush, at \$2 50 per cord	804	-
foot	004	12
Add ten per cent. for contingencies	2, 923 292	
Add tell per cents for contingencies	234	<u></u>
Total cost	3, 215	
To build two cribs, 32' × 25' × 20' at \$3,215 47, each	\$6, 430	94
B.—Estimated cost of one crib, $32' \times 20' \times 17'$.		
2,368 feet 12-inch square timber, at 24 cents per lineal foot	\$ 568	32
288 feet 3-inch plank, at \$15 per thousand	4	32
3,554 pounds iron bolts, at 10 cents per pound	355	40
6 pounds iron spikes, at 10 cents per pound		60
56 cords stone, at \$10 per cord	560	
63 cords brush, at \$2 50 per cord	16	88
foot	568	32
	2, 073	84
Add ten per cent. for contingences	2,013	
zzud son per commiter committences		_
Total cost	2, 281	22
To build 84 cribs, 32'×20'×17' at \$2,281 22, each	\$191,622	48
C.—Estimated cost of close-piling 655 feet to protect the sides of abrasion.	the cut fr	07%
For 16,375 feet piles, 25 feet long, at 10 cents per lineal foot For 1,310 feet of capping, 12 inches square, at 24 cents per lineal	\$1,637	50
foot	314	40
For 2,620 feet of stringers, 6 inches by 12 inches, at 12 cents per lineal foot	314	

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For driving 720 piles, at \$2 each	\$1,440 00
cents per lineal foot	218 40
cents per pound	1,368 00
per stringer	243 30
For framing and bolting 3,530 feet, at 12 cents per lineal foot	423 60
	5, 959 60
Add 10 per cent. for contingencies	595 96
Total cost	6, 555 56
D.—Estimated cost of dredging a channel 250 feet wide to a dep	th of 12 feet.
For dredging 60,000 cubic yards outside the point to sink the cribs and open a channel 12 feet deep, at 50 cents For dredging 59,842 cubic yards inside the river to deepen the	\$30,000 00
channel, at 30 cents per cubic yard	17,962 60
Total cost of dredging	47,962 60
To build 2 cribs, $32' \times 25' \times 20'$, as per estimates A	6,430 94
To build 84 cribs, $32' \times 20' \times 17'$, as per estimates B	191, 622 48
To build 655 feet of close piling, as per estimates C	6, 555 5 6

Respectfully submitted:

WM. T. CASGRAIN,
Assistant and Superintendent of Surveys.

252, 571 58

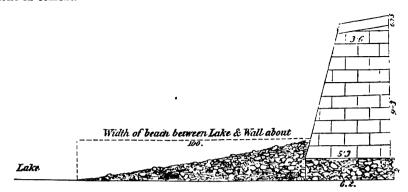
APPENDIX B.

Annual report upon the several civil works under the charge of Brevet Major General T. J. Cram, colonel of engineers, for the fiscal year commencing 1st July, 1866, and ending 30th June, 1867.

1.—SEA-WALL AT BUFFALO HARBOR, NEW YORK.

1. This work was commenced several years since. I find a small specific appropriation of \$349 made for it in 1863. It has by some means, but how derived I do not know, been extended from the south pier along the sandy shore southeasterly for a distance of 5,400 feet; (sketch A.) The design seems to have been to make this construction along nearly parallel to and coextensive with the city ship canal for a total length of 7,050 feet, or about 1½ mile. 4,081 feet is built with a coping, 1,319 is built without the coping, and foundation is laid without anything above it for 321 feet more, leaving about 1,020 feet of the original design upon which nothing has been done. The part of the wall which has been erected under the late Major Tardy's charge is much better constructed than any of the previous work. The cross section of the wall is here shown. The foundation rests upon clay, underlying the sand; the clay is met nearly at the level of the lake water; the foundation is of concrete,

made of cement and beach gravel; the wall is laid upon it with rough rubble stone in cement.



The coping is of flat stone, hardly as wide as the top of the wall; the land edge of the coping is bolstered up on "spalls," giving the top of the coping an incline towards the lake. In those parts which were built in the first year of the construction, the mortar is seen to be yielding to the influence of the climate. In 1864 there was a specific appropriation of \$37,500, and in 1866 another of \$31,000, for this work.

2. Under the appropriation of 1864, the late Major Tardy commenced operations upon the sea-wall in October, 1864, and from that time until June 30,

1866, expended \$35,554 55.

3. The operations under that officer, for the fiscal year commencing July 1, 1866, were as follows: Length of wall laid without coping, 160 running feet; laid in wall above foundation, 245 cubic yards; laid concrete in foundation upon which no wall was built, 221 cubic yards. Total expenditure in this fiscal year for wall and foundation, \$9,194 06; made up of materials purchased, \$1,035 87; labor, superintendent and foreman included, \$6,345 12; contingencies, \$1,813 07.

Hence, up to the termination of this fiscal year there has been expended a total of \$44,748 61. This, deducted from the sum of the appropriations of 1864 and 1866, leaves the available amount to carry on the work for the fiscal year commencing July 1, 1867, \$23,751 39. It seems that the late Major Tardy carried on the work not by contract; but as the law of 1866 required all to be done by contract that can be so done, it strikes me that the balance of the available means for this work should be done by contract, under the immediate supervision of a competent inspector.

Amount over and above the amount on hand required to be appropriated to complete the work to the extent of the original design, according to the rate

of cost of the wall for the last fiscal year, \$46,920.

I have not thought it advisable to recommend operations on this wall the present season, as there are other works of much more practical necessity to make preparation for at Buffalo. In fact, I see no particular necessity of extending this wall any further than it now is. It would be well, however, to complete the parts up to the present extent of the foundation which has been laid.

2.—HARBOR AT BUFFALO, N. Y.

This work was placed under my charge by orders from engineer department, March 28 last. After carefully examining this highly important harbor I submitted a special report upon the plan which I thought should be adopted, to adapt it not only to the present wants of commerce especially belonging to Buffalo, but likewise to the future enlarged demand of the lake commerce for vessels

intending to discharge at this port or seeking refuge there during heavy blows so prevalent at that end of the lake. (See sketch B.) The plan proposed consists of the following items:

1. Repair and protect the existing south and north piers.

2. Extend the south pier three hundred to six hundred feet on the line of its present direction.

3. Endeavor to obtain the consent of the legislature of New York, during next winter session, for the United States to remove two hundred or four hundred feet of the south end of Erie Basin breakwater.

4. Construct a breakwater, B², which should be about four thousand feet long and eight feet high above water. This would stand in twenty-five feet water to cover the harbor from the prevailing winds and secure a large space between

it and the shore for refuge.

5. To make examinations and a report in full for opening a ship channel from the lake, at what is denominated "South cut," directly to the Buffalo creek. The reply to this plan from the engineer department, dated July 9, was as follows: The department inclines to the approval of the project submitted in your report of the 20th ultimo, and directed operations for the present season to be confined to items 1, 2 and 3, in accordance with my plan, and to preparing detailed plans and estimates for the remaining improvements proposed by me.

6. Anticipating the approval of this or some other plan, I found it necessary to order my assistant, Captain F. Harwood, brevet lieutenant colonel, to make numerous measurements and examinations in order to obtain the condition of the parts of the piers requiring repairs, many lines of sounding in order to obtain an exact practical idea of the shape of the bottom, also many borings in the bed of the lake to ascertain the nature of the ground upon which the works are to rest. All these data it was necessary to obtain in order to intelligently draw the detail plans of the kind of construction, and before knowing what was to be advertised for proposals for furnishing materials and doing the work.

These examinations have more than occupied the party up to the expiration of the fiscal year to which this report belongs. I have no doubt, however, that they will be completed in season to enable me to perfect the plans, which it will take time to do, and put much of the work under contract the present season, so that materials can be accumulated and the work vigorously commenced by the spring of 1868. It takes a long time to collect sufficient materials for such an extensive work.

The amount of the available funds for this work at the close of the fiscal year was \$200,000, nothing having been expended of this up to that time.

3.—DUNKIRK HARBOR, NEW YORK.

This was also placed under my charge 28th March last. After a careful examination of it, aided by my assistant, Brevet Lieutenant Colonel F. Harwood, and I had obtained the requisite data, I submitted a special report upon the plan which I thought should be adopted, June 20, 1867, (see sketch C,) and which was—lst. To commence and rebuild upon the old under-water work, which remains as a foundation, the west pier P, and extend the work quite to the beacon light 2d. Afterwards, when we get appropriation sufficient for the whole work, to commence the breakwater B and complete it. Both to be built of timber cribs and timber superstructure filled with rubble stone and planked over, instead of a superstructure of concrete masonry. These views were approved by the engineer department in its reply June 22, 1867, with the exception that it preferred 20 to 18 feet for width of the breakwater. I shall give to it a thickness of 20 feet. The working plans and bills of materials in detail were made for P, and the materials and labor for classes for each were advertised for proposals. The proposal came in and the contracts have been made.

The cost of P, as per the contract, augmented by 10 per cent. thereon for contingencies, will be	\$37, 4 39 00
contract for P, including 10 per cent. for contingencies, will be	111,145 00
Total cost of P and B	148, 584 00
of the fiscal year in question	100,000 00
Required yet more to be appropriated to complete the plan	48, 584 00
•	

Owing to the long time necessary after a contract is made to accumulate sufficient quantity of materials, not more than 300 feet of P can be put in this season.

4.—Erie harbor, pennsylvania—(Sketch D.)

At the close of the fiscal year, June 30, 1866, there had been expended out of the \$15,000 allotted to this work from the general appropriation of 1864, not requiring advertising for proposals for material and labor, in repairs of the old north and south piers only, \$5,854 36. Up to that time this was all that was needed to be done in repairs of the old work, exclusive of the east breakwater.

In my inspection in the fore part of June last, I found that, since those repairs were made, several other holes and breaches had occurred in the under-water old timber work, and the stones fallen out into the harbor, caused by the old wooden pins, with which the timbers were bolted, being cut off by the action of the sand as if they were sawed, and by bad framing otherwise. To make the repairs required to stop, effectually, these holes, and to replace all decayed timber in both of the old piers, will require about \$10,000.

Again, see on the sketch the line of old breakwater, constructed under the direction of Major Maurice, corps of engineers, many years ago, extending from south pier to shore. I have had this thoroughly examined during this last fiscal year, and we find that there is a portion, commencing at the pier and going towards the shore, of 705 running feet, in which all the crib-work seems to have been swept away, except some of the piles. The average depth of water along this is about four feet, and if this part be rebuilt, it should be raised to a height of four timbers above water, filled with stones, and planked over, and should be twelve feet wide, which, at \$20 71 per running foot of piering, including the per cent. for contingencies, would cost

The remaining part of this breakwater, (779 running feet,) as we go towards the shore, shows a width of eight feet, and placing the old timbers in place upon which we could build a new work eight feet wide and three timbers above water, and drift-bolt the new down to the old under-water timbers, fill with stones, and plank over. This repair would average five timbers in height, two below and three above water, which, at \$12 62 per running foot of piering, including ten per cent. for contingencies, would cost.....

9, 831 00

\$13,408 00

500 00

Total cost of re-establishing the old breakwater......

\$22,739 00

In regard to the old line of west breakwater seen on the sketch, the sand has filled in so much between it and the lake that there is no necessity of rebuilding that work.

We thus have an estimate for the thorough repair and completion of all the harbor works to the amount of \$33,739. This will have to be appropriated to accomplish all these repairs. The plan decided upon for the improvement of this harbor, under the appropriations of 1866 and 1867, was to extend the north pier 500 feet out into thirteen feet depth of water below lowest stage, and to dredge out the whole space between the piers called the outer bar, so as to allow a uniform depth over that space of twelve feet below lowest known stage. The amount of these two appropriations was \$61,961, and required to be expended under contract to the lowest responsible bidder. Considerable time was necessarily consumed for making the contracts before we could begin the actual work at the harbor. On about the 20th March, 1867, the contracts were made, and from that time to June 30, 1867, there was expended \$6,214 91. The cost of extending the north pier, including ten per cent. for contingencies, will be \$34,339. Hence there will be left available out of those appropriations, for dredging and other purposes, \$27,622.

This is one of the harbors which I reported February 1, 1867, as being worthy of improving to a condition of allowing vessels of fourteen feet draught at all times to enter and depart, and I now give the estimate of the additional appropriation required to be made to all former appropriations for attaining this

object, including ten per cent. for contingencies, \$35,000.

Dredging outer bar — This was begun on the 16th day of April, 1867, and during the remainder of that month 1,878 cubic yards were raised and dumped. From that time to 30th June, 1867, (end of fiscal year,) no more was done, because of the stipulation in the contract that no dredging of the outer bar should be done in advance of the progress of the extension of the north pier. The dredging will be resumed and carried on in accordance with this stipulation.

5.—Conneaut harbor, ohio—(Sketch E.)

The appropriations of 1866 and 1867 for this amounted to \$30,513, both requiring to be done under contracts which were made October 1, 1866; and during the fiscal year in consideration there was expended to June 30, 1867, \$3,134 81; leaving July 1, 1867, available, \$27,378 19. The plan fixed upon for the improvement was to thoroughly repair the old piers, and extend the west pier into twelve feet water, and dredge the interior between the piers and outer bar to obtain twelve feet water throughout at low stage. The cost, according to the contract of the thorough repairs of the old piers, including the building up of the breached flaring 130 feet of east pier on a straight course for the same extent out

into the lake, will be \$9,064, including 10 per cent. for contingencies.

I advertised May 17, for proposals for dredging A very high single bid was received at \$1 25 per yard, owing to doubts entertained upon the character of the earth to be dredged, and the bottom upon which the extension of the west per for 350 feet is to be built. I had, after I inspected the work, 1st June last, borings made into the bottom. The auger penetrated to rocks, showing a regular dip to the northward of 7.7 feet in 360 feet, or a dip of one-fourth of an inch vertical to one foot horizontal. But in no place did we strike rock until we had descended more than twelve feet below low-water stage. Sand is the material to be dredged, and at the proposal I received, (\$1 25 per yard,) the total dredging contemplated in outer bar would cost, including 10 per cent. for contingencies, \$10,265.

I considered that bid, based upon a different view entertained by the bidder from my own concerning the character of the bottom, unreasonably high. I have therefore not made a contract for the dredging, but concluded postponing

the matter, and advertising again for dredging, at the same time of advertising for the materials and work for the extension of the west pier to be done next season to the extent proposed, should Congress make the necessary appropriations. The cost of the dredging to place the cribs, and for the bar and interior, ought not to exceed \$6,181; the cost of the extension of the pier, \$22,771, including 10 per cent. for contingencies. We therefore find that in addition to the appropriations of 1866 and 1867, we shall need an appropriation to complete the work to the extent and capacity planned for, of \$10,638.

6.—Ashtabula harbor, ohio—(Sketch F.)

The two appropriations for this in 1866 and 1867 amounted to the sum of \$78,708 32. The plan adopted for this was to—1. Thoroughly repair both of the old piers. 2. And to extend them to twelve feet water into the lake in parallel lines, doing away with the flare. 3. To dredge between old piers and the extension so as to obtain twelve feet water throughout below lowest stage.

Contracts were made for materials and labor for item 1, about the 1st October, 1866, and thorough repairs of the old piers upon the estimate of materials and labor for the work first made will cost, including 10 per cent. for contingencies, \$11,838; and at the close of the fiscal year ending 30th June, 1867, there had been expended \$5,944 03, leaving 1st July, 1867, a balance on hand of \$72,764 29.

From the survey made by W. T. Casgrain, under the direction of the superintendent lake survey, in May, 1865, upon which plans and estimates for the improvement, items 2 and 3, were based, we inferred that we should have to dredge in shale rock between the piers, also on the outer bar, in order to obtain twelve feet water below low stage. Much time was spent in studying plans and estimating for such a condition of the bottom, and it was upon that condition that I made my estimate upon which Congress appropriated in 1867, and that item 3 was advertised for proposals May 17, 1867. The lowest (and there was only one) bid was for the sand \$1, and for shale \$2 75, if no blasting was required; but if blasting was required, \$4 50 per yard. These conditions vitiated the bid, and in my inspection in the fore part of June, coming to the opinion that there was no evidence of rock exterior to the old piers that would require dredging, I ordered borings to be made to test the question. These borings show that we shall not come to rock, but be in sand for the whole depth of twelve feet below low stage, from A in the sketch for 450 feet out to the twelve feet depth of water in the lake.

But from the point A, as we go interior for 565 feet we come to rock that will have to be dredged to obtain twelve feet water for the harbor between the

old piers. This rock is overlaid with sand and gravel.

Now I dare not dredge the interior harbor close up to the piers to twelve feet depth for fear of undermining these old structures. I think they were mostly laid upon the sand without dredging to pose them upon the rock. But I think we can safely make a channel sixty feet wide in the middle of the water-way to the required depth without undermining, and as the new extensions are to be laid on a level bottom, twelve feet water, we can dredge the whole space between them on the outer bar to the required depth. The interior dredging to sixty feet width will be sand, 2,763 cubic yards; gravel, 627 cubic yards, and rock 3,892 cubic yards. The exterior dredging, including dredging to lay the piers, will be sand, 5,880 cubic yards. Since the examination I instituted was made, in June last, entire new plans and estimates have had to be made differing considerably from those made upon Mr. Casgrain's survey. I estimated upon the new survey the proposed extension of both piers to cost, including 10 per cent. for contingencies, \$38,526; and the dredging inside of the piers, and to place the cribs and to remove the outer bar, including 10 per cent. for contingencies, \$18,851:

hence after repairing old work, item 1, extending the piers by new work, item 2 and dredging, item 3, we shall probably have left about \$3,569, requiring no further appropriation to be made for this work.

7.—GRAND RIVER HARBOR, OHIO—(Sketch G.)

This harbor was commenced to be repaired, under the appropriation of 1864, upon the allotment of \$10,000 at first; afterwards increased to \$24,453 24, out of the general appropriation for repairs and preservation of lake harbors, works not requiring to be done by the lowest bidder after advertising for materials and labor.

There was a difference of opinion as to the manner and plan of repairing this work. I recommended that as the old flaring part of the west pier from "X" out to "Y" was breached and much of it gone, it would be best to repair that pier out to "X," and thence carry a work straight out parallel with the old east pier for an equal extent of the flare. I was overruled in this and ordered to repair the whole quite out to "Y," and to repair the whole of the east pier which was done, costing \$24,453 24; all this out of the appropriation of 1864. Of this amount there was expended in the fore part of the fiscal year commencing July 1, 1866, \$8,443 60, and the completion of the repairs of the old pier to their outer extremities has been accomplished.

Believing that we could remedy the evil of the flare by extending out the east pier by a new construction in a parallel direction with "X Y," I made plaus and estimates for doing so, and the sum asked for was appropriated 23d

June, 1866, to the amount of \$22,111 80.

I was also overruled in my recommendation as to the direction of extending the east pier, and required to make another survey, &c., which I made in the winter of 1867 on the ice. The result of this survey confirmed me in the opinion that the best plan was to extend the east pier from "E" to "E" around parallel to "X Y" of the west pier.

The engineer department, however, took a different view upon the question and ordered me to extend from "E" straight out on the prolongation of the old direction of the east pier, and called for an additional appropriation of \$60,000, which was made by act approved March 2, 1867, thus making available the appropriations of 1866 and 1867 for the extension of the east pier, and any other improvements that may be required for this work the sum of \$84,453 24.

The work of extension was put under contracts, materials and labor, about 20th March, 1867. Since that time and up to the close of the fiscal year ending 30th June, 1867, there has been expended out of this (\$84,453 24) the sum of \$3,143 02, leaving available at the end of that fiscal year \$81,310 22. The cost of extending the east pier three hundred and twenty feet will be, according to the contract, including 10 per cent. for contingencies, \$21,631; and after this is done we shall have the probable available sum for future improvements of this harbor, (in dredging probably,) \$33,040 30, requiring no further appropriation at present.

8.—CLEVELAND HARBOR, OHIO—(Sketch H.)

The sum allotted from the appropriation of 1864 for the repairs of this work was \$20,000. A less sum than this from that appropriation, however, has been applied to its repairs, being only \$12,310 79. Of this during the fiscal year commencing July 1, 1866, and ending June 30, 1867, there was expended \$1,949 27.

Insamuch as the principal part of the east pier had been monopolized by a railroad company, it was decided by orders from the engineer department that no repairs should be done to the east pier at the expense of the United States.

In my former reports I have fully set forth the uses and abuses of this pier, all of which still continues, and in consequence the old pier is fast going to destruction. The plan for enlarging this harbor was settled upon, consisting in an extension of both piers out into the lake as shown in the sketch in red; and for this purpose an appropriation was made by act approved 23d June, 1866, of \$59,806.

The contracts for the extension were made about the 10th of October, 1866, for materials and labor, and for the execution of the work. During the fiscal year ending June 30, 1867, there has been expended out of the appropriation, principally for materials, \$8,215 55. Making an expenditure out of this and of the appropriation of 1864, during the fiscal year commencing July 1, 1866, and ending June 30, 1867, of \$10,164 82. The amount available July 1, 1867, from the appropriation of 1866, was, therefore, \$51,590 45. The cost of the extension of the piers, according to the plan and upon the contracts made will be, including ten per cent. for contingencies, \$63,497. Required, therefore, to complete the present plan, an additional appropriation of \$3,691.

This is one of the harbors recommended in my report of February 1, 1867, as entitled to the consideration of being improved to the condition of allowing vessels of fourteen feet draught to enter and depart at all times. Should this enlargement of capacity, which is highly desirable, be ordered, there will be required for this purpose another additional appropriation of \$38,622.

9.—Black river harbor, ohio—(Sketch I.)

During the fiscal year commencing 1st July, 1866, and ending 30th June, 1867, there was expended in the completion of the repairs of both of the old piers, thoroughly, to their outer extremities, the sum of \$10,334 88. This was all from the appropriation of 1864. In a year or two hence there will probably be required an expenditure for stopping holes that may develop themselves in the old under-water work of west pier of some few thousands of dollars. The rebuilding of the outer extremity of the east pier has much improved the depth over the outer bar and restored the harbor to a condition of usefulness. The appropriation made in 1866 of \$10,000 for this work is still available for the future repairs above alluded to.

10.—HURON HARBOR, OHIO—(Sketch J.)

By act approved June 23, 1866, there was appropriated for this work the sum of \$39,000. The plan decided upon was to thoroughly repair both piers, including building up the cribs that had been swept away from the extremity of the east pier. After duly advertising, contracts were entered into for materials and work for this purpose about the 3d of October, 1866; the timber was all to have been delivered by the 10th of June, 1867. tractor failed to furnish all, and the difficulty was made good by advertising anew, and a contract was made for it with the lowest responsible bidder on the 22d of June, 1867. The percentage already retained from the first contractor was more than sufficient to make good all loss to the work by this failure. The amount expended out of the above appropriation during the fiscal year commencing the 1st of July, 1866, and ending the 30th of June, 1867, was \$10,960 24, leaving on the 1st of July, 1867, available, \$28,039 76. The cost of the repairs according to the contracts, including ten per cent. for contingencies, will be about \$25,070 15, which is less than the appropriation by \$13,929 85. No additional appropriation will therefore be required for this work.

11.—VERMILLION HARBOR, OHIO—(Sketch K.)

The amount appropriated for the repairs of this harbor by act approved June 23, 1866, was \$15,315 74. The piers were much dilapidated, and seven cribs of the lake extremity of the east pier have been breached away. The plan was, to rebuild these cribs for their whole width, and for the superstructure that was to be rebuilt, to make it 12 feet wide above water, and ballast by filling with stone and planking over. Contracts for the material and labor were made on the 10th and 20th of October, 1866. The amount expended to June 30, 1867, was \$11,987 16, leaving on hand July 1, 1867, only \$3,328 58. The total cost of thoroughly repairing the piers, including ten per cent. for contingencies, will be \$21,827, costing considerably more than was estimated previously to the appropriation being made. More work, however, will have to be done before exhausting the appropriation than was originally estimated for.

The first examination was made as well as it could be under the circumstances, but other damages have been found, by a more critical examination, in the old under-water west pier; and, since stopping a 400 feet breach in west pier, a cutting away of the shore at the inner extremity has occurred, requiring much work to secure the pier there against destruction, an additional sum, either by special appropriation or out of the general fund for "repairs and preservations," to the amount of \$6,511. With this I think we can have, before the expiration of the present summer, the work fully repaired. But if this is put off to another season it will probably cost much more, owing to the breaches that will occur

during the coming winter.

12.—SANDUSKY CITY HARBOR, OHIO—(Sketch L.)

For this there was appropriated by act of June 23, 1866, the sum of \$38,580 The plan decided upon was not at first to construct the side piers P P seen on the sketch, but to dredge a channel 400 feet wide across the narrow outer bar to a depth of 12 feet below the lowest known stage, and wait for the effects upon it, and afterwards to decide whether the piers would be necessary. Accordingly, after duly advertising for proposals, a contract was made for the dredging on the 12th day of October, 1866, to commence with one dredge as soon as the weather would permit. The weather, however, did not allow of dredging in that exposed place before the expiration of the fiscal year, June 30, 1867. The amount expended to June 30, 1867, was only \$2 10, laving, July 1, 1867, available, \$38,577 90, and no additional appropriation needs to be made for this until after the effects following the dredging may show the necessity of further appropriations.

13.—SURVEY AND IMPROVEMENT OF SANDUSKY RIVER, OHIO.

(See map of this river, engineer department.)

By act approved June 23, 1866, an order was made for a survey of Sandusky river, with a view of plans and estimates for its improvement. In October, 1866, I had the survey commenced, and completed in December following, costing in all (for the survey) \$468 90. This was defrayed out of the appropriation for "examinations and survey on the northwestern lakes." The maps of the survey were forwarded to the engineer department, with a report containing an estimate for improving by dredging between the town of Fremont, Ohio, and the Sandusky bay, December 4, 1866. The total amount of dredging estimated for the improvement of the river in seven different places was 185,075 cubic yards sandy earth to be excavated, so as to give a 12 feet channel from 160 to 200 feet wide at the various places. By act approved March 2, 1867, the sum appropriated for the improvement was \$20,000. In May following advertise-

ments were issued for proposals to do the work. A favorable contract made for it, as far as we have or shall obtain the means, June 5, 1867. amount expended on the improvements to June 30, 1867, was \$7 60, les amount available July 1, 1867, \$19,992 40. At the prices in the contract improvement of all the places will cost, including ten per cent. for continge \$54,967. Hence to complete the work upon this plan for the improvement should require an additional appropriation of \$34,967.

14.—Tolbdo harbor, ohio—(Sketch M)

By acts of June 23 and March 2, 1867, the sums appropriated for the provement amounted to \$40,000. The plan decided upon for this improvement amounted to \$40,000. The plan decided upon for this improvement to decide the channel to a width of 200 feet and to a depth of 1 below the lowest stage from buoy No. 1 to buoy No 7 seen on the sket extent of 4,800 feet. Advertisement was published inviting proposals A 15, and a contract was made October 12, 1866, to commence the dredgin continue it during that fall, weather permitting. But the weather did not dredging to be done so late in that season. In the winter, taking advant the ice, range signals were erected so as to guide the dredge in its work. dredging commenced the 27th of March, 1867, and up to the end of the year (June 30, 1867) 16,236 cubic yards were raised by one dredge, to away, and dumped. The actual number of working hours of the dredge 425. The number of days on which the dredge was prevented by bad we from working full work (ten hours) was 47, and by the breaking of the chinery, was 16. Amount expended on the work during the fiscal year of June 30, 1867, \$4,564 62, leaving available July 1, 1867, \$35,435 38.

I do not think it advisable to ask at present for an additional appropto be expended in this particular direction, from the mouth of the river usuch a short turn.

For so important a commercial place as Toledo is, and for its future grocommerce, the improvement should be one commensurate with the comme the place, and should be made upon a scale and direction adapting it for for vessels passing up and down the lake in all weather.

This is one of the harbors I recommended in my report of 1st Feb 1867, as worthy of being made so a vessel drawing fourteen feet could ent depart at all times, and estimated that it would cost \$469,664. This co plated a channel 300 fect wide. The plan, however, for making this one very best improvements would be to prolong the river straight from A out fifteen-feet water in the lake, in the direction of the red-arrow line seen of sketch, and dredge a channel 200 feet wide. which would be about four long, and to a depth of fifteen feet, using the materials to make the canal b that should rise five feet above water and dike the sides. But to obtain a estimate of the cost of such a work, it would require a previous examin and survey, with borings along the line. From my personal knowledge of Maumee river, for the canal and adjacent ground, every inch of which I been over, I am satisfied the material to be dredged would be highly favor for such a work, and, what is quite as important, that the force of the ou of the Maumee river would ever keep the canal free from deposits. I d think a better application of a few hundred dollars of the appropriatio "examinations and survey of the northwestern lakes" could be made the apply it to the examination for the improvement, herein suggested, throug

By order of the chief engineer, in 1863 and 1864 I made complete su and a detailed report, and selected sites for the temporary works for mi defences of the harbor. These surveys show that the defence of the teasy, and may be made very perfect.

15.—Monroe Harbor, michigan—(Sketch N.:

By act approved 23d June, 1866, there was appropriated for this work \$31,015 27. The plan adopted for its repairs was, 1st, to rebuild the outer breach part of south pier, and to repair the decayed and broken portion of the inner parts of that pier; 2d, to repair the west pier thoroughly; 3d, to construct a new work of 300 feet in length to connect the inner part of the west pier with shore, to stop a dangerous breach that had been opened here in the side of the canal, allowing sand to be driven through from the lake beach into the harbor at every blow from the northeast.

It required much labor in making measurements and plans and bills of materials to fit everything to the old work. Borings were made to determine how to place the construction of the new work, (item 3.) After duly advertising, inviting proposals, contracts were made 2d October, 1866, for the material and for doing the work. The delivery of materials and doing the work were pushed through the winter, and the energetic contractor, W. H. Most, taking advantage of the favorable condition of the ice which covered the harbor, all the exterior under-water work of item 1 was securely put in before the spring thaw came.

Up to the end (30th June, 1867) of the fiscal year there had been expended \$6,760 06, leaving available 1st July, 1867, \$24.255 21. By the price in the contracts, the cost of completing all the items, 1, 2, 3, including ten per cent. for contingencies, is \$12,484 36, leaving, after completing items 1, 2, 3, \$12,770 65, which can be applied to the harbor hereafter, as circumstances may develop. No additional appropriation seems necessary to be asked for at present for this work.

16.—St. Clair flats, michigan—(Sketch O.)

By act of 23d June, 1866, and of 2d March, 1867, there were sums appropriated for this improvement amounting to \$230,000; and by the last of raid acts Congress fixed the plan to be followed for the improvement, to consist of a straight, direct ship canal from sufficiently deep water at the mouth of the south pass of the St. Clair river, through the shoal, to sufficiently deep water in Lake St. Clair, an extent of about one and a half mile; the canal to be 300 feet wide in the clear—sides to be diked and banked, to be made five feet above water and fifty-eight feet wide on top, of the materials to be dredged out to make the water-way thirteen feet deep below lowest known stage.

After receiving orders to make preparations for commencing the work, advertisements were published inviting proposals for furnishing materials and doing the work, March 20, 1867. Owing, however, to my absence under orders from the War Department, the bids were not opened and the abstracts of them made out, so as to decide who were the lowest responsible bidders for materials and labor, until May 7, 1867. A difficulty arose in awarding the contracts, and this was not allayed before the expiration of the fiscal year, 30th June, 1867, so that up to that date nothing had been accomplished, but much office work.

Amount expended to 30th June, 1867, \$29 68, leaving available 1st July, 1867, \$229,970 32. My first approximate of the cost of the work was \$428,754. As yet I know of no reason why this estimate should be enlarged, though, when we come to exactly locate the work and take the soundings referred to, the lowest known stage, we may find a result differing somewhat, in the amount of dredging, from what I estimated and based upon the survey of 1856, made under the direction of Captain, now Major General, Meade. It was upon that survey, the last we have, that I made my estimate, and so stated in my first report, August 13, 1866, and in my revised report, December 10, 1866. Upon that basis, therefore, besides what was available July 1, 1867, we shall need an additional appropriation to be made by Congress for the completion of this work to the amount of \$198,754.

Notwithstanding the embarrassment arising to prevent putting this import in operation during the past fiscal year, I have no doubt that we commence active operations on the work by at least two dredges before the of September, 1867, and be able to push it to an entire completion, as Conwill make the needful appropriations.

17.—Saginaw River mouth—(Sketch P.)

By acts 23d June, 1866, and 2d March, 1867, the sums appropriated to

highly important work amounted to \$95,500.

This estimate, it is to be borne in mind, is based upon a survey made the direction of Captain Macomb, topographical engineer, in 1856, without resenting the character of the bottom to be dredged. Advertisement wa lished inviting proposals for executing this work August 28, 1866. A fav contract was made 5th October, 1866. Owing, however, to the lateness season, work could not be commenced that season, and the time for comp the work to the 30th November, 1868, should the materials to be dredged The plan fixed upon was to dredge a channel from the lower re the river proper, straight out into the lake, whatever materials might be fo the way, for a width of channel at bottom at first of 195 feet, with side of two horizontal to one vertical, and to a uniform depth of twelve feet below the lowest known stage of water, and to make it 200 feet wide after as we could obtain the means. Taking advantage of the ice during the I had made under my direction a complete set of soundings and boring the bottom to the required depth for dredging. These borings develo remarkable formation of earth.

I also had during the winter the channel to be dredged perfectly locate ranges on the mainland erected by which the dredges should be guided in a the channel to the proper width and alignment and the "bench mark" lished for the lowest stage of water, by which the depth of dredging she regulated, and the place marked for dumping. The operation of dredgended commenced 21st May, and on that day a furious northeast gale suddenly and drove the large dredge ashore; owing to this bad luck to the contained the bad weather it was not until the 12th June that he could recommend the property of the remaining sixteen working days of that month there were detailed.

During the remaining sixteen working days of that month there were dand dumped 3,426 cubic yards of very hard conglomerate clay, grav sand. This closes the operation up to the end of the fiscal year, 30th 1867. I am gratified to find that notwithstanding the hardness of the mathere is no doubt of our being able the present season to open a channel its close of navigation, that will admit vessels of ten feet draught, running the entire length of the new cut. The portion which will be dredged to the full depth of twelve feet, the work having been commenced on the mit of the bar and extended both ways. One of the most powerful material, sweeping the whole width, 195 feet, of the channel and to the depth at a swing, leaving the bottom as even as a house floor. Two dred at work and each performing admirably, giving assurance of a complete in the undertaking of the opening of a channel which before we combad to many the appearance of a quixotic effort, but which now be challenge admiration.

The amount expended to the 30th June, 1867, was \$422 32. The dredged not having been paid for until July, comes into the next fiscal expenditure.

Amount available 1st July, 1867, was \$95,077 68.

As yet, I have no evidence of its costing more than I estimated in annual report to complete this work. I do not therefore ask at presented ditional appropriation for this work.

18.-HARBOR AT AUSABLE RIVER, MICHIGAN-LAKE HURON-(Sketch Q)

After this was placed under my charge I found it necessary to make a tedious special study of the question. The survey that was made for this work by the lake survey assistant was defective in two essential points. No reference was given by which the low stage of water at the mouth of the river could be compared with the stage at the time of the survey, nor was there a bench mark made to which the stage, even at the time of the survey, can ever be identified. These are points too much neglected by officers and assistants of our corps, who are intrusted with these important surveys, and the officer who is called upon to make the detail plans and drawings, and bills of materials and the estimate of the cost of the work, is forced to grope in the dark for the proper data to work upon.

I planned a work, however, for a ten foot harbor, the stage of reference to be fixed hereafter, and it was approved, and advertised inviting proposals May 14, 1867, and reported an abstract of the bids received June 11, 1867. Owing to contention among the bidders arising from a difference of opinion as to what should be the construction of the law in reference to the principle of awarding contracts, this work has not been put under contract up to 30th of June, 1867. I have no expectation that it is possible to obtain a sufficient quantity of materials to judiciously commence work with until the 30th of May, 1868. From that time forward the work may be expected to be as rapidly pushed forward as the very unfavorable circumstances attending the place will permit.

By act approved 2d of March, an appropriation was made for this work of

\$50,000, and all of this was available 1st of July, 1867.

I have been able to make a close estimate of the cost of completing the work upon the plan fixed upon and the bids from responsible contractors for furnishing materials and doing the work, but not so close as I could have done had the faults in the survey not been perpetrated. The estimate of the cost, including ten per cent. for contingencies, is \$69,367. We shall, therefore, need an additional appropriation to complete the work of \$19,367. This, put with the \$50,000 already available, will give us as good a harbor at this site as the case admits, but which, however well we may first construct it, will need in a few years much dredging from the enormous quantities of sand which the river brings along from the interior into the lake to be deposited at the Louth.

19. IMPROVEMENT OF ST. MARY'S RIVER, MICHIGAN-(Sketch R.)

By acts of 23d of June, 1866, and 2d of March, 1867, the sums appropriated for this work amounted to \$100,000.

After fixing upon the plans for expending the money to the best advantage for the large and increasing commerce that is to be benefited by the improvement of this river, proposals were invited for dredging, August 23, 1866, and a favorable contract was made for dredging, (only at present, however, in middle channel Lake George, a shoal expansion of the river,) 5th of October, 1866. The work was to be commenced as soon as machines could be taken there and completed, either by November, 1867, or in the following season of 1868, according to the softness or hardness of material found necessary to excavate to obtain a channel not less than 200 feet wide at bottom, and of a uniform depth of fourteen feet below low stage at middle channel, and thirteen feet depth at sides, with side slopes of two horizontal to one vertical. It was not possible, owing to the lateness of the season, before the contract was concluded and approved, to get the dredges safely into position before the 22d of June, 1867. As soon as possible I sent my assistant, Captain Lydecker, corps of engineers, who fixed the range signals and established the low-water "bench-mark" to guide the dredges, and located the field for dumping. On the 29th of June dredging commenced in good carnest, and up to the 30th of June, 1867, there had been excavated eighty yards. From the 1st of July, 1867, onward there are two dredges at work day and night.*

Disbursed up to 30th of June, 1867, end of the fiscal year, on the

\$4 56, leaving available 1st of July, 1867, \$99,995 44.

I have seen no reason since sending in my annual report for fiscal year e 30th of June, 1866, to change the estimate I have made for the improve of the several places requiring work to be done in this great river. The there will yet remain to be appropriated to complete the improvements, it ing ten per cent. for contingencies, \$323,983.

20. Summary statement for the fiscal year commencing July 1, 1866, and e June 30, 1867.

Name of work repaired, improved, or surveyed for improving.	Amount expended under appropriation of 1864.	Amount expended under appropriations of 1866 and 1867.	Amount of full estimate for completion required to be appropriated.	Amount that can be pro- fitably expended in next
Sea wall, Buffalo	\$35, 554 35	\$9, 194 06	\$46,920 00	\$3
Buffalo harbor, New York				1
Dunkirk harbor, New York	.		48, 584 00	5
Erie harbor, Pennsylvania	5, 854 36	6,214 91	33, 739 00	5
Conneaut harbor, Uhio		3, 134 81	10,638 00	1 3
Ashtubula harbor, Ohio		5, 944 03		1 7
Grand River harbor, Ohio	8, 443 60	3, 143 62		9
Cleveland harbor, Ohio	1,949 27	8 215 55	3,691 00	6
Cleveland harbor, Ohio	10, 334 88			
Huron harbor, Obio		10, 960 24		2
Vermillion harbor, Obio			6, 511 00	1
Sandusky City harbor, Ohio		2 10		3
Sandusky City harbor, Ohio		*476 50	34, 967 00	ī
Toledo harbor, Ohio		4,564 62		3
Monroe harbor, Michigan		6, 760 06		
St. Clair fluts, Michigan			198 754 00	25
Saginaw river, Michigan				6
Aurable harbor, Michigan			19, 367 00	6 5
St. Mary's river, Michigan			323, 983 CO	é

^{*}For survey, \$168 90; for improvement, \$7 60; out of appropriation "Surveys and examinatio western lakes."

REMARKS.—This table does not contain the estimates for adapting Buffalo, Erie, Cleveland, and T vessels drawing fourteen feet of water; for these additional estimates see text to the report.

Of course it is understood that the foregoing report makes no record of done or expenditures made since 30th of June, 1867. Since that time a amount of materials have been delivered, and much work has been done on a works that were put under contract before the expiration of the last fiscal all of which, with the expenditure, will come properly in place in my next a report.

I have the honor to be, very respectfully, your most obedient servant,

T. J. ORAM, Colonel Engineers, Brevet Major Gener

Major General A. A. Humphrbys,

Chief of Engineers United States Army.

^{*} In July one dredge working twenty-one days, and two three days, relieving each at night, raised 15.287 cubic yards soft. In August two dredges, relieving each other at raised 32.561 cubic yards soft. Number of hours worked, 1,169½; which means that tantamount to one dredge working 1,169½ hours to raise 47,848 cubic yards soft mate that channel bed.—September 12, 1867.

B 1.

United States Engineer Office, Detroit, June 11, 1867.

Siz: I have the honor to make this report upon the contemplated harbor at Assable, Lake Huron:

I. In your order of 28th March last, assigning this work to my charge, I was directed to submit my views, in case of a dissent from Colonel Raynolds' plan, for this work. I do not dissent from the general plan as to direction, extent of piers, and width of channel-way; but I respectfully submit that a ten-feet depth of water for this harbor will cost about thirty thousand dollars less than to make a twelve-feet harbor, as proposed and estimated for by that officer. All acquainted with that place are of opinion that this will be ample for all the trade there. And owing to the enormous quantities of sand away up in the river, and the other causes for accumulating deposits at the mouth of the harbor when constructed with all the care possible, I am satisfied that to maintain even that depth (ten feet) we shall have to resort to dredging, or extend the piers indefinitely beyond the present proposed limits. A bar will form across the mouth in a few years after constructing the piers. The lowest bid for constructing the work for a twelve-feet harbor is \$82,892 72; the lowest bid for constructing the work for a ten-feet harbor is \$58,037 37. Which depth shall be adopted?

II. I have advertised and received proposals for each, as will be seen by the accompanying advertisement, and bids have been put in according to the advertisement, more especially according to the classifications (see last paragraph of advertisement) of work or labor and of materials, as shown on the abstract of bids herewith sent. To this classification, which is in harmony with the law making the appropriation, we must adhere, if we expect to hold the bidders up to the proposals, and successfully accomplish the work to good advantage.

For the materials: class 1, round piles; class 2, sawed timber and lumber;

class 3, all kinds iron material.

For the labor or work: class A, dredging to place cribs, framing, boxing, bolting, placing, driving piles in their corners, superstructure, filling the cribs with stone, slabs, and brush, framing, fitting joist, plank, &c.; class B, dredging channel between piers and just above piers in the river.

For each class of materials the lowest bidders are Carkin & Kimball, No. 3. For class A, labor or work, the lowest bidder is H. M. Mixer, No. 5. For class

B. labor or work, the lowest bidder is Hasbrouck & Conro, No. 4.

Observe: Henry M. Loud, No. 8, in his bid for the whole, set class 1 (round piles) lower than No. 3 did, but No. 8 bid for the whole or none. It will be seen by the abstract, for either a twelve or a ten-feet depth of harbor, the abovenamed are the lowest bidders.

I have to request the proper instructions from the proper authority to make the contracts with Nos. 3, 4, 5, as above named. I also request to be informed whether to contract for a twelve-feet or a ten-feet harbor.

Very respectfully, your obedient servant,

T. J. CRAM,

Colonel Engineers, Brevet Major General.

Major General A. A. HUMPHREYS,

Chief of Engineers United States Army.

P. S.—For a twelve-feet harbor we have proposals for constructing the same length of piering and planking it over, \$31,861, less than estimated for by Colonel Raynolds. This is the result of a more economical plan for cribs, and using slabs and brush to some extent, and less stone than proposed in his estimate.

To Contractors.

OFFICE LAKE AND RIVER HARBOR IMPROVEMENTS, Detroit, No. 111 Griswold str.

Written and sealed proposals for furnishing materials and doing the work be received at this office, addressed to the undersigned, until the 31st of 1867, for constructing the piers and dredging for a harbor at the mouth of ble river, Lake Huron, Michigan.

Materials required.

CLASS 1.

240 round white oak piles, 12 inch middle diameter, 30 feet long.

CLASS 2.

1, 560 side timbers, sawed, 12 by 12, 30 feet long.
120 side timbers, sawed, 12 by 12, 32 feet long.
360 side timbers, sawed, 12 by 12, 20 feet long.
2, 656 cross timbers, sawed, 12 by 12, 16 feet long.
1, 020 crib bottom, sawed, 8 by 12, 16 feet long.
52 brace timbers, sawed, 8 by 8, 9 feet long.
210 scantling, sawed, 3 by 4, 14 feet long.
1, 260 boards, sawed, 1 by 12, 20 feet long.
900 joists, sawed, 2 by 12, 9\frac{9}{4} feet long.

The total amount of the sawed stuff, 1.436,920 feet, board measure.

CLASS 3

1,455 bars 1-inch square iron, 16 feet long, 77,115 pounds. Wrought spikes, 6 inches long, 420 pounds. Wrought spikes, 4 inches long, 2,057 pounds. 10d. cut nails, 120 pounds.

1,661 covering plank, sawed, 2 by 12, 16 feet long.

CLASS 4.

2, 180 cords rubble stone, measured in cribs.

840 cords of slab.

840 cords of brush, measured under a pressure of 132 pounds to the the cord, after being put in the crib.

State the price per pile for class 1.

State the price per thousand feet, board measure, all sawed stuff, class:
State the price per pound for the iron, per pound for the spikes, per pouthe nails, class 3.

State the price per cord for stone, also for slabs, also for brush, class 4.

The government reserves the right of diminishing the above quantities sit be decided to make a ten instead of a twelve feet harbor.

The timber to be of white oak, white or Norway pine, all sawed trul squarely to dimensions stated, from good live timber, and to be free from knots, splits, shakes, and other defects tending to impair its durabil strength. One-fourth of the piles and timber to be delivered on or beforest day of July, 1867, and for four months in instalments of one-eighth me thereafter during navigation, and all of the remainder on or before the April, 1868.

Work to be done under water.

There are 60 cribs 30 feet long, 14 or 16 feet wide, to be placed in 10 or 12 feet depth of water, on a sand bottom, where the average depth of water is now about six feet; the total length of both piers to be 1,800 feet; a pile is to be driven in each crib-corner, and the cribs to be filled with brush, slabs, and stones, in proportions to be given. The amount of dredging to place the cribs in 12 feet water is 4,433 cubic yards.

After constructing the two lines of piers, 45,121 cubic yards, to make a twelve feet harbor, is to be dredged out from between the piers and up the river immediately above the piers. The earth to be dredged as all sand. In the cribs there will be about 94,080 running feet of twelve inch square timber, and about 28,560 feet, board measure, of boards and scantling in the cribs. State the price in the bid for dredging per cubic yard to place the cribs. State the price in the bid for framing, fastening, boring, and bolting, and putting in place the cribs per running foot of timber so framed and put into the cribs. State the price per M feet, board measure, for fitting and fastening in scantling and bottom boards.

The above amounts of work to be done will be about one-sixth less should a

10 feet instead of a 12 feet harbor be made.

Work to be done in superstructure above water.

There will be 1,800 running feet of pier work, 14 or 16 feet wide, and four timbers high, above the tops of the cribs and surface of the water, and to be filled with stones and covered with plank spiked down to the joists, which are to be grained and spiked into the cross ties. State the price in the bid for fra ing and fastening the superstructure per running foot of timber, of which there will be about 22,684 running feet in the superstructure. State t' e price for joining and spiking in the joists and fitting and spiking on the covering plank per thousand feet, board measure, of which there will be about 70,552 feet board

Bidders are cautioned to conform strictly with the manner above given in proposing their prices, both for the items in the classes of materials and for the items of work specified.

The time for completing all the work will be extended if required by the contractor until the fall of 1868.

It is highly desirable that the furnishing of all classes of materials and doing all the work should be under one contract.

The doing of all the work and furnishing all the stone, slabs, and brush, must be done by one contractor, though he may not furnish all other materials.

T. J. CRAM,

Colonel Corps Engineers, Brevet Major General.

Abstract of bids for furnishing materials and doing the work at Aus

		M	anufactured ma	terials bid	or.
		Class 1.	Class 2.	Class 3.—A	ll kinds of
Number of bid.	Name and residence of bidders and bondsmen.	240 round piles.	All sawed timber and lumber, 1,104 520 ft. b. m.	Drift bolt iron, 67,360 lbs.	Spikes, 2, 477 lbs.
1	Backus Bros., Ausable, Mich.; Standart Bros., Detroit, bondsmen.	Per pile. W. oak, \$8; h. pine, \$5; w. o. \$1,920; pine, \$1,200.	\$24, 851 7 0	Per lb.	Per lb.
2	James McGill, Aussble, Mich. No bonds- men offered in bid.		Per M.	· · · · · · · · · · · · · · · · · · ·	•••
3	Carkin & Kimball, East Saginaw, Mich. No.	\$2 90	\$15.90		
	bondsmen offered in bid. Hasbrouck & Conro, Milwaukee, Wis.; A.	696 00	16, 501 46 1,005,648 ft.	3, 974 24	198 16
•	Goodrich, Chicago, M. B. Medbury, Mil- waukee, bondsmen.	960 00	\$20, \$20, 112 96; 98.872 ft. \$16, \$1,581 95.	4,715 20	247 79
5	H. M. Mixer, Monroe. Michigan; N. M. Brooks,		18 00	6	8
6	Detroit, J. M. Sterling, Monroe, bondsmen. Alex, McDonell & Co., Hamilton, C. W. No		19, 881 36 18 00	4,041 60	198 16 10
١	bondsmen offered in bid.	1, 464 00	19, 881 36	5, 398 80	
7	Harvey P. Platt, H. F. Stock, Toledo, Ohio; A. C. McNairy, Henry M. Classiin, bondsmen.	i			•••
8	Henry M. Loud, Detroit, whole or none. No bondsmen offered, but good promised.	3 59 840 00	17 90 19, 770 90	3, 533 90	19 8 16
9	Loud, Priest & Gay, Detroit. No bondsmen offered, but good promised.		17 90 19, 770 90		

9	Loud, Priest & Gay, Detroit. No bondsmen	17 90 19, 770 90
	Abstract of bids for furnishing n	naterials and doing the
		Work and
		Cla
	Name and residence of bidders and bondsmen.	
Number of bid.		Filling cribs and superstructs 1,047 cords stone; 600 cords or brush.
Nump		
1	Backus Bros., Ausable, Mich.; Standart Bros., Detroit, Mich.,	Per cord.
2	bondsmen. James McCill, Ausable, Mica. No bondsmen offered in bid Carkin & Kimball, East Saginaw, Mich. No bondsmen offered in bid.	Stone, \$12 00; \$12, 5 Stone, \$17 90; slabs, \$1 40; b \$4 40. Stone, \$17,799 00; s \$340 00; brush, \$2,640 00.
4	Hasbrouck & Conro, Milwaukee; A. Goodrich, Chicago, M. B. Medbury, Milwaukee, bondsmen.	Stone, \$18 00; alabs, \$3 00; b \$3 00. Stone, \$18,846 00; s
5	H. M. Mixer, Monroe, Mich.; N. W. Brooks, Detroit, J. M. "Sterling, Monroe, bondsmen.	\$1,800 CO brush, ——. Stone, \$10 25; slabs, \$1 50; bi \$1 25. Stone, \$10,731 75; s
6	A. McDonell, Hamilton, C. W. No bondsmen offered in bid	\$900 00; brush, \$750 00. Stone, \$14 00; slabs, \$5 00; bt \$4 00. Stone, \$14.658 00; s
7	H. P. Platt, H. S. Stock, Toledo, Ohio; A. C. McNairy, Henry	\$3,000 00 ; brush, \$2,400 00.
8	M. Clafflin, bondsmen. H. M. Loud, Detroit, (whole or none.) No bondsmen offered, but good promised.	Stone, \$13 90; slabs, \$2 90; bt \$2 90. Stone, \$14,553 30; a \$1,740 00; brush,——,
9	Loud, Priest & Gay, Detroit. No bondsmen offered, but good promised.	\$1,740 OU; OFUSE,
!		

River, Lake Huron, harbor, for a depth of ten feet below ordinary low water.

	1		ng	Work or	labor classified an	d bid for.
			H .		Class A *	
Cost of class 1.	Cont of class 2.	Cost of class 3.	Total cost of all classes manu factured materials.	Dred'ing to place cribs on an even bottom in 10 feet of water, 3,962 cubic yards.	Framing cribs, boring, boling, boling, putting them in place, drying piles in their corners, and rwater work — 7,200 r. f. piles, 66 440 r. f. square timber.	Framing superstruc- ture, boring, bolting, & fastening in place, above water—22,684, r. f. square timber.
7. oak, \$1,920; f. pine, 1,200.	\$24, 851 7 0			Per cubic yd.	Per foot.	Per foot,
696 00	16, 591 46	\$4 , 180 80	\$21,379 26	\$ 0 44 1,743 28	\$9 15 11,046 00	\$0 14 3, 175 76
960 (10	21,694 91	4, 972 50	27, 627 41	1, 267 84	7,200 r.f. piles,161 c.,\$1,200; 66,440 r. f. sq. timber, 11 c., \$7,308 40.	2, 495 24
1, 188 (0	19, 821 36	4, 249 36	25, 318 72	1, 584 80	7, 364 00	2, 268 40
1,464 (4)	19, 881 36	6, 098 50	27, 443 86	55 2, 179 10 47	5,891 20	1,587 88
				1,862 14 40	12.9	9
849 00		3, 741 66	24, 355 56	1,584 80	9, 499-56	2, 245 79
••••••	19,770 90	• • • • • • • • • • • • • • • • • • • •				• • • • • • • • • • • • • • • • • • • •

at Ausable River, Lake Huron, harbor, &c.-Continued.

classified an	d bid for.			Ī	both classes	terials,
-Continued		Class B.			both	in in in in in in in in in in in in in i
Francing, fitting, and boring joint and plank; 70,552 feet b. m.	Francing, fitting, and splking scattling and bottom boards into cribs; 28,320 feet b.m.	Dredging channel be- tween plers and just above plers in river; 24,919 cubic yards.	Cost of class A.	Cost of class B.	Total cost of work or l	Total cost of all classes of materials, work, or labor.
Per M. ft.	Per M. ft.	Per yard.				
\$12 00 846 62	\$12 50 354 00	\$0 44 10, 964 36	Slabs, \$35,804 66 Brush, 37,604 66	\$10,964-36	\$46, 769 02 48, 569 02	\$68, 147 28 69, 947 28
8 00 564 41	8 00 226 56	32 7, 974 08	Slabs, 33, 708 45	7, 974 08	41, 682 53	69, 309 94
6 50 458 59	6 50 184 08	9, 22) 63	Slabs, 23, 491 62 Brush, 23, 341 62	9, 220 03	32, 711 63 32, 561 65	58, 030 37 57, 880 37
14 00 987 72	12 00 339 84	9, 967 60	Slabs, 28, 643 74 Brush, 28, 043 74	9, 967 60	38, 611 34 38, 011 34	66, 055 20 63, 455 2 0
5 00 352 76	8 00 228 56	39 9, 718 41 35 8, 721 65	30, 202 70	9, 718 41 8, 721 65	38, 924 35	63, 279 91

or labor cannot be separated in process of constructing pier.

Abstract of bids for furnishing materials and doing the work at Ausable river,

-		Ma	anufactured ma	terials bid f	or.
		Class 1.	Class 2.	Class 3—A	ll kinds
Number of bid.	Name and residences of bidder and bondsmen.	240 round piles.	All sawed timber and lumber, 1, 223, 944 feet, b. m.	Drift bolt iron, 76.315 lbs.	Spikes, 2,477 lbs.
1	Backus Bros., Aurable, Mich.; Standart Bros., Detroit, bondsmen.	Per pilc. W. oak, \$8 00; h. pine, \$5 00; w. oak, \$19 20;		Per lb.	Per lb.
2	James McGill, Ausable, Mich. No bondsmen offered in bid.	h. pine,\$12 00.			
3	Carkin & Kimball, East Saginaw, Mich. No bondsmen offered in bid.	2 90 696 00	15 90 19, 460 71	\$9 03.9 4,502 54	\$0 c 198 1
4	Hasbrouck & Conro, Milwankee, Wis. A. Goodrich, Chicago, M. B. Medbury, Milwaukee, bondsmen.	4 00 960 00	1, 125, 072 ft. \$20, \$22,501 44. 98,872 ft., \$16, \$1,581 95,	5, 342 05	247 7
5	H. M. Mixer, Monroe, Mich. N. M. Brooks,	4 95	18 00	06	0
6	Detroit, J. M. Sterling, Monroe, bondsmen. A. Mc Donell, Hamilton, C. W. No bonds-	1, 118 0 0 6 10	22, 030 99 18 00	4,578 90 08	198 1
7	men offered in bid. H. P. Platt, H. T. Stock, Toledo, Obio. A. C.	1,464 00	22, 030 99	6, 105 20	247 7
8	McNairy, Henry M. Clafflin, bondsmen. H. M. Loud, Detroit, (whole or none.) No	3 50	17 90	051	1 0
9	bondsmen offered, but good promised. Loud & Priest, Detroit. No bondsmen offered, but good promised.	840 00	21, 908 59 17 90 21, 908 59		198 1

Abstract of bids for furnishing materials and doing to

		Work
Number of bid.	Name and residence of bidders and bondsmen.	Filling cribs and superstr 2,180 cords stone; 840 co or brush.
1	Backus Bros., Ausable, Mich.; Standart Bros., Detroit, Mich.,	Per cord.
	bondsmen.	
2 3	James McGill, Ausable, Mich. No bondsmen offered in bid Carkin & Kimball, East Saginaw, Mich. No bondsmen offered in bid.	Stone, \$12 00; \$ Stone, \$17 90; #lab#, \$1 40 \$1 40. Stone, \$19,022 0
4	Hasbrouck & Conro, Milwaukee; A. Goodrich, Chicago, M. B. Medbury, Milwaukee, bondsmen.	\$1,176 00; brush. \$3,696 (Stone, \$18 00; slabs. \$3 00 \$3 00. Stone, \$39,240 00 \$2,520 00.
5	H. M. Mixer, Monroe, Mich.; N. W. Brooks, Detroit, J. M. Sterling, Monroe, bondsmen.	Stone, \$10 ; slabs, \$1 50 \$1 25. Stone, \$22.345 00 \$1,260 00; brush, \$1.050
6	A. McDonell, Hamilton, C. W. No bondsmen offered in bid	Stone, \$14 00; slabs, \$5 00 \$4 00. Stone, \$30,520 00 \$4,200 00; brush, \$1,360
7	H. P. Platt, H. S. Stock, Toledo, Ohio; A. C. McNairy, Henry M. Clafflin, bondsmen.	Wilder of Diaba, Miles
8	H. M. Loud, Detroit, (whole or none.) No bondsmen offered, but good promised.	Stone, \$13.90; slaba, \$2.90 \$2.90. Stone, \$30.302.00 \$2.436.00; brush, \$2.436
9	Loud & Priest, Detroit. No bondsmen offered, but good promised	44,100 00 , Di ueu, 44,100

^{*} All below must necessarily be classed together, a

I certify this to be a true abstract of the original bids.

9 Lond & Priest, Detroit. No bondsmen offered, but good promised ...

Huron, harbor, for a depth of 12 ft. below ordinary low water, crib-plan No. 2.

			ufac-	Worl	k classified and bi	d for.
			e men		Class A.*	
Cost of class 1.	Cost of class 2.	Cost of class 3.	Total cost of all classes manufac- tured materials.	Dred'ng to place cribs on an even bottom in 12 ft. water, 6,480 cubic yards.	Framing cribs, boring, boling, putting them in place, driving piles in their cours: under water work—7, 200 r. f. piles, 76, 502 r. f. square timber.	Framing superstruc- ture, boring, bolting. & fastening in place abovo water, 22, 684 r. f. square timber.
. oak, \$19 20 nc 12 00				Per cubic yd.	Per foot.	Per foot.
696 00 960 00	\$19,460 71 24,083 39	\$4, 709 10 5, 599 35	\$24, 865 81 30, 642 74	\$0 44 2, 851 20 32 2, 073 60	\$0 15 12, 484 80 7,200 ft., piles, 16‡ c., \$1,200, 76,032 ft.sq.timber, 11c., \$3,363 52.	\$0 14 3, 175 76 11 2, 495 24
1, 189 00 1, 464 00	22, 030 99 22, 030 99	4, 786 66 6, 364 90	28, 005 65 29, 859 89	2, 592 00 55 3, 564 00 47	10, 128 00 6, 658 56	2, 268 40 07 1, 587 88
840 00	21, 908 59	4, 214 29	26, 962 88	3, 045 60 40 2, 592 00	12. 9 10, 736 91	9. 2, 245 72

at Ausable River, Lake Huron, harbor, &c .- Continued.

classified and	bid for.				both	ي ڇ
-Continued.		Class B.				classes or labor.
Framing, Atting, and boring Joins and plank; 70,532 feet b, m.	Framing, fitting and spiking scantling and bottom boards into cribs; 28,320 feet b. m.	Dredging channel be- tween piers & just above piers in river; 42,300 enble yards.	Cost of class A.	Cost of class B.	Total cost of work or	Total cost of all cla
Per M. fl.	Per M. ft.	Per yard.				
\$12 00 846 6 2	\$12 50 354 00	\$0 44 18, 612 00	Slabs, \$59, 910 Brush, 62, 430		\$78, 522 38 . 81, 042 38	\$103, 388 19 105, 908 19
8 CO 564 41	226 56	32 13, 536 00	56, 683	33 13, 536 00	70, 219 33	100, 862 07
6 50 458 59	6 50 184 08	37 15, 651 00	Slabs. 39, 236 Brush, 39, 026		54, 887 07 54, 677 07	82, 892 72 82, 682 72
997 73	12 00 339 84	16, 920 00	Slabs, 47, 858 Brush, 47, 018		64, 778 01 63, 938 01	94, 637 90 93, 797 90
5 00 352 76	8 00 226 56	16, 497 00 35 14, 805 00	48, 891	95 14,805 00	. 16, 497 00 63, 696 95	90, 659 83

or labor cannot be separated in process of constructing pier.

T. J. CRAM, Col. Engineers, Rot. Maj. Gen.

B 2.

United States Engineer Office, June 15, 18

3

GENERAL: Since my letter of June 11 with abstract of bids Ausable h was forwarded, recommending awards to Carkin & Kimball (bid 3) for cl. 2, 3; to H. M. Mixer (bid 5) for class A, and to Hasbrouck & Conro (befor class B, trouble arises and comes from a practical attempt to carry out construction of the law which supposes we may award to a contractor who been the lowest bidder for each class of material or labor. Carkin & Ki (3) consent to contract for class of material or labor. Carkin & Ki (3) consent to contract for class B, being the lowest for each; Hasb & Conro (4) consent to contract for class B, being the lowest. But Mixe 5) declines to contract for class A, though he was the lowest bidder, or ground that he bid for the whole or none. I have called upon the next bidder, Alex. McDonell & Co. (No. 6) for class A; they say, "Nay, we be the whole or none." I have called on the next lowest bidder for class A, Loud, (No. 8;) he says, "I bid for the whole or none," and declines taking I have, therefore, upon this system of class contracting, no alternative call on the next lowest for A, Hasbrouck & Conro, (bid 4.) If I do ce

call on the next lowest for A, Hasbrouck & Conro, (bid 4.) If I do c them I have no doubt they will accept. You will see they bid for the Now let us figure up:

Carkin & Kimball get classes 1, 2, 3, costing the United States	\$21,
Hasbrouck & Conro get class B, costing the United States	
Hasbrouck & Conro (if we give it to them) class B, costing the	
United States	

1 of all cost of the work by class contract system	. 03, 0
	===
Now let us glance at the abstract.	

Mixer's bid for the whole straight through (No. 5) is lowest	57.
H. M. Loud's bid for the whole straight through (No. 9) is next lowest	63,
McDonell & Co.'s bid for the whole straight through (No. 6) is next	•
lowest	65,

It is therefore seen by the class system that the work will actually cost by \$5,180 42, than it would if we gave the contract to Mixer, who is the lefor the entirety. In other words, while we are surving by the class system picking here and there from the bids, to save from the bidders, we are absorbing the government in the present case of \$5,180 42, which is more ten per centum of the appropriation. Suppose by some time next week coax McDonald (the excellent Scotch mechanic) to consent to take A. coaxing little becomes the dignity of the government, when there are so good contractors who promptly come forward and bid upon the common system for the entirety. If he consent, however—

Carkin & Kimball get classes 1, 2, 3	\$ 21,	3
Hasbrouck & Conro get class B	7,	9,
McDonald (if he can be coaxed) gets A	28,	0

T	otal .	• .	· • •	• •	 ٠.	 	 	 • •	 	٠.	 ٠.	• •	• •		 • •	 57	, 3	٩
																		-
																		-

This is lower than Mixer's lowest bid by \$484 29. We should have contractors, fifteen contracts to be written out, six bonds, nine bondsmen. when Carkin & Kimball deliver classes 1, 2, 3, they will claim to be paided quit. Who then takes the risk and pays the storage and custodians these materials until they are put into the work? These are items of expensions of the storage and custodians.

and there will be extra expense besides on account of inspection, all far more than \$484 29. But I have no expectation McDonald will accept of A

Let us now consider the legality of awarding the entirety to Mixer, (bid No.

5,) who is the lowest responsible bidder for the whole work.

This harbor appropriation comes under the act approved March 2, 1867. In section two it reads: "Provided that no contract shall be made except after public advertisement for proposals, in such form and manner as to secure general notice thereof, and the same shall only be made with the lowest responsible bidder therefor." Now I admit there is some ambiguity as to what substantive noun the compound pronoun "therefor" refers to. If we go back, however, in the section, we shall find that it undoubtedly meant to refer to that part of the preceding proviso where it says, "and the money appropriated by this act shall be so applied as to complete or make the nearest approximation to completing the work for which each specific appropriation is made." I believe that "therefor" refers particularly to the work, and generally to the whole substantives in the sentence quoted; and the word work in the present application means the entire harbor at Ausable. Look at section two, act approved 23d June, 1866, and it will be seen the same interpretation is meant in this respect for the second sections of both acts.

Now it is certain that Mixer's bid for the work (the entire harbor) if accepted will enable us to reach further towards completing the work than we could by awarding to Carkin & Kimball and to Hasbrouck & Conro, by the sum of \$5,180 42. It seems to me the law will only be fully complied with by contracting with Mixer for the whole, instead of awarding to Carkin & Kimball classes 1, 2, 3, and to Hasbrouck & Conro classes A and B.

Section 3 says: "There shall be separate proposals and separate contracts for each work;" also, "separate contracts for each class of material or labor." In my judgment the latter clause does not mean there shall be separate contractors for each class, but that in making out the instrument or writing denominated articles of agreement, there shall be specified or contracted for in the writing the classes of material or labor separately, which is tantamount to separate contracts for each class of material or labor.

I believe, however, that separate contractors might be contracted with for classes, provided their joint sums or bids should be less than the lowest for the whole work. As would be the case if Mixer would accept A. allowing Carkin & Kimball to have 1, 2, 3, and Hasbrouck & Conro B, for then the work would cost \$52,693 96, less by \$5,186 41 than Mixer's bid. Since Mixer and McDonald & Co. decline the awards for A, and under the interpretation of the law, as I view it, having now examined it in all its bearings upon these questions, I feel it my duty as a faithful officer to withdraw my recommendations of awards in my letter of June 11, and now to recommend that the whole be awarded to H. M. Mixer.

I do not see how else we can fully comply with the law, and at the same time do justice to the bidders.

I have the honor to be, very respectfully, your obedient servant,

T. J. CRAM, Colonel Engineers, Brevet Major General.

Major General A A. HUMPHREYS, Chief of Engineers, U. S. Army.

В 3.

ENGINEER DEPARTMENT,

Washington, May 16, 18

SIR: The enclosed letter of the 7th instant from Brevet Major General Cram, United States army, colonel of engineers in charge of the improve of the St. Clair flats, Michigan, is respectfully submitted, with an abstract the bids received in accordance with the printed copy of the advertisement pended thereto, inviting proposals for furnishing materials and for executive work, including dredging of the channel.

General Cram recommends that the contract be awarded to John Brow

Thorold, Canada West, for the execution of the entire work.

It will be observed by reference to the abstract that the lowest bidde

materials and labor are respectively as follows:

John Brown, Thorold, Canada West.—For piles, sawed and hewn ti and for framing and putting in all sawed and hewn timber embraced in one to fourteen, inclusive, of "materials required" and item four of "work done" under the advertisement.

Detroit Bridge and Iron Works Company, Petroit.—For one inch roun nut and screw and two washer bolts, items fifteen and sixteen of advertise

H. M. Mixer, Monroe, Michigan.—For bar-iron for drift bolts, items atteen, eighteen, and nineteen of advertisement.

Buhl, Ducharme & Co., Detroit, Michigan.—For spikes, items twent

twenty-one of advertisement.

George P. Sanborn, Milwaukee, Wisconsin.—For dredging channel, ite of "work to be done."

R. A. Conolly, Chicago, Illinois.—For preparing and driving round item two of "work to be done."

tem two of "work to be done."

W. W. & E. T. Williams, Manlius, New York.—For preparing and d

sheet piles, item three of "work to be done.".

It is, therefore, recommended that the contracts for each class of material labor be awarded to the parties enumerated, they being the lowest bidders

responsible.)

It is admitted that it would be convenient and advantageous to aware contract to the lowest bidder for the entire improvement, but this course be setting aside the true intent of the law, which provides for separate confor each class of material and labor.

Again, by awarding the contracts separately it will be observed that the sagate of the lowest bids amounts to the sum of \$370,729 50. Wherea Brown's bid is \$411,627 55, an excess of nearly \$41,000 over the aggreg the lowest bids, and for the item of dredging alone his bid is in excess \$20,0

It will be observed that the proposals are for the execution of the whol ject, whereas the appropriation has been made for the execution of only on

of the project.

It is, therefore, recommended that contracts be entered into only for so of the material as may be advantageously used in executing the plan of import or as may be justified by the amount of the funds available—na \$230,000, leaving sufficient of the appropriation for contracting for work to be including dredging.

Any other course would result in procuring a large amount of material out means of putting it in place until Congress should make a further s

priation.

 ${f V}$ ery respectfully, your obedient servant,

A. A. HUMPHREYS,

Chief of Engineers, Major General Volunte

Hon. E. M. STANTON, Secretary of War.

DETROIT, May 7, 1867.

Sin: I send herewith an abstract of all the bids on a large sheet, with a printed copy of my advertisement for the materials and work for ship canal across the St. Clair flats.

Eleven contractors came forward and bid for furnishing all materials and doing all the work as per my request in printed advertisements; all these, however, bidding according to the laws requiring bids separately for each kind of material and for each kind of work. Twenty-five more bidders bid partially, some for furnishing piles, some for the timber, and others for the iron, materials, &c. Those who bid for furnishing all materials and doing all the work. I coliate from the abstract as follows:

	· - · - · - · · - · · · · · · · · · · ·
Names of bidders and residence.	Total amount of bids for all ma- terials and all work.
H. M. Mixer, Monroe, Michigan Ledlie & Corse, Chicago, Illinois. Harvey P. Platt, Toledo, Ohio Walton & Fuller, Detroit, Michigan, and Springfield, Massachusetts. John Brown, Thorold, Canada West Williams & Smith, Manlius, New York R. A. Conolly, Chicago Fox & Howard, Chicago Hasbrouck & Conro, Milwaukee Burt & Glasby, East Saginaw Michigan George P. Sanborn, Milwaukee	497, 187 40 551, 118 02 514, 252 96 411, 627 55 482, 206 87 453, 398 58 476, (63 33 463, 675 59 466, 574 14

The estimate of the probable cost, as seen in my report of December 10, 1866.

upon which Congress directed this plan to be adopted, is \$428,754.

Two of the above bids are below my estimate. The contractors making these bids are acknowledged to be the most experienced in similar work of all the others who have come in competition with them. Mr. Brown is the lowest bidder, and is represented as eminent in experience and abundant in means and every way highly responsible. He is the contractor who dredged for the Wel-He is the same who secured last season the contracts for Saginaw and St. Mary's rivers. I have no hesitation in recommending the contract for the St. Clair flats to be given to him on the above bid, and request the approval of the proper authority and instructions to forthwith make the contract with

My absence at Baltimore so long, under the orders of the War Department, has put me back some twenty days in everything.

There should be only one contractor on such a work, for all work and material.

Very respectfully, your obedient servant,

T. J. CRAM.

Colonel Corps Engineers, Brevet Major General. Major General A. A. HUMPHREYS,

Chief of Engineers.

To Contractors.

Office Lake Harbor Improvements, 111 Griswold street, Detroit, Michig

Written and sealed proposals for furnishing materials and doing the worbe received at this office, addressed to the undersigned, until the 10th of April, 1867, for constructing a straight ship canal from the mouth of Pass directly across the St. Clair flats, 300 feet wide and 13 feet deep, lowest stage water, to be diked with piles and timber on each side for its length to a height of five feet above water, and the earth dredged out to the canal to be put into the dikes and beyond them, so as to form the banks even in height with the dikes and of uniform width on top, and han outer slope going off into the lake water of two horizontal to one versor further details of construction, bidders are informed that they must person at this office, where models, drawings and specifications can be and all explanation given.

MATERIALS REQUIRED.

1. Five thousand one hundred and eighty rock elm or white oak st round piles, 12 inches at least, exclusive of bark, in diameter at the middl not less then 28 feet long.

2. Sixteen thousand four hundred sheet piles, 4 by 12, sawed, 18 feet white oak, rock elm, hard or white pine, that will drive well without spl width may vary from 10 to 12 inches, giving an equivalent amount in measure to what 12 inches in width would give for the total amount.

3. Eight hundred and sixty-four stringers, 5 by 6, sawed, 20 feet long

4. Eight hundred and sixty-four water-sills, 8 by 9, sawed, 20 feet long or white pine.

5. Eight hundred and sixty-four water sills, 6 by 12, sawed, 20 feet long or white pine.

6. Eight hundred and sixty-four binders, 4 by 6, sawed, 19 feet long or white pine.

7. Eight hundred and sixty-four fenders, S by 12, sawed, 20 feet long, oak or hard pine.

8. Eight hundred and sixty-four front caps, 12 by 12, sawed, 20 feet

heart of white oak or of hard pine.
9. Eight hundred and sixty-four back caps, 8 by 8, sawed, 19 feet long.

or white pine.

10. One thousand six hundred and forty front sidings, 10 by 12, sawe

feet long, hard or white pine.
11. One thousand eight hundred and twenty-two front sidings, 10 beawed, 18 feet long, hard or white pine.

12. One thousand six hundred and forty rear sidings, 8 by 12, sawed, 2

long, hard or white pine.

13. One thousand eight hundred and twenty-two rear sidings, 8 by 12, s.
18 feet long, hard or white pine.

14. Six thousand four hundred and ninety-five cross-ties, 8 by 10, sawe

feet long, hard or white pine or white oak.

The sawed stuff will amount to 3,684,278 feet board measure. The piles to be of best quality of timber, all sawed stuff, to be squarely and sawed to the dimensions stated, of good live timber, and to be free from a knots, splits, shakes, or other defects tending to impair its durability or street.

State the price in the bids for furnishing item 1, per pile delivered; state

price in the bids per thousand feet board measure, for furnishing all the sawed stuff delivered, the place of delivery being at the mouth of the South Pass, near "Jerry's ranch."

15. Two thousand five hundred and ninety one-inch round iron nut and screw (and two washers) bolts, 27 inches long from outside of head to point of screw.

16. Two thousand five hundred and ninety one-inch round iron nut and screw (and two washers) bolts, 25 inches long from outside of head to point of screw. Note.—These bolts may probably have to be varied from one-half to one

inch in length to suit variations in thickness of pile.

17. Six hundred and forty-eight bars one-inch square iron, 12 feet long, for drift-bolts, 25,754 pounds.

18. Six hundred and forty-eight bars one-inch square iron, 14 feet long, for

drift-bolts, 30,046 pounds.

19. Five thousand eight hundred and twenty-eight bars one-inch square iron, 16 feet long, for drift-bolts, 308,837 pounds.

20. One thousand seven hundred and twenty-four pounds ten-inch wrought

spikes, half inch in diameter.

21. Sixteen thousand nine hundred and forty-four pounds eight-inch wrought spikes, three-eighths inch in diameter.

The iron for the bolts and spikes to be of the best quality.

State the price per pound for furnishing and delivering items 15 and 16. State the price per pound for furnishing and delivering items 17, 18 and 19. State the price per pound for furnishing and delivering items 20 and 21.

The place of delivery being the same as above stated.

One sixteenth part of each class of the twenty-one foregoing items to be delivered on or before the 1st of July next, and the remainder in instalments of one sixteenth for every month thereafter during navigation until all shall have been delivered.

Whoever receives the contract will be required to furnish two responsible indorsers in a bond of indemnity to the United States to the amount of twenty per cent. of the total value of the materials contracted for.

State in the bids the names in full, and residence of the bondsmen.

WORK TO BE DONE.

1. The average depth of water all along where the canal is to be made was 6 feet 24 inches last November, and nowhere less than 34 feet, allowing scows to work with facility everywhere on and about the site.

The average thickness of the stratum or prism of earth to be excavated between the dikes is 6 feet 9½ inches; the mean lift to raise it to the surface of the

The length of the canal is to be 8,200 feet. water is 9 feet 7 inches.

Borings show earth easy of dredging and good for driving the piles; should it be found hard to drive through the upper crust, dredging will be done along where the dikes are to stand, to a sufficient depth to allow the piles to be driven to the depth of 24 feet below the surface of the water for the round and 17 feet for the sheet piles.

Earth to be dredged from between the dikes and put into the dikes and into the canal banks beyond, so as to make them into proper shape as described, 618,280 cubic yards; probable amount to be dredged to ease the driving of the piles, 35,000 cubic yards. State the price per cubic yard, measured in the natural bed or cut, for so dredging and placing the earth. Also, state the price per cubic yard measured in the dikes and banks for so dredging and placing the earth.

2. State the price for preparing and driving the round piles per pile.

3. State the price for preparing and driving the sheet piles per pile.

11 w----Vol. ii

4. State the price for framing and putting in place, according to plan specifications to be learned in this office, all the sawed timber, per runnin of timber of each size measured in the works, there being 4 by 6, 16,416 refeet; of 5 by 6, 17,280 running feet; of 8 by 8, 16,416; of 8 by 9, 17,280; or 10, 90,930; of 6 by 12, 17,280; of 8 by 12, 82,876; of 10 by 12, 65,59 12 by 12, 17,280—mill measure; in all 341,354 lineal feet.

The framing is exceedingly simple; all can be done with the adze, han and auger. For every bolt to be driven, a hole must be previously bord depth equal to the length of the bolt. A dovetail is to be cut on each end cross-ties, and corresponding notches in the embracing timbers. In the retimber work the ends of the siding timbers are to abut, not lap, except the fenders and water-sills, which are to join by halving. In each sheet-pil spikes are to be driven without splitting, which will require previous bord.

It is highly desirable that one contractor should be fortunate in putting bids so as to secure to himself the contract for furnishing all materials and all the work. The doing of all the work, however, to be under one contrabids justify. The act of Congress making the appropriation requires bid received and contracts made for each class of materials and for doing the separately, and it may happen that he who gets the materials may not geometric for the work. The lowest responsible bid secures the contract

It is pertinent to remark that from the beginning of the work at South to the completion, all the machinery and work will be, in almost all weath feetly protected, nor will any hindrance arise from passing vessels or rafts, until the new channel shall be completed, will follow the old crooked route the flats. These circumstances are of great consideration to the contract

The rule which has already been stated in reference to bondsmen and ind for materials applies to the contract for doing the work. Bidders will ticular to follow the instructions herein contained in writing out their pro Time for completing the work, on or before the spring of 1869.

T. J. CRAM,

Col. Corps Engineers, But. Maj. Gen. U. S. A., Sup't Harbor and River Improven

B 4.

Engineer Department,

Washington, May 21, 1

GENERAL: Your letter of the 17th instant, with abstract of proposals for at St. Clair flats, was submitted to the Secretary of War, with recommen from this department, which have been approved by him. You will the proceed to award the contracts to the lowest responsible bidders "for each of material and labor," as provided for in the third section of the act at March 2, 1867, upon their furnishing the requisite security for the faith formance of the same.

It will be seen by reference to the abstract of proposals, that the loweders for materials and labor are respectively as follows:

John Brown, Thorold, Canada West.—For piles, sawed and hewn and for framing and putting in all sawed and hewn timber embraced in to 14 (inclusive) of "materials required," and item 4 of "work to be done," the advertisement.

Detroit Bridge and Iron Works Company, Detroit.—For 1-inch rour nut and screw two-washer bolts, items 15 and 16, advertisement.

H. M. Mixer, Monroe, Michigan.—For bar iron for drift bolts, items and 19, advertisement.

Buhl, Ducharme & Co., Detroit, Michigan.—For spikes, items 20 and 21, advertisement.

George P. Sanborn, Milwaukee, Wisconsin.—For dredging channel, item 1 of work to be done.

R. A. Conolly, Chicago, Illinois.—For preparing and driving round piles, item 2 of work to be done.

W. W. and E. T. Williams, Manlius, New York.—For preparing and

driving sheet piles, item 3 of work to be done.

You will enter into contracts only for so much of the material as may be advantageously used in executing the plan of improvement to the extent practicable with the appropriations made for it, namely, \$230,000, leaving sufficient of that sum for the contract for work to be done, including dredging. Any other course would result in procuring a large amount of material without the means of putting it in place until Congress should make further appropriation.

Very respectfully, your obedient servant,

A. A. HUMPHREYS,

Brig. Gen. and Chief of Engineers, Maj. Gen. Volunteers.

Brevet Major Gen. T. J. CRAM, U. S. A.,

Colonel of Engineers, Detroit, Michigan.

B 5.—Abstract of bids for St. Cla

			For fu	nishing me	terials.
Number of bid.	Names of bidders,	5, 180 piles.	All sawed or hewed tim- ber -3,684,276 ft. board measure.	5,180 1-inch round nut and scrow (2 washer) bolts—37,555 lbs.	7,134 bars 1-inch square iron for drift bolts— 364,637 iba.
	H. M. Mixer, Monroe, Mich	Per pile. \$6 25	Per M. \$22 00	Per lb. \$0 091	Per lb. \$0 05
2	Joel W. Kelsey, Toledo, Ohio	4 48	•••••	•••••	
3	Loud, Priest & Gay, Ausable, Mich	6 57	25 00		
4 5	Gilbert W. Ledlie, East Saginaw Ledlie & Corse, Chigago, Ill		19 90	9. 9 12	5. 9 7‡
6	Harvey P. Platt, Toledo, Ohio	4 75	26 00	79	64
7	H. M. Loud, Detroit, Mich	6 50	Unintelligi	ble and no	
8	D. E. Rice, Detroit, Mich		28 00	8 1 121	5t -
10	Alex. McDonnell, Hamilton, C. W.	4 50		Each 1 15	61
ii	Wm. Sanborn, Port Huron Mich		24 00		
12	John Brown, Thorold, C. W	3 20	19 00	11	51
13	Brooks & Adams, Detroit, Mich		20 00		<u>-</u> ;- -
14 15	Buhl, Ducharme & Co., Detroit, Mich Detroit Bridge and Iron Works Co., Detroit, Mich.			9 71	5 1 .
16	W. W. Williams & Smith, materials, and W. W. & E. T. Williams, work, Manlius, N. Y.	4 00	24 70	11	6
17	Eugene St. Amour, Detroit, Mich				-
18 19	Elias Sims, Cleveland, Ohio	4 00	22 00	13	8
20	Fox & Howard, Chicago, Ill		20 00	124	8
21	Hasbrouck & Conro, Milwaukee, Wis		23 00	12	71
~	Hammer C. Fibbas, Datumit Mich		piles only.]
22 23	Henry C. Kibbee, Detroit, Mich	3 80 5 00	23 00	g	51
24	Burt & Glasby, East Saginaw, Mich		20 00	14	8
25	Farquhar McRae, Wallaceburg, C. W	3 88			
26	Mason, Doty & Luce, Detroit, Mich		22 40		-
27 28	John Trowbridge & Bros., Detroit, Mich		22 00 *19 00	• • • • • • • • • • • • • • • • • • • •	
29	John C. Valentine, East Saginaw, Mich L. M. Mason, Detroit, Mich		21 90		
30	Seth P. Cushman, Detroit, Mich	3 25			
31	A. A. McDonnell, Hamilton, C. W	5 60			
33	George P. Sanborn, Milwaukee, Wis		24 00 20 95	14	71
34	L. Reeve, East Saginaw, Mich E. Vonderbecke, Detroit, Mich	3 75	20 93	•••••	
35	Balphe C. Smith, Detroit, Mich				
36	Milton H. Butler, Detroit, Mich	3 45			
					- 1

^{*} All to be delivered in rafts. This condition vitiates bid.

Michigan.—Ship canal, straight cut.

· For doing work.												
yd.,	dikes yds.	dri- llos.	- L	For framing and putting in, per running foot.								
Dredging per cuble yd., measured in the cut or bed-653,280 yards.	Dredging per cubic yd., measured in the dikes or banks-653,280 yds.	For proparing and drawing the round piles.	For preparing and diving the sheet piles.	Binders, 4 in. by 6 in., 16,416 r. ft.	Stringers, 5 in. by 6 in., 17,280 r. f.	Back caps,8in. by 8in., 16,416 r. ft.	F. water sills, 8 in.by9in, 17,280 run. feet,	Cross-ties, 8 in. by 10 in., 90,930 run. feet.	B. water sills, 6in. by 13 in., 17,280 run. feet.	Fenders, b. sid'gs, 8 in. by 12 in., 82,876 run. feet.	Front siding, 10 in. by 12 in., 65, 596 run. feet.	Front caps, 12 in. by 12 in., 17,280 run. ft.
\$0 40 44	\$0 45 44	Per pile. \$3 00 5 90	Per pile. \$1 00 1 40	0 15		water, \$0 10	20 cents \$0 16	below w \$0 09	\$0.16 €	lineal foo Fenders, 16 c.r. si- ding, 10 c.	t. }\$0 11	\$ 9 11
44 48) pecifica	55) tions in	2 99 5 00 advertis	1 24 1 25 ement.	10	10 6‡	14 7‡	14 9	14 10	14 10	\$0 14 12	14 15	14 16
45	60	3 50 3 00	75 1 25	7 5	7 5	7 5	7 5	7 5	7 5	7 5	7 5	7 5
38	39	2 80	1 30	4 1	4‡	42	44	41	44	44	41	41
44	••••••	3 50	60	6	6	8	8	7	7	9	9	10
56 40 44 42	57 441	3 00 3 20 2 00 2 50 2 25	75 72 75 65	9 24 84	9 3 8 1	9 6 1 81	9 71 81	9 8 8 1	9 71 81	9 9 1 81	9 12 81	9 144 84
50 50 44		2 90 †1 40	81	6 cts.	per fo	ot abo	ve and	12 below 8	water. 8	8	8	8
					•••••						••••••	••••••
341		4 20 2 25	69	91	91	91	91	94	91	91	91	91

[†] Bids for driving not accepted on account of condition.

Abstract of bids for St. Clair

				Cost of -	,
Number of bid.	Names of bidders.	Piles-	Timber.	Washer bolts.	Drift bolts, from
1	H. M. Mixer, Monroe, Mich	\$32 375 00	\$81, 054 11	\$3,567.72	\$18 231 85
2	Joel W. Kelsey, Toledo, Ohio	23 206 40	402, 002 12		
3	Loud, Priest & Gay, Ausable, Mich	33, 670 00	92, 106, 95		
4	Gilbert W. Ledlie, East Saginaw, Michigan				
5	a Ledlie & Corse, Chicago, Ill	20,720 00	73, 317 13	4, 506 60	
6	Harvey P. Platt, Toledo, Ohio	24,605 00	95, 791 22	3, 379 95	23,701 40
7	H. M. Loud, Detroit, Michigan				
8	D. E. Rice, Detroit, Mich. b Walton & Fuller, Detroit, Mich. Alex. McDonnell, Hamilton, C. W. Wm. Sanborn, Port Huron, Mich. John Brown, Thorold, C. W.			3, 192 17	19,599 23
9 10	Alex McDennell Hemilton C. W.	31,060 00	103, 157 78	4,694 37	25, 324 59
ii	Wm Sanhorn Port Huran Mich	23, 310 00	81,004 11	3,957.00	
12	John Brown Thorold C W	16 576 00	70 001 98	4 131 05	19 143 44
13	Brooks & Adams, Detroit, Mich	20,010 00	0,001 20	2, 101 00	10,110
14	Brooks & Adams, Detroit, Mich			3, 379 95	18, 687 64
15	Detroit Bridge & Iron W'ks Co., Detroit, Mich.	l 		2.816 62	18, 687 64
16	c W. W. Williams & Smith, materials, W. W.	20,720 00	91,001 66	4, 131 05	21,878 22
	& E. T. Williams, work, Manlius, N. Y.	1		}	
17	Eugene St. Arnour, Detroit, Mich				
18 19	Elias Sims, Cleveland, Ohio	01 886 00	01 074 11	4 000 15	00 1-0 00
20	For & Howard Chicago, Ill	21,750 00	72 695 56	4,662 13	229, 170 90
20	rox at noward, Cuicago, m	20, 120 00	Piles.	2,002.07	29,010 90
21	Hasbrouck & Conro, Milwaukee, Wis	19, 425 00	27, 158, 40	4,506 60	27, 347 77
22	Henry C. Kibbee, Detroit, Mich. J. M. Jones, Detroit, Mich. Burt & Glasby, East Saginaw, Mich. Farquhar McRae, Wallaceburg, C. W.	,			
23	J. M. Jones, Detroit, Mich				
24	Burt & Glasby, East Saginaw, Mich	20, 305 60	73, 685 56	5, 257 70	29, 170 96
25	Farquhar McRae, Wallaceburg, C. W				
26	Mason, Doty & Luce, Detroit, Mich				
27	d John Trowbridge & Bros., Detroit, Mich		• • • • • • • • • • • • • • • • • • • •		• • • • • • • • • • • •
28 29	John C. Valentine, East Saginaw, Mich. L. M. Mason, Detroit, Mich.	· • • • • • • • • • • • • • • • • • • •		• • • • • • • • • •	• • • • • • • • • • • • • • • • • • • •
30	Seth P. Cushman, Detroit, Mich				• • • • • • • • • • • • • • • • • • • •
31	A A McDonnell Hemilton C W		•••••		• • • • • • • • • • • • • • • • • • • •
32	George P. Sanborn, Milwankee, Wis.	18, 130, 00	88 422 67	5 257 70	26 436 18
83	L. Reeve, East Saginaw, Mich.	20, 100 00	00, 120 01	4, 25.	20, 200 10
84	A. A. McDonnell, Hamilton, C. W. George P. Sanborn, Milwaukee, Wis. L. Reeve, East Saginaw, Mich. E. Vonderbecke, Detroit, Mich.				
35	Raipa C. Smith, Detroit, Mich				
36	Milton H. Butler, Detroit, Mich				

Bids for dredging not in accordance with advertisement. For all materials rehandled in dike extra per yard.
5 Wants explanation of measurement in embankment.
c If the earth is measured in scows, deduct 2 cents per yard, Too many conditions in bid for dredg d Will only furnish 1,000 piles. This condition vitiates bids for piles.

Michigan .- Ship canal, straight cut-Continued.

		Cost of-						
Dredging, measured in cut.	Drodging, measured in dikes. Preparing and driving round piles.		Preparing and driving sheet piles.	Framing sawed tim- ber.	Total cost of material.	Total cost of work.	Total.	
\$261, 312 00 287, 443 20	\$293, 976 00 287, 443 20	\$15, 540 00 30, 562 00	\$16, 400 00 22, 960 00	\$50, 560 00 36, 491 34	\$136 , 582 11	\$343, 812 50	\$480, 394 6L	
287, 443 20 316, 840 80		15, 488 20 25, 900 00	20, 336 00 20, 500 00		127, 478 28 149, 157 69	369, 709 12 401, 960 33	497, 187 40 551, 118 02	
293, 976 00	391, 968 00	18, 130 00	12, 300 00	23, 894 78	165, 952 18	348, 300 78	514, 252 96	
248,246 40	254, 779 20	14, 504 00	21, 320 00	16, 215 47	111, 341 68	300, 285 87	411, 627 55	
2:7, 443 20		18, 130 00	9, 840 00	27, 382 62	139, 411 05	342, 795 82	482, 206 87	
261, 312 00 29, 709 6 0	290, 709 60	10, 360 00: 12, 950 00		30, 721 86 30, 069 38				
274, 377 60		11, 655 00	10, 660 00	29, 015 09		325, 697 69		
287, 443 20		7, 252 00	13, 284 00	27, 308 32	131, 286 62		466, 574 14	
		11,655 00	11,316 00	32, 428 63	140, 113 35	280, 781 23	420, 894 58	
					•		••••••	

I certify this to be a true abstract of the original bids.

T. J. CRAM, Colonel of Engineers, Brevet Major General U. S. A.



B 6.

United States Engineer Office Detroit, June 11, 18

GENERAL: In justice to the engineer department I admit that the ab of bids sent with my letter of May 7th last recommended the award materials and work to John Brown. Bid No. 12 was made out in this off a manner which, with an omission of facts not then known, but since discommay have misled, and been the cause of the engineer department letter, 21, being sent to me, to the end of awarding contracts for the canal to sev different persons. The letter seems to take it for granted that the seven rewill take the respective contracts for the several items specified.

We shall see how far such a supposition may be realized. I have mad a corrected abstract, and classified the work and materials in accordance what I regard a proper construction of the law in reference to this part work. This abstract I desire to replace the one I before sent, and to hav returned to my office. I transmit the corrected one as my final official r herewith. As to awarding the iron to bid No. 1, (H. M. Mixer,) bid N (John Brown) is as low as Mixer's, and, besides, Mixer declines upo honest ground that he bid for the "whole or none." As to awarding the and screw washer bolts and bar iron (round) to Detroit Bridge and Works Company, (bid No. 15,) it will be seen that Brown's bid for these low as bid No. 15, and as that company expresses indifference to receiving contract, on the ground of risk and inconvenience of delivering, I see reasons for awarding it to John Brown.

As to awarding the contract for spikes to Buhl, Ducharme & Co., (bit 14.) it will be seen that Brown's bid, No. 12, is as low as Buhl, Ducharm Co.'s bid for these spikes; and as Buhl, Ducharme & Co. express indiffer to the contract, on account of risk in sending (to a place where they of government have no storehouse) kegs of the articles, there seems good reto award the contract to Brown for the spikes. The iron bolts and spikes stitute one "class" of material, and by making a contract for it with B we comply with the law in awarding this class of material to him, who is lowest bidder for all in the class.

As to awarding the contract to R. A. Conolly, (bid No. 19,) his bid wa the whole or none, so understood; besides, though he might be a respon bidder for the whole straight through, he is not a responsible contracte driving these piles, as would be required, at \$2 each, under an isolated apart from the rest of the work he bid for. Such a price is unreasonable would he take the contract for driving alone. On account of unreasonable article 1046 Army Regulations, Revised, tells us that part of the bid which: to driving the piles should be rejected; and, besides, there is another remuch dredging will have to be done to facilitate the driving. Who does if an independent contract be made for the driving alone? The truth is can't separate the dredging from the rest of the work of putting in the v work of the dikes, and afterwards securing them. This is a reason for pu the driving of the piles and all other work, including the dredging, unde class, designated Class A in the abstract. Each and every reason just assi would apply to every bid below \$5 per pile for driving, if we award for dr alone, independently of the other work to be done. Hence, we can only re Brown's bid as the lowest we can take to make a contract upon, and we can that only upon the ground of his having the award of all other work. Bro bid is the lowest—I may say, the only—bid for driving the round piles the can hope to contract for.

In regard to driving sheet-piling, precisely the same reasons, each and exapply to them as for the round piles, and more besides. These cannot

driven and spiked in a job disconnected with sawed timber-work of the dikes. No person could be induced to take a contract for doing it (unless he has the contract for the whole work) at any reasonable price. Brown's is the lowest reasonable, responsible bid that the preparing, driving, and spiking can be done for, and that, too, in connection with other work. What does the preparing mean? Brown was the only one of the contractors who examined the model and could take in all the measuring; who had the wit to discover that the sheetpiles would have to be jointed with a plane on both edges. Now, to joint an oak stick on both edges, eighteen feet long, twelve inches wide, and four inches thick, is no small job. Williams bid for the award of all he bid for, or none, (bid No. 16.) He therefore declines the contract, virtually, for driving sheetpiles, and the same is the case with every other bidder unless he can have the whole he bid for awarded to him.

We now come to the question of awarding the dredging to George P. Sanborn, Milwaukee, (bid No. 32.) In relation to this bid there is considerable to be said. Some days after sending my letter of May 7, with the abstract of bids, I discovered that this was a fraud, which had I known when I opened the bids I would have rejected in toto, and it should never have appeared on an abstract of mine. The evidence of this piece of sharp practice became conclusive some days since. Conro (who bid No. 21) drew up bid 32, signed Sanborn's name, and put it in, not for Sanborn to get the contract, but for Hasbrouck & Conro to get the dredging, provided their bid (No. 21) should fail to be the lowest. It proves that Sanborn was a "straw," used by Conro to obtain for Hasbrouck & Conro the advantage of two bids to each of their fellow bidders having one; and this was all unknown to me until since the abstract was made out and my letter of 7th May was sent. I find that Sanborn used to be a bookkeeper to Conro, and knows no more about constructions than the most inexperienced. Conro has seen me twice since, and acknowledged, when I charged him with perpetrating the trick, that he did it so as to secure to himself the dredging in case his bid (No. 21) should fail to secure the whole. There never has been a word, verbally or in writing, from Sanborn to me, claiming the contract or acknowledging the bid, or conferring any authority for Conro to use his (Sanborn's) name in a manner which, without such written authority, is tantamount to a forgery. I stamp this "straw" bid (No. 32) as a transaction worthy of a trickster. Conro, first, on seeing that Brown, for the entirety was the lowest, went and proposed to Brown to take him (Conro) in as partner on Brown's bid. Brown's reply was that "the contract had not yet been awarded, consequently he could give no definite reply to such a proposition." Conro has since been active to secure for himself the dredging on the fraudulent bid in the name of Sanborn, and the pertinacity of his acts show a desire more for a haul upon the public crib than for the interest of this important public work. I regard his use of a "straw" bidder as an unfair attempt to overreach those who put in their bids honestly, according to the understanding. Conro has had the barefacedness recently to call on me a third time, claiming that I should award the dredging to Sanborn, in order that he (Conro) might get the contract. Never presenting any authority, written or verbal, from Sanborn for anything concerning bid No. 32, or anything else. It is needless to say I have treated this claim of Conro with silence; and now, in making up my final abstract herewith sent, I mark upon the abstract a rejection of the whole bid, (No. 21,) for reasons herein stated and thereon assigned, under the head of remarks.

As to awarding the items specified in engineer department letter, May 21, to John Brown: 'The letter arrived at my office during my absence at Buffalo, and was opened under the instructions I left, for opening all official letters by my clerk. Brown called to see about the award, and the letter was shown to him. He replied: "I can't take such a contract; my bid was for the whole or none,

the same as all who bid for the entirety. Nobody can expect that such a can be carried on by several contractors. One can do the work well, acco to the general's plan; two would be in each other's way, and would ineviclash." I have not seen Mr. Brown since, but he left this message as his o reply to the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of awarding him parts of his bid instead of the whole the proposition of the whole the proposition of the whole the proposition of the whole the proposition of the whole the proposition of the proposition of the whole the proposition of the whole the proposition of the proposition of the whole the proposition of the whole the proposition of the proposition of the whole the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition of the proposition

I have now reported in full upon the proposition in engineer department of May 21, to assign the work to seven contractors, and the conclusion is that we cannot expect to accomplish the object by assigning contracts for ent "kinds" of material or different "kinds" of work, but that we could secured a highly favorable contractor for the interest of the work by assigned the contract by classifying the labor or work, and by "classifying" matericant contemplated in the law, according to my construction, and as I have do

my final abstract, after a careful reperusal of all the bids.

The law says, "no contract shall be made except after public advertise for proposals, &c., and the same shall only be made for the work, (work ing in this case ship canal, St. Clair flats,) with the lowest responsible ! therefor, upon security deemed sufficient in the judgment of the Secreta War." Now, this don't oblige contracts to be made for different kinds of or work, nor for different kinds of material. Now, in regard to section "there shall be separate proposals, and separate contracts for each work also for each class (not kind) of labor, and also for each class (not kind) of terial for each work." Now, I do not infer this to mean there shall be se contractors even for each class of labor or material. Much less do I infe it means separate contractors for each kind of labor or each kind of ma but that it does mean that in drawing up the instrument called contract shall be stated in it separate classes of labor and of material specified an tracted for, so that kinds, quantities of measure or weight, and prices, &c. be clearly understood, and thus avoiding the former system of lumping, ble to frauds, and injustice to the public interest, and whereupon many piers were so slightingly constructed. I hold this to be a sound interpret applicable to every work coming under that law generally. Now, in reg the particular work (ship canal, St. Clair flats) under consideration, the another feature of the law, which says the money shall be applied to ca out a certain plan. The plan, to be properly executed, must be done by p all work of whatsoever kind in it under one contractor; it is for this that I classify in the abstract all work to be done in one class, A. The rials are classified into three. The propriety of giving all three of these of to the same contractor, who has class A, may have not been so obvious engineer department as I will now make it. We have no storehouse, no no boom, at the place where the work is to commence. Suppose we co with three different contractors for the materials to be delivered. Each del we are bound to receive and pay. Who is, after delivery to the United S to have the care, or to store, or to guard, so much valuable property a point, where there is not a house, dock, or inhabitant within several n You must see the risk to the United States would be very great. Put a materials and work under one responsible contractor; he makes his own storehouses, shop, and boom, and relieves the government from all risk expense of custodianship for materials. I should take care to draw the tract with a clause to this end; also, with a clause to fill the condition i last paragraph but one of your May 21 letter. It will be seen by the ab that Brown (bid No. 12) is as low and lower than anybody else's bid upon one, class two, class three, and for class A; he is lower than any other be bona fide, responsible bidder. I hold that under the law he is entitled to the tract for the work, as a whole. I also hold that under the law he is entit the award for each class, not only of materials, 1, 2, 3, but likewise for the of labor, A, since the bid 32 is rejected in toto, and utterly discarded. Eve

were admitted among honest bids, Brown, under the law, would be entitled to the award, upon the ground, "and the same (contract) shall only be made for the work with the lowest responsible bidder therefor." Now as to security. Before writing my recommendation of 7th May, Brown presented satisfactory names to me, and I doubt not they will be perfectly satisfactory to the Secretary of War, as his bondsmen. He went so far as to say, "I will, if required, put in deposit in gold, in the government bank at Detroit, funds to any amount you may require, to the credit of the work, as additional security." I cannot say that Brown will, now it has become so late, accept the contract; but I think he would if awarded to him in the same good faith in which he bid. For reasons and facts herein given, I submit the matter and recommend that the award of all be made to John Brown (bidder No. 12) for the whole work, and that I be authorized to draw up the contract as soon as possible, or we shall consume another season in preliminaries to another annual detriment to the commerce to the amount of half a million of dollars, more than enough to construct the whole work.

I have the honor to be, very respectfully, your obedient servant,

T. J. CRAM,

Colonel Engineers, Brevet Major General.

P. S.—The letter to which this is in reply came while I was absent in Buffalo. I returned, and next day but two had to go to Cleveland on duty. It has been impossible to put all the facts in shape before. Every day is precious, and we should, in my opinion, get this work under contract immediately, on account of getting materials to begin with this season, and for the general interest of the work.

T. J. CRAM.

Major Gen. A. A. HUMPHREYS, Chief of Engineers, U. S. A.

B 7 .- Abstract of bids for furnishing materials

		Class	sification of	materials to	be furnish	
		Class 1.	Class 2.		Class 3, actured ire	
Number of bid.	Joel W. Kelsey, Toledo, Ohio. Loud, Priest & Gay, Ausable, Michigan. Gilbert W. Ledlie, East Saginaw, Mich. Ledlie & Corse, Chicago, Ill. Harvey P, Platt, Toledo, Ohio. H. M. Loud, Petroit, Mich. D. E. Rice, Detroit, Mich. Walton & Fuller, Detroit, Mich.	5,180 piles.	All sawed or hewed timber.	5,180 1-inch round, nut, and serew 2-washer bolts, 37,555 lbs.	7,124 bars 1-inch square iron for drift-bolts; 364,637 lbs.	
1	H. M. Mixer, Mouroe, Michigan	Pr.pile, \$6 25	Pr. M.b.m. \$22 00	Per lb. \$0 09½	Per lb.	
2	Joel W. Kelsey, Toledo, Ohio	4 48				
3	Loud Priest & Gay, Ausable, Michigan	6 50	25 00			
4				9, 9	5, 9	
5	Ledlie & Corse, Chicago, Ill		19 90	12	71	
6			26 00	9	61	
7 8	H. M. Loud, Detroit, Mich.	6 50	Unintelligi	bleandnot		
9			28 00	8± 12±	51	
10	Alexander McDonnell, Hamilton, C. W		22 00	each I 15	61	
11	Wm. Sanboru, Port Huron, Mich	5 00	24 00			
12	John Brown Thorold C W	3 20	19 00	71	5	
13	Brooks & Adams, Detroit, Mich.		20 00			
14 15	Buhl, Ducharme & Co., Detroit, Mich			9 74	5± 5±	
16	W. W. Williams & Smith, materials; and W. W. & E. T. Williams, work, Manlius, N. Y.	4 00	24 70	11	6	
17	Eugene St. Amour, Detroit, Mich	6 00				
18	Elias >ims, Cleveland, Ohio. R. A. Conolly, Chicago, Ill.	4 00	22 00	13		
20	Fox & Howard, Chicago, Ill	4 00	20 00	121	8	
~ -	Los to Homma, Chicago, Milliannian		Piles only.			
21	Hasbrouck & Conro, Milwaukee, Wis	3 75	23 00	12	71	
22	Henry C. Kibbe, Detroit, Mich	3 80	**********			
23 24	J. M. Jones, Detroit, Mich. Burt & Glasby, E. Saginaw, Mich.	5 00 3 92	23 00 20 00	9	5 1 8	
25	Farquhar McRae, Wallaceburg, C. W	3 88	20 00	14		
26	Mason, Doty & Luce, Detroit, Mich		22 40			
27	John Trowbridge & Bros., Detroit, Mich	6 16	22 00			
28	John C. Valentine, E. Saginaw, Mich		19 00			
29	L. M. Mason, Detroit, Mich.		21 90			
30	Seth P. Cushman, Detroit, Mich					
32	George P. Sanborn, Milwaukee, Wis		24 00	14	71	
33	L. Reeve, E. Saginaw, Mich		20 95			
34	E. Vanderbecke, Detroit, Mich	3 75				
35	Balphe C. Smith, Detroit, Mich	3 50		********		
36	Milton H. Butler, Detroit, Mich	3 45				

doing work at St. Clair flats, Michigan.

Classification of work to be done.

All the different kinds of work or labor here below have such dependence on each other that they must be put in one class of labor—class A.

rard, bed;	rard,	driving es.	Aing		For fr	aming an	d putti	ing in pe	r running	g foot of t	imber.	
Dredging per cuble yard, measured in the cut or bed; 633,280 yds.	Dredging per cuble yard, measured in the dikes or banks, 633,280 yds.	For preparing and druhe the round piles.	For preparing and driving the sheet piles.	Binders, 4 in. by 6 in., 16,416 r. f.	Stringers, 5 in. by 6 in., 17,280 r. f.	Back caps, 8 in, by 8 in., by 16, 116 r. f.	F. water sill, 8 in. by 9 in., 17,280 r. f.	Gross-ties, 8 in. by 10 in., 90,930 r. f.	B. water sill, 6 in. by 12 in., 17,280 r. f.	R. siding and fenders, 8 ln. by 12 ln., 82,876 r. f.	Front siding. 10 lp. by 12 in., 65,596 r. f.	Front caps, 12 in. by 12 in., 17,280 r. f.
\$0 40	\$0 45 44	Pr. pile. \$3 00 5 90	Pr. pile. \$1 00 1 40	\$0 15 a	bove \$0 06	water a \$0 10	nd 20 \$016	c. under \$0 09	\$0 16	er lineal Fenders, 16c., r. si- ding, 10c.	foot. }\$011	\$ 0 11
44 48} with spe	55 1	2 99 5 00 ns in ad	1 24 1 25 vertisem	10 6 ent.	10 6 1	14 7‡	14 9	14 10‡	14 10	14 12	14 15	14 16
45	60	3 50 3 00	75 1 25	7 5	7 5	7 5	7 5	7 5	7 5	7 5	7 5	7 5
38	39	2 80	1 30	41	41	41	42	41	41	42	41	42
44	 	3 50	60	6	6	8	8	7	7	9	9	10
56 49 44}	57 44½	3 00 3 20 2 00 2 50	75 72 75	9 21	9 3	9 6 1	9 7‡	9 8	9 71	8 1 9	9 12	9 14‡
42 50		2 25	65	81	81	81	81	81	84	81	81	81
50 44	· · · · · · · · · · · · · · · · · · ·	2 90 1 40	81	8	8 6 p	er foot 8	above 8	and 12 8	per foot	below w	ater. 8	8
	· · · · · · · · · · · · · · · · · · ·											
	· · · · · · · · · · · · · · · · · · ·	4 20 2 25	69		91	94		94	91	94	91	94
J45 !		2 20	09	91	91	91	91	94	94	94	94	:

Abstract of bids for furnishing materia

-	1		1			_
				C	Cost of class	3.
Number of bid.	Names of bidders.	Cost of class 1-piles,	Cost of class 2—timber.	Washer bolts,	Drift bolt from.	
1	a H. M. Mixer, Monroe, Michigan	\$32, 375 00	\$81,054 11	\$3, 567 72	\$18,231 85	20
2	Joel W. Keisey, Toledo, Ohio	23, 206 40				
3	Loud. Priest and Gay, Ausable, Mich	33, 670 00	92, 106 95	• • • • • • • • • • • • • • • • • • • •	 	ļ
4 5	Gilbert W. Ledlie, East Saginaw	00 700 00	22 212 12	4 FOC CO		1:
6	b Ledlie & Corse, Chicago, Ill	20,720 06	73, 317 13 95, 791 22	3 370 05	27, 347 77 23, 701 40	11
7	H. M. Loud, Detroit, Mich	21,000 00	50, 151 22	3, 319 90	20, 701 40	1 "
8	D. E. Rice. Detroit. Mich.			3. 192 17	19, 599 23	1
9	D. E. Rice, Detroit, Mich	31,080 00	103, 159 78	4, 695 37		1
10	Alexander McDonnell, Hamilton, C. W	1				
11	Wm. Sanborn, Port Huron, Michd John Brown, Thorold, C. W					١.,
12	d John Brown, Thorold, C. W	16,576 00	70,001 28	2,816 62	18, 231 85	լ է
13	Brooks & Adams, Detroit, Mich		•••••			·:
14 15	e Buhl, Ducharme & Co., Detroit, Mich			3, 379 95		1,
16	f Detroit Bridge and Iron Works	90 200 00	01 001 66	4 121 05	18, 687 64 21, 878 20	i,
10	W. & E. T. Williams, work; Manlius, N. Y.	20, 120 00	32,002 00	4, 101 00	21,010 20	•
17	Eugene St. Amour. Detroit. Mich.					l
18	Elias Sims, Cleveland, Ohio				•••••••	
19	k R. A. Conolly, Chicago, Ill	21,756 00	81,054 11	4, 882 15	29, 170 96	2
20	Elias Sims, Cleveland, Ohio	20,720 00	73, 685 56 Piles,	4, 694 37	29, 170 96	1,
21	j Hasbruck & Conro, Milwaukee, Wis	19 495 00	97, 158 40	4,506,60	27, 347 77	l ı.
22	Hanny C Kibba Datroit Mich	1	- 1			1 1
23	J. M. Jones, Detroit, Mich. & Burt & Glasby, E. Saginaw, Mich. Farquhar McRae, Wallaceburg, C. W. Mason, Doty & Luce, Detroit, Mich.				• • • • • • • • • • • • • • • • • • • •	
24	k Burt & Glasby, E. Saginaw, Mich	20, 305 60	73, 685 56	5, 257 70	29, 170 96	1,
25 26	Farquhar McRae, Wallaceburg, C. W				• • • • • • • • • • • • •	
26	Mason, Doty & Luce, Detroit, Mich				• • • • • • • • • • • • • • • • • • • •	
27	John Trowbridge & Bros., Detroit, Mich	• • • • • • • • • • • • • • • • • • • •		••••••	• • • • • • • • • • • •	-•-
28 29	l John Trowbridge & Bros., Detroit, Mich John C. Valentine, E. Saginaw, Mich L. M. Mason, Detroit, Mich	•••••		•••••	•••••	• • •
30						
31	A. A. McDonnell Hamilton C. W.					
35	a George P. Sanborn, Milwaukee, Wis	18, 130 00	88, 422 67	5, 257 70	26, 436 18	i.
33	L. Reeve, E. Saginaw, Mich					
34	E. Vanderbecke, Detroit, Mich					
35	Balphe C. Smith, Detroit, Mich				• • • • • • • • • • • • • • • • • • • •	
36	A. A. McDonnell, Hamilton, C. W. Scierge P. Sanborn, Milwankee, Wis. L. Reeve, E. Saginaw, Mich. E. Vanderbecke, Detroit, Mich. Balphe C. Smith, Detroit, Mich. Milton H. Butjer, Detroit, Mich.		· · · · · · · ·		• • • • • • • • • • • • • • • • • • • •	• • •
			<u></u>		!	

I certify this paper to be a correct abstract of the original bids.

a Bids for the whole or none; declines the award for iron. b All material rehandled in dikes, 15 cents extra per yard. Bid for dredging not in accordance with c Wants explanation of measurement in embankment. Explanation never been given. Bid for each of On examination of the original bid, it is discovered that a note accompanied the bid to the effect the prices in his bid should be reduced as low as any other bidder. Hence the corrections in class 3. separately.

a Declines the award on account of risk in sending kegs of spikes to a place where it would not be a Declines the award of boits for the same reasons as above given by Buhl, Ducharme & Co.

If the earth is measured in scowa, deduct \$2 per cubic yard. Bid for the whole or none. Too many a Bid for whole or none. Piles separate from the other work could not be driven for this price. Bid for whole or none.

A Bid for whole or none. Piles separate from the other work could not be driven for this price. Bid is for the whole or none; so understood at engineer's office, Detroit.

Bid for the whole or none; so understood at engineer's office, Detroit.

Bid for driving not accepted on account of containing a condition. Bid for the whole or none. To will only furnish 1,000 piles. This condition vitiates the bid for piles.

All to be delivered in rafts. This condition vitiates the bid.

In my first draft of abstract, I supposed this to be an honest bid from Sanborn himself, like other b of May 7, to the engineer department, I discovered that it was a "shyster" bid. Sanborn was a 21, (Hasbrouck & Conro.) I now reject the whole of this bid, No. 32, as a bid none but a trickster woo nying this abstract to the engineer department, June 11, 1867. I regret that such a bid should have did a correct abstract of the orderinal bids.

doing work at St. Clair flats, Michigan-Continued.

		Cos	t of class A	۷.				i i
Total cost of class 3.	Dredging, measured in cut.	Dredging, meanured in dike.	Preparing and driving round piles.	Preparing and driving sheet piles.	Framing sawed tim- ber.	Total cost of material.	Total cost of work.	Complete cost of the canal.
\$23, 153 00	\$261, 312 06 287, 443 20	\$293, 976 00 287, 443 20		\$16, 400 00 22, 960 00	\$50, 560 80 36, 491 34	\$136, 582 11	\$343 , 812 5 0	\$480, 394 61
33, 441 15 28, 761 47	287, 443 20 316, 840 80		15, 488 20 25, 900 00	20, 336 20 20, 500 00	46, 441 72 38, 719 53	127, 478 28 149, 157 69	369, 709 12 401, 960 33	497, 187 40 551, 118 02
31, 712 40	293, 976 00	391,968 00	18, 130 00	12, 300 00	23, 894 78	165, 952 18	348, 300 78	514, 252 96
22, 355 23 23, 374 35	,,	254, 779 20	14, 504 00	21, 320 00	16, 215 47	108, 932 51	300, 285 87	409, 218 38
27, 689 39	287, 443 20		18, 130 00	9, 840 00	27, 382 62	139, 411 05	342, 795 82	482, 206 87
36, 386 61 35, 638 79	261, 312 00 290, 709 60	290, 709 60	10, 360 00 12, 950 00	11, 808 00 12, 300 00		139, 196 72 130, 044 35	314, 201 86 346, 028 98	453, 398 58 476, 063 33
33, 814 51	274,377 60		11,655 00	10, 660 00			325, 697 69	
36, 295 46				13, 284 00		131, 286 62	335, 287 52	466, 574 14
••••••								
33, 560 68	225, 381 60		11,655 00	11, 316 00	32, 428 63	140, 113 35	290, 781 23	490, 894 58
••••••	••••••••••			•••••	•••••			••••••

tiement. But the bid is for the whole or none.

bone.

he was not posted with the prices of iron and bolts and spikes in the States, he requested that for these articles the lowest responsible accepted bid for the work, whether regarded as to be awarded in aggregate or by classes

keep them until taken off their hands by the government.

ditions in bid for dredging. No definite conclusion can be drawn except that the bid is higher than others, driving unreasonable.

must be banded. This price, \$1 40, conditional upon their not requiring to be banded with iron.

for the whole or none, and so received it. After sending off that preliminary abstract, accompanying letter 'straw' in the matter. The bid had been made out and signed with Sanborn's name by Conro, who bid No. off on the government. The whole chain of facts connected with it are fully set forth in my letter accompanal abstract of mine. The bid, however, was understood at my office to have been for the whole or none.

T. J. CRAM, Colonel of Engineers, Brevet Major General.

B 8.

MILWAUKEE, June 19, 18

SIR: A letter or appeal to you, bearing date the ninth instant, a co which is annexed, was forwarded to you through General T. J. Cram. I reference to letting of the government work upon the St. Clair flats, and explain itself.

I now beg leave to enclose you, in connection therewith, my views, as a by the advice of counsel, in answer to what I understand to be General C views in the premises, and which I have reduced to writing, and also and

hereto.

With great respect, your obedient servant,

GEORGE P. SANBOR

Major General A. A. Humphreys, Chief of Engineers, U. S. Army, Washington, D. C.

An act of Congress of the United States entitled "An act making approtion for the repair, prescription, and completion of certain public works be fore commenced under authority of law, and for other purposes," appropriation in the words following, to wit:

"For improvement of St. Clair flats in Michigan, one hundred and fifty sand dollars, to be expended in accordance with the plans and specificatio Colonel T. J. Cram, in his report of December tenth, eighteen hundred

sixty-six."

In and by the second section of this act it is provided, "That no conshall be made, except after public advertisement for proposals in such formmanner as to secure general notice thereof, and the same shall only be with the lowest responsible bilder therefor, upon security deemed sufficient

the judgment of the Secretary."

Section three of the act provides, "That whenever the Secretary of shall invite proposals for any works, or for any material or labor for any wo there shall be separate proposals and separate contracts for each work, and for each class of material, or labor fire each work, and he shall report to gress, on the first Monday of December next, all the bids with the names o bidders."

The enclosed public advertisement, inviting sealed proposals for furniseach class of materials and labor for the proposed improvement of the St.

flats, was given.

In pursuance with the said act of Congress, and the foregoing notice, I, Ge P. Sanborn, a resident of Milwaukee, Wisconsin, and a citizen of the Un States of America, submitted my written and sealed proposals for furnise ach class of materials and labor separately.

John Brown, a resident of Thorold, Canada West, and a subject of G

Britain, in like manner submitted his proposals.

That upon opening such proposals so submitted, it was ascertained that proposal or bid for the required dredging was \$22,914 80 lower than any of and that John Brown's bid was the lowest for the balance of the required wand materials; and that for the required materials and work, as an entity John Brown's was \$11,679 73 lower than my combined bids for the work a entirety.

The bids also show that by awarding the dredging to me under my therefor, and awarding to John Brown the balance of the materials and we to which he may be entitled as the lowest bidder under the act of Cong and said notice, that the cost of the entire work to the government would

\$22,864 80 less than the bid of John Brown for the required materials and

labor as an entirety.

General T. J. Cram, notwithstanding the plain and explicit language of the third section of the act of Congress referred to, and its evident intent and spirit, and of his previous explicit construction thereof in his said notice, in which he says: "The act of Congress making the appropriation requires bids to be received and contracts made for each class of materials, and for doing the work separately; and it may happen that he who gets the material may not get the contract for the work. The lowest responsible bid secures the contract."

Now he contends that John Brown, whose bid, though higher than mine for the dredging by nearly \$23,000, but lower than any other for the work as an entirety, is still entitled to the contract for the whole; notwithstanding my lower bid for the dredging, which dredging of itself constitutes a very large and important portion of the entire proposed improvement, and will amount, as shown by the lowest bid therefor, to the sum of \$225,381 60, while the entire proposed improvement, upon the lowest bid therefor, amounts, as an entirety, to but the sum of \$409,627 55.

General Cram's expressed reasons for his "recent construction" of this act, (I mean by "recent" as contrasted with his previous construction, in his notice under which and the act of Congress bidders were guided in becoming competitors for the contracts,) as I understand them, are certainly very peculiar and unprecedented both in theory and practice, which, in a court of justice giving a judicial construction to the act, would hardly be regarded as equitable guides in

getting at the intent of Cougress.

Many of his expressed views, as I understand them, are in fact mere matters of possible expediency on the part of the government, or rather of convenience to its officers in superintending the work when performed under one contract, than when performed, as specified in the act of Congress, under more than one contract. For instance, it is claimed that the work is an entirety, and that the dikes or piers are composed in part by combination of the piles, timbers, and the earth dredged out of the channel, and that the combination of the different work to be performed in order to make a complete whole is so inseparably connected in its execution that it cannot be performed under distinct contract for its respective parts; and hence the further claim is founded on this specious argument that the dredging, the materials, and the work cannot be classed at all, and must therefore be let under one contract.

This line of argument virtually characterizes the act of Congress as an impracticable act, impossible of being carried out, and necessarily must claim that the work contemplated by it can only be executed by disregarding its positive provisions in regard to the mode of its execution. This result is unavoidable if

General Cram's expressed views are tenable.

I claim that it is apparent from the act that Congress intended that the work should not be awarded merely to jobbers in contracts, and hence required a classification of the work and materials to enable the skilled mechanics of the country, and others, to compete for and participate in the performance of public work in the departments in which they are conversant.

To the entire species of argument made against my claim to this dredging contract in question, I urge—what was undoubtedly familiar to Congress when they framed and passed the act in question—that structures and improvements of every character, whether for private or public purposes, are the result of the combination of the different classes of skilled labor in connection with the combined use of the different class of materials adapted to the design, in the adaptation of which different class of materials the respective class of employed labor is skilled. So well is this understood by the government, as well as by private citizens, that in such structures engineers or architects prepare the specifications for each class of skilled labor, and the class of material used by each. Bids are

taken separately by class, and contracts made accordingly; then, in order the combination of the different classes of labor and material may produce intended design, the contracts made with each class of contractors provide

and prescribe the duties of each.

The objections started by General Cram to the awarding the contract for dredging to me in this instance are so novel and unexpected, and, according my understanding of the act of Congress in question, are so unjust, that I aggrieved and compelled to appeal to General A. A. Humphreys, chief of eneers; and, accordingly, on the 13th day of June, 1867, I forwarded thro General Cram my appeal, a copy of which is herewith given.

I do hereby respectfully beg to submit herewith, under the supervision of counsel, my views in support of my rights under my bid for the dredging tract for the dredging to be done in the contemplated improvement of the

Clair flats.

Respectfully yours,

GEORGE P. SANBORI

MILWAUKEB, June 9, 186

DEAR SIR: On the 10th day of May I bid for the materials, labor, and dr ing for the improvement of the St. Clair flats, under and in accordance with no from T. J. Cram, inviting proposals for doing said work and furnishing said terials, a copy of which I hereby enclose. I was the lowest bidder for dredging, and I suppose I am entitled to the contract under section three of law making appropriations for the repairs, preservation, and completion of tain public works heretofore commenced under the authority of law, and for o purposes, approved March 2, 1867. I also think I am entitled to claim the dr ing under General Cram's notice inviting proposals, as he has classified the and materials to be furnished, and it will be noticed that he also stated in advertisement for proposals that the above mentioned law requires bids t received and contracts made for each class of materials, and for doing the separately. I have applied to General Cram, by an agent, since the bids been opened, and was informed by General Cram that he did not intend to s rate the work if he could help it, but to give the contract for the whole work materials to Mr. Brown, who is the lowest bidder, as an entirety. I am read give the required security for faithful performance of the contract for doing dredging, and I am ready to take any other portion of the work in addition the whole, if it should fall to me.

The best part of the season for doing such work is passing away, and desirable to get at the work as soon as possible; therefore I should like to k

at as early a day as is consistent if I am to have the contract.

I appeal this case direct to you, because, from the experience of the past weeks, I do not see any hope of General Cram's forwarding my claim upor merits under the law, as I understand it.

He insists that the contract for the entire contemplated improvement and

terials must be embraced in one contract, which view is directly contrary to understanding of the law and his notice, and as I am advised by my countries construction of the act is contrary to its plain intent and meaning.

bis construction of the act is contrary to its plain intent and meaning.

Very respectfully, your obedient servant,

GEORGE P. SANBORI

Major General A. A. Humphreys, Chief of Engineers, United States Army.

To contractors.

OFFICE LAKE HARBOR IMPROVEMENTS, No. 111 Griswold Street, Detroit, Michigan.

Written and sealed proposals for furnishing materials and doing the work will be received at this office, addressed to the undersigued, until the 10th day of April, 1867, for constructing a straight ship canal from the mouth of South Pass directly across the St. Clair flats, three hundred feet wide and thirteen feet deep below lowest stage of water, to be diked with piles and timber on each side for its whole length, to a height of five feet above water, and the earth dredged out from the canal to be put into the dikes and beyond them, so as to form the canal banks even in height with the dikes, and of uniform width on top, and having an outer slope going off into the lake water of two horizontal to one vertical. For further details of construction, bidders are informed that they must be in person at this office, where models, drawings, and specifications can be seen, and all explanation given.

MATERIALS REQUIRED.

1. Five thousand one hundred and eighty rock elm or white oak straight round piles, 12 inches at least, exclusive of bark, in diameter at the middle, and not less than 28 feet long.

2 Sixteen thousand four hundred sheet piles, 4 by 12, sawed, 18 feet long, white oak, rock elm. hard or white pine, that will drive well without splitting; widths may vary from 10 to 12 inches, giving an equivalent amount in board measure to what 12 inches in width would give for the total amount.

3. Eight hundred and sixty-four stringers, 5 by 6, sawed, 20 feet long, hard pine.

4. Eight hundred and sixty-four water-sills, 8 by 9, sawed, 20 feet long, hard or white pine.

5. Eight hundred and sixty-four water-sills, 6 by 12, sawed, 20 feet long, hard

or white pine.

6. Eight hundred and sixty-four binders, 4 by 6, sawed, 19 feet long, hard or white pine.
7. Eight hundred and sixty-four fenders, 8 by 12, sawed, 20 feet long, white

oak or hard pine.

8. Eight hundred and sixty-four front caps, 12 by 12, sawed, 20 feet long.

heart of white oak or of hard pine.

- 9 Eight hundred and sixty-four back caps, 8 by 8, sawed, 19 feet long, hard or white pine.
- 10. One thousand six hundred and forty front sidings, 10 by 12, sawed, 20 feet long, hard or white pine.
- 11. One thousand eight hundred and twenty-two front sidings, 10 by 12, sawed, 18 feet long, hard or white pine.
- 12. One thousand six hundred and forty rear sidings, 8 by 12, sawed, 20 feet long, hard or white pine.
- 13. One thousand eight hundred and twenty-two rear sidings, 8 by 12, sawed, 18 feet long, hard or white pine.
- 14. Six thousand four hundred and ninety-five cross-ties, 8 by 10, sawed, 14 feet long, hard or white pine, or white oak.

The sawed stuff will amount to 3,684,278 feet board measure.

The round piles to be of best quality of timber; all the sawed stuff to be squarely and truly sawed to the dimensions stated, of good live timber, and to be free from rotten knobs, splits, shakes, or other defects tending to impair its durability or strength.

State the price of the bids of furnishing item 1, per pile delivered.

State the price in the bids per thousand feet, board measure, for furnishin the sawed stuff delivered, the place of delivery being at the mouth of the S Pass, near "Jerry's ranch."

15. Two thousand five hundred and ninety 1-inch round iron nut and so (and two washers) bolts, 27 inches long from outside of head to point of so

16. Two thousand five hundred and ninety ditto, 25 inches long from out of head to point of screw.

Note —These bolts may probably have to be varied from one-half to one in length, to suit variations in thickness of pile.

17. Six hundred and forty-eight bars 1-inch square iron, 12 feet long, for bolts, 25,754 pounds.

18. Six hundred and forty-eight bars 1-inch square iron, 14 feet long, for bolts, 30,046 pounds.

19. Five thousand eight hundred and twenty-eight bars 1-inch square 16 feet long, for drift bolts, 308,837 pounds.

20. One thousand seven hundred and twenty-four pounds 10-inch wro spikes, one-half inch in diameter.

21. Sixteen thousand nine hundred and forty-four pounds 8-inch wrospikes, three-eighths inch in diameter.

The iron for the bolts and spikes to be of the best quality.

State the price per pound for furnishing and delivering items 15 and 16 State the price per pound for furnishing and delivering items 17, 18, and

State the price per pound for furnishing and delivering items 20 and 2 place of delivery being the same as above stated.

One sixteenth part of each class of the twenty-one foregoing items to be livered on or before the first of July next, and the remainder in instalme one sixteenth for every month thereafter during navigation until all shall been delivered.

Whoever receives the contract will be required to furnish two responsib dorsers in a bond of indemnity to the United States, to the amount of 2 cent. of the total value of the materials contracted for.

State in the bids the names in full and residence of the bondsmen.

WORK TO BE DONR.

1. The average depth of water along where the canal is to be made we feet two and a half inches last November, and nowhere less than three half feet, allowing scows to work with facility everywhere on and about the

The average thickness of the stratum or prism of earth to be excavate tween the dikes is six feet nine and a half inches; the mean lift to raise the surface of the water is nine feet seven inches. The length of the case to be \$,200 feet. Borings show earth easy of dredging, and good for driving piles. Should it be found hard to drive through the upper crust, dredging be done along where the dikes are to stand to a sufficient depth to allow the to be driven to the depth of 24 feet below the surface of the water, for the rand 17 feet for the sheet piles.

Earth to be dredged from between the dikes and put into the dikes, and the canal banks beyond, so as to make them into proper shape as desc 618,280 cubic yards. Probable amount to be dredged to ease the driving piles, 35,000 cubic yards.

State the price per cubic yard, measured in the natural bed or cut, dredging and placing the earth. Also, state the price per cubic yard, mes in the dikes and banks, for so dredging and placing the earth.

2. State the price for preparing and driving the round piles per pile.

3. State the price for preparing and driving the sheet piles per pile.

4. State the prices for framing and putting in place according to plans and specifications, to be learned in this office, all the sawed timber per running foot of timber of each size, measured in the works, there being of 4 by 6, 16,416 running feet; of 5 by 6, 17,280 running feet; of 8 by 8, 16,416; of 8 by 9, 17,280; of 8 by 10, 90,930; of 6 by 12, 17,280; of 8 by 12, 82,876; of 10 by 12, 65,596; of 12 by 12, 17,280, mill measure—in all, 341,354 lineal feet.

The framing is exceedingly simple; all can be done with the adze, hand-saw, and auger. For every bolt to be driven a hole must be previously bored to a depth equal to the length of the bolt; a dovetail is to be cut on each end of the cross-ties, and corresponding notches in the embracing timbers. In the running timber-work the ends of the siding timbers are to abut, not lap, except the caps, fenders, and water sills, which are to join by hewing. In each sheetpile three spikes are to be driven without splitting, which will require previous

It is highly desirable that one contractor should be fortunate in putting in his bids so as to secure to himself the contract for furnishing all materials and doing all the work. The doing of all the work, however, to be under one contractor, if the bids justify. The act of Congress making the appropriation requires bids to be received and contracts made for each class of materials and for doing the work separately, and it may happen that he who gets the material may not get the contract for the work. The lowest responsible bid secures the contract.

It is pertinent to remark, that from the beginning of the work at South Pass to the completion, all the machinery and work will be in almost all weather perfectly protected, nor will any hindrance arise from passing vessels or rafts, which, until the new channel shall be completed, will follow the old crooked route across the flats. These circumstances are of great consideration to the

contractor.

The rule which has already been stated in reference to bondsmen and indemnity for materials, applies to the contract for doing the work.

Bidders will be particular to follow the instructions herein contained in writing

out their proposals.

Time for completing the work, on or before the spring of 1869.

Col. Corps of Engineers, Brevet Maj. Gen. U. S. A., Superintendent of Harbor and River Improvements.

NOTE .- I claim that General Cram has made four classes of this work: First, dredging; second, driving round pile; third, driving sheet pile; fourth, framing and putting in place all sawed timber.

GEORGE P. SANBORN.

B 9.

MILWAUKER, WISCONSIN, June 27, 1867.

Sin: Referring to my letter of the 19th instant, with which I enclosed to you my letter of appeal relating to the subject of my bids for public work upon the St. Clair flats, and the accompanying documents, suggesting argument in support of my appeal, I now beg leave to enclose to you copy of a series of interrogatories which have been propounded to me by General Cram, with my answers thereto appended, and a copy of my letter to General Cram, the originals of all of which were this day forwarded to him.

I ask that these may be considered in connection with my letter of appear you, dated June 9, instant, and accompanying document

Very respectfully, your obedient servant,

GEORGE P. SANBOR

General A. A. HUMPHRBYS,

Chief of Engineers, Washington, D. C.

MILWAUKEB, June 27, 186

SIR: The euclosed document contains copy of a series of interrogatories, pounded to me by Mr. E. Cram, upon the subject of my bid for, and the let of, the government work upon, the St Clair flats.

My answer is also appended to each one in its order.

The answers are submitted with entire respect and deference.

You will observe that in my letter on appeal to General A. A. Humph dated June 9, instant, forwarded through you, I distinctly declare my willing to accept contracts for all the classes of work awarded to me.

Very respectfully, your obedient servant,

GEORGE P. SANBOR

Brevet Major General T. J. CRAM, Detroit.

June 25, 186

SIR: In compliance with instructions of General T. J. Cram, colonel University

States corps of engineers, I ask you the following questions:

Question 1. Will you withdraw your bid so far as relates to the dred and driving the round and sheet piles in the contemplated construction straight ship canal across St. Clair flats, and refuse to take an award of tracts, unless the entire work is awarded to you?

Answer. No. I adhere to my claim to have awarded to me that portion

the work for which I was the lowest bidder.

Question 2. Will you, in case you decline to answer question one affitively, make an affidavit, stating whether your bid, in response to the actisement of General T. J. Cram. colonel of engineers, published in the De Advertiser and Tribune on the 29th March, 1867, inviting sealed proposal constructing a straight ship canal across St. Clair flats, from the mouth of Seas, was made by Conro, in your name, for the purpose of enabling the Conro, of the firm of Hasbrouck & Conro, or of enabling the said Hasbrouck Conro, to get, through an award to your said bid, the job or contract from for the dredging, and for doing the other work, and for furnishing the mate one or all of these jobs, in case Hasbrouck & Conro should fail to get these their own bid; and that it was understood between Conro and yourself that would transfer to him the jobs aforesaid, one or all, in case Hasbrouck & Should fail, and in case the bid in your name should succeed, in securing

award?
Please forward the affidavit in due form, if made, without delay, to B
Major General T. J. Cram, colonel United States corps of engineers, De
Michigan.

Answer. I decline to make any statement under oath on the subject, advised of some act of Congress, or some positive regulation from proper dements, requiring the same. I am willing to state the facts upon my honor I will not volunteer an extra-judicial oath. My counsel inform me that have been unable to discover any act of Congress, and they are ignorant of department regulation, requiring such an oath.

Supposing that any statement of facts contemplated by the second intertory would be useless, as not meeting your approval unless verified by affid I have neglected to append any such, but will cheerfully do so, without oath, upon your suggestion, or under oath if required by any act of Congress or de-

partment regulation.

Question 3. In the event of your also declining to make the affidavit called for upon the points above explained in question two, will you then make an affidavit in due form, in your own terms, setting forth all the facts as you understand them, in relation to your said bid, and forward the same without delay to General Cram?

Answer. I decline making any affidavit for the reasons given in response to question two. I will make any statements of facts, in either form, as before suggested, if you will state more explicity what facts or circumstances you wish me to testify to. You say, alluding to a contemplated affidavit, "setting forth all the facts, as you understand them, in relation to your said bid." I cannot begin to conceive what range you would have me take in such statements. I had supposed that all material facts already appeared of record.

These questions are put to you in compliance with suggestions in a recent letter of instructions received by General Cram from the engineer department.

Very respectfully, yours,

E. CRAM, Clerk.

GEO. P. SANBORN, Esq., Milwaukee, Wis.

B 10.

ENGINEER DEPARTMENT, Washington, June 18, 1867.

GENERAL: I have to acknowledge the receipt, on the 17th instant, of your communication of the 11th instant, containing a newly arranged abstract of proposals for the St. Clair flats improvement, with important information added, not contained in the abstract forwarded by you on the 7th May, and in which materials are distributed into classes in a manner altogether different from the classification used in the previous abstract.

The request for the return of the first abstract cannot be complied with, since it has formed the subject of action in the engineer department and by the Secretary of War, and has become part of the official records of the War

Department and of the engineer bureau.

The classification adopted by you in the first abstract is correct, and conforms to the letter and spirit of the laws of 1866 and 1867. You there presented five

classes of materials and four classes of labor or work.

The division of the three kinds of iron required into three subjects of contract is perfectly proper. The lowest bidders may, without assigning any reason, decline to contract, but the statement that there is any greater difficulty in the way of delivering one load of iron of one sort than there is in the delivery of three loads of three different sorts is unfounded, and forms no good reason for declining the award.

The advantage resulting from one person or firm having the contracts for all the material and all the labor are well understood here, but the law overrides

all such considerations.

Congress having distinctly and unequivocally specified that the money appropriated shall be expended by contract, and by separate contracts for each class of material and for each class of labor, it remains simply to carry the law into effect.

In your communication of the 11th instant you state, "the law says no contract shall be made except after public advertisement for proposals, &c., and

the same shall only be made for the work, [work meaning in this case canal, St. Clair flats,] with the lowest responsible bidder therefor," &c.

From what source was this quotation of the law made? The words "fo work" have no existence in the copies of the late laws furnished to engineer department, or distributed from it.

The language of the law is explicit and positive. There can be no que as to its meaning; it admits of no discretion except when, from the natu the work to be done, the same cannot, in the judgment of the Secretar made the subject of contract.

The Secretary has decided the question in the case of river and harbor w They must be carried on by contract in the manner specified by law.

Further, articles of Army Regulations of 1863 are quoted by you as riding the positive and specific requirements of laws passed in 1866 and It would seem to be needless to inform you that, in such cases, the law and cannot be qualified.

Prior to presenting the subject to the Secretary of War for his revisits deemed essential to have the written refusal of the bidders to enter into tracts for such portions of material or labor for which they may be the lounless they shall have awarded to them the whole of the work of improve or none.

It is also desirable that you ascertain directly from Mr. Sanborn, of waukee, all of the facts of the case in relation to his bid for materials and the assertions of Mr. Conro being ex parte, and hence inadmissible. Mr. born's affidavit to the facts of his bid being genuine and made in good should be required.

The contract may be awarded to Mr. John Brown for the entire work, it shall appear that all the bidders lower than he is for such class of ms and labor, as specified in your first abstract, shall have declined, in writing award to them for such classes of material and labor for which they a lowest responsible bidders.

Conditions as to time in the execution of the contracts that will cause e rassment and delays in the execution of the work of improvement w carefully avoided.

Very respectfully, your obedient servant,

A. A. HUMPHREYS,

Chief of Engineers, Major Gene

Brevet Major General T. J. CRAM, U. S. A., Colonel of Engineers, Detroit, Michigan.

B 11.

United States Engineer Office,

Detroit, July 3, 18

Sir: In reply to the suggestions and instructions in the engineer departement to me of June 18, 1867, in relation to bids and awards for St.

flats, I have to report as follows:

It is assumed in the letter that I adopted a classification in my first absent to the department May 7th. This assumption is entirely contrary:

letter, spirit, and meaning of the author of that abstract.

In my final official abstract, sent with my letter June 11th, I made a fication by which I abide From the tenor of the letter one might infer that been judged as striving to get the work under contract in violation of

Nothing in my acts or correspondence can warrant such a reflection; on the contrary, that correspondence clearly evinces a determination to aid the engineer department to the end of having it to put under contract in strict accordance with law. I am asked from what source I quoted a portion of the laws. I answer, from the copy sent to me from the engineer department. Now, I see by inspection, the quotation was correct, but that I put one branch of the bracket in the wrong place. The following should have been the mode: "Provided that no contract shall be made except after public advertisement for proposals, &c., and the same [meaning contract] shall only be made with the lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder that lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder that lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor." ("Therefor" meaning the work in this lowest reapproxible bidder therefor.")

In reply to "It is deemed essential to have the written refusal of the bidders to enter into contract for such portion of material or labor for which they may be the lowest, unless they shall have awarded to them the whole of the work of improvement," this suggestion has been complied with as far as possible Sanborn (bid 32) refuses; "Hasbrouck & Conro," bid 21, could not be found by my clerk, who was sent to Milwaukee to see him and Sanborn. A week has been spent in sending him after the bidders; some were found; some be could not find; some would not say whether they would sign a refusal, but thought they must have time to think of it; wished not to say or do anything, provided by holding back they could get the whole reopened to be again advertised, &c. He discovered that this is the plan of the "ring" in which Conro is conspicuous, who is endeavoring, if he cannot get the job of dredging through Sanborn's bid, to prevent Brown from getting the award, and to have the work readvertised.

Papers No. 1, No. 2, No. 3, No. 4. No 5, attached to this report, show, however, that John Brown, bid 12, H. M. Mixer, bid 1, W. W. & E. T. Williams, bid 16, Buhl, Ducharme & Co., bid 14, and Detroit Bridge and Iron Work Company, bid 15, have declined. There are some others that have not been reached on the subject; when their refusals are received I will forward them.

I come now to that part of said engineer department letter which says: "It is desirable that you ascertain directly from Mr. Sanborn, of Milwaukee, all the facts of the case in relation to his bid for materials and labor, &c. Mr. Sanborn's affidavit to the fact of his bid 32 being genuine and made in good faith should be required."

To carry out this suggestion I sent my clerk to Milwaukee, who saw and presented to Sanborn paper X, herewith embodied:

X

STATE OF WISCONSIN, County of Milwaukee, ss:

George P. Sanborn, of the city of Milwaukee, in the State of Wisconsin, being duly sworn, deposes and says: That, in response to an advertisement of Brevet Major General T. J. Cram, colonel United States corps of engineers, published in the Detroit Advertiser and Tribune on the 29th of March, 1867, inviting sealed proposals for furnishing materials and doing the work for constructing a straight ship canal from the mouth of South Pass directly across St. Clair flats, I bid for furnishing the material and doing the work by a written proposal signed by one Conro in my own name; that the said bid was made for the purpose of enabling the said Conro, of the firm of Hasbrouck & Conro, to get, through an award to my said bid, the job or contract from me for the dredging and for the furnishing the materials and for

doing the other work—one or all of these jobs—in case Hasbrouck & Coshould fail to get these on their own bid, and that it was understood between Conro and myself, before said bid in my name was put in that I would train to him the jobs aforesaid, one or all, in case Hasbrouck & Conro's should and in case the bid in my name should succeed to secure an award.

Which paper states the facts as I have understood them and now be them to be—as told to me by Conro, and admitted to be by Conro's law (Van Dyke) on his visit with Conro to me, claiming the award for Saubor Conro's benefit. Sanborn's expression was, "I presume Conro and Get Cram understand the matter alike." Sanborn then refused to make affidavit. Then he was asked to make an affidavit that the bid was made in faith for his own benefit, as expressed in paper Y. He refused to make affit to this.

Y.

STATE OF WISCONSIN, County of Milwaukee, ss:

George P. Sanborn, of the city of Milwankee, in the State of Wisconsin. duly sworn, deposes and says: That, in response to an advertisement of B Major General T. J. Cram, colonel United States corps of engineers, publ in the Detroit Advertiser and Tribune on the 29th of March, 1867. inv sealed proposals for furnishing materials and doing the work for construct straight ship canal from the mouth of South Pass directly across St. Clair I bidf or furnishing the materials and for doing the work by a written proj signed by one Conro in my own name and by my authority, which authority given by me to the said Conro in writing previous to his, the said Conro's, ing and handing in the said bid; that said bid was genuine and made in faith and in my own interest alone, and was not made, in whole or in thereof, in the interest of the said Conro, or in the interest of Hasbrou Conio, of Milwaukee, Wisconsin, or of either of them; that I authorized said Conro to make out the said bid as to prices for materials and for before the said bid was put in; that the aforesaid bid was for the who the materials and for the whole work advertised for, or for none of materials or work, and it was so meant by me before said bid was put opened; that I have not answered this bid for the benefit of the said Con Hasbrouck & Conro since the same was put in and since it became put known, nor have I assumed the bid since it became publicly known for purpose of turning the benefit of an award of a contract to me upon it eith part or in whole of what was bid for over the said Conro or Hasbrouc Conro; and that the copy of said bid hereto annexed is, to the best of my lief, true.

The following quotation was then put to Sanborn: "Will you, then, an affidavit in due form in your own terms, setting forth all the facts, as understand them, in relation to your said bid, and forward the same wit delay to General Cram?" His answer was: "I decline making any affida

All of which is respectfully submitted to accompany my remarks upor appeal of George P. Sanborn to the Chief of Engineers, which appearance B.

Very respectfully, your obedient servant,

T. J. CRAM,

Colonel Corps Engineers, Brevet Major Genera

Major Gen. A. A. Humphrbys, Chief of Engineers. No. 1.

BAY CITY, MICHIGAN, June 25, 1867.

GENERAL CRAM: As I bid for the entirety of St. Clair flats improvement. and knowing that the successful completion of the plan, though in its parts very simple and in its whole easy to construct if all be made under one contractor, and all of us, as I understood the matter, having bid for the whole or none, and knowing from my long experience that two or more contractors on such a job would inevitably clash, I hereby decline to contract for parts of the work, unless I am awarded the whole, as being the lowest responsible bidder for the whole material and work. But I am ready at any time to accept the award of the whole upon my hid as put in with its accompanying note, if the engineer please, in relation to iron bolts and spikes.

Respectfully yours,

JOHN BROWN.

I certify the above to be a true copy of John Brown's original letter declining to contract for parts of his bids, but expressing his willingness to contract for all his bid.

> T. J. CRAM, Colonel Engineers, Brevet Major General.

No. 2.

Monror, June 14, 1867.

DEAR SIR: I desire to withdraw my proposal to furnish materials for the construction of a ship canal across the St. Clair flats. Very respectfully, yours, &c.,

H. M. MIXER.

Maj. Gen. T. J. CRAM, Detroit, Michigan.

I certify that the above is a true copy of the original.

T. J. CRAM.

Brevet Major General, Colonel Corps Engineers.

No. 3.

DETROIT, June 24, 1867.

We, the undersigned, W. W. & E. T. Williams, who bid for doing the work on St. Clair flats canal, according to items in General Uram's advertisement of 29th March, 1867, decline taking the work simply for driving sheet piles at 60 cents each, as our bid contemplated all the work or none.

W. W. & E. T. WILLIAMS.

General CRAM.

United States Engineer.

I certify that the above is a true copy of the original.

T. J. CRAM.

Brevet Major General, Colonel Corps Engineers.

No. 4.

DETROIT, June 28, 18

We, the undersigned, decline the award of contract for furnishing nuscrew and washer bolts for 9 cents per pound, also for iron bars for drift at 5½ cents per pound, also spikes for 7 cents per pound, delivered at Sout mouth, for the St. Clair ship canal, near where was "Jerry's ranch."

BUHL, DUCHARME &

General T. J. CRAM, Detroit, Michigan.

I certify that the above is a true copy of the original.

T. J. CRAM, Brevet Major General, Colonel Corps Engin

No. 5.

DETROIT, MICHIGAN, July 3, 1

DEAR SIR: We herewith withdraw proposition of April 10, 1867, to bolts, nuts, and washers for St. Clair flats ship canal improvement.

Respectfully yours,

DETROIT BRIDGE AND IRON WORK By WM. C. CALHOUN, Secre

General T. J. CRAM,
Superintendent of H. R. Improvement.

I certify that the above is a true copy of the original.

T. J. CRAM,

Brevet Major General, Colonel Corps Engis

B 12.

United States Engineer Office, July 5, 1

Sir: In reply to your letter of the 25th ultimo, transmitting to me copies of letter of appeal, June 9th, to the Chief Engineer, and of the agr to sustain the appeal (drawn out by Conro's lawyer) purporting to be from P. Sanborn, Milwaukee, claiming the award of dredging for St. Clair fle canal, I have the honor to submit the accompanying papers, marked A, cing my abstract of bids 21, 32, and 12, with copies of original bids attact the remarks thereon, and my advertisement, B, containing the letter of ap Chief of Engineers and agreements to sustain it, purporting to be from Ge Sanborn, but in reality from lawyer Van Dyke, Conro's attorney, push an award to Sanborn's bid for the benefit of Conro; C, containing my and declinatures from bidders received in accordance with suggestions gineer department letter to me 18th June, 1867, and the following remarks the reference is occasionally and necessarily made to contents of A, B.

I. As to awarding the drudging to George P. Sanborn, of Milwauk. No. 32,) see abstract herewith sent of bids 12, 21 and 32, and certified of the original bids attached.

In relation to this bid 32 I have to remark, some days after sending my of May 7th, with the abstract of bids for items not classified, to the endepartment, I discovered that this bid 32 was a fraud, which had I known

I opened the bids, I would have rejected in toto, and it should never have appeared on an abstract of mine. The evidence of this piece of sharp practice

became conclusive some days afterwards.

Conro, who in the name of Hasbrouck & Conro bid No. 21, drew up bid 32, signed Sanborn's name and put it in, not for Sanborn to get the contract, but for Hasbrouck & Conro to get the dredging, provided their bid No. 21 should fail to be the lowest. It turns out that Sanborn was a "straw" used by Conro to obtain for Hasbrouck & Conro the advantage of two bids to each of their fellow-bidders, having one which was unbeknown until since that abstract and my letter of 7th May were sent. I find that Sanborn used to be a book-keeper to Conro, and is now a broker in small business, and knows no more about constructions than the most inexperienced. On opening the bids I mistook him for another Sanborn. Conro has since twice acknowledged perpetrating the trick, and that he did it to secure to himself the dredging in case Hasbrouck & Conro's bid No. 21 should fail to secure the whole

There never has been a word, verbally or in writing, from Sanborn to me, claiming the contract or acknowledging the bid, or conferring any authority for Conro to use his (Sanborn's) name in the matter, which makes bid 32 without such written authority tantamount to a forgery. I stamp this straw bid No. 32 as a trans-

action worthy of a trickster.

Conro first, on seeing that Brown for the entirety was the lowest, went and proposed to Brown to take him (Conro) in as a partner on Brown's bid; Brown's reply was that the contract had not been awarded, consequently he could give no definite reply to such a proposition. Conro has since been active to secure for himself the dredging on the fraudulent bid in name of Sanborn, and Conro's pertinacity shows a desire more for a hand upon the public crib than for the interest of this important public work. I regard his use of a straw bidder as an unfair attempt to overreach those who put in their bids honestly.

Conro has had the barefacedness a third time to call on me, the last time with a lawyer, (the same who now makes appeal to the Chief Engineer ostensibly for Sanborn.) This lawyer is behind the scenes pushing Sanborn, the straw bidder, forward, but really in the interest of Conro, and wrote out the appeal to the Chief of Engineers in Sanborn's name, claiming that the dredging should be awarded to Sanborn, to the end that he (Conro) may get the contract through

Sanborn

It is needless to say I have treated this claim of Conro's with silence, and in making up my final abstract, which was sent to the department June 11, I marked upon the abstract a rejection of the whole bid No. 32, for reasons herein stated and thereon assigned under the head of remarks.

Sanborn has never written a word to me—made any claim upon me in writing or by any authorized agents upon the subject. He has thought proper, by his counsel, only to communicate directly with the Chief Engineer upon the claim

for the award.

The reasons and facts above stated are in my estimation sufficient for the

Secretary of War to reject the bid 32 in Sanborn's name.

But there is another reason, not before mentioned, that bid 32 for dredging should be rejected in comparison with others for the reason that in his bid for dredging he does not conform to the advertisement in an essential point. (See his original bid for dredging.) He does not propose for the dredging which is to be measured after being put in the dikes and banks, as will be seen by the copy of his bids.

The following extracts from my advertisement will prove this defect:

(a) "Earth to be dredged from between the dikes and put into the dikes and canal banks beyond, so as to make them the proper shape as described.

"State the price per cubic yard, measured in the natural bed or cut, if dredging and placing the earth. Also state the price per cubic yard, mea

in the dikes and banks, for so dredging and placing the earth"

The defect, an essential one, in the bid is apparent. On the ground of non-compliance with my adver isement I reject bid No. 32 for the dredging awarded to him upon the price for that only to be measured in the cut, price is to be paid for that part, or the whole of what in the construction I find it necessary to measure in the dikes and banks? The advertisement the plan of carrying on the work show that some may have to be measure way and some the other.

Nor has any authority been presented to me, verbally or in writing, giving the least reason to hope, much less to believe, that the persons named in h

as bondsmen will consent to sign a bond for Sanborn.

II. In relation to argument B to sustain Sanborn's appeal, Sanborn's lacopies section one of the act correctly, but in his argument he gives the to the practical interpretation of that section, not deigning a word upon it to quote it. I will endeavor to elucidate the clause which says "to be exp in accordance with the plans and specifications of Colonel T. J. Cram report of December 10, 1866." That plan is set forth in the report, and templated beginning the work at the mouth of South Pass, and carrying a wood and iron work of both dikes, the dredging to fill the dikes, and to the banks and to make the water way all simultaneously, thus pushing wise by short sections at a time through the whole line of the work, comp the whole, section by section, as we progress.

And this I submit is an essential and important feature of section one, in the of execution; and this is the proper mode of engineering the work, and practimplies one contractor for all the different kinds of labor involved in carrying oplan. To separate the labor into kinds or items, and award so as to have secontractors, would greatly embarrass the work. They would inevitably Common sense runs counter to such a mode of awards, which, if made a dent, will be the means of leading to inextricable involvements, as will be

further on

I hold that all the different kinds of labor for this should constitute one which I have characterized by A in my final abstract, forwarded June 11,

The lawyer positively perverts the second section of the act in skippi sential features and quoting what suits his client—virtually ignoring othe tions of the section. I here supply the ignored parts, which any one cafor himself by reading the section through:

(b) "And provided, That * * * the money approp by this act shall be so applied as to complete or make the nearest approtion to completing the work for which each specific appropriation is made

(c) "Provided," That no contract shall be made except after public adv ment, * * and the same shall only be made with the l responsible bidder therefor upon security," &c.

The word "same" refers to "contract" and the word "therefor" ref "the work" expressed in second proviso. This is the plain common

English of the law.

The first part, b, omitted by him, but now supplied, will settle the quagainst the lawyer, even supposing Sanborn's bid not ruled out in toto, part, for the dredging, for it will be seen in passage c that John Brown (bi 12) declines to take an award for part or parts, but will take the bid, moby his note, for \$409,218 38, or at his bid, excluding the note, \$411,627 55. The last paragraph but two of my advertisement reads:

(d) "It is highly desirable that one contractor should put in his bids so

secure to himself the contract for furnishing all materials and doing all the work. The doing of all work, however, to be under one contractor, if his bids justify."

All the bidders for the entirety responded to the first part of d, and put in their bids expecting to be awarded the whole or none; and hence it was so remarked on my final official abstract. None but Conro (bid 21) and Sanborn (bid 32) have as yet shown the meanness to repudiate that understanding. They come forward after the bids are made public to claim awards for any kind or item they bid for or the whole they bid for. Now, suppose (what I do not believe will be the case) that my ruling, rejecting bid 32 in toto or rejecting the part for dredging, be overruled by the Secretary of War, and the award for dredging made to Sanborn. In such an event we have to look to some other bid than John Brown's (bid 12) for all materials and all work other than dredging. The next lowest to Brown's bid for the materials (of those whom we have any evidence will accept partial awards) is bid No. 32—this same Sanborn's—amounting to \$140,113 35. The next lowest bid to Brown's for all work other than dredging (of those whom we have any evidence will accept partial awards) is bid No. 21 (Hasbrouck & Conro's—the same Conro who is at the bottom of the appeal) amounting to \$51,330 09. Bid 32 (Sanborn) for dredging amounts to \$225,481 60. Total cost of the work, \$416,825 04.

But John Brown, who bid for the entirety, will still hold to his bid, and accept the award of the whole, at \$409,218 38. Difference in favor of the

United States if given to Brown, \$7,606 66.

I here ask the sapient lawyer which award would fulfil the law quoted and marked b? and which award will fulfil the meaning of quotation c, requiring the contract to be made only with the lowest responsible bidder (not bidders) therefor?

Further comment is unnecessary to convince that if the dredging be awarded to Sanborn we must, as a consequence, award all the materials to him, and, as another consequence, all the work other than dredging to Hasbrouck & Conro, and thus would the letter and spirit of the law be violated; on the contrary, by awarding the whole to Brown, the letter and spirit meaning of the law would be fulfilled.

III. Further in relation to the argument of Sanborn's lawyer to sustain the appeal. He quotes section third of the act: (e) "There shall be separate proposals, and separate contracts for each work, and also for each class of material or labor for each work," and upon this goes into much special pleading to convince the Chief of Engineers that the law requires the award to be given to Sanborn. Nothing more clearly shows a brain muddled with the greediness for government contracts than this special pleading, full of speciousness and rank sophistry. conducts his argument by assuming that "separate contracts" necessarily means separate contractors, and holds that we are bound by the law to award in a manner implying a separate contractor for each kind or item of labor or material. This is one part of his foundation, which I think is no better than sand, at all events. I differ essentially from his construction, and will briefly show the fallacy of such inidea. In this work there are not less than seventeen different items or kinds of material and labor. To admit a contractor for each, therefore, implies eightyfive copies of articles of agreement, thirty-four bonds to be written out, duly signed and acknowledged, fifty-one bondsmen, and seventeen contractors. small army of contractors would be in each other's way, each claiming to do his item of work, be paid for it, and to be quit of his job without regard to the work of the other sixteen. Nothing satisfactory can be accomplished in this way, and no responsible contractor would undertake a bid under so diluted a division, in two kinds, of material or labor; such is the practical tendency of admitting his construction. I do not infer "separate contracts" to mean there shall be separate contractors for each item, or even "kind" or even "class," of material or labor.

I apprehend the meaning to be that it is one duty in drawing up articles of ment to specify therein separate classes of labor or material, and to the en kinds, quantities, (r,) by weight or measure, and prices shall be clearly designated therein; and this separation in the articles of agreement is tantamount to "secontracts," all being legally permitted in the same instrument and under or tractor. If I am decided to be lame in this point of my construction by vising authority, then (e) of the 3d section comes in direct conflict with (c) 2d section of the act. In such an ambiguous conflict, to what shall we as a precedent? I answer, to previous practice, which has been that in the articles of agreement I have made on behalf of the United States in refere the works named in the act to which sections two and three belong, in the two years, I have done precisely in accordance with my views as above and contrawise to the construction of Sanborn's lawyer; every one of articles of agreement (and several of which contain "separate contracts been approved by the proper authority.

To depart from this practice (which I hold is strictly in accordance w law) now because this lawyer has come into the arena, and make contract his sophistication of section three, and to allow ourselves to be blinded cloud of dust he has raised to the nullification of the provisions of section (which he skipped,) would, I respectfully submit, lead us into many diffi-

besides the palpable violation of the law.

Another important difference between this lawyer and myself is the assumes in his argument to sustain the appeal that "class of material "class of labor" mean in practical engineering the same as "kinds or imaterial," and "kinds or items of labor." I deny that, from the English la or the practice of the engineer any such assumption is admissible as appearance.

to the work in question.

What does "class of material" mean in this connection? It is inte by experienced engineers and contractors of constructions as meaning placed together which have something in common and a necessary praction. And what does "class of labor" mean? It means those kinds which have something in common practical connection, and which can separated without injury and risk in the execution of the work of impro-To say that "items" or "kinds" constitute classes in the meaning of is begging the whole question at issue. To award to separate contractors " or "items" is endangering the whole structure and practically nullifying of Congress, as I have already shown. The lawyer in his argument ins that in "said notice" (meaning my advertisement) I classified material an A similar idea is also held out in the engineer department letter to me of 18, that I classified labor and material in my abstract sent May 7. I here fully but emphatically deny having made a classification either in my ad ment or in that abstract. I did, however, make a classification in my final abstract sent with my letter of June 11, just as shown on the abstract A with sent. I hold that this shows the proper practical engineering classi in connection with the work in question. In my letter before to the e department upon "classes" and "kinds" or "items" I have said enough t without repetition here, that such as I have made should be the classific we classify at all.

Whose business is it to classify? Congress has not classified the labor terial. The engineer department has not, nor has the Secretary of W it the province or right of a particular bidder (who has never been a conseeking an award upon a "shyster" bid to make the classification?

I submit that the engineer in charge can with propriety, until instructional higher authority to the contrary, classify legally not only labor but materiac cordance with this right I classified as shown on my abstract of June 11 this right we must hold if we expect successfully to prosecute the work.

I have denied classifying in my advertisement. I only numbered items in it with the numerals 1, 2, 3, 4, &c. To call this a classification is an assumption not warranted, and only resorted to by a special pleader, to raise a mist behind which to conceal the weakness of his cause.

The numbering of the items in the advertisement was to guide the bidders in the manner of putting down their prices as far as possible in a uniform manner, and to facilitate computations of the bids in my office, and nothing else, as had been the practice for other works. If the numbering in the advertisement is to be wrested into a classification and used as a pretext by the lawyer for subserving the particular interests of the man who put in spurious bid 32, what is to become of the rights of the other 35 bidders? Would they not have a right in equity to a reconsideration of the whole matter? And this would lead to the blotting ontall we have done and pricking anew by readvertising, and thus another retardation of the work for another season, to the money detriment to commerce of another half million of dollars.

The interpretation which in the foregoing remark I have given to the sections 1, 2, 3, are the same I had the honor to present to the engineer department in my letters of June 1st and 3d, in relation to Erie and Grand River harbors, and of June 11th, in respect to St. Clair flats; and it was upon the same interpretation of sections 1, 2, 3, all considered fairly in their relations to each other, that I thought it my duty to recommend, as I did in my letter of May 7th, the award of the entirety of the St. Clair flats work to John Brown, who was and is the lowest responsible bidder therefor.

Since perplexing questions, however, (in respect to Erie, Grand River, St. Clair flats and Ausable,) have been turned back upon me, I have thought it advisable to consult the highest authority within my district for an interpreta-

tion of the law.

I have obtained the opinion, and must respectfully insist upon its forming a part of this paper in answer to Mr. Sanborn's appeal.

The opinion speaks for itself, and here follows:

practical questions which present themselves.

United States District Attorney's Office, Eastern District of Michigan, Detroit, June 26, 1867.

GENERAL: I have to acknowledge your communication of June 25th, requesting my opinion as to the construction of the act of Congress approved March 2, 1867, entitled "An act making appropriations for the repair, preservation, and completion of certain public works heretofore commenced under the authority of law, and for other purposes," Sess. L, 1867, p. 418, with reference to certain

It is a cardinal rule that in the construction of statutes, each section is to be

interpreted with reference to the other sections and the law as a whole.

I first observe that the appropriation for St. Clair flats improvement is to be expended in accordance with your plans and specification by section 1, and that by section 2 the money voted is to be made to go as far as possible towards finishing the work, and that by the same section the honorable Secretary of War is to expend the money by contract, (except in cases of examinations and surveys.) In regard to the aforementioned matters the law is clear.

Section 2 further provides that the work shall be let to the lowest responsible bidder therefor. This means clearly, in my opinion, to the lowest single individual, partnership, or company who shall put in a bid for the entire public improvement; the word "therefor" signifying the public work advertised to be

contracted for.

In the light of this latter portion of section 2, the language of section 3, were it otherwise ambiguous, (which I think it is not,) becomes plain.

13 w----Vol. ii

There shall be separate proposals and contracts for each work and BACH OF MATERIAL OR LABOR for each work.

I think this is by no means to be interpreted as requiring a separat vidual contractor for each class. It evidently means that each single shall classify and itemize his bid.

This provision is made, undoubtedly, for two reasons: first, because the is to be done according to certain plans; and second, to prevent the

sometimes resulting from a lumping bid.

Any other construction than that which I give I apprehend would the appropriation; for it would be impossible, in the first place, to find who would contract for portions of the work; and secondly, if the former tion is met, such persons would inevitably disagree and clash, so as to or destroy the work.

But I regard sec. 2 as IMPERATIVE in requiring that the whole work

be let to ONE contractor.

Permit me to express the opinion that the early letting and completion work is very highly desirable for the interest of our commerce.

Very respectfully, your obedient servant,

ALFRED RUSSELL. United States District Atto

Brevet Major General T. J. CRAM, Coloncl of Engineers, Detroit.

I certify that the foregoing is a true copy of the original letter in ever point, single line and double line of underscoring.

T. J. CRAM,

Brevet Major General, Colonel Engis

Since receiving this official opinion, in an interview with Hon. J. M. H one of our most eminent lawyers, and who helped to make the law, h precisely the same interpretation, corresponding with my own previous

pretation, also with Mr. Russell.

I remark upon the fling at Brown in B. Conro's lawyer's argument with Sanborn's name, because of Brown's residing in Canada, that t manly shaft is thrust with an ill grace, since "Hasbrouck & Conro" has their residence for many months, if not years, and one still lives, in O Brown was for three years the able, faithful, and efficient contractor for the States at Fort Niagara. Because he hails from Canada now, I required present bondsmen who should be citizens of the United States, with w promptly complied.

There is much besides what I have remarked upon in this paper in the of appeal and the argument B full of gammon, to which I have made n notes, which may be seen by any one wishing to follow out his argumen

To sum up, we are forced to the following conclusion:

1. That Sanborn was not a bona fide bidder, as will be proven by have heretofore reported, and in his refusal to make affidavits or to gi satisfactory answer in relation to bid 32. (See my report C.)

2. That I did my duty in rejecting bid 32 in toto, (as soon as I found deceit,) which Sanborn has (subsequently to the opening of the bids) be forward to claim an award upon, for the benefit of Conro.

3. Whether the bid be allowed to stand rejected in toto at the War I

ment or not, the part for the dredging must be rejected.

4. That therefore Brown's bid 12 is the lowest responsible bid for the and even for the item of dredging.

5. That Brown is entitled to the awards for the entirety on bid 12 (w

all or the dredging alone of bid 32 be rejected or not rejected by the Secretary of War) by all that is fair, and by the complete and full opinion of the United States district attorney.

In conclusion, I have to request that this paper, with the accompanying papers, may be laid before the honorable Secretary of War as early as possible, for a decision of the question as to whom the contract or contracts shall be awarded.

I have the honor to be, very respectfully, your obedient servant, T. J. CRAM.

Colonel of Engineers, Brevet Major General.

Major General A. A. Humphreys, Chief of Engineers, U. S. A.

B 13.—Abstract of bids for furnishing mater

		Classi	fication of	materials t	o be far	
		Class 1.	Class 2.	Class 3. Manufactured		
Number of bid.	Name of bidder.	5,180 piles.	All sawed or hewed timber and lumber.	5,190 1 inch round, nut, and screw 2-washer bolts, 37,555 pounds.	7,124 bars 1-inch sq. fron for drift bolts; 364,637 pounds.	
21	Hasbrouck & Conro, Milwaukee, Wis	Pr. pile.	Pr. M. b.m. Piles only. \$23 00	Per. lb.	Per lb	
32	George P. Sanborn, Milwaukee, Wis	3 50	\$23 00 24 00	\$0 12	\$0.7	
-	Goorge I. Sanooin, minwaukee, Wis	3 30	22 00	14	1	
12	John Brown, Thorold, C. W	3 20	19 00	{ 11 7±	5- 5	

Abstract of bids for furnishing mate

				Class 3.					
Number of bid.	Name of bidder.	Cost of Class 1.	Cost of Class 2.	Nuts and screw wash- er bolts, iron.	Drift-bolt iron.	Spikes, wrought iron.			
			Piles.						
21	a Hasbrouck & Conro, Milwaukee, Wisconsin.	\$19, 425 00	\$27, 158 40	\$4,506 60	\$27, 347 77	\$1,960 14			
32	b George P. Sanborn, Mil- waukee, Wisconsin.	18, 130 00	88, 429 67	5, 257 70	26, 436 18	1,866 80			
12	c John Brown, Thorold, C.W.	16, 576 00	70, 001 28	{ 4, 131 05 { 2, 816 62	19, 143 44 18, 231 85	1, 493 44 1, 306 76			

a Bid for whole or none-so understood at the engineer office, Detroit.

a Bid for whole or none—so understood at the engineer office, Detroit.

b In my first draught of abstract, I supposed this to be an honest bid from Sanborn himself, like out ter, May 7, to the engineer's department, I discovered that it was a shyster bid. Sanborn was a mbronck & Conro.) I now reject the whole of this bid, No. 32, as a bid none but a trickster wou ng this abstract to the engineer department, June 11, 1867. I regret that such a bid should disgrace. Additional remarks since my abstracts of May 7 and June 11: I now reject the bid for dredging c On a re-examination of the original bid it is discovered that a note accompanied the bid to the articles the prices in his bid should be reduced as low as any other bidder; hence the corrections by classes apparation.

by classes separately.

This abstract, as far as bids 21, 32, and 12 are concerned, is precisely like that sent May 7, 1865. June 11, after I discovered the fraud in bid 32.

In my abstract sent May 7 there was no classification either of material, or work, or labor; thin Nor was there any classification in my advertisement. The classification I knew could be made been the practice in my office in numerous contracts I had before made, and all sanctioned in the contracts.

doing work at St. Clair flats, Michigan.

Classification of work to be done.

All the different kinds of work or labor here below have such dependence on each other that they must be put in one class of labor—Class A.

cut or	yard es or	driving L	driving	_	For fr	aming a	nd putt	ing in pe	r runnin	g foot of t	imber.	
Dredging per cubic mensured in the cr	Dredging per cubic yamesaured in the dikes banks; 653,220 yarda.	For preparing and drupe the round piles.	For preparing and drup the sheet piles.	Binders, 4 in. by 6 in., 16,416 r. f.	Stringers, 5 in. by 6 in. 17,280 r. f.	Back caps, 8 in. by 8 in, 16,416 r. f.	F. water sill, 8 in. by 9 in., 17,280 r. f.	Cross ties, 8 in. by 10 in., 90,930 r. f.	B. water sill, 8 in by 12 in., 17,280 r. £	R. siding and fenders, 8 in. by 12 in., 82,676 r. f.	Front siding, 10 in. by 12 in., 65,596 r. f.	Front caps, 12 in. by 12 in., by
		Pr. pile.	Pr. pile.						1			
\$0 42		\$2 25	\$0 65	\$0 81	\$0.81	\$0.84	\$0 81	\$0.81	\$0 8 <u>1</u>	\$0 8 1	\$0 8 <u>1</u>	\$0 8 <u>1</u>
34	ļ	2 25	69	91	91	91	91	91	91	91	91	91
38	\$0 39	2 80	1 30	41	42	41	42	41	, 41	41	41	48

doing work at St. Clair flats, Michigan-Continued.

	c	ost of Class	A.		=	to #	anal.
Dredging monsured in cut.	Dredging measured in dikes.	Preparing and driv- ing round piles.	Preparing and driving sheet piles.	Framing sawed tim- ber.	Total cost of all material	Total cost of all labor work.	Complete cost of the ca
\$274, 377 60 225, 381 60 248, 246 40	\$254, 779 20	\$11,655 00 11,655 00 14,504 00	\$10,660 00 11,316 00 21,320 00	\$29, 015 09 32, 428 63 16, 215 47	\$140, 113 35 { 111, 341 68 { 108, 932 51	\$325, 697 69 280, 781 23 } 300, 285 87	\$420, 891 58 { 411, 627 55 { 409, 218 38

for the whole or none, and so received it. After sending off that preliminary abstract and accompanying letin the matter. The bid had been made out and signed with Sanborn's name by Conro, who bid No. 21, (Hasupon the government. The whole chain of facts connected with it are fully set forth in my letter accompanyof mine. The bid, however, was understood at my office to have been for the whole or none.

and-compliance in an essential resture with my active listenent to which this bid was in response.

This is the lowest responsible accepted bid for the work, whether regarded as to be awarded in aggregate or

the parts seen in this in red ink and the remarks, and this is like the one I sent as my final official abstract

or were numbered in items for more convenience in bidding intelligently and for computing costs in my office. up the contract to meet the conditions prescribed in section three of the law, as had, for the last two years, Not a word of objection had, to my knowledge, been raised there until the question of St. Clair flats came up.

T. J. CRAM, Colonel Engineers.

Copy of original bid No. 21.

We propose to furnish materials, and do the work, to construct the sta

General T. J. CRAM,

United States Engineer, Detroit, Michigan:

Dredging between the dikes and placing the earth in the canal banks

A. E. GOODRICH, Chicago, M. B. MILBURY, Milwaukee,

Bondsmen.

I certify the above to be a true copy of the original bid.

T. J. CRAM,

Colonel of Engineers, Brevet Major Gene

Copy of original bid No. 32.

General T. J. CRAM,

United States Engineers, &c.:

I propose to furnish all materials and do the work for constructing the from the mouth of South Pass across the St. Clair flats, 300 feet wide, a required depth, to be diked with piles and timber on both sides, in account plans and specifications now on file at your office and advertisem follows:

Round piles, each.

Sawed lumber and sheet piles, per M, board measure.

Round piles driven, each.

Sheet piles driven, each.

Screw-bolts, per pound.

Inch square drift-bolts.

Spikes, per pound.

Framing and placing timber, per lineal foot.

Dredging, measured in excavation, per cubic yard.....

GEORGE P. SANBORN, Milwan

JOHN J. DETON, of Milwaukee, O. J. HALL, of Milwaukee,

for Bondsmen.

I certify this to be a true copy of the bid 32, which was written conro and signed by Conro with Sanborn's name.

T. J. CRAM.

Colonel of Engineers, Brevet Major Gene

Copy of original bid No. 12.

DETROIT, April 10, 1867.

SIR: I propose to furnish all the material and to do all the work, according to the advertisement for constructing a straight ship canal from the mouth of South Pass directly across the St. Clair flats, 300 feet wide by 13 feet deep, at the following rates:

MATERIAL.		
Round piles, each	\$ 3	50
Sheet piling, stringers, water sills, binders, caps, sidings, and cross-	19	00
ties, per M, board measure	19	
Round iron bolts, nuts, screws, and washers		11
Square iron ditto, per pound		$0.5\frac{1}{4}$
Spike, wrought iron, per pound		08
•		
WORK.		
Earth to be dredged from between the dikes and put into the dikes and into the canal banks beyond, measured in natural bed or cut		38
Measured in the dikes or banks, for so dredging and placing the earth,		
per cubic yard		39
Preparing and driving piles, each	2	80
Preparing and driving sheet piles, each	1	30
Framing and putting in place all the sawed timber, per lineal foot		$04\frac{3}{4}$

This bid is put in, paying duties on piles; if no duties, thirty cents less than in my bid.

In case the front piling should be required to be driven battring, fifty cents per running foot extra.

JOHN BROWN.

Sureties:

E. TROWBRIDGE & HENDRY. Detroit.

T. J. CRAM, Esq.,

Col. Corps of Engineers, Brevet Maj. Gen. U. S. A., Sup't Harbor and River Improvements.

Note.—The figures in red, under the head "complete cost," show what Brown's total bid would amount to, at prices $7\frac{1}{2}$ cents, 5 cents, and 7 cents per pound for nut and screw bolts, iron, and spikes, such as Brown proposed in a note accompanying this bid, and referred to in my letter of 11th June to engineer department, and which note has since been disallowed by the department.

T. J. CRAM.

This is a true copy of the original bid.

T. J. CRAM, Col. Corps Engineers, Brevet Major General.

DETROIT, May 2, 1867.

DEAR SIR: Not being fully posted in price of iron and bolts, I will say that, if I am fortunate enough to have the whole contract awarded to me, I will furnish those articles at as low a figure as any bid you may receive.

My object in making this offer is that I may control the whole work, and then

there could be no difficulty between contractors.

JOHN BROWN.

General T. J. CRAM.

I certify the above to be a true copy of the original note accompanying Brooriginal bid. None of these bids were opened, owing to my absence, unto 5th of May, and in the hurry of recording so many bids, this note es observation until after my abstract of 7th May was sent off. If this note accepted, Brown's bid will be less, as shown by the red figures in the abherewith sent.

T. J. CRAM, Col. Engineers, Brevet Major Gene

To contractors.

OFFICE LAKE HABOR IMPROVEMENTS, No. 111 Griswold street, Detroit, Michigan

Written and sealed proposals for furnishing materials and doing the work be received at this office, addressed to the undersigned, until the 10th April, 1867, for constructing a straight ship canal from the mouth of Sout directly across the St. Clair flats, 300 feet wide and 13 feet deep, below stage of water; to be diked with piles and timber on each side, for its length, to a height of five feet above water, and the earth dredged out the canal to be put into the dikes and beyond them, so as to form the banks even in height with the dikes, and of uniform width on top, and an outer slope going off into the lake water of two horizontal to one vertically across the state of the same state.

For further details of construction, bidders are informed that they must person at this office, where models, drawings, and specifications can be seall explanations given.

MATERIALS REQUIRED.

1. 5,180 rock elm or white oak straight round piles, 12 inches, at least, sive of bark, in diameter at the middle, and not less than 28 feet long.

2. 16,400 sheet piles, 4 by 12, sawed, 18 feet long, white oak, rock ele or white pine, that will drive well without splitting; widths may vary ft to 12 inches, giving an equivalent amount, in board measure, to what 12 in width would give for the total amount.

3. 864 stringers, 5 by 6, sawed, 20 feet long, hard pine.

4. 864 water sills, 8 by 9, sawed, 20 feet long, hard or white pine.

5. 864 water sills, 6 by 12, sawed, 20 feet long, hard or white pine.

6. 864 binders, 4 by 6, sawed, 19 feet long, hard or white pine.

7. 864 fenders, 8 by 12, sawed, 20 feet long, white oak or hard pine.

8. 864 front caps, 12 by 12, sawed, 20 feet long, heart of white oa hard pine.

9. 864 back caps, 8 by 8, sawed, 19 feet long, hard or white pine.

10. 1,640 front sidings, 10 by 12, sawed, 20 feet long, hard or white p 11. 1,822 front sidings, 10 by 12, sawed, 18 feet long, hard or white p

12. 1,640 rear sidings, 8 by 12, sawed, 20 feet long, hard or white pi

13. 1,822 rear sidings, 8 by 12, sawed, 18 feet long, hard or white pin

14. 6,495 cross-ties, 8 by 10, sewed, 14 feet long, hard or white pine oak.

The sawed stuff will amount to 3,684,278 feet, board measure. The piles to be of best quality of timber; all the sawed stuff to be squarely an sawed to the dimensions stated, of good live timber, and to be free from knots, splits, shakes, or other defects tending to impair its durability or st

State the price in the bids of furnishing item 1, per pile, delivered. State the price in the bids, per thousand feet, board measure, for furnishing item 1, per pile, delivered.

all the sawed stuff delivered, the place of delivery being at the mouth of the South Pass, near Jerry's ranch.

15. 2,590 1-inch round-iron nut and screw (and 2 washers) bolts, 27 inches

long from outside of head to point of screw.

16. 2,590 1-inch round-iron nut and screw (and 2 washers) bolts, 25 inches long from outside of head to point of screw.

Note.—These bolts may probably have to be varied from $\frac{1}{2}$ to 1 inch in length to suit variations in thickness of pile.

17. 648 bars 1-inch square iron, 12 feet long, for drift-bolts, 25,754 pounds.
18. 648 bars 1-inch square iron, 14 feet long, for drift-bolts, 30,046 pounds.

19. 5,828 bars 1-inch square iron, 16 feet long, for drift-bolts, 308,837 pounds.

20. 1,724 pounds 10 inch wrought spikes, ½ inch in diameter.
21. 16,944 pounds 8 inch wrought spikes, 3 inch in diameter.

The rim for the bolts and spikes to be of the best quality.

State the price per pound for furnishing and delivering items 15 and 16. State the price per pound for furnishing and delivering items 17, 18, and 19. State the price per pound for furnishing and delivering items 20 and 21.

The place of delivery being the same as above stated.

One-sixteenth part of each class of the twenty-one foregoing items to be delivered on or before the 1st of July next, and the remainder in instalments of one-sixteenth for every month thereafter during navigation until all shall have been delivered.

Whoever receives the contract will be required to furnish two responsible indorsers in a bond of indemnity to the United States to the amount of 20 per cent. of the total value of the materials contracted for.

State in the bids the names in full and residence of the bondsmen.

WORK TO BE DONE.

1. The average depth of water all along where the canal is to be made was six feet two and a half inches, last November, and nowhere less than three and a half feet, allowing scows to work with facility everywhere on and about the site.

The average thickness of stratum, or prism of earth, to be excavated between the dikes is 6 feet 9½ inches; the mean lift to raise it to the surface of the water is 9 feet 7 inches. The length of the canal is to be 8,200 feet. Borings show earth easy of dredging and good for driving the piles; should it be found hard to drive through the upper crust, dredging will be done along where the dikes are to stand, to a sufficient depth to allow the piles to be driven to the depth of 24 feet below the surface of the water for the round, and 17 feet for the sheet piles.

Earth to be dredged from between the dikes and put into the dikes, and into the canal banks beyond, so as to make them into proper shape as described, 618,280 cubic yards; probable amount to be dredged to ease the driving of the piles, 35,000 cubic yards.

State the price per cubic yard, measured in the natural bed or cut, for so

dredging and placing the earth.

Also state the price per cubic yard, measured in the dikes and banks, for so dredging and placing the earth.

State the price for preparing and driving the round piles, per pile.
 State the price for preparing and driving the sheet piles, per pile.

4. State the prices for framing and putting in place, according to plans and specifications, to be learned in this office, all the sawed timber, per running foot of timber of each size, measured in the works, there being of 4 by 6 16,416 running feet; of 5 by 6, 17,280 running feet; of 8 by 8, 16,416; of 8 by 9, 17,286; of 8 by 10, 90,930; of 6 by 12, 17,286; of 8 by 12, 82,876; of 10 by 12, 65,596; of 12 by 12, 17,280, mill measure—in all, 341,354 lineal feet.

The framing is exceedingly simple; all can be done with the adze, han and auger. For every bolt to be driven a hole must be previously bored depth equal to the length of the bolt. A dovetail is to be cut on each e the cross-ties, and corresponding notches in the embracing timbers. In the ning timber-work the ends of the siding timbers are to abut, not lap, e the caps, fenders, and water-sills, which are to join by halving. In each pile three spikes are to be driven without splitting, which will require vious boring.

It is highly desirable that one contractor should be fortunate in putting bids so as to secure to himself the contract for furnishing all the material doing all the work. The doing of all work, however, to be under one contributed by its justify. The act of Congress making the appropriation requires be received and contracts made for each class of materials and for doin work, separately, and it may happen that he who gets the material may the contract for the work. The lowest responsible bid secures the contract

It is pertinent to remark that from the beginning of the work at South to the completion, all the machinery and work will be in almost all weather feetly protected, nor will any hindrance arise from passing vessels or rafts, until the new channel shall be completed, will follow the old crooked across the flats. These circumstances are of great consideration to the continuous control of the con

The rule which has already been stated in reference to bondsmen and in the nity for materials, applies to the contract for doing the work. Bidders we particular to follow the instructions herein contained in writing out the posals. Time for completing the work, on or before the spring of 1869.

T. J. CRAM,

Colonel Corps of Engineers, Brevet Major General U. S. Army, Superintendent Harbor and River Improvement

B 14.

United States Engineer Office, Detroit, Michigan, August, 18

SIR: I herewith forward the abstract of bids upon the St. Clair fla provement, as the result of the new advertising for proposals, in obedie your letter of instructions to me of July 16, 1867. You will perceive th classifications in the advertisement and abstract correspond with the class tions made in your instructions, your "first class," "second class," "third of under the head of materials, being designated by the letters B, C, D. Ina as your instructions divide the work to be done under the head of "labor two classes, first and second, which I have designated by E and F, it b necessary, for the security of the dikes while in progress of construct introduce another class, which I have designated by A. This is continger will be necessary upon class F being assigned to another contractor that to whom E might be awarded. Item 5 under class E is, of course, conti upon the same event. It would not be wise to be unprovided with the of staying and securing the wood-work of the dikes, as we should have under separate contractors for E and F, who would inevitably, in spite the precautions we can take to make them work in harmony, sometimes as their interests would be affected.

There is another point to be explained: As near as I can foresee, the c gent dredging item 1, class E, also dredging class F, will, for the interest work, have to be measured, say, one-half in natural bed and one-half in and dikes; hence, in making up the costs, add one-half the price bid mes

in natural bed to half the price bid measured in banks and dikes, and multiply the mean thus obtained by the number of yards, and we obtain the cost of E and F. It is upon this principle that the sums in columns headed "Total cost of class E" and "Cost of class F," marked in the abstract with a red star, have been computed to ascertain who is the lowest bidder for each class, E and F.

For the entirety, the lowest bid is seen to be No. 5, John Brown, \$397,125 68. Of those who have put in their bids according to instructions in the advertisement, the lowest bidders for separate classes are-

For class A, bid No. 3, Fox & Howard	\$750 0	0
For class B, bid No. 5, John Brown	14,504 0	0
For class C, bid No. 9, J. S. Miner	58, 027 2	8
For class D, bid No. 10, Moses Hill	20,502 4	16
For class E, bid No. 3, Fox & Howard	64, 393 3	2
For class F, bid No. 5, John Brown	224, 126 5	0
Total cost by having six contractors	382, 303 5	6

In regard to bidders and their bondsmen, named in the bids-

For class A, bid No. 3, I know nothing of their means or pecuniary responsibilit**y.**

For class B, bid No. 5, bidder perfectly good in means and pecuniary respon-

sibility; also the bondsmen.

For class C, bid No. 9, bidder doubtful as to efficiency and pecuniary responsibility; bondsmen considered good in so far as pecuniary responsibility is con-

For class D, bid No. 10, considered to be good in means and pecuniary responsibility.

For class E, bid No. 3, I know nothing of the bidders' means or pecuniary responsibility, nor of the responsibility or pecuniary means of the bondsmen. For class F, bid No. 5, bidder and bondsmen considered good in means of

machinery to carry out the work; also in pecuniary responsibility.

It will be seen at the foot of the abstract that another bid (seen in red ink) for the entirety was put in. I have forwarded the calculations of this bid. This bid I would reject for its non-conformity to the advertisement. The non-conformity was, I think, purposely done. On the 6th day of August I wrote to the bidder to the effect that he must write me at once whether this bid was intended for the whole or none, or whether he would accept, if awarded to him, any class designated A, B, C, D, E, F, upon which he bid. He replied by telegraph that he would write. At least eight mails have since come from Chicago, but no letter upon the subject. I have no confidence in him or his bondsmen, and do not believe he would accept of class F without having other classes, which are very high, assigned to him, in order to make up for the loss on F at his prices. It will be perceived his bid for the entirety is \$406,371 12. I submit the bid, however, for rejection by the United States, or for acceptance, as the higher authority may elect. If the award is made for the entirety to the lowest responsible bidder, who bid according to the advertisement, (bid No. 5,) the approximate cost of the work will be \$31,628 32, (a,) less than my original estimate. But if the awards be made to the lowest bidder for classes as above described, (rejecting the bid in red,) the approximate cost is less by \$36,450 44, (b,) than my original estimate, and we should have the disadvantage of several contractors. Difference between (a) and (b) is only \$4,822 12, not enough to balance the advantage to be gained by having one responsible contractor for the entire work, and as the second section of the act making the appropriation directs that the contract for each work shall only be made with the lowest responsible bidder, therefore I can see no reason why this advantage should not be available to this work. See accompanying opinion of United States di

attorney for eastern district of Michigan.

In conclusion I would remark that in case awards should be made by clit would be well if authority be given to contract with the next lowest resible bidder and so on, should any who are the lowest in the abstract faccept, as I now think one or two would. In the advertisement the riprejecting all or any of the bids is reserved to the United States as directed Very respectfully, your obedient servant,

T. J. CRAM,

Colonel of Engineers, Brevet Major General A. A. HUMPHREYS,

Chief of Engineers United States Army.

UNITED STATES DISTRICT ATTORNEY'S OFFICE, EASTERN DISTRICT OF MICHIGAN, Detroit, June 26, 19

GENERAL: I have to acknowledge your communication of June 25, reing my opinion as to the construction of act of Congress approved Ma 1867, entitled "An act making appropriations for the repair, preservatio completion of certain public works heretofore commenced under the aut of law, and for other purposes," (Sess. L. 1867, p. 418,) with reference tain practical questions which present themselves.

It is a cardinal rule that in the construction of statutes each section is interpreted with reference to the other sections and the law as a whole.

I first observe that the appropriation for St. Clair flats improvement is expended in accordance with your plans and specifications by section that by section 2 the money voted is to be made to go as far as possible to finishing the work, and that by the same section the honorable Secret War is to expend the money by contract, (except in case of examination surveys.) In regard to the aforementioned matters the law is clear.

Section 2 further provides that the work shall be let to the lowest respectively. This means clearly, in my opinion, to the lowest single vidual, partnership, or company who shall put in a bid for the entire improvement; the word "therefor" signifying the public work advertised contracted for.

In the light of this latter portion of section 2, the language of section 3 it otherwise ambiguous, (which I think it is not,) becomes plain. There she separate proposals and contracts for each work and BACH CLASS OF MAT OR LABOR for each work.

I think this is by no means to be interpreted as requiring a separate vidual contractor for each class. It evidently merely means that each bidder shall classify and itemize his bid.

This provision is made undoubtedly for two reasons: 1. Because the w to be done according to certain plans; and, 2. To prevent the frauds some

resulting from a lumping bid.

Any other construction than that which I give I apprehend would nulli appropriation; for it would be impossible, in the first place, to find person would contract for portions of the work; and, secondly, if the former obj is met, such persons would inevitably disagree and clash, so as to del destroy the work.

But I regard section 2 as IMPERATIVE in requiring the whole work show

let to ONE contractor.

Permit me to express the opinion that the early letting and completion of this work is very highly desirable for the interest of our commerce.

Very respectfully, your obedient servant,

ALFRED RUSSELL, United States District Attorney.

Brevet Major General T. J. CRAM, Colonel of Engineers, Detroit.

I certify that the above is a true copy of the original letter.

T. J. CRAM,

Colonel Engineers, Brevet Major General.

B 15.

To contractors.

OFFICE LAKE, HARBOR AND RIVER IMPROVEMENTS,

No. 111 Griswold street, Detroit, Mich.

All proposals heretofore made for St. Clair flats are rejected, and new written and scaled proposals for purchasing material and doing the work will be received at this office, addressed in duplicate to the undersigned, until the 5th August, 1867, for constructing a straight ship canal from mouth of South Pass directly across St. Clair flats, three hundred feet wide and thirteen feet deep, below lowest stage of water, to be diked with piles, timber, and earth on each side for its whole length (about one and a half mile) to a height of five feet above water. The filling for the dikes and the rest of the canal banks to be done with the material to be dredged out from between the dikes to form the canal waterway; and the material to be put into the dikes and beyond and contiguous to them, so as to form the canal banks even in height with the dikes, and of uniform width on top, and having an outer slope-off into the lake water of two horizontal to one vertical.

For further and minute details of construction bidders must call in person at this office, where models, drawings, and specifications can be seen, and will be explained between the hours of noon and 1 p. m.

Materials to be furnished.

CLASS A.

Item 1.—(Contingent and may not be required.)—15,000 running feet, 6 to 10 inches diameter, round, straight, small timber or poles, in sticks of various lengths, not less than 20 feet, however.

CLASS B.

Item 1.—5,180 rock elm or white oak, straight, round piles, 12 inches diameter at least, exclusive of bark, at the middle, and not less than 28 feet long, of best quality of timber, straight and otherwise good for driving.

CLASS C.

Item 1.—16,400 sheet piles, 4 by 12, sawed, 18 feet long, white oak, rock elm, hard or white pine, that will drive well without splitting; width may vary from 10 to 12 inches, giving an equivalent amount in board measure to what 12 inches in width would give to the total amount.

Item 2.—864 stringers, 5 by 6, sawed, 20 feet long, hard pine.

Item 3.—864 water-sills, 8 by 9, sawed, 20 feet long, hard or white pine.

Item 4.—864 water-sills, 6 by 12, sawed, 20 feet long, hard or white Item 5.—864 binders, 4 by 6, sawed, 19 feet long, hard or white pine

Item 5.—864 fenders, 4 by c, sawed, 19 feet long, nard or white pind Item 6.—864 fenders, 8 by 12, sawed, 20 feet long, white oak or hard Item 7.—864 front caps, 12 by 12, sawed, 20 feet long, heart of white

of hard pine.

Item 8.—864 back caps, 8 by 8, sawed, 19 feet long, hard or white p Item 9.—1,640 front sidings, 10 by 12, sawed, 20 feet long, hard o pine.

Item 10.-1,822 front sidings, 10 by 12, sawed, 18 feet long, hard o

pine.

Item 11.—1,640 rear sidings, 8 by 12, sawed, 20 feet long, hard or wh Item 12.—1,822 rear sidings, 8 by 12, sawed, 18 feet long, hard or wh

Item 13.-6,495 cross-ties, 8 by 10, sawed, 14 feet long, hard or white oak.

The sawed stuff will amount to 3,684,278 feet, board measure, and all squarely and truly sawed to the dimensions stated, of good, live timber, be free from rotten knots, splits, shakes, or other defects tending to imdurability or strength.

CLASS D.

Manufactured iron.

liem 1.—2,590 1-inch iron nut and screw (and two washers) bolts, 27 inches long from outside of head to point of screw. 2,590 1-inch iron nut and screw (and two washers) bolts, 25 inches long from outside of head to point of screw. (Note.—These bolts may probably have to be varied from one-half to one inch in length, to suit variations in thickness of piles.)

ltem 2.—648 bars one-inch-square iron, 12 feet long, for drift-bolts—25,754 pounds; 648 bars one-inch-square iron, 14 feet long, for drift-bolts—30,046 pounds; 5,582 bars one-inch square iron, 16 feet long, for drift-bolts—308,837

pounds.

Item 3.—1,724 pounds 10-inch wrought spikes, ½-inch in diameter; 16,914 pounds 8-inch wrought spikes, 3-inch in diameter.

The iron for the bolts and spikes to be of the best quality.

Labor or work to be done.

CLASS E.

Item 1.—(Contingent and may not be required.)—Dredging, say 35,00 yards, to ease the driving of the piles, if the engineer should find it need and putting the same into the canal banks, to be measured either in batthe natural bed before being dredged, at the option of the engineer in characteristics.

Item 2.—Preparing and driving the round piles, so that these points 24 feet below said lowest stage of water, and to be in line, and without to the heads of piles such as to prevent them from being well should faced to receive the watersills or other timbers to be connected with the according to plans. Borings have shown the earth to be good for drivinholding the piles.

Item 3.—Preparing, driving, and spiking sheet piles, so that their shall be 17 feet below said stage of water, and joined so closely to each that the earth filled into the dikes will not wash through the joints, bored where spiked to the side timbers, and heads not to be split in the

so as to injure them.

Item 4.—Framing, fitting, putting in place, boring, fastening toget into piles by spikes and bolts, including cutting the bars into bolts; all timber or lumber, (besides sheet piles,) of which there will be about running feet of various sizes, (as seen in items 1 to 13, class C.) Whe whether nut and screw or drift, are inserted, boring must be previous for the whole depth of wood the bolts are to penetrate. For the fend binders, the round washers, heads, and nuts of the screw bolts must be sunk into the timbers; the side pieces and cross-ties to be framed togeth the double dovetail joint.

Item 5.—Framing, fitting, putting in place, and fastening all smal timbers or poles, if needed, according to the judgment of the enginabout 15,000 running feet. This is contingent, and may not be requirement, however, be bid for.

CLASS F.

Item 1.—Dredging between the dikes, so as to make the prescribe water-way, and putting the earth dredged into the dikes and banks, s make them of the shape described, namely, about 58 feet uniform width including the width of the dikes; 5 feet high above water, and outer s into the lake water two feet horizontal to one vertical. The appramount to be dredged for these purposes is about 618,280 cubic yar amount so to be dredged and so to be placed to be measured either in bed before dredging, or in the dikes and banks after they are made into at the option of the engineer in charge. No distinction must be made bidder as to "soft" or "hard." Borings have shown the earth to be dredging, but the bidder takes all risks on the character of the earth.

One-sixteenth part of the items of each class of material to be delivered quired, by the engineer in charge, on or before the 1st of September next, remainder in instalments of one-sixteenth for every month thereafter navigation, if required, until all shall have been delivered that may be to complete the work, which it is expected will be completed, should priations continue to be made, on or before the close of navigation in 18

The dredging and formation of the banks to commence simultaneous to keep pace with, and to be subordinate to, the construction of the woo of the dikes, with at least two dredges of power to raise 600 cubic yaday each, and as many additional dredges are afterwards to be put into the safety and experience of the disconstruction with safety and the disconstruction with safety and the disconstruction with safety and the disconstruction with the d

It is highly desirable that a bidder for the entirety should be so fortunate in putting in his prices as to be the lowest, or as low as any for each class.

The right of rejecting all bids, or any of them, is reserved to the United States. Bidders are notified that the contractor for class E will be required to revive at the site of the improvement, or at some convenient point in that vicinity, an materials of wood and iron used in the work, in such quantities and at such times as the engineer in charge of the improvement may direct, and will be responsible for the safe-keeping of the same.

The place of deposit for materials will be near where was "Jerry's ranch;" and bidders are informed that there is no dock, no house or inhabitant within

seven miles of the site of the work, except the light-house.

Bidders will please conform to the following form in writing out their bids, and forward in duplicates, filling the blanks with prices and attaching a copy of this advertisement to the bid:

this advertisement to the bid:
The undersigned proposes to furnish for St. Clair flats improvement— CLASS A.—Item 1, for —— cents per running foot. CLASS B.—Item 1, for —— dollars per mile.
CLASS C.—Items 1 to 13 inclusive, for —— dollars per thousand feet, board
measure.
CLASS D.—Item 1, for —— cents per pound; item 2, for —— cents per pound; item 3, for —— cents per pound, and to do the work.
CLASS E.—Item 1, for —— cents per cubic yard, measured in natural bed; for —— cents per cubic yard, measured in dikes and banks of the canal; item
2, for — dollars per pile; item 3, for — dollars per pile; item 4, for — cents per running foot; item 5, for — cents per running foot.
CLASS F.—Item 1, for —— cents per cubic yard, measured in the natural
bed; for —— cents per cubic yard, measured in dikes and banks of the canal. ———————————————————————————————————
 ,
 ,
Bondsmen.
O 1 M T O

General T. J. CRAM,
United States Engineer Officer in charge.

The bidder will please write his name in full, in a legible hand; also, his residence; and his two bondsmen must sign their names below his, stating their residence, and be citizens of the United States.

Each bidder must state in his proposal whether he wishes the award of the whole he bids for or none. Bids for items in a class, without covering all the items therein, not acceptable.

The amounts stated in the foregoing items subject to be varied as the engineer in charge may find necessary.

T. J. CRAM,

Colonel Corps Engineers, Brevet Major General.

14 w----Vol. ii

B 16.—Abstract of bids for furnishing materia

	Name of bidder.
_,	
bid.	•
r of	
ppe	•
Number of	•
-	
1	J. E. & D. E. Baily, and Patrick Smith, Cleveland, Ohio; S. S. Stone and W. H. Trescott,
_	Cleveland, Ohio, bondsmen.
2	Wm. E. Standart, Elias Sims, and E. M. Peck, Cleveland, Ohio; Jas. M. Coffinburg and J. F. Card, Cleveland, Ohio, bondsmen.
3	Henry Fox and W. B. Howard, Chicago, Illinois; J. E. Miller and W. G. Lewis, Chicago, Illinois, bondsmen.
4	Hosea Thomas Stock, Toledo, Ohio; Harvey P. Platt and E. S. Platt, Toledo, Ohio, bonds-
5	men. John Brown, Thorold, Canada; E. Trowbridge and Geo. Hendric, Detroit, Michigan, bonds-
6	men. Hasbrouck & Conro; Stephen C. Walker and Albert E. Goodrich, Chicago, Illinois, bonds-
- 1	men.
8	D. E. Rice, Detroit, Michigan; W. Buchanan and J. Carroll, Detroit, Michigan, bondsmen Brooks & Adams, Detroit, Michigan; E. B. Ward and Christian Buhl, Detroit, Michigan,
9	bondsmen. J. S. Miner, Detroit, Michigan; L. M. Mason, and Moore, Foot & Co., Detroit, Michigan,
10	bondsmen.
	Moses Hill, Cleveland, Ohio; Jacob Lawman and Albert T. Slade, Cleveland, Ohio, bonds- men.
11	Charles F. Dunbar, Erie, Pennsylvania; Franklin Lee, Buffalo, New York, and S. Dunbar, Toledo, Ohio, bondsmen.
12	Bubl, Ducharme & Co., Detroit, Michigan
13	Jos. T. Walton; H. P. Baldwin, Detroit, Michigan, and H. A. Fuller, Springfield, Massachusetts, bondsmen.
14	Stephen C. Walker, Chicago, Illinois; Andrew J. Wright and Charles G. Wicker, Chicago, Illinois, bondsmen.

doing work on St. Clair flats, Michigan.

cation of		es bid fo ials.	r furnishi	ing ma-		Cost of clas	ses of materi	al.	A, B, C,
Class B.	Class C.	Man	Class D.	iron.					sterials,
Item 1.—5,180 round pilen.	Items 1 to 13, inclusive.—All sawed or hewed timber and lumber, 3,684,272 ft., b. m.	Item 1.—5,180 nut and serew 2-in. weaher bolts; 1-inch round iron, 37,555 lbs.	Item 2,-7,124 1-inch square bare, 12, 14, and 16 ft. long; drift bolt iron, 364,637 lbs.	Item 3.—Wrought spikes, 5 and 10 inches long, 4 and 8 inch indiameter, 18,668 lbs.	A.	В.	C.	D.	Total cost of all classes of materials, and D.
Pr. pile. \$4 00	Pr. M.	Pr. lb. \$0 08	Pr. 15.	Pr. lb.	\$750 00	\$20,720 00	\$73, 685 44	\$20, 673 15	\$115, 828 5
4 50	21 00	071	041	06 <u>‡</u>	1, 950 00	23, 310 00	77, 369 71	21, 396 96	124, 026 6
3 00	20 00	121	08	09	750 00	15, 540 00	73, 685 44	35, 545 45	125, 520 8
4 75	25 00	09	041	09	1,500 00	24, 605 00	92, 106 80	21, 468 73	139, 680 3
2 80	17 70	07	041	07	900 00	14, 504 00	65, 211 61	23 775 56	104, 391 1
3 75	23 00	12	07	· 08	900 00	19, 425 00	84, 738 25	31, 524 63	136, 587 8
	18 00	08	05	06\$			66, 316 89	22, 496 34	
	15 75				ļ. 		58, 027 28		
		06}	04.6	06.9				20, 502 46	
5 00	23 00	081 09	04‡ 07	06 1 08	1,500 00	25, 900 00	84, 738 25	21, 819 73 30, 397 98	142, 536 2
5 00	22 00	15	09	121	1, 200 00	25, 900 00	81, 053 98	40, 784 08	148, 938 0

Abstract of bids for furnishing materials an

				Clas	a B.
	Name of bidder.	gent. — ing 3,5	contin- Dredg- 00 c.y. to be driv- piles.	1 971	
Number of bid.		Measured in banks, not in scows.	Measured in natural bed, not in scows.	Item 2.—Proparing and driving round piles. 5, 180.	Item 3.—Preparing, driving, and fast- oning sheet piles—16,400.
1	a J. E. & D. E. Bailey, and Patrick Smith, Cleveland, Ohio; S. S. Stone and W. Trescott, Cleveland, Ohio,	Pr. c. y. \$0 33	Pr. c. y. \$0 30	Pr. pile. \$3 00	Pr. pi
2	bondsmen. b Wm. E. Standart, Elias Sims, and E. M. Peck, Cleveland, Ohio; Jas. M. Coffinburg and J. F. Card, Cleveland, Ohio	44	42	3 50	10
3	bondsmen. c Henry Fox and W. B. Howard, Chicago, Illinois; J. E. Miller and W. G. Lewis, Chicago, Illinois, bondsmen.	35	25	2 25	1
4	d Hosea Thomas Stock, Toledo, Ohio; Harvey P. Platt and E. S. Platt, Toledo, Ohio, bondsmen.	.39	34	5 00	1 5
5	e John Brown, Thorold, Canada; E. Trowbridge and Geo. Hendric, Detroit, Michigan, bondsmen	33	33	2 50	13
6	f Hasbrouck & Conro; Stephen C. Walker and Albert E. Goodrich, Chicago, Illinois, bondsmen. g D. E. Rice, Detroit, Michigan; W. Bnchanan and J. Carroll, Detroit, Michigan, bondsmen.	40	30	2 25	
8	Brooks & Adams, Detroit, Michigan; E. B. Ward and Christian Bubl, Detroit, Michigan, bondsmen.			ļ	ļ
9	J. S. Miner, Detroit, Michigan; L. M. Mason, and Moore, Foot & Co., Detroit, Michigan, bondsmen.	•••••		·····	
0	Moses Hill, Cleveland, Ohio; Jacob Lawman and Albert T. Slade, Cleveland, Ohio, bondsmen.	•••••	•••••	· ····	
11	Charles F. Dunbar, Erie, Pennsylvania; Franklin Lee, Buffalo, New York, and S. Dunbar, Toledo, Ohio, bonds- men.		•••••		
12 13	Buhl, Ducharme & Co., Detroit, Michigan. A Jos. T. Walton; H. P. Baldwin, Detroit, Michigan, and	50	42	3 50	
14	'H. A. Fuller, Springfield, Massachusetts, bondsmen. i Stephen C. Walker, Chicago, Illinois; Andrew J. Wright and Charles G. Wicker, Chicago, Illinois, bondsmen.	34	24	3 50	1 :

c We wish the award of the whole, or of classes C, D, E, or F, either or all.
b Desire the award of all in classes A, B, C, D, and E, or none, and whole in class F or none.
c Will take whole work or a part.
d The whole or none.

d The whole or none.

Bid for the whole, but will accept classes E and F, if awarded, or any class upon which he may

Will take any portion or the whole.

Will take any of the items at the price named.

A I bid for the whole or any class that may be assigned me.

The bidder does not conform to the advertisement in an essential point.

I certify that the above bids, Nos. 1 to 13, inclusive, have been truly entered, and that this is a to

I certify that No. 14 is a true abstract of a bid which was put in, but which did not conform to ayed three days longer than necessary for him to reply and make his bid to conform to the advert

work on St. Clair flats, Michigan .- Continued.

of sad pri	ices bid for lal	oor.			, D, E, and ensured in i dikes and contingent	Total approx		
	ontin- stural	Class F.		n nat-	C, D, E, measured nal dikes	the work, classes A, B, C D, E, and F.		
Item 5, contingent.—Framing, fitting, putting in, and fast ring round piles, 15,000 running feet.	tal cost of class E, supposing the contingent dreigner, and treigner, measured half in natural bod and half in dises and banks.	the canal and dumpi and beyond to form the perfect in si	es to make water-way ng into dikes I them so as canal banks hape, 618,280 s, (approxi-	Cost of class F; balf dredging measured in dikes or banks and half measured in nat- ural bed or cut.	Total approximate cost of classes A, B, C, D, E, and F, supposing half of the drasging measured in natural bed and half measured in canal dikes and banks, including, by the same rule, contingent dredging in class E.	If all dredging measured in dikes or banks, including contingent dredg- ing in class E.	If all dredging measured in natural bed, including contingent dredging in class E.	
Item 5, contingen putting in, and 15,000 running	Total cost of class E, gent dreigng meas bed and half in dik	Measured in the canal banks as completed, not in scows.	Measured in the natural bed be- fore dredging, not in scows.	Cost of class F; dikes or banks aral bed or cut	Total approximate er. F. supposing half natural bed and ha banks. including, dredging in class	If all dredging n banks, includin ing in class E.	If all dredging rebed, including in class E.	
Pr. r. ft. \$0 05	\$84,677 48	Pr. cubic yd. \$0 38	Pr. cubic yd. \$0 35	\$225, 672 20	\$426, 178 27	\$435, 977 47	\$406, 379 07	
04	80, 468 32	44	42	265, 860 40	470, 375 39	477, 908 19	463, 842 59	
œ	64, 393 32	45	35	247, 312 00	437, 226 21	469, 890 21	404, 562 21	
06	101, 037 48	39	34	225, 672 20	466, 390 21	491, 722 21	450, 058 21	
04	68, 608 01	39	331	224, 126 50	397, 125 68	414, 128 38	380, 122 98	
05	66, 279 32	42	32	228, 763 60	431, 630 80	464, 294 80	398, 966 80	
	· · · · · · · · · · · · · · · · · · ·							
	• • • • • • • • • • • • • • • • • • • •	45	371	255, 040 50				
04	67, 611 24	50	42	284, 408 80	494, 556 27	520, 687 47	468, 425 07	
08	79, 881 86	36	42	200, 941 00	429, 760 92	453, 150 72	406, 371 12	

T. J. CRAM, Col. of Eng'rs, Brevet Major General.

of all bids received and which conform to the requisitions set forth in the advertisement hereto attached.

T. J. CRAM, Col. of Eng'rs, Brevet Major General.

tisement, and that the bidder, after being apprised of the defect, has not compiled, although this has been de-

B 17.

ENGINEER DEPARTMENT,

Washington, August 17, 1867.

GENERAL: Your letter, enclosing an abstract of bids for the St. Clair flat improvement, made in conformity with engineer department letter of July and the terms of your advertisement of July 23, was received at this office of the 16th instant.

You will make the awards of the contracts by classes to the lowest responsible bidders for each class, and if any should fail to comply with the terms of their bids, you will award to the next highest responsible bidders, with the

understanding that the delinquents will be held accountable to the United State

to the extent of the guarantee accompanying the bids.

Your attention is also called to the concluding paragraph of department lette to you of the 21st May last, viz: "You will enter into contracts only for a much of the material as may be advantageously used in executing the plane improvement to the extent practicable with the appropriations made for inamely, \$230,000, leaving sufficient of that sum for the contract for work to be done, including dredging.

"Any other course would result in procuring a large amount of materix without the means of putting it in place until Congress should make further

appropriations."

Very respectfully, your obedient servant,

I. C. WOODRUFF, Lieut. Col. of Engineers, Brevet Brig. Gen. U. S. Army, Assistant in charge.

Brevet Major General T. J. CRAM, U. S. A., Colonel of Engineers, Detroit, Michigan.

B 18.

Engineer Department,
Washington, August 28, 1867.

GENERAL: Your letter of the 23d instant, enclosing contract in duplicat entered into with John Brown, of Thorold, Canada West, with bond for th faithful performance of the same, for material for the St. Clair flats improve

ment, and contracts in duplicate with the same, with bond, for labor for the same improvement, has been received.

The contracts are approved, and you will notify the contractor accordingly Very respectfully, your obedient servant,

I. C. WOODRUFF, Lieut. Col. of Engineers, Brevet Brig. Gen. U. S. Army, Assistant in charge.

Brevet Major General T. J. CRAM, U. S. A., Colonel of Engineers, Detroit, Michigan.

B 19.

United States Engineer Office, Detroit, June 12, 1867.

SIR: Herewith I forward abstract of bids received on my invitation for dredging Sandusky river, Ohio, below the town of Frémont. See printed notice hereto

attached. Thomas Dunbar & Co. being the lowest responsible bidder, I have to request authority to make the necessary contract with him, having already notified him that he is the lowest bidder.

I have the honor to be, very respectfully, your obedient servant, T. J. CRAM,

Colonel of Engineers, Brevet Major General.

Major General A. A. Humphreys, Chief of Engineers United States Army.

To contractors.

OFFICE OF LAKE HARBOR AND RIVER IMPROVEMENTS, No. 111 Griswold street, Detroit.

Written and sealed proposals will be received at this office, addressed to the undersigned, until the 31st day of May, 1867, for dredging in the Sandusky river, below Frémont, Ohio, to obtain twelve feet water.

Places to be dredged.	Cubic yards at each place.	Width of chan- nel to be ob- tained.
1. At Whitacre bar 2. At bar below Nigger Point 3. At South Creek bend. 4. At point two and a half miles below Frémont. 5. At point three-fourths mile below Whitacre bar. 6. At point one-third mile lower down. 7. At outer bar, mouth of the river	13,176 22,916 15,503 10,333	Feet. 160 200 200 200 200 200 200 160

The earth dredged to be dumped at such places that it will not wash in so as to form another shoal.

The government reserves the right to diminish the amounts of dredging above specified, so as to keep the work within the appropriation. The work to be commenced by the middle of June, and continued during the season with dredges of power at least sufficient to raise five hundred yards per twelve hours each. State the price per cubic yard for dredging, towing, and dumping at each place specified; also, the names and residence of bidder and the two bondsmen. The material is soft for dreding, and all the places to be dredged only some few miles below the town of Frémont. For further particulars apply in person, at this office, where the map of the survey can be seen.

T. J. CRAM, Colonel Corps of Engineers, Brevet Major General.

Abstract of bids for dredging Sandusky river, Ohio.

and residence.	Names of bondsmen and residence.	Price bid for dredging per cubic yard.	Total cost, 185,075 cubic yards.
John F. Hosch, Mohawk, Her- kimer county, N. Y.	Elliot Harnun and Willard Johnson, Fulton, Oswego county, N. Y.	38 cents; all places meas- ured in dump scows.	\$ 70, 328 5
Harvey P. Platt and H.S. Stock, Toledo, Ohio.	A. C. McNair and Henry M. Claston, Cleveland, Ohio.	For all except outer bar, 331 cents; outer bar, 39 cents, measured in dump scows.	105,649 cu.yd \$35,568 50: 79,426 cu. yd \$30,976 14.
Thomas Dunbar & Co., Toledo, Ohio.		27 cents; all places meas-	\$49,970 9
	Mohawk, Herkimer county, N. Y. Harvey P. Platt and H.S. Stock, Toledo, Ohio. Thomas Dunbar & Co., Toledo,	Mohawk, Herkimer county, N. Y. Harvey P. Platt and H.S. Stock, Toledo, Ohio. Thomas Dunbar & Co., Toledo, Fillmore, Frémont, Ohio.	Mohawk, Herkimer county, N. Y. N. Y. Harvey P. Platt and H.S. Stock, Toledo, Ohio. Thomas Dunbar & Co., Toledo, Fillmore, Frémont, Ohio. Is all Johnson, Fulton, Oswego county, N. Y. A. C. McNair and Henry For all sexcept outer bar, 33‡ cents; outer bar, 39 cents, measured in dump scows. You can be described by the county of the coun

I certify this to be a true abstract of the original bids.

F. J. CRAM, Colonel Engineers, Brevet Major General.

B 20.

United States Engineer Office, Detroit, August 17, 1867.

SIR: The following special report upon Vermillion harbor, Ohio, I think my duty to make, and which will explain itself.

In my estimate for repairs and preservation of this work I made it amount to only \$15,315 94 in the (my) report of 20th February, 1865. Congressing the constant of the sum would be all that would be required to complete the repairs and preservation of the works. I have or shall soon have expended all the appropriation, and find that I have done considerable more work than I estimated for During the present summer more critical examinations have been made than was possible to make in the winter of 1865, and we have found serious damage

to the under water work, deep down, which may have been produced since the winter examination.

The old under work of west pier, much of it, was made with miserable rout timber, no better than poles, and put together in the roughest manner—no bett than a boy's "cob house." Wooden pins were used, which have been cut of the control of t

by the sharp sand moved by the current, and the logs been washed out, at stones too. Several of these large holes have been repaired, at much expens by means of bundles of brush and sheet piles, in a way that could not be do by contract, and thus far, I hope, we have been successful in preserving sever hundreds of feet of that old pier not foreseen to be in danger.

But another danger now threatens. Having stopped a 400 feet breach in or west view (estimated for its said report) and rebuilt that a current new sets it.

west pier, (estimated for in said report,) and rebuilt that, a current now sets is shore along the west side of that pier, cutting away the bank and threatening several hundred feet of the low part of that old pier, away in-shore, and which am fearful, unless repaired this season and preserved, will be found gone nesspring.

I have gone through with a careful estimate of what it will cost to save the whole of the existing old west pier, and prevent the in-shore breach, and built

the low part up to the proper height above water, and ballast it with stone, putting in all new timber that will be required—in short, doing justice to the work, and which would make it last many years to come—and find it will cost only about \$8,000 more than I shall have left of the appropriation specific. There are two reasons why I wish to do this complete repairing the present season.

1. The contractor is there ready with his force and willing to do it, straight on, and the timber contractor will, if soon notified, deliver all timber at one cargo.

The bills are all made out, and detail plans.

2. It will in all probability cost less by one hundred per cent. if preserved now than it will next year, if we wait for another specific appropriation, because the damage is going on in an increased proportion with the time of postponement.

Now I have to request authority to draw upon the appropriation of 1864 for "repairs and preservation of lake harbor works" (not requiring advertising for proposals) to the amount of \$8,000—say \$9,000 for the purpose stated in this special report—and which had better be now applied to this harbor than to any other in my district.

I have the honor to be, very respectfully, your obedient servant,

T. J. CRAM,

Colonel Corps Engineers, Brevet Major General.

Major General A. A. HUMPHREYS,

Chief of Engineers United States Army.

P. S.—I have now on hand \$9,062 88, left of the said appropriation for 1864. Some of it will be wanted, say about half, possibly, at Erie—old interior part of north pier.

B 21.

Report of the survey of Grand river harbor, made in accordance with instructions from the engineer department, of date September 19, 1866. Survey made in the month of January, 1867, by T. J. Cram, colonel engineers, brevet brigadier general.

1. The suggestion made in the instructions to make this survey "soon after the abatement of the autumnal gales, or about the last of October," was attempted to be carried out in good faith. My then assistant, Brevet Major Farquhar, captain of engineers, was sent to the harbor with instructions to make the survey, but soon returned, reporting his failure in consequence of stormy weather. The absence (on leave) of the major and the continued stormy weather prevented the survey from being made until the present month, when, the ice having formed sufficiently strong to bear a working party, I took the survey into my own hands, and completed the field-work on the 19th instant, in snow up to the waist.

The soundings were taken with the utmost accuracy, the holes having been cut through the ice and the depths of water measured exactly as represented on the accompanying map, A. The whole network was laid out on the ice in rectangles, and the holes cut accordingly. The soundings have been taken from deep water of the interior of the harbor and carried out over the bar in the flaring mouth to 15 feet water in the lake, in exact accordance with the instructions. The level of the surface of the water, when the survey was made in August, 1865, was referred to a bench-mark. On the 18th instant, (January, 1867,) by my levelling, the surface of the water was referred to the same bench-mark, and found exactly 0.314 foot lower than it was in the former survey.

Map B, accompanying this report, is a representation of the survey of August, 1865, and by comparing it with A, will enable the department to judge of the effect of the outflow in sweeping out the sand since I put this work in order by

stopping the breaches in both piers, for it was just about the time of the closing of the breaches that the survey of the summer of 1865 was made.

In making the comparison of the soundings care should be taken to apply

the number 0.314 foot properly.

2. Comparisons showing the fruits resulting from the complete repair of the

old piers.

The portion of the area of the plotted soundings limited between the lines c d and a b on map A, and C D and A B on map B, may be regarded as the horizontal projection of the bar obstructing navigation at the mouth of this harbor. The mean of all the soundings between the said limits in A is (January 1867) 9.508 feet.

The mean of all in B was (August, 1865) 8.683 feet. From the latter subtract 0.314 foot, and we shall have 8.369 feet, as the mean depth for survey B

referred to the same stage, with the survey A of the present winter.

Hence, since August, 1865, we perceive that the mean top surface of the obstruction between the said limits has been lowered by 1.139 foot, or about 13½ inches, which may be regarded as the measure of the good effect in moving away the general obstruction resulting from the repairs.

But the mean depth of water over an obstruction gives no just idea of the capabilities of the channel for navigation. The least depth, direction, and width of channel way are required to be known, before the navigator will attempt the

passage.

It will be seen that at the stage of water and least depth for A, no vesse drawing over 7½ feet can pass the obstruction, supposing no ice at the presentime.

On examination of the map B, of the survey of August, 1865, we find that then the least depth in the channel over the bar was just at the neck of the flare, where, in fact, the water was only 4½ feet deep—the outflow of the rive then being principally through the old east pier into the basin, thence through the loose sand into the lake.

The difference (3 feet) of the least depths, augmented by 0.314 foot, gives u 3 314 feet as the measure of the good effect in deepening the channel, caused by the repairs of the piers, and compelling the outflow to act more directly

upon the obstruction.

These results have been realized, under the force of the outflow of the river animated by one vernal and two autumnal freshets, since the piers were repaired

The dotted lines show the present direction of the channels over the obstruction, the most westerly one being rather the best, and more strongly characterized in depth. Between them there are indications of a middle ground forming These indications teach us that to contract the mouth, by an extension of the east pier around parallel to the west pier, is the best way to remedy the evi of the flare, which was unfortunately given to its outer part.

The width of the entrance would be ample after making the contraction, and

access easy.

In regard to the outflow of the river, referred to in the instructions: This is the greatest, and of much strength, when the winds blow from a southeasterly around by south, and from a southeasterly direction. Then the lake water is driven away from the harbor, and the current of the river has its greatest velocity. In such a blow for twelve hours, last autumn, the waters in the harbors on our shore of this lake fell from three to five feet. After the succeeding lull, the inflow is proportionally great.

When the wind blows from the west around by north, and from east, the velocity of outflow is least, and the effect of the current upon the bar is less and less in proportion to the absolute strength and duration of the gale. After

the lull, the river again resumes its effect upon the bar.

3. In conclusion, the "oldest and best-informed inhabitants" of the place

informed me that before and up to the time of commencing the flare of the west pier there was ample water for the purposes of this harbor, whether viewed in regard to the commerce then existing at the place or in the aspect of a harbor of refuge, and that immediately after the flaring position was constructed the water began to diminish in depth at the mouth, and the harbor rapidly deteriorated from having a twelve-feet entrance to the condition in which I found it in September of 1864, and as explained in my report of the 26th of that mouth. From that report I quote the following, which is just as applicable now as it was then:

"Were we to act merely to subserve a supposed existing commerce at Fairport, inasmuch as there is no commerce there demanding a harbor, we should let the whole harbor go by the board and leave it to its fate. But if it be deemed of sufficient importance, as a harbor of refuge, to justify its restoration, then it would be best to repair it to the full extent required, but not to repair in part unless we have means enough to restore the whole [meaning to include the channel] to a full degree of usefulness for this (refuge) purpose. Any other course of repairs or plan of restoration coming short of this end would be tantamount to throwing money away upon it."

I was directed to go on by the engineer department and to repair all the old piers, though I gave reasons in the same report why it would be better and cost less to abandon the old flare of the west pier and extend a work for "preservation" out parallel to the east pier. The sum of \$24,453 24 has been expended under the order. My original estimate for the thorough repairs of all the old piers ordered by the department was \$30,726. I am happy to find that all has been accomplished for less, by \$6,272 76, than the estimate, and the piers are now stronger than ever before, and the work better done by thirty-three per

cent. than was the original work.

The above expenditure has been made under the act of 1864 making appropriation for "repairs and preservation" of works. Under this wording I held that we were authorized to construct a work from an angle of the west pier straight out, parallel with the east pier, as a means of "repairing" the damage done by the sand having accumulated to block the channel entrance. No one will deny that the channel, whether natural or artificial, is the main vital part of the harbor. For "repairing and preserving" the vital part all the other parts of a harbor work are but means to an end; and upon such means as I suggested as adapted to the end of "repairing and preserving" the channel, I still think the money under that act could have been legally applied, instead of following the old good-for-nothing, nay, worse than nothing, flaring part around to its extremity. But I was overruled

around to its extremity. But I was overruled.

Foreseeing that such an expenditure would not accomplish the object with which we started, viz., the restoration to a harbor of refuge, I submitted a plan and estimate, in my annual report of 1864, for extending the east pier around parallel, as a last resort, to the flaring part of the west pier, giving reasons in full in that report and in a subsequent one, dated August 31, 1866, for assigning such a direction for the extension in preference to constructing it on the prolongation of the straight line of the east pier. It is unnecessary here to repeat those reasons. The recent survey confirms me in the belief that the direction I suggested for the extension will best contribute to the end of restoring the harbor to its original capacity for refuge and for remedying the evil produced by the flare.

Congress made an appropriation for the exact amount I estimated. The estimate was for a work denominated a close "pile pier," and not for a crib work, as the latter would cost twenty-five to thirty per cent. more. The engineer department letter of August 23 last, however, seems to favor a crib work.

The proposals I received were predicated upon the plan of the pile pier, which

I hope the department will sanction for whatever direction the extension may be made to follow.

I take it the object still is to restore the harbors to a condition for refuge. The sooner this is done the better, and the successful bidder should be contracted with immediately to proceed to furnish the piles and timber. If it be put off until the opening of spring we cannot possibly get the materials in time to make but very little of the extension next summer.

I believe I have now complied with all the requirements of the order of 19th

September, 1866.

I have the honor to be, very respectfully, your obedient servant, T. J. CRAM,

Colonel of Engineers, Brevet Brigadier General.

Brig. and Bvt. Maj. Gen. A. A. HUMPHREYS,

Chief of Engineers.

DETROIT, January 24, 1867.

SIR: I herewith transmit my report of the survey of the harbor of Grand river, Ohio, made in virtue of engineer department instructions of September 19, 1866.

Observe: The successful bidder for furnishing the piles, stone, and for doing the work, was the firm J. E. & D. E. Baily; for furnishing the square timber and lumber, N. W. Brooks & Co.; and for furnishing the iron, Cleveland Union

Iron Works. Better contractors could not be desired.

The Bailys put in, supposing the extension to be a pile pier work, but will hold to their bid, even though a crib work be made. The notice for proposals was duly published when we advertised for other works, and this formality and delay have been complied with. Any direction the department may feel authorized to give, I submit had best be done soon, or we may forfeit the advantage of the bids of the above named contractors.

This is the season for cutting and gathering the piles, and for cutting the

logs for the square timber and lumber.

Very respectfully, your obedient servant,

T. J. CRAM.

Col. Corps of Engineers, and But. Brig. Gen'l.

Brig. and Bvt. Maj. Gen. A. A. HUMPHRBYS,

Chief of Engineers.

B 22.

Engineer Department,

Washington, February 8, 1867.

GENERAL: Your report of the 24th ultimo, upon the proposed improvement at the harbor of Grand River, Ohio, accompanied by a map of your survey, made in the month of January, has been carefully considered, but the department does not concur in the proposed bend in the extension of the east pier, for the reason, mainly, that the flow outward would have a tendency to scour the bottom close along that pier only, which vessels entering in stress of weather would, of course, by the narrowing of the channel-way, be compelled to hug, and thus be forced against it, to the injury both of the vessel and the structure.

The comparison of your recent survey with that of August, 1865, leads to the conclusion that the extension of the east pier on the present line of direction, would produce a favorable result; the latter channel at the present time being

nearly on the prolongation of the east pier, resulting evidently from the repairs of the breaches in the two channel piers, which now restrict the flow of water in

a straight course into the lake. (See comparative sketch herewith)

You are authorized to contract for the materials for the east pier extension, with the distinct understanding that if for pile pier, the fact of a clay bottom be established before commencing the structure, but if the bottom be of drifting sands, then the structure shall consist of cribs with stone ballast.

Very respectfully, your obedient servant,

A. A. HUMPHREYS, Chief of Engineers.

Brevet Brig. General T. J. CRAM, U. S. A., Colonel of Engineers, Detroit, Michigan.

B 23.

United States Engineer Offick, Detroit, June 18, 1867.

Siz: I herewith transmit my report upon Dunkirk harbor, New York, the result of my examination of that harbor, and a study of the question. As soon as I receive the approval of the proper authority I will advertise for proposals, with a view of getting the work under contract as soon as possible.

I have the honor to be, very respectfully, your obedient servant,

T. J. CRAM,

Colonel Corps of Engineers, Brevet Major General.

Major General A. A. Humphrbys, Chief of Engineers United States Army.

Report upon the harbor of Dunkirk, New York, by T. J. Cram. Brevet Major General, Colonel Corps of Engineers, June 20, 1867.

The order from the engineer department of 28th of March, assigning this work to my charge, under the appropriation by act of 2d of March, 1867, directs me to follow the plan presented by Major Tardy, late of the corps of engineers, unless I dissent, in which event, by the same order, I am directed to report my views.

Having recently made a thorough inspection of the harbor, and having had certain data which I directed to be obtained by my assistant, Brevet Lieutenant Colonel F. Harwood, captain corps of engineers, returned to me in actual measurements by that officer, I am enabled now to report my views as follows:

1. I cheerfully agree with the plan proposed by the late Major Tardy in so far as the extent of the west pier, P, and the breakwater, B, and their locations and heights above water, are concerned, and seen on the accompanying tracing from the map of the survey of 1866, and in reference to which he made the estimate upon which Congress appropriated \$100,000. I also concur with him in dissenting from the further attempt to carry out the plan of three detached outer breakwater constructions to be placed in eighteen feet water, and recommended by the "board of engineers lake harbors and western rivers, July 12, 1854." What was executed, about 250 feet, at great cost, has never been of any advantage to this harbor, but of positive disadvantage to vessels in a storm seeking the entrance, nor would it be of half the advantage, if completed in toto, as would be the pier P and breakwater B.

I must, however, express my dissent from the plan proposed by Major Tardy

in regard to the thickness to be given to P and B, also in respect to building of the superstructure above water, of B, in masonry. Instead of twenty-five feet in thickness for P and B, I would make P sixteen feet and B eighteen feet, and have the superstructure of both of timber and rubble stones, and well planked over. On the lakes we have the experience of two very expensive piers in what is called stone masonry superstructure upon crib foundation. They have not lasted as well as properly constructed timber and rubble-stone work without very expensive repairs, and the interest at six per cent. per annum on the excess of first cost of the masonry over the timber and rubble-stone superstructure is much more than would be sufficient to keep the latter in perfect repair for all time. I shall go more into this comparison in my report upon Buffalo harbor constructions.

I think the true policy is to construct P first and completely, and afterwards, when we get an appropriation sufficient in advance for the whole of B, to construct it, but not to begin on B until enough has been appropriated to complete it.

These two works, P and B, completed, Dunkirk will have as good a harbor as the case admits; and with only P completed the harbor will be greatly improved, certainly enough to justify the cost. The supposing that P and B will obviate the necessity of dredging occasionally from the railroad piers out to the beaconlight passage into the outer water must not be encouraged. Artificial dredging will have to be resorted to for preserving a twelve-feet channel at low stage.

2. I have all the plans in detail and bills of materials made out for P, and

2. I have all the plans in detail and bills of materials made out for P, and shall be ready, on receiving from the engineer department an approval of my views, to put the work in advertisement for construction.

I have the honor to be, very respectfully, your obedient servant,

T. J. CRAM,

Colonel of Engineers and Brevet Major General.

Brigadier General A. A. HUMPHREYS, Chief of Engineers and Major General of Volunteers.

B 24.

United States Engineer Office, Detroit, July 25, 1867.

SIR: I send herewith an abstract of bids for Dunkirk harbor, New York, from which it will be seen that for the material and labor the lowest responsible bidders are

Both, with the bondsmen named, are responsible. As soon as the preper authority may give me instructions I will draw up the requisite articles of agreement and bonds of indemnity, and have them duly executed.

Respectfully, your obedient servant,

T. J. CRAM,

Colonel of Engineers and Brevet Major General.

Major General A. A. Humphreys, Chief of Engineers United States Army.

Abstract of bids received for Dunkirk harbor, New York.

		Prices	Prices bid for furnishing classes of material	nishing clas	1 Jo 808	naterial.		اد	[]=-{	Price	bid for cla	Prices bid for classes of labor.	or.		10 da
		Class A.	Class B.		Class C.			alteta) bas		Class D	Ď.			, and)
Number of bid.	Names and roadences of bidders and of bondsmen.	Item 1.—116 piles.	Item I to 29.—699.428	Item 1. — 42,666 lbs.	Item 2.—203 lbs. 6d. spikes.	Item 3.—24,994 lbs. 5d. splkes.	Item 4.—72 lbs. nails.	m mori—O seed of other m	Cost of clauses A, B, materials.	Jeel .7 MT, EE I med I	Item 2.—20,399 r. feet.	Item 3.—1,183 cords.	tem 4.—63,403 feet, d.m.d	Cost of class D—labor.	Entire cost of materials
	*W. H. Mott, Detroit, Mich.; M. E. Stan- dart, Z. T. W. Richardson, Cleveland,	Per pile. \$6 50 754 00	139	8 0 05. 8 05.	Per 18.	50.00	Per 18. 5 08	\$2,381.88	\$22, 838 59		Per r. foot. Per r. foot. \$0 14 \$0 12 4, 718 56 2, 417 E8	Per cord. \$15 00 17,745 00	Per M. \$7 00 443 85	\$25, 355	29 \$48, 193 88
GR.	Ohio, bondsmen. A. F. Gay, Detroit, Mich.; H. M. Lond. A. B. Loud, Ausable, Mich., bondsmen.														
es	Brooks & Adams, Detroit, Mich.; E. B. Ward, C. H. Buhl, Detroit, bondsmen.		15,686 82 28,686		_· ·										
4	Thos. J. Skidmore, Fredonia, N.Y.; O. W. Johnson, Fredonia, W. H. Briscoe,	16 00 1, 856 00		2, 986 62	8 8	10 249 40	→	3,260 64	21, 762 62	6,740 80	3,05985	4, 196 90	15 00 951 12	24, 947 77	46, 710 39
10	Andrew Spaulding, Buffalo, N.Y.; O. Frederick Balm, Lewis J. Bennett,	969 969 969	24 00 16, 306 27	2, 559 96	16 24 16 24	199 55 æ	2,4	2, 780 76	19, 783 03	12 4,044 48	9 1, 835 91	10 00 11, 830 00	9 00 570 67	18, 281 06	38, 064 09
•	Alex. McDonell, Hamilton, C.W.; W. H. Douglas, Ambrose Clark, Buffalo,	464 90	22 00 14, 947 41	2, 773 29	8 18 57	224 46 99	92	3,023 22	18, 434 63	3, 370 40	1, 631 92	9 75	14 17 788	17, 424 28	35, 858 91
~	King & Vincent, Erie, Pa.; James Hos- kinson, Elljah Babbitt, Erie, bondsmen.	1, 160 00	24 00 16, 306 27	2, 133 30	838	10 249 40	28.	2,410.20	19, 876 47	15, 055 60	3,059 83	12 00 14, 176 00	12 00 760 96	23, 072 41	42, 948 BB
	Stanton Gere, Charles E. Hubbell, Geddes, N. Y., bondsmen.			1, 911 43	13.86	169 59	4	2, 099 71							

§ Will secept the award of the whole, or classes A, B, D. | Dosines the whole, but would take the contract without item 1, class C. In letter of 13th, says whole or none.

Icertify the above to be a true copy of the abstract of the original bids.

* Will accept the award of the whole or nune bid for.

Hid for whole or none to be awarded.

This accept the award for all classes, or any class proposed for.

T. J. CRAM, Colonel of Engineers, Brevet Major General.

B 25.

United States Engineer Office, Detroit, June 27, 1867.

SIR: I herewith forward my report (commenced 20th June, and closed this morning) upon Buildo harbor, in response to engineer department's letter to me of March 28, 1867, placing the work under my charge.

It is an important subject, one which has required much physical and mental

labor, patience and perseverance, on my part, to comprehend.

I found it necessary to direct borings, soundings, and other measurements to be made, so as to know upon what and in what we have to build. These have not all as yet been returned to me. They are, however, in progress, and as soon as received I shall be able to make out detailed plans of construction and issue proposals. None of these, however, until after the receipt of the approval from proper authority of my views.

1 have the honor to be, very respectfully, your obedient servant, T. J. CRAM.

Colonel Engineers, Brevet Major General.

Major General A. A. HUMPHREYS, Chief Engineer, United States Army.

Report upon the harbor of Buffalo, New York, by T. J. Cram, brevet major general, colonel corps engineers, June 20, 1867.

In the orders March 20, 1867, from engineer department, assigning this work to my charge, it is said, in case I dissent from the plan of construction proposed

by the late Major Tardy, I am requested to submit my views.

I have recently examined this harbor with much care, and in the mean time witnessed the dangers and difficulties of vessels entering it under one of those "three days' storms" which not unfrequently occur there. I have not only witnessed for myself the practical defects and wants of the harbor, but I have consulted with the old and experienced lake navigators who have been familiar with its working since it was first improved, and with all the subsequent changes or modifications in character, kind, and dimensions of lake vessels, sail and steam, and which have been constructed to keep pace with the growth and demands of commerce. My consultations with these men have been of much benefit, begetting a spirit of discussion which has not failed to elicit much practical information that the engineer should by no means neglect, if he wishes to do justice to this highly important harbor in any future work that may be adopted for its improvement and completion.

Never before having had any official connection with the work, I had of course no bias or predilection in favor of any particular plan. Nor did I know what plans had been projected by any officer preceding me. It became my duty, however, on receipt of the order placing it in my charge, to study the question in all its bearings, not only in reference to the immediate wants of the commerce now especially tending to Buffalo, but also in reference to the wants of the future of this commerce in its demands for safety for vessels, whether intending to discharge at Buffalo, or seeking refuge there during heavy blows so

prevalent at that end of the lake.

I.—HISTORY OF THE HARBOR WORKS AND THE PLANS FOR ITS IMPROVEMENT
PRIOR TO THE PRESENT TIME.

Sketch A shows the first improvement ever made at the mouth of Buffalo creek.

In the summer of 1818, William Peacock made a survey for this harbor.

The State of New York, in 1819, loaned \$12,000 for its construction to a committee of citizens on security to execute the work.

Samuel Wilkensen superintended the construction, which was commenced in 1820, and finished in two hundred and twenty-one days, in the year 1821. south pier consisted of timber cribs, filled with stone and brush, and extended one-fourth of a mile into thirteen feet of water. The north pier consisted of a double row of piles filled with brush and sand. The work cost \$14,000, and furnished a good harbor. The first lake steamer, Walk in the Water, was lost, and the second, the Superior, was built in Buffalo in 1822, and ready to enter the lake in the spring of 1823. The completion of the harbor was considered a great success, demonstrating that harbors could be built by piers at the mouths even of the small streams of value to the infaut commerce of the lakes. The completion of the harbor, and the success attending it, were the strong inducements the Erie canal commissioners had in deciding them to extend their canal to Buffalo and terminate it there, rather than leave its terminus at Black Rock, whose harbor works (called General Porter's harbor) are also shown on A. In 1826, the Congress of the United States appropriated \$15,000 for the improvement of the Buffalo harbor; upon that the United States engineer took possession and made it a government work. He broke up the top part of the pier Mr. Wilkensen had built to build upon the under water part a new superstructure; before this was put on, however, much of the Wilkensen work, left after tearing off the top, was destroyed by the first gale of wind. The government has in all, up to include 1864, appropriated and expended upon the piers at this harbor the aggregate sum of \$266,794. For this aggregate, and the sum of \$14,000, first cost of the original piers, making \$280,794, we have as a result two piers, marked S and N on sketch B; the south fifteen hundred feet in length, and the north six hundred and thirty feet in length, built with cribs of wood, filled with rubble stone under water as a foundation, and of a kind of stone and cement masonry above water. The cross sections of these show they were built with an outside shell filled with a kind of a grouting, not deserving the name of concrete, for the heart. These are now in a damaged state, and their repairs will furnish the subject of chapter IV in this report.

II .- DIFFICULTIES OF THE HARBOR.

The sand moved by the littoral current is carried along parallel with the shore, which is at the southward, forming an accretion back of the pier, and carried after a while around the head of S and deposited in a bar at the very mouth of the harbor; and frequently a current exists drawing the sand further into the harbor, which is the effect of so many draughts upon the lake water to feed the Erie canal, as are seen by the feeders (called slips) 1, 2, 3, map B. The Buffalo creek above the harbor is also drawn very liberally upon through the other slips represented on B for water which likewise goes to feed this canal. The Buffalo level of Erie canal is thus furnished with water; and the several mills some distance down towards Black Rock are also furnished with all their power through these slips, drawing a vast quantity of water during summer and winter from the harbor and creek. The outflow of Buffalo creek under such circumstances is not sufficient to sweep out the lake sand that comes around the head of the pier, and hence the harbor channel is obstructed by this sand deposit, and it always will be, however much we may extend the piering, and there are no means except artificial dredging occasionally, as it may be required, to remedy the evil, unless preventive jetties or piers are thrown out from the shore at a considerable distance south of the harbor to act as sand catchers.

The city of Buffalo is at considerable expense annually for dredging this deposit, and I do not see how this tax upon commerce can be obviated unless by

the means suggested; but this dredging is an inconvenience and expense, for so large a commercial place, of minor importance compared with other disadvan-

tages now to be explained.

Erie breakwater.—This is seen in location on map B, and was constructed at great expense by the State of New York, to cover a basin in which canal boats and other vessels in connection with the canal business could lie in safety from the great swells produced by the southwest winds, which are the prevailing, and are those under which all the difficulties of entering the harbor of Buffalo are severely felt. Previous to the construction of that breakwater a vessel coming in under these winds, if she could not round up in time to make a lee on the north side of pier S, would ground on the soft bottom near the shore north of pier N, and sustain comparatively little injury. But since that breakwater was constructed, projecting as it does so near to pier S, vessels coming in under stress by these storms are very liable either to strike on the breakwater or on the sand deposit before they can make sufficient lee. No small number of valuable vessels have been forced on the southern part of this breakwater, and destroyed sometimes, both vessel and cargo. The breakwater is stone superstructure, similar to S, resting on a crib under-water foundation. Sometimes the vessel in attempting to escape the breakwater in wearing runs head on against the pier S.

The breakwater, as well as S, are both in a much injured condition, in consequence of these collisions. Such masonry work as the pier and breakwater are constructed of have no more, if as much, capacity to resist the shocks than a well-constructed timber and rubble-stone superstructure, as is already evidenced by this stone pier, and the stone pier at Cleveland, neither of which has resisted the shocks and washing of the waves, and action of the frost, as well as

a wood and rubble-stone superstructure.

Every vessel owner and shipmaster in Buffalo, of any experience, condemns, in bitter terms, the Erie breakwater. All say that one of the improvements needed for the harbor is to remove about 400 feet of the south end of that breakwater. This would give a chance for the vessels under those southwestern gales to enter without striking. They also say, that in any well-devised plan for the improvement by enlarging the harbor, there will be no difficulty in obtaining the consent of the New York legislature authorizing the United States engineer to remove a portion of the south end of that obstruction, and use the materials, free of charge, in any work of improvement of the harbor. I agree fully with their condemnation of the south part of that structure, and I think that one has only to draw a line from the southwest into the mouth of the harbor, and he will see how difficult it would be, under a violent wind from that direction, for a vessel to obtain sufficient lee, under S, before striking the breakwater, especially as vessels have been so greatly increased in length since that work was completed.

III .- PLANS FOR IMPROVING THE HARBOR.

What I shall propose will compensate, in measure, in covering the basin from the effects of the prevailing wind just in proportion as we may take away from the breakwater, simply by an increase of the pier S, so as to leave the same security to vessels in the basin as there now is, and at the same time give us more extent of lee for vessels on north side of S, and also, at the same time, more space between the breakwater and the pier, and insure a third advantage besides, which will consist in affording more section of channel for drawing the water through to feed the canal, and, consequently, less velocity of currents to move the sand into the mouth of the harbor; or it will insure an equivalent by an increased effect of the outflow of the creek in its efforts to force back that sand. These ideas are illustrated on sketch C. Draw through any point p, on the prolongation of S, a line from the southwest, the di-

rection of the prevailing winds. It is seen this line cuts off a portion x from the breakwater, which will be found from the general equation-

$$x=\frac{2}{3}d$$

from which, by assigning a given number of feet to d, we shall find the corre-

sponding values in feet for x.

Build up a good substantial 12-inch square timber and rubble-stone extension of S, say for a length d of 600 feet, we shall find x = 400 feet. By such an extension we could, therefore, afford to cut 400 feet off from the south end of the Erie breakwater without any material injury to the security needed in the basin, and gain all of these advantages stated. If d is made 300, x will become 200 feet. In the foregoing formula and the reasons leading to it is the key to a successful and positive plan for the improvement of this harbor, in so far as the wants of commerce immediately centring at Buffalo are concerned. It will be perceived that, so far as the improvement consists merely in extending S, I concur with the plan upon which the late Major Tardy made his estimate, not, however, committing myself to the plan in respect to the mode of construction in the length of extension he estimated for.

The plan of Major Tardy (referred to in the order calling for my views in case I dissent) I find, in following down the history of this work, is the same as that proposed on the map compiled by Captain (now Brevet Brigadier General) Woodruff, in 1856, which carries on it the plan proposed on a map of a survey under Captain Williams, topographical engineer; as far back as 1839; and it was upon that plan that Major Tardy made his estimate, which plan is shown on map B, and contemplates not only an extension of S by 2,160 feet, but also the construction of an outer breakwater 3,690 feet long, both shown in blue on B, (the breakwater indicated in blue dotted on D.) This outer breakwater, I presume, was intended to cover the harbor so as to give room for refuge in comparatively tranquil water during prevailing storms. From this location for the breakwater it is easy to see that in those storms the proximity of its southern extremity (575 feet) to the extension of the pier, if both were constructed to the extent proposed in that plan, would produce a difficulty precisely similar to that now experienced in consequence of the Erie basin breakwater. The distance, 575 feet, was probably thought, in 1839, to have been ample for the wearing of vessels of the size then engaged in the commerce. But since that time lake vessels have made great strides in dimensions. Now there are many sail vessels on the lakes, little, if any, short of 300 feet in length, including bowsprit, and they will continue to increase in length. Our present large vessels would be in imminent danger in attempting to enter under a furious southwester with a passage of only 575 It should be at least 900 feet, or three times the length of vessel. an opening so wide as this would cause a tremendous swell in the harbor, under the influence of the prevailing winds and at the very time when the breakwater would be most needed. Besides, in that location of such a breakwater, vessels under those storms would not venture to run down and come up around its northern extremity to obtain a lee under it.

In reading over Captain Williams's report, as well as that of Major Tardy, on this proposed breakwater, I find cheapness of construction the chief argument for its adoption—an argument, in my judgment, of little force, unless the break-

water, when constructed, would answer all purposes.

From the foregoing objections it will be inferred that in the plan of extending the light-house pier, S, 2,160 feet, and constructing that breakwater for an extent of 3.690 feet, as represented in blue on map B, I do not concur. I would not extend S (the present light-house pier) more than 600 feet. For the purpose of refuge, however, a breakwater, B 2, should be constructed, and is seen located in full red on map D, its northern extremity, leaving an opening of 1,500 feet between it and the edge of the shoal where red buoy No. 2 stands, and having a length of about 4,000 feet, and a direction at an angle of 74° with the direction of the most violent storms, and of about 59° with the direction of the prevailing storms, and having an offing of about 2,000 feet from the twenty-feet curve of soundings. The breakwater would stand in about twenty-five feet depth of water. This breakwater to be constructed in the best manner, with timber cribs having compartments filled with rubble stone, its cross-section to rise with a batter to surface of water and then vertically six feet above high water, not with masonry, but with 12-inch square timber superstructure, filled with stone and planked over.

It will be seen by a simple inspection of the map that vessels under the most violent storms could run into the south opening and immediately come to anchor under the lee of this breakwater, or make the harbor; and that they could easily run down under prevailing winds, and wear up between the northern extremity

and the shoal, and make the harbor.

IV .- CONDITION OF EXISTING PIERS S AND N.

Having examined these, I am prepared to report upon their condition generally. Captain Tardy's annual report of 1865 sets forth the necessity of protection and repair, and considerable has since been done under his direction to this end; much, however, remains to be done, and as much of the money we have in hand as will be necessary should be applied to this purpose as follows:

have in hand as will be necessary should be applied to this purpose as follows:

1. Protect the whole foot of the lake slope of the stone pavement of the pier S by a close row of piles, and forming a crib-work interior to and connected with this row of piles, and make a superstructure of timber with cross ties, and fill with rubble stone and plank over all. This wooden superstructure to be upon the plan of my "pile pier" to some extent, and to be five feet above the surface of the water and twelve feet wide. This is an improvement upon the mode that was being followed for this protection; what has been done, however, will work into the mode now proposed, and this, if well constructed, will break the force of the sea and prevent it from washing out the stones of the pavement on that side. It will be necessary to put a close row of piles in several places along on the north face of the pier where the old piles are gone and attacks have much dilapidated that pier, and thus prevent further destruction on that side.

2. The outer portion of the stone superstructure of the north pier, N, has been destroyed, and the stones tumbled into the water. This should be rebuilt, not with masonry as before, but with a good construction of timber and rubble stone, with clusters of spring piles at the head. In these two piers, S and N, we have an example of stone superstructure, now, after the lapse of not many years, where masonry, such as it was, has to be sustained and protected by a wooden structure, demonstrating in a striking degree the want of good foresight in

building a superstructure with that kind of masonry.

The late Major Tardy gave us an estimate of the cost of a concrete masonry superstructure similar to the construction of S for his proposed extension of that pier, and also for his proposed breakwater, (represented by A and B, map B.) and made it amount to nearly \$290 per running foot of piering, making the total cost of both about \$1,697,000. We have already seen the results, in the old pier S, of a shell of masonry of stones, not very large, laid in cement, and a heart of a kind of concrete filling. No such masonry can withstand the shocks and washing of the sea, the displacement of the stone by the frost, nor the shocks of vessels against it, so effectually (experience teaches us) as a well-constructed timber and rubble-stone work. The interest on the difference between the first costs of the two modes of construction would far more than keep the latter in repair forever. Your masonry, to withstand such causes of destruction, must be of large, flat cut stone, dovetailed, bonded, dowelled, clamped, and bolted to-

gether. I have directed certain measurements and examinations by boring to be made and returned to me by Lieutenant Colonel Harwood, my assistant, which are well under way. When I receive the data I shall be able, and not before, to plan the work and estimate the cost of all that I propose, and shall recommend in this report, for the government to do for this harbor. It cannot amount to as much as estimated by Captain Tardy for the plan he advocated.

V.—CUT THROUGH FROM LAKE TO BUFFALO CREEK FOR A SHIP PASSAGE CALLED "SOUTH PASSAGE."

In chapter II, of this report, reference is made to jettees, or a work to the southward, with a view of stopping the sand accretion back of, and preventing it from passing around the end of S, and forming a deposit in the mouth of the harbor. I find an old idea now revived in the minds of many intelligent persons who have been long acquainted with the harbor of Buffalo, that a passage once properly opened in the place indicated by C, on map D, would be a decided advantage to the harbor. A canal there, 200 feet wide, with pier or piers projecting into the lake to fifteen feet water, which would require the pier on south side to be 450 feet in length, would allow water from the lake, under the prevailing winds, to flow freely into Buffalo creek, and probably improve the inner, and give more force of outflow to the creek to sweep out the deposit at the mouth of the present harbor; if the canal should be kept open, vessels in the Buffalo trade would realize greater facility for ingress into and egress from the creek. I do not think such a work would be attended with any injurious effect upon the present harbor; on the contrary, it would, for some years to come, arrest the sand and cause the accretion to form back of the south pier of C, until in time it would pass around the end of that pier and form a deposit at the lake mouth of C which would stop that channel unless artificial dredgings be resorted to, just as it is now required to remove the deposit at the mouth of the present As this is a favorite project with some, and probably would be the subject of memorial to the department unless considered in this report, I have thought it advisable to touch upon the subject, reserving any report of the cost and precise direction of the work until further data be obtained to make the estimate upon. It would not be a very costly work in comparison to the benefit it would be to the interior harbor. Buffalo creek proper, which is now much crowded and restricted in room for vessels to wind, is certainly a question worthy of careful consideration, but whether it is a work to be done by the United States or by the city is another question, upon which I will not undertake herein to express an opinion. It will be time enough for this when the data which has been ordered to be obtained shall be received.

VI.-RECAPITULATION.

It will be seen by one reading of the whole of this report that my views may be summed up as follows:

1. Repair and protect the existing piers S and N.

2. Extend the pier S by 300 to 600 feet.

3. Endeavor to get the State of New York to allow us to remove 200 to 400 feet of the south end of Erie basin breakwater. The extension of S, though we are refused to take from that breakwater, will give great help to the vessels seeking a lee under the pier to make the entrance.

4. Construct the breakwater B2, on map D, instead of the one estimated for

by the late Major Tardy.

5. Make a report in full upon project C when required.

This is the order, in my opinion, in which the improvements of Buffalo harbor should be carried on. As soon as I receive from the department an approval or disapproval of the views expressed in this report, I shall have the

data furnished which will enable me to form the plans for the construction and specifications for advertising for proposals. I do not wish to make detail plans, specifications, and estimates, before I know what the department will or will not approve in relation to the general project.

I have the honor to be, very respectfully, your obedient servant,

T. J. CRAM.

Colonel Corps of Engineers, Brevet Major General.

Major General A. A. HUMPHREYS, Chief Engineer United States Army.

Abstract of amounts full estimate for its entire and permanent completion for each work, and abstract of amounts that can be profitably expended in the next fiscal year, from July 1, 1867, to June 30, 1868, for each work.

Name of work.	For its entire and perma- nent com- pletion.	Am'nt that can be pro- fitably ex- p'd in next fiscal year.
Sea-wall, Buffalo	‡3,691 00 6,511 00	\$30,000 00 150,000 00 50,000 00 50,000 00 34,000 00 70,000 00 22,000 00 63,497 00 9,839 84 30,000 00 19,992 40 35,435 35,435 35 5,724 00 250,000 00 65,000 00
St. May 8 Hvol	363, 363 VV	00,000 0

I certify this to be a true abstract.

T. J. CRAM, Colonel of Engineers, Brevet Major General.

^{*}Cannot be reported until I get all the data for making the working plan and estimate.
† This does not include the sum of \$35,000, required in addition if the harber should be improved to allow vessels of fourteen feet draught to enter, as per my report, 1st February, 1867.
† Besides this, there will have to be appropriated \$38,622, to make this a harbor for admission and departure of vessels drawing fourteen feet water.
† The amount I estimated in my report 1st of February, 1867, to make this a harbor to allow vessels to enter and depart drawing fourteen feet water, was \$469,664. No appropriation should be expended until perfect plans can be made, requiring first surveys and examinations.

Abstract exemplifying the commerce to which each work contributes for the fiscal year from July 1, 1866, to June 30, 1867.

Amo't of revenue collected at the port during the fig- cal year as duties on im- ports.	\$554,778 40 1,1557 37 2,11557 37 33 25 104,537 48 355 44 35,629 38
Near what fort, light-house, or port of entry the work is situated.	Fort Porter; light-house on pier. Fort Porter; light-house on pier. Light house on main land, beacon on brenkwater. Light house on main land, beacon on pier—range light. Beacon on pier. Light-house on main land, beacon on pier. Light-house on main land, beacon on pier. Light-house on main land, beacon at entrance. Light-house on main land, beacon at entrance of bay. Light-house on pier. Light-house on pier. Light-house on pier. Light-house on pier. Light-house on main land at entrance of tyer. Light-house on main land at entrance of river. Light-house on main land at entrance of river. Light-house on Tawas Point, 16 miles off.
Collection district in which the work is located.	Buffalo Creek, N. Y. Buffalo Creek, N. Y. Dunkir, N. Y. Dunkir, N. Y. Berle, Fron Cuyaboga, Ohio Cuyaboga, Ohio Cuyaboga, Ohio Cuyaboga, Ohio Cuyaboga, Ohio Sanduaky, Ohio Sanduaky, Ohio Port Huron, Mich Port Huron, Mich Port Huron, Mich Port Huron, Mich Port Huron, Mich Port Huron, Mich
Total tonnage measurem'ni of all vessels enfered and cleared.	19, 127, 276 458, 295 431, 278 13, 228 13, 228 14, 328 10, 08 1, 99, 339 11, 149, 887 20, 880 11, 149, 887 11, 149, 887 11, 149, 887
Number of entrances and clearances, sail and steam vessels.	24 4 4 9 4 9 4 9 9 9 9 9 9 9 9 9 9 9 9 9
Name of work.	Bea-wall, Buffalo, N. Y. Buffalo harbor, N. Y. Erie haktor, N. Y. Erie haktor, Penn. Conneaut harbor, Ohlo Grad River harbor, Ohlo Black River harbor, Ohlo Black River harbor, Ohlo Black River harbor, Ohlo Black River harbor, Ohlo Sanduaky river harbor, Ohlo Sanduaky river murvey and improvement. Toledo harbor, Ohlo Monroe harbor, Mich Beginaw river, Mich Beginaw river, Mich Sagnuaw river, Mich Sagnuaw river, Mich Sagnuaw river, Mich Sagnuaw river, Mich Sagnuaw river, Mich Sagnuaw river, Mich Sagnuaw river, Mich

* No reply from collector. † Not obtainable. ‡ All commerce between upper and lower lakes. § All commerce from and to Lake Superior not obtained.

T. J. CRAM, Colonel of Engineers, Brevet Major General.

I certify that this is a true abstract as far as I have been able to obtain.

Abstract of contract for each class of materials or labor for each work.

Name of work.	Name of contractor and what he contracts for.
Sea-wall, BuffaloN. Y	Not yet under contract.
Buffalo harbordo Dunkirk harbordo	Not yet under contract. Alex. McDonell, for furnishing wood and stone material, and
Erie harborPenn	for doing all work; R. Nelson Gere, all iron materials. Brooks & Adams, for all timber; James Loveday, for all iron; Vincent & King, for stone and piles, and for doing all work on piers; Lee & Dunbar, for dredging bars.
Conneaut harborOhio	Kenneth McKenzie, for all timber and stone, and for doing all work on repairs of old piers; James Loveday, for iron materials.
Ashtabula harbordo	Kenneth McKenzie, for all timber and stone, and for doing all work on repairs of old piers; James Loveday, for iron materials.
Grand River harbor do	J. E. & D. E. Bailey, for piles, stone, spikes, and doing all work; Brooks & Adams, for timber; James Loveday, for iron materials.
Cleveland harbordo	J. E. & D. E. Bailey, for furnishing stone and doing all work; Patrick Smith, for piles; J. Dwight, Palmer & Wright, timber; James Loveday, iron.
Black River harbordo	Not under contract.
Huron harbordo	W. H. Mott, for all work; F. D. Ketchum, stone, spikes, and timber; Brooks & Adams, timber; Jas. Loveday, iron.
Vermillion harbordo	W. Nicoll, for all work; Standart & Richardson, timber; F. D. Ketchum, stone and spikes; James Loveday, iron.
Sandusky City harbor.do Sandusky river survey and	Thomas Dunbar & Co., for all dredging.
improvementOhio	Thomas Dunbar & Co., for dredging improvement; surveying not done by contract.
Toledo harbordo Monroe harborMich	Thomas Dunbar & Co., for all dredging. W. H. Mott, for all work; J. M. Sterling, all timber, iron, spikes, and stone.
St. Clair flatsdo	John Brown, for all work, and furnish all wood materials. Moses Hill, all iron materials for improvement straight ship canal.
Saginaw riverdo Ausable harbordo	John Brown & Co., for dredging. Carkin & Kimball, for all wood and iron materials; — —,
St. Mary's river do	work for putting in the piers, stone, and dredging. John Brown & Co., for dredging middle channel, Lake George.

I certify this to be a true abstract.

T. J. CRAM, Colonel of Engineers, Brevet Major General.

Abstract of contracts for each work, with names of contractors.

Name of work.	Names of contractors.
Sea-wall, BuffaloN. Y	
Buffalo harbordo	Not under contract.
Dunkirk harbordo	Alex. McDonell, Hamilton, C. W.; R. Nelson Gere, Syracuse, N. Y.
Erie harbor Penn	Brooks & Adams, Detroit, Mich.; James Loveday, Cleve land, Ohio; Vincent & King, Erie, Penn.; Lee & Dunbar Buffalo, N. Y.
Conneaut harborOhio	Kenneth McKenzie, Ashtabula, Ohio; James Loveday, Cleveland, Ohio.
Ashtabula harbordo	Kenneth McKenzie, Ashtabula, Ohio; James Loveday, Cleve- land, Ohio.
Grand River harbordo	J. E. & D. E. Bailey, Painsville, Ohio; Brooks & Adams, Detroit, Mich.; James Loveday, Cleveland, Ohio.
Cleveland harbordo	J. E. & D. E. Bailey, Painsville, Ohio; Patrick Smith, J. Dwight, Palmer & Wright, and Jas. Loveday, Cleveland, Ohio.
Black River harbordo Huron harbordo	Not under contract. W. H. Mott, Detroit, Mich.; F. D. Ketchum, Huron, Ohio; Brooks & Adams. Detroit, Mich.; James Loveday, Cleve-
_	land, Ohio.
Vermillion harbordo	W. Nicoli, Detroit Mich.; Standart & Richardson, Cleveland, Ohio; F. D. Ketchum, Huron, Ohio; Jas. Loveday, Cleve- land, Ohio.
Sandusky City harbor.do Sandusky river survey and	Thomas Dunbar & Co., Toledo, Ohio.
improvementOhio	Do. do.
Toledo harbordo	Do. do.
Monroe harbor Mich St. Clair flats do	W. H. Mott, Detroit, Mich.; J. M. Sterling, Monroe, Mich. John Brown, Thorold, C. W.; Moses Hill, Cleveland, Ohio.
Saginaw riverdo	John Brown & Co., Thorold, C. W.
Ausable harbordo	Carkin & Kimball, East Saginaw, Mich.
St. Mary's riverdo	John Brown & Co., Thorold, C. W.

I certify this to be a true abstract.

T. J. CRAM, Colonel of Engineers, Brevet Major General.

APPENDIX C.

U. S. Engineer Office, Oswego, N. Y., August 29, 1867.

Siz: I have the honor to transmit herewith my report of progress upon the works of harbor improvement in my charge during the year ending June 30, 1867. I also report, as far as practicable, upon the works placed in my charge this last spring, upon which little has yet been done, partly because the plans have only recently been determined upon, and partly from the obstacles which the provisions of the acts of June 23, 1866, and March 2, 1867, place in the way of executive officers, and which provisions, instead of facilitating the speedy, economical, and proper execution of the works, have, in my opinion, just the opposite effect.

Very respectfully, sir, your obedient servant,

C. E. BLUNT,

Lieutenant Colonel of Engineers, Brevet Colonel U. S. A. Major General A. A. Humphrbys,

Chief Engineer United States Army.

IMPROVING HARBOR OF BURLINGTON, VERMONT.

The appropriation made under this title by the act of March 2, 1867, was \$80,000. The appropriation of the previous year was \$27,672 20 for extension and repair of breakwater at Burlington, Vermont. A special board of engineers visited Burlington in June, 1867, and recommended the extension northwardly of the present breakwater, in a direction nearly parallel to the shore line, with a rectangular cross section of 30' width and rising 2' above extreme high water; the depth at low water along this line being about 30'. An additional length of 1,500' is desirable, and the estimate of amount needed for completion (\$333,442) is based upon this length. The amount now available will, it is hoped, be sufficient to build a length of 450'. The project of the board of engineers having been approved by the engineer department, proposals have been invited and contracts entered into, and it is hoped that some progress can be made in the work during the present season.

It has not been practicable to procure, in time for this report, the exact commercial statistics of Burlington for the last fiscal year, but it is estimated that the quantity of lumber received was about 100,000,000 feet, board measure; the gold value of the principal imports \$1,000,000; and the amount of tonnage averaging about 350,000. Burlington is a port of entry, in the district of Ver-

mont, fifty miles distant from Fort Montgomery, New York.

There are two beacon-lights upon the present breakwater, the northern of

which must be removed to the extremity of the extension.

If the extension of the breakwater is to be further prosecuted until completed, it is very desirable that the work should go on continuously, and an appropriation of \$100,000 could be profitably expended during the year ending June 30, 1869, provided the restrictions imposed by the present law are removed and some discretion allowed the engineer officer in charge. It is believed that these restrictions are entirely inconsistent both with the economical and proper execution of public works of this nature.

Abstracts of proposals and contracts are sent herewith.

IMPROVING HARBOR OF PLATTSBURG, NRW YORK.

The appropriation of \$26,000 made for this purpose by the act of March 2, 1867, being based upon the estimate of Brevet Brigadier General Reese, for the repairs of the breakwater only, contracts have been made for that purpose, and it is hoped that the work will be well advanced before the close of navigation. It being found upon examination, however, that the width of the renewed superstructure could be somewhat reduced, without disadvantage, so that no new stone would be needed, it is probable that a small balance will remain after these repairs are executed. This balance, with an additional grant, should, it is believed, be applied in the removal by dredging of a shoal lying between the wharves and the breakwater, which is a great obstruction to navigation.

A series of soundings were made by Brevet Major Allen, in January, 1867, and a report with maps sent to the engineer department. In this report dredging was recommended, the estimated cost, (based upon a depth of thirteen feet at low water) being \$84,326 50, but it is believed that, upon this basis of dredging contracts made for other harbors this year, the quantity necessary to be removed will require an appropriation of only \$20,000, besides the balance of \$2,000 or \$3,000 which may remain of the last one. Plattsburg is a port of entry, in the northern New York district. There are two light-houses on the breakwater. The nearest fort is Montgomery, at Rouse's Point, twenty miles distant.

It has not been practicable to procure this year's commercial statistics, but a good idea of the business of the place, and of the improvement required, is

given in the following quotation from a printed petition to Congress, dated

February, 1866:

"The harbor of this place is the best harbor of any extent or importance on the west side of the lake, being protected on the west, north, and east by the shores of Cumberland bay, and on the south by the breakwater. As early as 1812, Plattsburg was, and still continues to be, a military post of the general government. The government have erected extensive barracks and quarters for military purposes upon their own ground next south of the wharves and bordering upon the lake. This military post, as early as 1820, was connected with Sackett's harbor, on Lake Ontario, by a military turnpike, with a view to the protection and defence of the frontier.

"The government also, in 1838-9, erected a breakwater 855 feet long by 41 wide, of timber and stone, at a proper distance south of the wharves, to protect the harbor against south winds, which are the prevailing and strongest winds upon the lake, and the only ones to which the harbor is exposed. By the erection of the breakwater this harbor was rendered secure to vessels while

loading and unloading at all times.

"The government grounds lie immediately south of the harbor, the banks of which are high and steep, and are composed of sand, loam and clay. The banks between the wharves and the government grounds, owned by individuals, are protected by a crib work of stone and timber; but the banks of the government grounds, being unprotected and exposed to the waters of the lake, have washed away to considerable extent within the last ten years. The materials thus detached from the banks have washed down into the harbor below the breakwater and formed a sand-bar across the channel and in front of the south wharf, and been deposited generally over the entire harbor and along both wharves to such an extent as seriously to interfere with navigation in times of low water.

"The sand-bank amounts to a total obstruction of the channel for 150 feet in front of the south wharf. The depth of water in 1839, when the breakwater was built, was from ten to fifteen feet on the present location of the sand-bank. The average depth of water upon the sand-bank is now but two feet, and during

low water in summer and fall the sand-bank is scarcely covered.

"The breakwater seems to have been located upon the assumption that the channel would remain or be kept clear and unobstructed. It will be seen by reference to the maps that the former course of steamboats and vessels, till within a few years, was in a direct line from and to the wharves past the west end of the breakwater. But since the formation of the sand-bank aforesaid which now extends into the harbor 150 feet east of the south wharf, vessels arriving from and departing for the south are compelled to make a circuit around the sand-bank, which is accomplished with danger, difficulty and delay, in dark nights, and with unfavorable winds. The harbor is no less important to the commerce of the lake and no less national in its character at the present time than in 1839, when government built the breakwater. The following items of export from this port during the past season of navigation will give some idea, (though imperfect,) of the extent and amount of exports therefrom, being wholly the productions of this country, to wit:

Iron in blooms and other forms, pounds	
Nails, 29,000 kegs, pounds	2,900,000
Starch, 1,786 barrels, 1,753 bags, 230 casks, pounds	789,800
Flour, barrels	7,516
Provender, pounds	1,026,222
Lumber, pieces	2,443,906
Shingles	4,087,000
Barrels of heading	11,448
Barley, pounds	4,201,105
Oats, pounds	4,879,696

"During the season of navigation two large steamers arrive daily at evening, and most of the season after dark, en route for Whitehall and Rouse's Point, and in consequence of the sand-bar their landing and departure are effected with difficulty and delay. During a portion of the past season, for want of water, these boats were compelled to leave passengers and freight, and receive the same at the northern corner of the railroad wharf, and at times were unable to effect a landing at any point. There is also a daily line by steamboat, between this port and Whitehall, from early in the spring till late in the fall. There are four other steamers and two propellers on the lake, whose business is carrying freight and towing canal boats and barges to and from Whitehall and Rouse's Point. These steamers arrive and depart at least four times a week, receiving and discharging freight of various kinds, and each generally have in tow from six to twenty canal boats and barges. There is also a daily ferry by steamboat between Plattsburg and Burlington during the season. This port is the terminus of the Plattsburg and Montreal railroad, which brings to it a large amount of exports to be transported through the lake to market, and conveys back merchandise to persons and places along the line of the road. It is also the terminus of a plank road leading to Clinton prison and up to the Saranac river into the timber and iron regions. The village of Plattsburg contains a population of over four thousand persons, and not only the inhabitants of this village, but the whole country back for about thirty miles, are wholly supplied with merchandise, and partly with provisions, through this port. The population thus supplied is about twenty thousand."

An appropriation of \$20,000 is asked for the next fiscal year; this sum is believed sufficient for the completion of the improvement. The prevention of further injury by the construction of crib work along the shore on the public

land is a separate matter; this would probably require \$10,000 more.

Abstracts of proposals and contracts are sent herewith.

IMPROVEMENT OF OGDENSBURG HARBOR, NEW YORK.

This harbor was examined in June, 1867, by a special board of engineers, who considered the projects of improvement submitted by Brevet Brigadier General Reese, corps of engineers. These projects embraced the construction of piers for the confinement of the current of the Oswegatchie river, as well as dredging in various portions of the harbor. The report of the board, which was approved by the engineer department, recommended the expenditure of the appropriation (\$40,000) in dredging, exclusively, in various portions of the harbor and channels. It is believed that the amount available is sufficient, and no further appropriation would be needed for several years, the rate of deposit not being very rapid. The contract has been made, and it is hoped that considerable work will be done before the close of the present season.

Ogdensburg is a port of entry in the district of Oswegatchie, New York. It has a harbor light. The town is equidistant (120 miles) from two forts, Mont-

gomery, at Rouse's Point, and Ontario, at Oswego.

The commercial statistics of the year could not be obtained in time for this report. The town is the centre of a large and increasing business, which will be greatly facilitated by the dredging proposed.

Abstract of proposals and contracts are sent herewith.

IMPROVEMENT OF OSWEGO HARBOR, NEW YORK.

The operations for this object have been the repair of the United States pier and dredging.

Contracts (which were reported last year) have been made for the materials and labor required to keep the pier in order up to the close of the season of 1868. The dredging, which was carried on under a per diem contract up to June,

1867, has resulted in opening for vessels a considerable portion of the harbor

along the western end of the pier and vicinity.

The contract for dredging under the new appropriation has been made on terms somewhat more favorable than the last, and it is expected that considerable work can be done this season. The amount expended up to June 30, 1867, was for dredging, \$29,451 50; materials and labor on pier, \$5,517 14; contingencies, \$736 56.

As reported last year the repairs of the old pier must be made every year, whenever damages are caused by heavy gales and by the action of ice. The dredging, too, (though more will be done with the available funds than was anticipated.) should be continued. For these objects an additional appropriation of \$25,000 is needed for the next fiscal year.

It is believed that should this sum be granted no further dredging will be needed for several years, or until causes steadily in action shall have resulted

in a further accumulation of material requiring removal.

But, as the damage to the old wooden pier is constant, an annual expenditure for its repair and re-enforcement will be necessary. This annual expenditure is estimated at \$12,000.

A suggestion has been made, by various persons interested, that the present light-house pier should be extended northwardly into the lake some four hundred or five hundred feet, the object being to cut off the reflected action of the waves from the eastward, inside the harbor. This object is undoubtedly desirable, and I should recommend the construction of the suggested pier (which would cost about \$50,000) were it not that I think it would be found very much in the way, and possibly have to be removed should the harbor ever be enlarged by the construction of a new United States pier further out in the lake, in place of the present one. The harbor accommodations are even now very cramped for the business of the place, and would prove altogether inadequate should that business increase, as it is believed it will, so that a new harbor pier will probably become necessary before many years.

The complete business statistics of the city for the last fiscal year could not be obtained in time for this report; the value of the exports, however, during

the period, was \$1,079,320.

The place is the centre of a very large commerce by lake, railroad, and canal, principally in grain, breadstuffs, and lumber, and it is important that the harbor should be maintained in adequate condition for this object and as a harbor of refuge.

Oswego is a port of entry. There is a stone light-house on the United States pier, and Fort Ontario is on the east side of the river, within the city

limits.

The abstracts of contracts, &c., required by law, accompany this report.

IMPROVEMENT OF LITTLE SODUS HARBOR, NEW YORK.

Contracts were made (and reported) last fall for the improvement needed in this harbor, as far as the first appropriation would permit. Under these contracts work was commenced in the spring, and is still progressing, though not as rapidly as was hoped, owing to delays on the part of the principal contractor, which have not been quite satisfactorily accounted for.

Up to the 30th of June two cribs, each thirty feet in length, had been sunk, and seven others were in progress. A considerable quantity of timber was

also on hand.

The dredging machines had deepened the channel to twelve feet, low water, for a width of about eighty feet and length of about four hundred feet. These operations are still going on; and it is hoped that by the close of the season a length of pier of four hundred and fifty feet will be completed, and a practicable, though narrow, channel dredged throughout to twelve feet at low water.

The project of improvement reported last year contemplated the extension of the west pier to about 1,300 feet in length, the construction of a short east pier, and the dredging of the channel between them. Besides this, it was thought necessary to close the opening, about one thousand feet in length, between the west pier and the shore. This closure is still considered necessary.

The appropriation of \$50,000 made last spring will be expended in these operations, and it is estimated that an additional sum of \$25,000 will suffice for the entire completion of all that is needed. This completion is, however, not permanent, as the piers are of wood and must decay, and the channel will very probably fill up to a certain extent, but it is not probable that any further

expenditures will be necessary for eight or ten years.

The harbor is in the collection district of Oswego, which is the nearest port of entry, (sixteen miles,) and where the nearest light-house and fort (Ontario) are situated. Its business and commerce are now quite important, though great results are anticipated by the inhabitants from the opening of the harbor and the construction of a railroad connecting with the New York Central. There is no doubt of its value as a harbor of refuge.

A new contract for dredging by the yard has been made, and will go into effect upon the close of the previous one. Nothing had been paid out up to

June 30, except for contingencies, (\$185 28.)

The abstracts of proposals, &c., required by law, are transmitted herewith.

IMPROVEMENT OF BIG SODUS HARBOR, NEW YORK.

Contracts were made and reported last fall for the improvements needed in this harbor, as far as the first appropriation would permit. Under these contracts work was commenced in the spring and is still progressing, though not as rapidly as was hoped, owing to delays on the part of the principal contractor, which have not been quite satisfactorily accounted for.

Up to the 30th of June about 400 feet of the west pier had been rebuilt up to low-water mark, and a considerable quantity of timber for further work had arrived. The dredging machines had cut off the point of the spit, just inside the entrance, down to eight feet depth. These operations are still going on, and it is hoped that by the close of the season a length of pier of 800 feet will be

rebuilt and a practicable, though narrow, channel dredged throughout.

The project of improvement reported last year contemplated the rebuilding of the entire west pier, with a head adapted to a beacon light. The sum now available is believed to be sufficient for these and all other objects for the improvement of this harbor, and no further appropriation is asked for. In the course of eight or ten years, however, the pier, being of wood, may need repairs, and further accumulations of materials may require additional dredging.

This harbor is in the collection district of Oswego, which is the nearest port of entry, thirty miles distant, and where the nearest fort (Ontario) is situated.

There is a light-house at the entrance.

The business and commerce of the place are now quite insignificant, though great results are anticipated by the inhabitants from the opening of the harbor and the construction of a railroad connecting with the New York Central. There is no doubt of its value as a harbor of refuge.

A new contract for dredging by the yard has been made, and will go into effect upon the close of the previous one. There was expended up to June 30, \$2,215, 35 for dredging and \$550 58 for contingencies.

Abstract of proposals and contracts are sent herewith.

SURVEY AND EXAMINATION OF PULTNEYVILLE, NEW YORK.

These were made during the month of June, 1867, and a tracing showing the result of the former is transmitted herewith.

There is now no harbor worthy of the name, and, as at other points on the lake, piers and dredging must be resorted to if one is desired. These piers (an east pier 850 feet long and a west pier 900 feet long) would cost \$68,238 44, and 32,000 cubic yards of dredging, at fifty cents, \$16,000; to which add contingencies of all kinds, \$2,761 56; total cost of improvement, \$87,000; which might be granted in two consecutive equal appropriations.

The information obtained in relation to this place may be conveniently given in the form of extracts from a report made to me by my assistant, Brevet Major C. J. Allen, captain corps of engineers, and from letters from persons interested.

Major Allen reports: "The village of Pultneyville claims a population of between four hundred and four hundred and fifty. It contains two stores, one hotel, one planing mill, one tannery, (not in operation,) a custom-house, and post office.

"The receipts from customs last year amounted to (in gold) \$1,500. About 700,000 feet (board measure) of lumber imported last year, (from the Canadas,) and a few horses. Six vessels, averaging 100 tons burden, are owned in the village. About fifteen vessels (same draught) enter and depart from the port annually. A daily line of steamers touches at the place during the season of pleasure travel, en route from Lewiston to Montreal. There is nothing that can strictly be called a harbor. Several small cribs and one long wharf jut out boldly into the lake, and are perfectly inaccessible to boats in anything like a rough

"A small stream, called Salmon creek, I think, sluggish and discolored, and averaging say thirty-five feet in width, extends from the planing mill to the lake, for which distance it is said to be navigable for vessels drawing seven feet of Beyond the bridge it extends, perhaps, three or four miles up the country,

but has barely water enough to float a fishing smack.

"The banks of the stream are not liable to be washed by freshets, so that no obstructions at the mouth are to be feared from this source. Between the mill and the lake the stream winds through a swamp of muck, sand, gravel, and swamp grass; a very narrow entrance only is left for vessels, a bar having choked up the former entrance, attributable, I think, to sand carried up by northwest winds.

"To form a harbor, then, (there being no natural harbor there,) will require, first, the excavation of all that part contained within the irregular polygon (A, B. C. D. F-see sketch;) and secondly, the construction of two piers or jettees occupying about the position indicated in the drawing."

It is claimed that a railroad is to be run from Canandaigua to Pultneyville

conditionally upon these piers being built by the government.

"The nearest place of importance is Palmyra, upon the line of the New York Central railroad. A line of stages is the only means of communication between the two places at present.

"There is a blast furnace within three or four miles of Pultneyville, working ore obtained from beds in that vicinity, and which may be removed to Pultney-

ville if the harbor is rebuilt.

"Some farming produce is exported, one item of which, some forty thousand bushels of apples annually, may be mentioned. Land sells for about one hundred dollars per acre.

S. C. Cuyler, esq., United States deputy collector at Pultneyville, writes as

follows to my civil assistant, James S. Lawrence, esq.:

"The yearly trade at this port, foreign and domestic, is over \$100,000, con-

sisting of grain, lumber, and fruit.

"The arrivals of schooners and steamers were last year eight-five in number; aggregate tonnage 45,000, ranging from twenty ton vessels to one thousand. This, however, is below the average of years.

"Amount of duties paid last year \$1,500; would be vastly increased with a harbor.

"Our trade would be increased with a good harbor in various ways. We have an extensive iron-ore bed some five miles from here, where are now located two blast furnaces. Some of their iron has been shipped at this port; more would have been but for the need of a harbor. Those furnaces are now using coal brought from Erie, Pennsylvania. This is the most favorable point to obtain it. One cargo of 315 tons was landed here last week.

"This port would be the terminus of the Northern Central railroad from Baltimore, which is now completed as far as Canandaigua, twenty-eight miles from us A survey for the construction of said road to this point has already been

ordered."

Captain H. N. Throop, general superintendent of the Lake Ontario steamer

line, who resides at Pultneyville, writes me as follows:

"Pultneyville has been a port of entry for many years. It is in the collection district of Genesee. I learn from the deputy collector of the port (S. C. Cuyler) that the amount of revenue collected during the fiscal year ending 30th June, 1867, was \$1,200, received principally from duty on lumber."

As the shipments from this place for several years past have to a large extent been in small quantities by daily steamer, it is difficult to come at the quantity or value. There have however, been shipped from this place, during a season, 34,000 to 35,000 barrels of fruit, valued at \$150,000. Grain, also, has been shipped from this place to a considerable extent.

There are owned and belonging to the place, or port, schooners Rival, say 360 tons, A. Allen, 300 tons, Sylph, 250 tons, J. J. Nile, 175 tons, Union, 60 tons,

and William Doran.

The only line of side-wheel American passenger steamboats running on Lake Ontario touch at this place daily, wind and weather allowing a landing at the imperfect harbor.

The amount of imports would be vastly increased at this place by an improvement of the harbor, permitting loaded vessels to enter and discharge; consequently the amount of customs revenue would be increased in proportion.

Near this port are two blast furnaces, requiring harbor facilities for shipping

iron and receiving Pennsylvania coal.

There has been expended for harbor improvements at this place, at various times, by private enterprise, an aggregate of \$25,000 to \$35,000, but always with too much effort to do too much with too little money. The work to a considerable extent has failed to be permanent. As a harbor of refuge, this place is on the line of the largest amount of trade and navigation between Lake Ontario and the western lakes. As stated above, Pultneyville is a port of entry in the collection district of Genesee. New York.

The nearest light-house is Sodus Point, eleven miles, and the nearest fort (On-

tario) at Oswego, forty miles.

GENESEE RIVER HARBOR, NEW YORK.

During the year ending June 30, 1867, 864 linear feet of the west pier have been rebuilt, and since that date contracts have been entered into for the completion of this pier, and the construction of the east pier (nearly 2,500 feet long.) The west pier repairs will be finished in 1867, and those of the east pier will, it is expected, be commenced.

No appropriation was made for this harbor in the spring of 1867, it being supposed that the balance then on hand would be sufficient to complete the work. Should there be no failure in the contract for labor on the east pier this expectation may still be realized; but if from any cause this contract should not be fully executed, the failure would probably involve additional expense, and

require an additional appropriation.

The unexpended balance August 1, 1867, was \$58,570 59.

Charlotte is a port of entry in the district of Genesee; there is a light-house

on the west pier. The nearest fort is Ontario, at Oswego.

The commercial statistics for the last year have not been obtained, but the importance of the harbor is sufficiently shown by the fact that it is the port of the flourishing city of Rochester, and also a harbor of refuge.

Abstracts of proposals and contracts are sent herewith.

IMPROVEMENT OF OAK ORCHARD HARBOR, NEW YORK,

The projects for repairing and extending the pier and dredging the channel having been approved, contracts have been made for labor and materials, and it is hoped that something can be done this season.

The appropriation of \$87,000, made last March, is believed to be sufficient to

complete the improvement desired.

Oak Orchard is a port of entry in the collection district of Genesee, thirty miles west of the nearest light-house, (at Charlotte,) and forty-five east of the nearest fort, (Niagara.)

The business and commerce of the place are now quite insignificant. There is a good depth of water, however, in the creek, and when improved the harbor will be valuable as a harbor of refuge, and its business will probably increase. Abstracts of contracts and proposals are sent herewith.

IMPROVEMENT OF HARBOR OF OLCOTT, (BIGHTEEN-MILE CREEK,) NEW YORK.

The projects for the construction of piers and dredging in the channel and harbor having been approved, contracts have been made for labor and materials, and it is hoped that something can be done this season.

The estimate for completion of the work is \$118,000; an appropriation of \$60,000 having been made last March, an additional grant of \$58,000 for the next fiscal year is desired.

Olcott is a port of entry in the collection district of Niagara, eighteen miles

east of the nearest fort and light-house of the same name.

The business and commerce of the place are at present small, but will probably largely increase when the harbor improvements are complete. The harbor will then also be valuable as a harbor of refuge, which are few in number upon Lake Ontario. Should it be selected as the outlet of the Niagara ship canal it will acquire a great importance.

Abstracts of contracts and proposals are sent herewith.

C. E. BLUNT,

Lieutenant Colonel of Engineers, Brevet Colonel U. S. A.

Oswego, N. Y., August 29, 1867.

Abstract of proposals and contract for materials and labor for repair of Plattsburg breakwater, 1867.

Names.	White pine timber, 12 by 12 in. sq.	Round white pine timber.	Rag bolts, 14 in. in diameter, and 2 feet long.	Construction of breakwater.
S. Hart and O. J. Jennings	Per l. ft. \$0 34	Per l. ft. \$0 17	Per 1b.	Perr. ft. \$19 00
Luther Whitney, (contractor for every class)	25	10	6	12 00

Abstract of proposals and contract for improvement of harbor at Ogdensburg-dredging.

					Per	cubic	yard.			
Names.	:	Price.	Boulders and coarse gravel.	Sand and mud.	Mud.	Sand,	Muck, clay, sand, gravel, and boulders not over one cubic yard in size.	Gravel.	Boulders.	Sand and coarse gravel.
Clark & Douglass H. Van Slyck Hiram Sharp John F. Hosch J. McDonald and W. W. W R. N. Gere Peter D. Toble Albert A. Dodge, (contrac W. Doty and Cornelius Di B. F. Wilson and R. Johns Edwin Allen D. G. Fort and Joseph Ow	Vrighttor)ay	271 34 17 38	\$0 39 6 50 40 4 50	\$0 29 30 60	\$0 40	\$0 45 40	\$0 291	\$0.90	\$1 50 50	20 5

Abstract of proposals and contract for dredging in Oswego harbor, N. Y., in 1867 and 1868.

	Price per cubic yard for-								
Names.	Sand, gravel, &c.	Sand, mud, and fine gravel.	Tearing up and removing old crib and contents, and all stone within 50 feet of pier.	All other stone.	Removing old timber crib-work and con- tents, and stone over 800 lbs. in weight.	Stone under 800 lbs. in weight.	Old crib-work, and tim- ber and stone.	All materials.	Price per day of ten hours for heavy stone, old timber,
Thomas J. Strong		\$0 23	\$1 25	\$ 0 50				\$0 68	\$69 5
James M. Baker P D. Tobie		25 49			\$0.99	\$ 0 5 0			\$55 or
H. Van Slyck A. P. Grant R. R. Dodge John W. Allen A. R. Wright		33 29 244 24 49	1 00				\$0 99 9d		per bo \$60 0

Abstract of contract for improvement of Genesee River harbor at Charlotte, N.Y.

	T. Parsons.	Jennings & Hart.	Wm. Burke & Co.	R. Goraline.
Fisited pine timber, per linear foot. 12 by 12 in. square pine timber, per linear foot. Fise pisals, per thousand feet. Ene plank, per thousand feet. Evek elm plank, per thousand feet. Evek elm plank, per thousand feet. Evend iroz, per pound. Spikes, per pound. Spikes, per cond. Labor for construction of sloping cribs, each (west side). Labor for constructing wing on south end of west pier, per lin. ft. Labor for repairing breach in west pier, (80 ft. long.) per lin. ft. Construction of east pier, per linear foot.	8 75 5 00 35 00		\$0 04.7 07	\$75 00 5 00 3 50

Abstract of proposals and contracts for dredging Little Sodus and Big Sodus harbors in 1867 and 1868.

Name.	Little Sodus harbor.	Big Sodus har- bor.
Heman Van Slyck J. W. P. Allen Charles Doolitile R. Nelson Gere R. Nelson Gere R. Dodge A. P. Grant Dasiel G. Poot and Joseph Owens William J. Baker † Willard Johnson Lewis J. Rennett Cark & Donglass Asquitss R. Wright Thomas J. Strong and James H. Sherrill	\$0 30 24 29 33 251 281	Price per c. yd., \$0 35 24 29 33 325 291 29 41 42 28 28 38

^{*} Contractor, Big Sodus harbor.

Abstract of contracts for improvement of harbor at Ollcott, N. Y.

	Thomas Parsons.	Albert A. Dodge.
It by 12 inch square pine timber, per linear foot. Flaind pine timber, per linear foot. Fine plank per thousand feet, board measure. Em plank per thousand feet, board measure. Fine each. Smibbing posts, each. Em tre nails, each. Entre boils, per pound. Eag or drift boils, per pound.	17 22 00 22 00 6 25 6 00 07	\$0 06 <u>\$</u> 05 \$
inch wrought spikes Stone, pur cord broging, per cubic yard. Tusber, per linear foot. Iva and spikes, per pound.		34

[†] Contractor, Little Sodus harbor.

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	Per linear foot of pier.	8 8 88 88 88 88 88 88 88 88 88 88 88 88	: :
	Per lb. of iron and spike.	द्विक्रम क्षा क्षा क्षा क्षा क्षा क्षा क्षा क्षा	
abor.	Per linear foot of tree- allag	ਰੈਂ ਕ	
and l	Per M. feet b. m. of plank,	% <u>5</u>	
tion	Per linear foot of suubbing posts,	2 3 3	
Construction and labor.	Per linear foot of piles,	g 28 28 18 19 19 19 19 19 19 19 19 19 19 19 19 19	::
Con	Per M. feet b. m. of timber,	\$0 0 1	
	Per linear loot of tim- her,	20 00 00 00 00 00 00 00 00 00 00 00 00 0	<u> </u>
-	Per enbic foot of tim- her.	8	$\overline{\vdots}$
,b1	Dredging, per cuble yas	10	38
	Stone, per cord.	## ## ## ## ## ## ## ## ## ## ## ## ##	$\overline{\parallel}$
	8-inch wronght spike.	0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Ī
Iron.	Iron washers, per lb.	d	<u>:</u>
Ir	Hag or drift bolts, per lb.	8	<u>:</u> :
	Screw bolts, per lb.	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	<u>: :</u>
	Elm tree-nails, per		<u>:</u> :
	Elm tree-nails, each.	g a 4 - 4 m a a a a m m m m m m m m m m m m m m	
	Suubbing posts, per cubic foot,		<u>::</u>
	Snubbing posts, each.	\$7400 0740 BB BB BB BB BB BB BB BB BB BB BB BB BB	
	Snubbing posts, per linear foot,	2,64 B	<u> </u>
	Біјев, евсћ.	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	
ï.	Piles, per linear foot.	288 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	::
Lumber	Elm plank, per M, b. m.	83899 25899 838 838 839 889 889 889 889 889 889	
	Pine plank, per M, b.m.	<u></u>	
	Flat'd tim, other than pine, per M, b. m. & l. f.	<u> </u>	
	Flatted pine timber, per M, b. m.	65 65 65 65 65 65 65 65 65 65 65 65 65 6	
	Flutted pine timber, per cubic foot,	**************************************	::
	timber, per M ft., b.m. Flatted pine timber, per linear foot.	1	<u></u>
	timber, per lin. ft.	\$: : : : : : : : : : : : : : : : : : :	
-	13in, by 12in, sq. pine	<u> </u>	<u> </u>
	Names of bidders.	Dwight Keep. H. F. Cadyll and C. B. Morse. Van Wagner & Matthews Charles E. Case, (dead) Thomas Purons. Thomas Purons. Glark & Moore Glark & Moore W. W. Cremell Harrington & Moore W. W. Wright and others W. W. Wright and others Albert A Deage. Chas. A Janolds, (no guarantee) O. P. Lockwood O. P. Lockwood O. P. Lockwood W. W. Britt Shaw, Vincent & Co Charles B. Birdaull Glark & Huntington Biggs & Huntington Biggs & Huntington J. D. Cooper W. M. Prist J. D. Cooper Van Walkenburg & Poterson W. M. Prist John Hecker J. M. Collins & Co Lewis J. Bennett J. M. Collins & Co J. Pennings J. W. Gore J. Wallins & Co J. J. Farrons J. M. Gollins & Co J. J. Farrons J. M. Gore J. Wallins & Co J. J. Farrons J. M. Collins & Co J. J. Farrons J. M. Gore J. Wallins & Co J. J. Farrons J. M. Gore J. J. W. Collins & Co J. J. Farrons J. M. Gore J. Wallins & Co J. J. Farrons J. M. H. Menett J. M. H. Menett J. W. Gore	Edwin Allen
		Devige H. F. Consultation of the consultation	PAIN

Abstract of proposals for improvement of Genesee River harbor, at Charlotte, New York.

		I	umber	•			Ire	on.				La	bor.	
Names of bidders.	2 3	12 in. by 12 in. square timber, per lin. foot.	Pine plank, per M.	Elm plank, per M.	Rock elm, per M, b. m.	Round iron, per lb.	Long screw bolts, per lb.	Rag or drift bolts, p. lb.	Spikes, per lb.	Stone, per cord.	For construction of slop- ing cribs, each, west side.	For constructing wing on south end of west pier, per linear foot.	For repairing breach in west pier, 80 feet long, per linear foot.	Construction of east pier, per linear foot.
	Cto.	Cts.				<u>٠</u> .	Cts.	C:	Cu.					
W. H. Crennell	17	22	\$22 00	\$91	\$25	· · ·		0.5.	•••.					
W. J. Pace	16	20	20 00	20	30									
Parnons.		26	8 75	5	35	51			7	\$6 00	\$75 00	\$5 00	\$3 80	88
I. H. Coats	17	24	28 00	25	25				l		7.0			
ennings & Hart	16	22	27 00		17									7
R. G. Briggs	18	28	26 00	28	40									l š
. Caldwell	16	22	30 00	20	25									
W. Rankin	18	24	24 00	22	24		12	8		7 00				\$7 for 1
		~-						"				300 ft.	& 29 fo	rbal'ne
. Peterson	16	21	22 00	20	26									8 9
A. McAllister		22	22 00		24					••••				8
George Hardison	l			~~	~*		l						•••••	10 0
C. J. Strong		34	39 00	30	35								•••••	10.
F. Bebee		33	25 00		60									
C. H. Mils	19	24	25 00		30					•••••				
ohn T. Parsons			25 00		30		71	7	••••					
ewis J. Bennett	• • • •					61								
acob Bann						0.5	1 42			6 25				
eorge Tvler					· · · ·	51			7	6 70				
rati & Co						5			7	0 10				
Villiam Burke & Co			••••	• • • •										
						4.7	• • • •		7					
. S. Page			• • • • • •	••••		51			8	7 00				
. B. Benson			• • • • • •	••• <i>•</i>		61			81				• • • • • •	
ey & Ellis			• • • • • •	· • • •		·:			· · · :	7 00				
P. Pairchild			•••••			6	1		81			••••		
ames Cochrane						::			:	6 00	·			• • • • • •
L Goraline						5			7}		75 00	5 00	3 50	
.F. Parsons						51			71					
. C. Dowling								• • • •		6 50			• • • • • •	
bert McIntyre						···		••••		11 00				
F. S. Grant, Lynn						9.			12	6 20			•••••	
N. Collins & Co									8					
J. Degraw						10			12	8 00				
. Wagner				· • • •					·	6 75				
harles E. Case				· • • •		6	·	<u>-</u> -	9					
aine & Pritchard							7	7	71	j				
N. Gere			t .	1		4.6	1	1	71		1			

Abstract of contract for material and labor for constructing the Burlington breakwater.

Names of contractors.	Hemlock timber, 12 by 12in. square, per lin. ft.	Pine timber, 12 by 12 in. square, per lin. ft.	3-inch pine plank, per M feet b.m.	3-inch hemlock plank, per M feet b. m.	Tree-nails, each.	8-inch wrought spikes, per lb.	Rag bolts, per lb.	Stone, per cubic yard.	Construction per M ft. b. m. of timber and lumber used.
C. J. Degrawtimber.	\$0 14	\$ 0 17	\$30 00	\$18 00	\$0 06				
R. N. Gereiron						\$0 06. 8	\$ 0 04.95		
Luther Whitney stone .							 .	\$1 00	
Jennings & Hartlabor .					ļ				\$4 37 <u>1</u>

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Abstract
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o REPO	MI OF THE SECRE
Construction per M b. m.	2
Construction per M b. m. of timber and lumber used.	\$7 00 9 50 4 374
Construction per sq. foot of one side of break- water, (19,200 feet.)	80 80 80 80
Construction per linesr.	00 8C#
Construction per M b. m.	88 88
Construction per enble or lin, foot of timber.	\$ 0 08
Stone, per cubic yard.	20 20 20 20 20 20 20 20 20 20 20 20 20 2
Rag bolts, per lb.	#0 074 10 10 9 9 4. 95
8-inch wrought spikes, per lb.	\$0 08 100 112 112 100 6.8 6.8
Т 1996-паіл, 68ср.	\$0 06 10 10 8 8 4 4 4 7 7
3-inch hemlock plank, per M b. m.	888888888888888888888888888888888888888
3-inch pine plank, per M b. m.	#8888888888888888888888888888888888888
For portion of hemiock timber, delivered by Oct. 1, 1867, per lin. ft.	8 8 81
For portion of pine tim- ber, delivered by Oct. 1, 1867, per lin. ft.	\$ \$ \$
Pine timber, 12 by 12 in. square, per lin. ft.	\$ 2,5 2,5 2,5 2,5 3,5 3,5 3,5 3,5 3,5 3,5 3,5 3,5 3,5 3
Hemlock timber, 12 by 12 in. square, per lin. ft.	82 22 23 24 25 25 25 25 25 25 25 25 25 25 25 25 25
Names of bidders.	Walter Doty and C. Daly C. J. Degraw, (contractor for timber). George P. Woods L. B. Flatt Thomas J. Strong & Sherrill R. N. George Contractor for Iron). Benjamin H. Hine. Wright, Potter & Hubbard Luther Whitney, (contractor for stone). Jennings & Hart, (contractor for labor). A. Martin A. Chapman.

York.
Nevo
harbor,
Orchard
Oak
improving
for
contracts
9
Abstract

	N N N N N N N N N N N N N N N N N N N	C. A. Harrington and C. H. Moore, contractors for stone Albert A. Dodge, contractor for iron and construction. Thomas Parsons, contractor for timber Edwin Albert, contractor for timber Edwin Albert, contractor for timber Edwin Albert, contractor for timber Edwin Albert, contractor for timber Edwin Albert, contractor for timber Edwin Albert, contractor for timber Edwin Albert, contractor for timber Edwin Albert, contractor for timber Edwin Albert, contractor for timber Edwin Edw
	Square pine tim- ber, per lin. ft.	98 01
Lumber.	Flatted pine tim. ft.	4 41 0
	Oak piles, each.	28 28
	Snubbing posts, each,	00 9
	Elm tree-neils, each.	10 0
	Pine plank, per M b.m. Fim plank, per M b.m.	3 00 822
	Serew bolts, with 12-in, nuts, per lb.	90 08
Iron.	Rag or drift bolts, per lb.	\$0 061 \$0 051 \$0 071 \$6 47 \$0 29
	8-inch wrought spikes, per lb.	\$0 07
	Stone, per cord.	\$6 47
	Dredging, per cub.	8
Construction and labor.	Per linear foot of timber.	10 0 0 +ED 0\$
pus uc	Per lb. of iron rods and spikes.	0 0

Abstract of proposals for materials and labor for improvement of Oak Orchard harbor, New York.

	Per lin. foot of tree-nails.	៩ : : : : : : : : : : : : : : : : : :
	Per linear foot of pier, (construction.)	8 17 00 00 00 00 00 00 00 00 00 00 00 00 00
	Rag bolts, per lb.	\$ m
	Iron rods and spikes, per lb.	द्व भाग व्यावस्य मान प्र
Labor	Per linear foot of pilea.	88 28
1	Per im. foot of snabbing posts.	8 9 9
į	Per M of plank, b. m.	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
İ	Per M of timber, b. m.	
	Per linear foot of timber.	G. G. G. G. G. G. G. G. G. G. G. G. G. G
	Per cable foot of timber.	8 9
	Dredging, per cubic yard	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
	Віоле, рет согд.	8 8 8 8 8 8 8 8 9 9 9 9 9 9 9 9 9 9 9 9
	8-inch wrought spikes, per lb.	8 - 1 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2
١.	Cast-fron washers, p. lb.	8
Iron	Iron rods, per lb.	8 0
	Rag or drift boits, p. lb.	40 go agage - 1. e gat
	Screw bolts, with 14-in. nuts, per 1b.	2.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
	Elm planks, per M b. m.	2
	Pine planks, per M b. m.	2 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
1	Stringers, cross-ties, and anabig posts, per cub, ft.	g : : : : : : : : : : : : : : : : : : :
	Elm tree-nails, per M	3
	Elm-tree nails, each.	8 4 0 0 0 0 44 04000044
	gurpping bosts, esch.	\$ 1- 54 640 WWA
Lumber	Saubbing posts, per lin.	§ 8 = 8 & A
	Oak piles, each.	9 0 00 744 0
	Oak piles, per lin. foot.	8 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
	Flatted pine timber, per M b. m.	<u>8</u> 5888
	Flatted pine timber, per linear foot.	8 2 8 2 8 2 E 2 8 2 E 2 E 2 E 2 E 2 E 2
	Square pine timber, per M. b. m.	588
	Square pine timber, per linear foot	9 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
	Names	Charles S. Birdsall Will. Pirk Will. Rights & Co. Will. When burk & Co. Payre & Pritchard Thomas Clement Thomas Clement Charles J. Degraw Pratt & Co. Fratt & Co. W. W. Brigge & Huntington J. N. Collins & Co. W. W. Brigge & Huntington J. M. Collins & Co. W. W. Wright and others W. W. Wright and others W. W. Wright and others W. W. Wright and Others M. Childs A. Clarke and W. H. Douglass J. H. Childs E. Cadyll and H. B. Moore Lewis Jones Peer, (ded.) M. Haller Charles E. Case, (ded.) M. H. Mills Edwin Allen John W. P. Aller John W. P. Aller A. P. Grant, A. B.
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C 1.

Oswego, New York, June 20, 1867.

SIR: By direction of General Bache, I have the honor to transmit herewith the record of proceedings of the board of engineers, which convened in obedience to Engineer Order No. 50, at Ogdensburg, New York, on the 19th instant, for the consideration of plans of improvement of that harbor.

Very respectfully, sir, your obedient servant,

C. E. BLUNT,
Lieut. Col. Engineers, Bvt. Col. U. S. A.,
and junior member of board.

Major General A. A. Humphrrys, Chief of Engineers, Washington City.

In pursuance of Engineer Order No. 50, dated June 15, 1867, convening a board of engineers composed of Brevet Brigadier General Hartman Bache, colonel of engineers; Brevet Brigadier General George Thom, lieutenant colonel of engineers; Brevet Colonel C. E. Blunt, lieutenant colonel of engineers, the board assembled at Ogdensburg, New York, on Tuesday, the 18th of June, 1867, upon the call of its president, "for the purpose of considering and reporting a plan for the improvement of the harbor of Ogdensburg."

Present-all its members.

The board made a partial examination of the harbor, and afterwards adjourned until Wednesday, the 19th of June, at 8 o'clock a. m.

Pursuant to adjournment, the board met at 8 o'clock a. m. on Wednesday, the 19th of June.

Present—all its members.

The board completed the examination of the harbor, after which all the information in the possession of Brevet Colonel Blunt was laid by him before the board, including the following papers, viz:

1. Map of the village, harbor of Ogdensburg, by John W. Tate, civil engineer,

1846.

2. Map of Ogdensburg, "showing the termini, &c.," on a scale of 400 feet to

1 inch, dated February, 1852.

3. Report of Brevet Brigadier General C. B. Reese, captain of engineers, to the engineer department, upon the improvement of the harbor of Ogdensburg, dated November 7, 1866, with accompanying map.

After a full discussion of the subject before the board it was

Resolved, That, in the opinion of the board, the improvement of the harbor of Ogdensburg should be confined, for the present at least, to dredging the selected channels and removing therefrom all boulders, &c., to a depth sufficient for the purposes of trade, and that it will be time enough, in case the channels so deepened be not permanent, to construct piers or dikes, which, in consequence of the complications naturally belonging to the case, in addition to the necessity of maintaining two outlets from Oswegatchie river to the deep water of the St. Lawrence, should only be resorted to as the last alternative.

The business for which the board assembled being completed it was adjourned

sine die.

HARTMAN BACHE,

Colonel Engineers, Brevet Brigadier General. GEO. THOM,

Lieut. Col. Engineers, Brevet Brigadier General. CHAS. E. BLUNT, Lieut. Col. Engineers, Brevet Colonel. 0 2.

FORT MONROE, VIRGINIA, February 6, 1867.

GENERAL: In compliance with the instructions contained in your letter of December 15, 1866, requiring an examination of the harbor of Plattsburg, New York, and estimate of cost necessary to remove a shoal said to exist between the breakwater and the main land, "if necessary for the use of the harbor as a harbor of refuge," I have the honor to state that the examination has been made by my assistant, Brevet Major C. J. Allen, corps of engineers, and a copy of his report to me, with map, is herewith transmitted.

By an examination of the map, it will be seen that the harbor, though much contracted by deposits formed since the construction of the breakwater, can still be used as a harbor of refuge for vessels drawing fifteen feet of water. The shoals formed between the breakwater and wharf heads, and due no doubt to the existence of the breakwater itself as well as the wharves, have materially obstructed the use of the harbor for even light-draught vessels discharging or taking

cargoes at this port.

To remove these shoals and secure a depth of water generally that existed as shown on the map of 1844, giving a depth of say nine feet at the wharf heads, will require the removal of 25,000 cubis yards of deposit, which, estimated at fifty cents per cubic yard, will cost \$12,500. An outlay of this amount of money, in the manner suggested, would, it is thought, make the harbor what it should be for

the commercial interests of the place.

But it is understood that the department desires to improve the harbor as a harbor of refuge only. With this view I would respectfully recommend that the area of the harbor covered by the breakwater, and contained between it and the blue line shown on the map, be dredged to give a depth of eleven feet of water, to afford additional room for vessels of the largest class likely to enter this port. This will require 16,000 cubic yards of dredging, which, estimated at fifty cents per cubic yard, will cost \$8,000. This estimate is based on the supposition that a dredge can be procured on Lake Champlain, or that one can be taken into the lake from elsewhere, say through the Champlain canal. After having informed myself on these points I will notify the department.

I am, very respectfully, your obedient servant,

C. B. REESE.

Brevet Brigadier General and Captain of Engineers.

Brevet Major General A. A. Humphrbys, Chief of Engineers U. S. A., Washington, D. C.

> FORT MONTGOMERY, Rouse's Point, N. Y., January 30, 1867.

GENERAL: I have the honor to submit the following in regard to the harbor of Plattsburg, New York. The important part of the harbor, as will be seen by the tracing here forwarded, lies in the immediate vicinity of the breakwater and wharves. This I found considerably filled up by deposits of mud, sand, &c., forming, in several cases, bars which seriously obstruct the navigation of the harbor for vessels of from six to seven feet draught.

I think that, from the natural formation of the harbor, the deposits would be likely to increase steadily, though not at the rate it has since the erection of the present breakwater. The harbor is partially land-locked, with a semicircular sweep from Cumberland Head around to the mouth of the Saranac river, which latter empties into the bay a little to the north of the railroad wharf. The

winds from the south, southeast, and east, force the waters of the lake up into this bay around the north and south ends of the breakwater. This body of water is in time thrown back by the north beach, losing its velocity, and resulting in

a deposit.

The Saranac river brings down in its course a considerable amount of earth, and large quantities of saw-dust from the various saw-mills situated upon its banks and worked by the power of the stream. The current of the river, meeting that of the lake, forms eddies and counter currents, and causes the mud and saw-dust deposits in the immediate vicinity of the railroad wharf.

The waters of the lake, forced up by the wind, impinge against the breakwater on the sea-side, and seem to separate into two bodies; one passing around the north end of the structure into the harbor formed by the breakwater and the

wharves, and the other passing around the south end and into the harbor.

These bodies of water encounter each other in the immediate vicinity of the docks, and also impinge against the wharves, losing their velocities and allowing the earth brought up by them to fall to the bottom. Much of the sediment is, I think, carried up from the various islands situated south of Plattsburg, but I think that the greater part is washed from the banks of the government reservation situated south of the entrance to the harbor. Under heavy south and southeast winds the water is forced up against this shore, and, following the course of the shore line, is carried into the harbor. The gravel occasionally met with on the bars around the wharves is, I think, brought up by the in-shore ice, as it moves up in the spring, and which, upon the melting of the ice, falls to the bottom.

The present breakwater, from its position, not only greatly contracts the harbor, but is, I think, one cause of the shoal having formed so rapidly. A location further seaward, parallel to its present position, and an extension north and south, would seem to be the more desirable.

The remedies, in the order of their importance, are, I think, first, dredging out the shoals; second, protecting a portion, at least, of the shore to the south, by

a slight revetment of crib-work.

In regard to the depth of water necessary, merchants and vessel owners are of opinion that from seven to eight feet, at most, of water will be ample for the largest vessels that now enter the harbor. This depth can be attained by a small

outlay of money.

I quote a portion of your letter of December 18, ordering an examination of the harbor: "To carry out the wishes of the department, I desire you to make such examination as you can of the harbor, and indicate on the tracing sent herewith the depth of water on the shoal and its extent, and estimate the number of cubic yards that will have to be excavated to give a depth of thirteen feet water."

I was obliged to reject the map of 1844 sent me, as the harbor has undergone such changes, both natural and artificial, that an entirely new survey was needed. Thirteen feet being stated in your letter to be the requisite depth of water, all that portion under the necessary course of vessels not having this depth must be considered shoal. The red lines on the tracing bound the portion to be exce-

vated; the outer line showing the present thirteen-foot line. The southerly portion of this shoal is narrower than that to the north, but a greater depth of excavation is requisite. Immediately in front of the wharves are shoals of considerable magnitude. Were a depth of water required here simply to facilitate navigation, as it now is, the cost of excavation would be trifling in comparison with the amount required to give a depth of thirteen feet. give a depth of thirteen feet then, requires the removal of 168,653 cubic yards, at fifty cents = \$84,326 50.

A great portion of This examination was attended with many difficulties. the time the thermometer ranged from 8 to 20° below zero, with heavy wind

and snow. The severe cold rendered it impossible, in many cases, to use the instrument when it was important to do so. Knowing your anxiety to get your report in immediately, I did not delay further, but made up my map from what I had taken thus far. The soundings were all obtained by cutting through the ice and dropping the lead.

Very respectfully, your obedient servant,

CHAS. J. ALLEN,

Brevet Major U. S. A., First Lieutenant Corps of Engineers.
Brevet Brigadier General C. B. RBESE,
Captain Corps of Engineers.

C 3.

Burlington, Vermont, June 15, 1867.

Siz: By direction of General Bache, I respectfully transmit herewith the record of proceedings of the special board of engineers, which met here to-day in obedience to Engineer Order No. 43, to consider the subject of Burlington breakwater. The map or tracing mentioned therein, and which I received from the engineer department, is also enclosed. The red line marked upon it "B B," indicates nearly the direction spoken of in the record.

Very respectfully, sir, your obedient servant,

C. E. BLUNT,

Lieut. Col. Engineers, Brevet Colonel, junior member of board.

General A. A. Humphreys,

Chief of Engineers, Washington City.

In pursuance of Engineer Order No. 43, dated June 6, 1867, convening a board of engineers composed of Brevet Brigadier General Hartman Bache, colonel of engineers; Brevet Brigadier General George Thom, lieutenant colonel of engineers; Brevet Colonel C. E. Blunt, lieutenant colonel of engineers, the board assembled at Burlington, Vermont, on Saturday, June 15, 1867, upon the call of its president, for the determination of the position of the breakwater for the protection of the harbor of Burlington, Vermont, and the mode of its construction.

Present, all the members.

All the information in the possession of Brevet Colonel Blunt was laid before

the board, including the following papers, viz:

1. Copy of a survey made by Brevet Brigadier General C. B. Reese, captain of engineers, dated 1866, showing the position of the breakwater proposed by him.

2. A study of proposed crib for Burlington breakwater, with plans, elevation and section, made by Brevet Colonel C. E. Blunt, lieutenant colonel of engineers.

After a full discussion of the subjects before the board, it was

Resolved, That the position of the breakwater shall be from the north end of the old breakwater, on a line, in about 29 feet of water, parallel with the general direction of the shore, until it meets a line drawn from about the middle of the wharfage to Rock Point. That the mode of construction of the breakwater

shall be by crib-work, loaded with stone, the cross-section of which shall be rectangular, with a base of thirty feet and a height of forty feet.

There being no further business before the board, it adjourned sine die.

HARTMAN BACHE,
Col. Engineers, Brevet Brigadier General.
GEO. THOM.

Lieut. Col. Engineers, Brevet Brigadier General. CHAS. E. BLUNT,

Lieut. Col. Engineers, Brevet Colonel U. S. A.

C 4.

Engineer Department, Washington, February 19, 1867.

SIR: I respectfully return herewith the letter of Hon. Z. Chandler, chairman Committee on Commerce United States Senate, enclosing form of a joint resolution in relation to increasing depth of harbors on the lakes to fourteen feet, &c., referred to this department for report, and have the honor to submit

the following:

As no communications had been made to this department from the officers in charge of the lake harbor improvements, nor from other sources, showing an imperative necessity for increasing the depth of the harbors so as to admit of fourteen feet draught, the attention of those officers was directed to the subject upon the receipt of the resolution, and an immediate report was called for. These reports have been received. The harbors are planned for vessels drawing twelve feet, although some now admit vessels of a greater draught.

The economy and security of lake navigation would be materially enhanced by increasing the size and draught of vessels engaged therein; and it has always been foreseen that increased depths would be demanded from time to time by the growing wants of lake commerce. It is not necessary that all the harbors should be equally deep; it will be sufficient for the present that an increased depth should be secured for those where the greatest amount of shipping enters, either for cargo or for safety, and that the channels of the rivers connecting the lakes should be of corresponding depth.

The wants of commerce would be met by deepening the following harbors and channels so as to give a depth of fifteen feet at the lowest stage known. This would secure a depth of sixteen feet at average low water, and admit at all times vessels of fourteen feet draught. The estimates of cost are in addition

to the estimates for completion, submitted in previous reports:

Lake Michigan.—1. For Chicago, \$25,000; 2. For Milwaukee, \$47,000; 3. For Manitowoc, \$50,000; 4. Aux Becs Scies, \$65,000; 5. Grand Haven,

already asked for; 6. St. Joseph, already asked for.

Lake Superior.—1. For Superior City, \$50,000; 2. For Marquette, already asked for; 3. For Sault St. Marie canal, a new canal and locks must be built, and when completed the present canal and locks must be enlarged. Not estimated for.

Connection of Lake Huron and Lake Superior .- 1. For St. Mary's river, \$100,000.

Connection of Lake Huron and Lake Erie.—1. For St. Clair Flats, \$40,000. Lake Erie.—1. For Toledo, \$550,000; 2. For Cleveland, \$130,000; 3. For Erie, \$92,000; 4. For Buffalo, already estimated for.

Lake Ontario.—1. For Genesee river, \$25,000; For Oswego, \$25,000. Total for the lakes exclusive of the cost of Sault St. Marie canal, \$1,200,000. It is to be remarked that until the new canal and locks at Sault St. Marie.

with the increased depth, are built, it will be unnecessary to deepen the St.

Mary's river and the harbors on Lake Superior.

Omitting Lake Superior and St. Mary's river, the estimated cost is \$1,050,000. The reports of Brevet Brigadier General T. J. Cram, Lieutenant Colonel C. E. Blunt, and Major J. B. Wheeler, are transmitted herewith.

Very respectfully, your obedient servant,

A. A. HUMPHREYS,

Brigadier General and Chief Engineer.

Hon. E. M. STANTON, Secretary of War.

Report upon the question of increasing the depth of lake harbors so as to admit vessels of fourteen feet draught, by T. J. Cram, colonel corps engineers, Brevet Brigadier General United States Army; made in accordance with instructions from the engineer department, of February 1, 1867.

FEBRUARY 14, 1867.

I. Taking the harbors of Chicago and Buffalo for the terminal points of lake navigation, as the commerce of the lakes now stands, the depth of water which it is practicable to give and maintain at these two harbors should, in my judgment, rule for the important intermediate harbors and channels in the chain of navigable water, including the St. Clair flats, as also rule the improvements of St. Mary's river, its ship canal into Lake Superior, and the depth that is in contemplation to give on the mitre-sill of a ship canal, intended by the people of the great northwest, to be made on the American side around the Falls of Niagara. To dig out the intermediate harbors between Chicago and Buffalo, and extend their piers so as to gain depth to fourteen feet, without first carefully considering the foregoing named improvements, would, in my opinion, be a wrong

beginning.

The question arises as to which are the important intermediate harbors, or works, between Chicago and Buffalo. Their importance rests upon consideration of the magnitude, present and prospective, of the commerce and of the wants of the general navigation for refuge where the harbors are. Upon these considerations, as a stand-point, I have no difficulty in determining, in my own mind, the works within my superintendency which are entitled to receive the increased depth. They are: Eric, Pennsylvania, Cleveland, Ohio, Toledo, Ohio, St. Clair flats, St. Mary's river (in certain places) and St. Mary's canal. Although the St. Mary's river and its canal are not located exactly intermediate, they nevertheless are of too much importance, in view of the future immense magnitude of the Lake Superior commerce in its relation to the general lake trade, to be overlooked in the project now under consideration. The other harbors within my superintendency I would leave with twelve feet water wherever it is practicable to obtain and maintain that depth, the estimates for which are already in the engineer department.

The phrase "to admit vessels of fourteen feet draught" is indefinite, and this very indefiniteness will probably lead to discrepancies in the estimates of the several officers charged with estimating the cost of deepening the channels at the harbors in the various lakes. Shall the estimate be based upon a depth of fourteen feet below high stage during the season of navigation? or medium stage? or average low stage? The lowest stage of water occurring in the season of navigation is the one whose surface should be taken as the plane of reference in as-

signing the depth for the improvement.

Now, it is of the highest importance that the terminal points, Chicago and Buffalo, should have water of sufficient depth at all times of blows, so prevalent here, that vessels can enter and depart with their cargoes without detention, or risk for want of depth or width of water way. For this purpose, were it left to

my judgment, I should make the phrase to mean fourteen feet below the lowest This would insure fifteen feet at average low water, and I would known stage. assign thirteen feet below the lowest known stage in the season of navigation, for the depth at Erie, Cleveland, Toledo, St. Clair flats, and the places in the St. Mary's river and on the mitre-sill of its ship canal, which with the upper part of that river connects the navigation with Lake Superior. This would insure fourteen feet at average low water. In regard to the estimates for improving the Chicago and Buffalo harbors to the required depth, these not being within my superintendency, I doubt not will be made by the officers respectively in charge of them. Now, it must not be inferred that I advance the opinion that when the Chicago harbor is completed and completely dredged out by artificial means, the force of the outflow of its river, alone, will keep it clear to the depth proposed, especially as the city is striving to turn the course of that river into the Illinois river and to draw the waters of the lake through in the same direction. Nor do I know that the outflow at Buffalo will alone be sufficient to keep that harbor clear to the depth proposed.

The commerce, however, has already attained to such a magnitude at these points, as to justify artificial aids to the force of the outflow in successive dredgings to maintain depths in all ordinary times of fourteen feet below the lowest

stage that occurs in the season of navigation.

II.—ESTIMATES OF COSTS OF THE PROPOSED DEEPENING AND MODIFICATIONS OF BRIE HARBOR, PENNSYLVANIA.

The estimate for dredging the outer bar to obtain twelve feet, and for extending the north pier, has already been made in my annual report of 1865, and the sum appropriated to the amount of \$36,961. I make the estimate of the cost of the additional items, viz., outer dredging, inner dredging, further extension of pier, and the restoration of the old interior north breakwater, to amount to \$54,666; therefore, the cost of deepening. &c., to the standard proposed, is \$91,627.

1. Cleveland harbor, Ohio.—In my annual report of 1865 I estimated for extending both piers of this harbor, and thereby increasing its capacity, at a cost of \$59,806. I estimate the additional work, viz., dredging in order to deepen it, and a further extension of each pier by about 110 feet to preserve the channel, to cost \$38,622. This added to the foregoing will bring the total cost, in order to bring this important work to the standard proposed, at \$98,428.

2. Toledo harbor, mouth of Maumee river, in the Maumee bay, Ohio.— The amount appropriated by Congress last session was \$20,000, I make the amount required to pay for dredging the channel, in addition to that contemplated with the above sum, to a greater width and depth, straight out, in a northeasterly direction from fifteen feet water in the river, with a channel width of 300 feet to sufficiently deep water in the lake, dumping in the north channel so as to dam that, and force all the water through the new channel, to be \$449,664; this added to the \$20,000, will bring the improvement to the

standard proposed, and to cost \$469,664.

3. St. Clair flats, Michigan.—At its last session Congress appropriated for this \$80,000. In my "revised report" of December 10, 1866, I estimated the cost of improving the existing crooked channel to as high a degree as the case admitted, at \$383,293, and no modification of the plan can be made if we follow this old channel; and as it contemplates thirteen and a half feet of water, it will be seen that the above sum is the estimated cost of bringing the work to the contemplated standard. But, in the same report I presented reasons for making the improvement by a direct short ship canal across the flat, in preference to expending any more money for dredging the existing channel; and I estimated the cost of the canal at thirteen feet deep at lowest stage, 300 feet wide between banks, and the banks to be revetted and raised vertically five feet above the water, and to be fifty-eight feet wide on top, and to bear trees, at the total sum

of \$428,754. Hence it will be seen that this is the estimate of the cost of this mode of improving the navigation to the standard depth contemplated without other modifications.

4. St. Mary's river, Michigan.—Congress appropriated at its last session for this the sum of \$50,000. In my report upon this improvement, August 23, 1866, five places were considered requiring improvement, costing \$423,983. Recently I have learned of another place in the channel, just below the canal, where over the natural bottom, for an extent of 300 feet channelwise, the water was only ten feet eight inches to eleven feet deep last summer; the bottom here is hardpan. The cost of removing this obstruction will be \$13,334, according to the best data I can now obtain; this added to the foregoing sum, will make the estimate for the river, exclusive of the canal, to bring the navigation to the

standard proposed, to the sum of \$437,317.

5. St. Mary's ship canal.—The depth of water on the mitre-sill of this important work last summer was only ten and two-thirds feet. By the process of flooding, however, vessels passed drawing eleven feet. That stage of water may be regarded as the lowest, and it affords good data for determining the precise modification necessary to adapt the canal to the depth of thirteen feet below the lowest stage, the same as contemplated for the proposed canal across St. Clair flats. I have made myself acquainted with the details of construction exhibited by the official drawings of the St. Mary's canal. I find that it admits of being excavated throughout its length, and the bottoms of the locks of being made to occupy a lower position, so as to give thirteen feet of water on the mitre-sill, without destroying any of the existing work, except some immaterial, and the grillage part of the lock foundations. These foundations may be removed and new foundations and under walls put in under the present lock walls without destroying the latter. The practical engineering for these modifications is quite simple, and safe against the destruction of the existing works.

But the full and complete estimate of the cost cannot be made until after a personal inspection of the work itself; and after that it would take two weeks to make up the estimate. The modifications could be accomplished probably in one year. But in the mean time, for the season of navigation what would become of Lake Superior trade? This is a question difficult to reconcile with the work necessary to the modifications. The period of time hence is not equal to that which has elapsed since the conception of this work, before the Lake Superior trade will become so great that the present system of its single locks will be inadequate for the business, and an additional side canal will be demanded. Then will be the time to construct the new parts with adequate depth, and soon after the new shall be completed, to modify the old canal to the same draught. In conclusion, I express the opinion that beyond these already considered, it is not necessary for all reasonable purposes of lake navigation to attempt to give more depth of water than we now expect to obtain under the existing plans and estimates for the other harbors of less importance within my superintendency.

I have the honor to be, very respectfully, your obedient servant, T. J. CRAM,

Colonel of Engineers, Brevet Brigadier General.

Brig. and Bvt. Maj. Gen. A. A. HUMPHRHYS,

Chief of Engineers.

OFFICE LAKE SURVEY, Detroit, February 4, 1867.

SIR: In reply to your circular of the first instant, I have no hesitation in saying that the general commerce of the lake would be greatly benefited if vessels having a draught of fourteen feet could be used. To do this, however, the

first point to be attained is to secure that depth in the communications between the lakes, and from Lake Ontario to the ocean; that is, in the St. Mary's canal; the flats of Lake George, St. Mary's river; St. Clair flats; around the Falls of Niagara, and the Canadian canals around the falls in the St. Lawrence river. None of these works being under my charge, I know nothing of the probable expense that would be incurred. Until these improvements are made, it would be neither necessary nor expedient to secure a greater depth of water in the harbors than has heretofore been proposed.

There are many of the harbors on the lakes where, owing to the small volume of water in the streams forming them, it would be inexpedient to attempt getting a greater draught than now obtains, as it could not be maintained without

constant dredging.

I beg leave respectfully to submit the following in regard to the several har-

bors for which I have been called on to make estimates:

Superior City.—The additional length required to carry the proposed piers to the depth of fourteen feet would not exceed two hundred feet on each pier, and the additional expense would not probably exceed \$25,000. The water in the St. Louis river, which forms the harbor, is abundant to maintain the depth proposed.

Ontonagon.—The stream is not believed to be sufficient to preserve the ad-

ditional depth even if the piers were carried out.

Engle harbor.—The estimate submitted was for the depth of water in the channel of fourteen feet. This was considered necessary to enable vessels drawing twelve feet to enter when there was any sea running, the entrance being exposed and the bottom rock. To give the same facilities if vessels drawing fourteen feet were used, there should not be less than sixteen feet in the entrance. This would treble the amount of rock excavation, and involve an additional expense of not less than \$292,032.

Lac La Belle.—The harbor at this place is a small interior lake in which is ample depth of water. The improvement is a canal connecting this with Lake Superior and piers to protect the entrance. These piers were to be carried to sixteen feet of water; it is not probable that they have been founded sufficiently deep to admit of dredging between them to the depth proposed, nor is it at all probable that the flow of water from the poud would be sufficient to maintain a depth of water in the channel of over twelve feet. It would be necessary to have additional data in regard to the amount of discharge before the expediency of attempting to get greater depth in the channel could be determined upon. The proposed improvement if made would probably cost \$250,000, if it should become necessary to rebuild the piers.

Marquette.—The proposed improvement at this point consists of a breakwater to afford shelter from the force of the lake waves. The depth of water behind this proposed breakwater being greater than fourteen feet, no additional

expense would be incurred.

As Sable river, Lake Huron.—The stream is not sufficient to maintain a greater depth than that which has been proposed.

Very respectfully, your obedient servant,

W. F. RAYNOLDS,

Brevet Colonel U. S. A., Major Engineers.

Major General A. A. Humphreys, Chief of Engineers U. S. A., Washington, D. C.

Boston, February 13, 1867.

SIR: In compliance with request contained in department letter of 1st instant, to report my views as to the necessity or expediency of increasing the depth of

the lake harbors under my charge, so as to admit vessels of fourteen feet draught

of water. I have the honor to report as follows:

These harbors are Oswego, Great Sodus and Little Sodus, N. Y. All the information which I have obtained up to this time leads me to believe that a depth of twelve feet has been heretofore considered ample. Of course a harbor which will admit a draught of fourteen feet is better than one of less depth, and there may be circumstances (such as the construction of a ship canal around Niagara Falls) which would so develop the lake commerce as to render advisable the building of larger vessels and an increased depth of water to accommodate But in view of the uncertainty of further appropriation and the actual condition of things, I cannot recommend that a depth over twelve feet should be attempted in the dredging intended with the funds now available for these harbors, and I feel very doubtful whether any part of the additional amounts recommended in my last report (viz: \$60,000 for Oswego, \$50,000 for Little Sodus, and \$80,000 for Great Sodus) should be used in obtaining a greater depth than twelve feet. If, however, the sum of \$25,000 in addition to these amounts were granted this year for each harbor, I should recommend that the increased depth required for fourteen feet draught be now given.

Very respectfully, sir, your obedient servant,

C. E. RLUNT,

Major Engineers, Brevet Lieut. Colonel U. S. Army.

General A. A. HUMPHREYS,

Chief Engineer U. S. Army, Washington City.

MILWAUKEE, WISCONSIN, February 6, 1867.

General: I have the honor to acknowledge the receipt of engineer department circular dated February 1, 1867, asking for a report upon the expediency of increasing the depth of the harbors on Lake Michigan so as to admit vessels of fourteen feet draught.

My opinion is that, as a general rule, the principal harbors on Lake Michigan

should be deepened so as to admit vessels drawing fourteen feet of water.

Vessels, generally propellers, drawing, when loaded, thirteen feet, have entered and gone out of the harbors of Chicago and Milwaukee during the year 1866, and at Racine, during the same period, a line of propellers drawing twelve and one-half feet have entered and left.

The commerce on this lake has increased immensely during the past few years, far beyond the most sanguine expectations of twenty years ago, when

twelve feet of water was the usual depth selected.

This increase has augmented the number of vessels employed, and enlarged

the size until a draught of twelve feet is not uncommon.

This increase in size is due to the immense grain trade of this country, and is limited only by the depth of the harbors and channels which are used by the carriers.

As an evidence of the greatness of this carrying trade, I will state that during this year, 1866, from the port of Milwaukee alone there were shipped by lake vessels 10.939,561 bushels of wheat, (nearly eleven millions,) nearly 500,000 barrels of flour, nearly 500,000 bushels of corn, over one and one-half millions of bushels of oats, and large quantities of rye and barley.

From Milwaukee you can easily judge what Chicago sends by the lake. Add to these the shipments from Racine, Sheboygan, and Manitowoc on the west shore, and the fast growing trade, especially in lumber, from ports on the east shore, we see we have a carrying trade that requires a larger number of vessels

and of a larger size.

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Our harbors should conform to the necessities of the case, and if practicable, at a slight addition to their cost, be deepened to at least fourteen feet. The limit to the draught of vessels sailing to and from Milwaukee and Chicago will be governed at all times by the depth of water in the channel through the St. Clair flats. As this channel is deepened, so will the grain vessels to and from these two harbors be increased in size, and a deepening of the harbors required.

We will now consider the deepening of the harbors on Lake Michigan in-

trusted to my charge:

1. Aux Becs Scies.—Examining the map of this harbor furnished by General Cram, with his report and plan in 1864, we notice a bar with a depth of twelve feet upon it, running parallel to and distant about 800 feet from the shore. width of this bar is about 100 feet. To improve this harbor so as to admit vessels of fourteen feet draught, we have two plans before us-one to extend the piers out and over this bar, and dredge the whole water-way to the required depth; and the other to extend the piers to fifteen feet of water, dredge between the piers, and a channel through the outer bar. Making no allowance for greater depth than fourteen feet, which should be done where the vessel actually draws fourteen feet, but estimating only for this depth of water throughout, the first plan would require an extension of about 1,000 feet running of pier work, and the removal of about 18,677 cubic yards of earth and sand. The cost would be \$92,838 50. The other plan would require about 300 feet of pier work, and about the same amount of dredging. The cost would be about \$34,388 50. This latter plan would be recommended by me. I must here state that these figures are based on General Cram's map, and as several years have elapsed. since this survey, I would recommend a new examination to be made of this locality.

2. Grand Haven.—This harbor was discussed in my report dated November 30, 1866. If the south pier is extended out the 600 feet recommended, the current of the river will scour out the channel to the required depth. The probable

cost would be, in addition to the present appropriation, \$40,111 21.

- 3. Black Lake harbor.—To improve this harbor, as required above, would require an addition of 2,150 running feet of pier work and 23,889 cubic yards of dredging. The cost of this would be about \$196,469 50 in addition to what has been asked for.
- 4. St. Joseph harbor.—When the improvement recommended in my report of November 30, 1866, is made, vessels drawing fourteen feet of water can enter. The amount asked for was \$22,459 84.
- 5. Chicago harbor.—This harbor would require, in addition to the present improvements contemplated, the removal of about 5,000 cubic yards of earth and sand, costing about \$2,500.
- 6. Kenosha harbor.—This harbor would require an additional 228 running feet of pier work and 15,704 cubic yards of dredging. The cost of this addition will be about \$26,890.
- 7. Racine harbor.—This harbor would require, in addition to the amount asked for in my report before referred to, the sum of \$7,423 50 to remove about 14,847 cubic yards of sand and earth from the new channel.
- 8. Milwaukee harbor.—This harbor has its piers in fourteen feet of water, and, taking the measurement of 1865 as a guide, there will be required to remove about 16,981 cubic yards of earth and sand from between the piers, to make a channel of fourteen feet throughout. I am confident that a bar has been forming opposite the mouth of this harbor during the past year, and that it will be safe to double this amount of dredging. I would, therefore, ask for \$17,000 for this harbor.
- 9. Sheboygan harbor —This harbor would require 560 running feet of pier work, in addition to the present proposed improvement, and 19,447 cubic

yards of dredging. The cost of this additional improvement will amount to \$56,483 50.

10. Manitowoc harbor.—This harbor would require 128 feet of additional pier work, and 21,333 cubic yards of dredging. The cost of this would amount to \$21,354 66.

RECAPITULATION.

Amounts required to improve the harbors on Lake Michigan under the charge of Major J. B. Wheeler, corps of engineers, so that vessels may enter with a draught of fourteen feet.

No.	Name.	Amount asked for.	Remarks.
1 2 3 4	Aux Becs Scies	\$34, 388 50 40, 111 21 196, 469 50 22, 459 84	Already asked for. Already asked for.
5 6 7 8 9	Chicago Kenosha Racine Milwaukee	2,500 00 26,890 00 7,423 50 17,000 00	·
10	Sheboygan	56, 483 50 21, 354 66 425, 080 71	

I am, general, very respectfully, your obedient servant,

J. B. WHEELER,

Major of Engineers.

Major General A. A. HUMPHREYS,

Corps of Engineers, Chief of Engineers U.S.A., Washington, D. C.



APPENDIX D.

St. Paul, Minn., September 14, 1867.

GENERAL: I have the honor to submit the following report of the operations conducted by me for the year ending June 30, 1867:

I was assigned to this field of duty by order from the engineer department, dated July 31, 1866. The duties were the examination and survey of the Mississippi river and its tributaries, between the Falls of St. Anthony and the Rock River rapids, viz., the Minnesota, St. Croix. Cannon. Zumbro, and, also, of the Fox and Wisconsin, as provided for in the act making appropriations for the repair, &c., of certain public works of the United States, approved June 23, 1866. This act stipulates that the navigation shall be made for vessels drawing four feet of water, economizing the water by dams, locks, sluice-ways, and all other means calculated to insure the passage of steamers of four feet draught at all navigable seasons. The subject of constructing railroad bridges across the Mississippi river between St. Paul and St. Louis, at such places, and upon such plans of construction, as will offer the least impediment to the navigation of the river, was also confided to me. A report on these matters was required, by the act before named, in time to be laid before Congress at its next session.

These duties were immediately entered upon by me, and a preliminary report,

as complete as could be made at the time, was rendered to the engineer department under date of January 21, 1867. This, with the report of the Chief of Engineers, was printed as H. Ex. Doc. No. 58, 39th Congress, 2d session.

As probably always occurs in such hastily prepared and hastily copied and printed reports, some serious errors appear in the printed copy, so as to make it a somewhat unreliable reference for the information designed to be conveyed. Thus, at the bottom of page 26, I am made to speak of "encouraging the water," when I said "concentrating the water." At the bottom of page 22, the print is "17,000 cubic yards," and "2,300 cubic yards." It was meant to read 17,000 cubic feet per sec., and 2,300 feet per sec. The diagram on page 69 was reduced one-half (\frac{1}{2}) its linear scale in the publication, but the scale is given in the text as it was on the original. Numerous other corrections might be made, but these will suffice to put any one on their guard in consulting the printed document.

I will here repeat the estimates submitted by me which were approved by the

Chief of Engineers:

1.—For improvement of the Mississippi river.

For lock and dam at Meeker's island	\$ 235, 665	00
For building and operating two dredges and snag-boats	96,000	00
For experimental dam at Prescott island	5, 000	00
For experimental dam at Wacouta channel	3,000	
For experimental beacons		00
Total first year	340, 465	00

2.—For improvement of the Minnesota river.

From Yellow Medicine to its mouth, by dams and locks giving four feet water, two hundred and thirty-seven (237) miles, as follows:

Total	775, 500	00
Little rapids to Mendota, 31½ miles	4, 500	00
Upper Le Sueur to foot of Little rapids, 39½ miles	126, 000	00
Mankato to Upper Le Sueur, 31½ miles	160, 000	00
Fort Ridgely to Mankato, 73½ miles		
Yellow Medicine to Fort Ridgely, 60 miles		

A desirable improvement by the following, securing two or three feet water,

was estimated as ionows:	
For removing snags and boulders throughout	\$37,000 00
For dam and lock at Little Falls	
For expense annually of scraper and dredge boat	20,000 00

Total for Minnesota river, giving 2 to 3 feet draught.... 117,000 00

For improving the Wisconsin river.

For constructing dredge and snag-boat	\$ 25, 000	00
For operating same for one year	15, 000	00

For additional surveys.

For Mississippi, above Falls of St. Anthony	\$ 7,500 00
rapids	35,000 00
For the Wisconsin river	
Total	50,000 00

By the act approved March 2, 1867, appropriations were made as follows:

1.-For the Mississippi river.

For building and operating two dredge and snag-boats..... \$96,000 00

2.—For the Wisconsin river.

For building and operating one dredge and snag-boat...... \$40,000 00

3.-For the Minnesota river.

No special appropriation was made for the surveys estimated for, but a general appropriation in its stead of \$125,000 for examination and survey of western and northwestern rivers, and section five (5) of this act directed the surveys above estimated for and numerous others to be made out of this amount. The letter to me from the Chief of Engineers, of May 16, 1867, set apart the liberal allowance of \$40,000 of this appropriation for surveys and examinations under my charge.

The full amount of my estimate, \$50,000, could not then be promised, in view of other surveys of great importance required to be done by this appropriation. The amount allotted to me, \$40,000, has proved as much as I could advantageously make use of this season, which has been one exceedingly unfa-

vorable for surveys on the upper Mississippi, on account of high water.

I will now resume the report of operations since the date of the published

preliminary report, dated January 21, 1867.

Owing to want of means, the parties engaged in office work were reduced soon after, as fast as the maps were placed in condition to easily be completed by others hereafter. Two assistants had, however, to be kept continuously on the Minnesota survey, and five on the Mississippi surveys. By orders from the engineer department of March 22, subsequently modified by one of March 26, I was made a member of the board on the improvement of the Des Moines rapids, to meet at Keokuk, April 15, 1867. I was engaged personally on this duty until May 15. After this, by authority, I visited St. Louis and Cincinnati, to examine steamboats, with a view to procuring a suitable one for dredging and snagging on the upper Mississippi and Wisconsin rivers.

I returned to St. Paul on the 9th of June. At this time the Mississippi river was, at this place, thirteen and a half feet above low water, and rising rapidly. It continued to rise till June 14, being then fifteen and a quarter feet above low water. By June 23 it had fallen to twelve feet above low water. It then began to rise again, and on June 30 was sixteen feet above low water. On the 9th of June the Wisconsin river, at Portage, was five feet above low water. It continued to fall throughout the rest of this month, being at its close two and a half feet above low water. So far, all the streams were too high to carry on advantageously any surveys other than gauging them, and this

was begun at once.

The preparation of drawings and specifications for different kinds of boats

required was also urged forward. On the 22d of June an advertisement was made for proposals—first, for selling to the United States steamboats to be used in scraping the sand-bars; second, for wrecking the steamer Northern Light; and, third, for removing snags and boulders from the Minnesota river—as it had become obvious that we should not be able to do anything this season with boats of our own construction, and it was hoped private parties might be able to perform the work advantageously to the public with means already provided. A favorable contract for removing snags and boulders from the Minnesota was ultimately made with Sanford A. Hooper, of Belle Plains.

The results of further operations do not come within the fiscal year ending June 30, and as my time now, in this most active period, does not allow for a full report thereon, and as the results themselves will not be fully determined till the close of the season, I deem it best to defer any further report of operations

till then.

All the works in my charge are being urged forward with all the speed consistent with accuracy and due regard for economy.

The surveys on the Mississippi and Wisconsin are progressing, and on the

latter will be completed this season.

The following table gives the expenditures made by me from appropriation for examination and survey of western and northwestern rivers, for the year ending June 30, 1867:

Applied to—	3d quarter, 1866.	4th quarter, 1866.	1st quarter, 1867.	2d quarter, 1867.	Total.
Mississippi river		\$14,504 29	\$5,406 03	\$3,901 61	\$26,727 74
Minnesota river		3,654 22	1,034 55	990 40	6,018 42
Cannon river		1,019 80	144 37		1, 292 76
Zumbro river Fox and Wisconsin		622 50	192 50		815 00
rivers		1,060 14	452 31		1,512 45
St. Croix river					
Grand total					36, 366 37
	I	Į.	1	, ,	

The season's work will, it is believed, enable us to complete the report on the subject of bridging the Mississippi between St. Paul and St. Louis, and the determination of a definite plan for improving the Wisconsin and parts of the Mississippi river.

Independent, however, of these plans, to be submitted in a future report, I deem it exceedingly desirable to have the following amounts appropriated for

continuing the works next year:

First, for continuing the survey of the Mississippi river above Rock	
island	\$50,000
Second, operating two (2) snag and scraper boats on Mississippi river	36,000
Third, experimental dam at Prescott	5, 000
Fourth, experimental dam to deepen Wacouta chute	5, 0 00
Fifth, dam and lock at Little falls, Minnesota river	60,000
Sixth, dam and lock at Meeker's island, Mississippi river	235, 665

The first item, of \$50,000 for surveys on the Mississippi, is very necessary. A thorough knowledge of the shifting sand-bars will require, for several years, a special survey to be repeated over the bad shoals, such as those at the mouth of the Chippewa river, so that the changes can be compared—the proper method devised for overcoming the difficulties navigation now experiences. Besides these repetitions, two more years of favorable stages will enable us to extend the

survey throughout the whole of the shoal river, and the boats and other material now on hand will enable us to much increase the work done, with the same expenditure as heretofore

The second item, for operating snag and scraper boats, will have to be provided in advance annually, if the method succeeds in producing the desired result. But it is probable that the full amount will only be required in extreme low water years, so that there will not be another amount required for the year

succeeding a high-water season, after the plan is fairly in operation

The third and fourth items, for experimental dams at the head of Prescott island and Wacouta chute, are specially desirable, as affording a test of this method of improvement applied to the upper Mississippi. I much regret that these mounts were not appropriated as estimated for last year, so that, by the end of the season, the result would be known to guide me in adopting a final report. The fifth item, \$60,000, for dam and lock at Little falls, on the Minnesota, is for an improvement demanded beyond any question; the benefit of the amount appropriated for removing the snags and boulders will be very imperfectly realized, unless the falls are also made navigable.

The sixth item is called for by the same reasons mentioned in the printed report, to which an item of such importance will render it worth while to refer.

The following table contains the names of the different rivers, the collection districts in which they are located, the nearest ports of entry, and the amount of revenue collected during the last fiscal year. I give them as required by instructions, but not as an indication of the value of the commerce benefited by the improvements proposed:

Work.	Collection district.	Nearest port of entry.	Revenue collected.
Mississippi river Mississippi river Mississippi river Wisconsin river	Minnesota Minnesota Minnesota Galena Dubuque Galena		5, 141 62 9, 950 71

Respectfully submitted:

G. K. WARREN,

Major Engineers, Brevet Major General U.S. A. Brevet Major General A. A. Humphrrys,

Chief of Engineers, U.S. A.

TREASURY DEPARTMENT, September 25, 1867.

Sin: Your letter of the 16th instant, requesting "information as to the amount of revenue collected at the following ports of entry, for the fiscal year ending June 30, 1867, viz., Dubuque, Iowa; Galena, Illinois, and Green Bay, Wisconsin." has been received.

In answer thereto, I would call your attention to an extract of a report made by the Commissioner of Customs, to whom your letter was referred, and who reports "that the amount of revenues collected at Dubuque during the time specified was \$5,141 62, and at Galena \$9,950 71. Green Bay, Wisconsin. is a port of entry of the collection district of Milwaukee, and no separate return of

revenue collected there exists in this office. The amount collected in the district of Milwaukee, which includes Green Bay, Southport, Racine, Sheboygan, and Depere, ports of entry, was \$83,815 19."

I am, very respectfully,

J. F. HARTLEY,

Assistant Secretary.

G. K. WARREN,
Major of Eng's and But. Major Gen. U. S. A., St. Paul, Minn.

APPENDIX E.

UNITED STATES ENGINEER'S OFFICE, Davenport, Iowa, September 10, 1867.

GENERAL: I have the honor to make the following report in reference to the works under my charge, as required by engineer circular No. 11, dated Washington, June 10, 1867.

IMPROVEMENT OF THE DES MOINES RAPIDS OF THE MISSISSIPPI RIVER.

Result of resurvey, with plan adopted and items of expenditure under that plan.—Having been assigned to the duty of superintending the improvement of the Des Moines rapids, by Special Orders No. 379, dated Adjutant General's office, August 3, 1866, I proceeded without delay to Keokuk, Iowa, and after a careful examination of the rapids, together with the reports and surveys which had been previously made with a view to the improvement of navigation, I determined to make a careful resurvey of the rapids, together with a section of the river above and below the same, and to consider not only the old plan of improvement, but every other one which might seem worthy of attention. The results of this resurvey, together with the details of the plan based thereupon, were communicated to the bureau in my report of January 1, 1867.

It will be remembered that the original plan of improvement contemplated widening, deepening, and in some degree straightening the original channel, by excavating the rock from the bed of the river at such points as required it. This plan was, after careful investigation, rejected as involving too much ex pense, requiring too long a time to complete, and failing when finished to give an adequate improvement. In lieu thereof I recommended the construction of a lateral navigation canal, extending along the Iowa shore from Keokuk to the village of Nashville, a distance of seven and six-tenths miles, and that the improvement should be completed to Montrose by making a thorough cut, two hundred feet wide and five (or six) feet deep, along the natural channel at the upper chain. This plan I became convinced would give the best, cheapest, and simplest improvement, all things being considered, that could be devised, and could be much more readily carried into execution than any other. In view of the great importance of the work, and the necessity of its being as permanent as possible, I recommended that the canal should be made three hundred feet wide in both excavation and embankment, and five (or six) feet deep at extreme low water; that there should be two lift-locks and one guard-lock, each three hundred and fifty feet long between the gates and eighty feet wide at the top; that the embankment should be made twenty feet wide on top, be carried up to four feet above the highest known flood, and be covered outside, inside, and on the top with well-constructed rip-rap of broken stone.

My report was duly transmitted to the House of Representatives by the Chief Engineer of the army, accompanied by a letter dated February 5, 1867, and, after having been considered in the Committee on Commerce, provision was made

in the general appropriation bill for carrying the plan into execution. tions were offered to the plan in the Senate, which resulted after debate in the rejection of the item making the appropriation called for, and substituting therefor the sum of \$500,000 "for improving navigation at Des Moines or lower rapids, according to such plan as the Secretary of War shall, on the report of a board of engineers, approve." Accordingly, on the 22d of March, 1867, the Chief of Engineers issued an order by the authority of the Secretary of War, convening a board of engineers to meet at Keokuk, Iowa, to consist of Brevet Brigadier General T. J. Cram, corps of engineers; Brevet Colonel John N. Macomb, lieutenant colonel of engineers; Brevet Major General J. H. Wilson, lieutenant colonel thirty-fifth infantry; Brevet Major General G. K. Warren, major of engineers; Mr. W. Milnor Roberts, superintending engineer Ohio river improvements, with Brevet Lientenant Colonel P. C. Hains, captain of engi-The subject of the improvement of the Des Moines rapids was neers, as recorder. committed to the board "without restrictions." The board met at Keokuk. Iowa, on the 16th of April, adjourned to Davenport, Iowa, on the 30th of April, and continued in session till the 13th of May. All the maps and plans which I had prepared, together with the reports of the previous surveys, were laid before the board, and, after a careful and exhaustive consideration of the entire subject in all its aspects, they recommended the plan which I had adopted, making no changes except in details, and fixing one point which I had left open for the decision of the bureau. The general arrangement and location of the canal are left as suggested in my report of January 1, but the embankment is reduced to ten feet in width on the top, with a rip rap covering two feet thick, and is to be carried only two feet above extreme high water, instead of four feet. The prism of the canal is left at 300 feet wide in embankment, but reduced to 250 feet in excavation; the minimum depth of water is fixed at five feet, maximum depth eight feet.

It will be observed that while these changes are all of a character to decrease the cost of the improvement, they diminish its strength and capacity in a like degree. In these matters I have deferred to the greater experience and observation of a majority of the board, without entirely yielding my convictions on all

the points involved.

The conclusions and recommendations of the board having been concurred in by the engineer department, were submitted to the Secretary of War, and having received his approval, I was instructed, July 19, 1867, "to proceed at

once to carry out the plan of improvement reported by the board."

In pursuance of these instructions, I advertised for proposals for excavating the prism and building the embankment of the canal, to be opened September 4. This part of the improvement was selected, after conferring with the board of engineers, as the first to be commenced, for the reason that the embankment, being throughout most of its length in the river, will require a more uncertain length of time for its completion, and will involve greater risk of being delayed, than the locks. After partial completion it will afford the means of protecting the locks from interruption or damage by high water or ice. In addition to this being all "section work," it will require no contract for "materials," and only one for labor; whereas, if the present oppressive laws governing contracts should be strictly adhered to, the locks would require contracts for masonry, carpentry, and smith-work, as well as for several different classes of materials. Finally, it will be seen hereafter that the \$700,000 already appropriated will complete nearly the entire embankment, leaving only the locks and the channel improvement at the "upper chain" to be provided for by future appropriations.

The resurvey of the Des Moines rapids was paid for out of the appropriation "for the examination and survey of western and northwestern rivers," and cost

88.059 67.

In carrying into execution the plan of improvement adopted, \$3,993 50

had been expended on the 1st of September for the purpose of paying contingencies of engineering and civil engineer assistants engaged in laying out and preparing the work for the contractors. This is charged to the appropriation "for the improvement of the Des Moines or lower rapids of the Mississippi river."

The excavation and embankment, together with the items connected therewith, in the report of the board of engineers dated July 20, 1867, are estimated to cost \$1,150,353; but it will be seen from the abstract of proposals herewith that this work will be contracted for at a saving on the estimates of \$438,694. Without making a corresponding reduction in the cost of the locks and channel improvements, it is estimated that \$1,479,647 will be required for the entire and permanent completion of the work in accordance with the plans adopted. namely: for contingencies in the completion of the embankment and excavation, \$100,000; for the locks, gates, and channel improvement, \$1,379,647, of which \$1,200,000 can be profitably expended on the work during the fiscal year ending June 30, 1869, exclusive of the sum already appropriated. But the entire improvement can be economically finished by the 1st of November, 1869, and hence I feel it my duty to respectfully recommend that the total sum requisite, namely, \$1,479,647. shall be appropriated at the next session of Congress. This will result in an actual saving of money to the government, and will not necessitate drawing money any more rapidly from the treasury than if it were provided for by two or more appropriations.

This work is situated in the first collection district of Iowa, 250 miles from Chicago, the nearest port of entry, and 208 miles from St. Louis, the nearest

port of delivery.

During the fiscal year ending June 30, 1867, \$547,558 80 were collected at Chicago on customs; \$18,935 38 were received as emolument fees; 640 vessels, with a registered capacity of 93,964 tons, are reported as owned in the district; 11,374 arrived at that port, and 11,497 cleared from it, during the year.

At St. Louis the receipts on customs during the fiscal year ending June 30, 1867, were as follows:

2001,	
Duties on imports	\$967, 597 33
Tonnage duties	31. 529 61
Tonnage tax, &c	14, 167 78
Hospital duties	8,721 46
Total	1,022,016 18

Two hundred and one steamboats are reported as belonging to the port, with a registered capacity of 89,515 tons, and 81 barges, flats, &c., of 15,790 tons.

The secretary of the Chicago Board of Trade, in his ninth annual report, showing the trade and commerce of that city for the year ending March 31, 1867, states the shipment of flour (reduced to wheat) and grain to have been 66,736,660 bushels, an increase over the year 1865-'66 of 13,524,436 bushels, and 10,252,550 bushels over and above the shipments of any other year in the history of Chicago; also that 672,769 head of hogs and 25,998 head of cattle were packed at that place. The receipts of lumber were 730,057,168 feet of lath, 123,992,400 pieces, and 400,125,250 shingles, showing an increase over the preceding year of 82,901,434 feet of lumber, 57,897,300 pieces of lath, and 89,227,900 shingles.

For a detailed statement of the amount of commerce and navigation which would be benefited by this improvement, I respectfully refer to my report of

January 1, since which time I have gathered no new statistics.

An abstract of the proposals for the "section work" (opened on the 4th instant) is forwarded herewith, from which it will be seen that Messrs. William

Hennegan & Son, of Mt. Vernon, Ohio, are the lowest bidders, and William Armstrong & Co., of Philadelphia and Keokuk, are the next in order. The contract not having been entered into yet, no "abstract of contracts" can be forwarded at this time. Specifications of the section work are submitted herewith.

ROCK ISLAND RAPIDS OF THE MISSISSIPPI RIVER.

In my report of January 1, 1867, I made a detailed statement of the results of the resurvey of these rapids, and the plan adopted for their improvement. It will be remembered that in an extent of fourteen miles they afford eleven miles of good navigation, and that only three miles are dangerous or difficult to pass during the low water stages. The river bed itself, being much narrower than at the lower rapids, and the fall of twenty-two feet being distributed nearly equally over a distance of fourteen miles, it was determined to adhere to the original plan of excavating and straightening the natural channel. This plan, involving the removal of about 57,500 cubic yards, received the approval of the engineer department, and Congress made an appropriation of \$200,000, which, with the \$100,000 previously appropriated, but unexpended, gave the sum of \$300,000 for the prosecution of the improvement Accordingly, on the 10th of April I was directed by engineer letter to invite proposals for doing the work. This was done immediately, and on the 5th of June following a letting was made at which Charles G. Case & Co., of Fulton, New York, were found to be the lowest responsible bidders. In pursuance of instructions from the engineer department, dated June 12, 1867, a contract was made with them on the 28th of June, by which they are bound to begin the work one month after the date of contract, or as soon thereafter as the water shall reach a stage four feet above the low water of 1864, to work at such points as the engineer in charge shall direct, and to remove at least 5,000 cubic yards of rock per month. The contractors are authorized to use coffer dams, chisels, or sub-aqueous blasting, as they may think best, but all tools, implements, and materials of whatever character must be furnished at their own expense, the government paying only for work done in accordance with the specifications of the contract.

The contractors are men of experience and energy, and may be depended upon to accomplish what they have undertaken. So far, however, owing to the unusual continuance of high water throughout the summer, they have not been able to fairly begin work. They have provided themselves with two of Osgood's patent rock chisels, weighing nearly eight thousand pounds each, (operated by machinery similar to that used for pile-drivers,) one dredge, all the lumber and iron required for coffer dams, and the necessary steamboats and barges for towing and handling their materials. Up to the present time, on account of high water, they have not been able to do more than to put their chisels, dredges, and barges into position, and to test the various parts of the machinery appertaining thereto. The chisels are just fairly getting to work on one of the upper reefs of Duck Creek chain, and to-day the contractors are beginning to construct a large coffer dam immediately below the point at which the chisels are working, for the purpose of cutting through the principal chain at Duck creek and taking out the points marked on our charts.

No estimate will be made of work done till the end of the current month, and hence I have no items of expenditure under this plan to report, except such as are involved in contingencies and engineering while preparing the work for the contractors. Under these heads \$5,853 73 have been expended, duly accounted for, and charged to the appropriation for the "improvement of the Rock Island rapids of the Mississippi river."

It is estimated that \$813,601 80, including the \$300,000 already appropriated, will be required under the plan adopted for the entire and permanent completion of this improvement, and that \$513,601 80 can be profitably expended during

the fiscal year ending June 30, 1869. This sum should be appropriated and made available as soon as possible, in order that advantage may be taken of every favorable opportunity presented by low water in the river for doing the work. If the necessary funds are provided, there is no good reason that can now be foreseen why the improvement of both the upper and lower rapids may not be completed by the 1st of November, 1869.

The upper, or Rock Island, rapids are situated in the second Iowa collection district; the nearest port of entry is Chicago, at which the collections upon cus-

toms amounted to \$547,558 80 during the last fiscal year.

Abstracts of proposals and contracts, with the names of the contractors, for this work are herewith submitted, together with the report of my assistant, Brevet Lieutenant Colonel P. C. Hains, captain of engineers, to which, and the accompanying diagram showing the stages of the water at this place during the last seven years, the attention of the department is invited.

LAWS GOVERNING CONTRACTS.

Before leaving this part of my report, I desire to call the attention of the department to the laws governing contracts, so far as the same concern and are applied to the construction of public works. Section 3 of the act of Congress approved March 2, 1867, "making appropriations for the repair, preservation, and completion of certain public works," requires that "whenever the Secretary of War shall invite proposals for any works, or for any materials or labor for any works, there shall be separate proposals and separate contracts for each work, and also for each class of materials or labor for each work." Section 3 of an act approved June 23, 1860, "making appropriations for the legislative, executive, and judicial expenses of the government," &c, provides that no contract, except for army supplies, shall be made by the War Department, "except under a law authorizing the same, or under an appropriation adequate to

its fulfilment."

In the works under my charge, it would be possible, by a close adherence to the law first cited, to make a technical division of the labor and materials required into several more classes than is customary among civil engineers engaged in the construction of similar works. For instance, the work of excavating the prism and building the embankment of the canal for the improvement of the Des Moines rapids, instead of being considered as one class, under the general head of "section work," is capable, as will be seen by referring to the specifications, of being divided into a number of items, such as "earth excavation," "rock excavation," "embankment," "lining," "puddling," "riprap," &c.. &c. As a matter of course it is, therefore, quite possible to invite separate proposals, and to make separate contracts, not only for these different items, but for the different kinds of materials required in the same; but it would be hardly possible for any engineer or set of contractors under such a construction of the law to complete the work in question at anything like a proper cost, or within a reasonable length of time. Take, for instance, the items of "earth excavation" and "rock excavation," and suppose separate contracts made for them. It will be seen at once, that as the river bank at the points where it is crossed by the canal is composed of mingled detritus, sand, gravel, broken rock, and rock in strata, it would be almost impossible, and certainly unprofessional and extravagant, to attempt to remove these different materials by different contractors. But the difficulty and confusion would not end here, for these materials are necessary in the river wall of the canal, and must be used as "lining." "puddling," "embankment," or "rip-rap," according to quality. Preparing, hauling, and placing them in the work would necessarily involve new items of labor, and require new contracts and contractors, and this would result in interminable trouble and confusion. It is not necessary to follow this subject further, although by so doing it might easily be shown that in the case of the

work involved in the construction of locks, a rigid adherence to the provisions of the law would lead to similar confusion, and would result in an equally wasteful expenditure of the public money. I have not been able to devise any other way of carrying on the works under my charge than by giving a liberal construction to the law and dividing the labor into general classes, as indicated above, and also in my correspondence with the bureau. But even with this I have no hesitation in saying that the law, so far as it applies to works which ought to be done by contract, should be repealed, or at least so much of it as requires contracts for different kinds of labor and materials to be made with separate parties after separate advertisements for proposals. I am convinced that the law, in its present shape, rigidly construed, will materially delay the works under my charge, and make them cost the government twenty-five per cent.

more than they would with the law repealed.

I have also to call the attention of the bureau to the law limiting contracts to the amount already appropriated; in other words, requiring new contracts to be made for each new appropriation. It seems to me, after careful observation, that this law should also be repealed, or so amended as to leave it discretionary with the War Department to make new contracts or not, as might seem most advantageous to the public interests. Many contractors, in bidding for labor or materials on works to be completed by successive appropriations, would be willing to take the risk of Congress providing the necessary money, and would therefore propose cheaper rates for the entire work than they could otherwise possibly afford. This was particularly the case with nearly all of the contractors who proposed for the work of improving the Rock Island rapids, and I have no doubt the successful bidders would gladly have reduced their bid twenty-five per cent. could they have had any assurance that they would be permitted to do the entire work without making a new contract. How they could afford to make such a reduction will become plain enough when it is remembered that they will be compelled to make almost as large an outlay for boats, machinery, and lumber to do \$300,000 worth of work as would be necessary to do \$800,000

I have also been somewhat embarrassed in carrying out the instructions of the department in regard to beginning the Des Moines rapids improvement, by the absence of any general law, so far as I can ascertain, regulating the condemnation of private property for public use, or even providing for the assessment and payment of damages to private parties, in cases like the above, where it is necessary for the improvement to cross private lands, and to take materials from those adjacent. Section 7 of the act approved May 1, 1820, for the "establishment and regulation of the Treasury, War and Navy Departments," specially provides: "That no land shall be purchased on account of the United States, except under a law authorizing such purchase." It may be that the statutes of Iowa regulating such matters are so framed as to permit the general government to take such action in the case under consideration as any chartered company would be required to take in a similar case. I am investigating this point now, but it has occurred to me that in any event the passage of a general law by Congress for the regulation of such matters when the government is an interested party, would materially facilitate the location and construction of such public works as are intended to improve navigation or to extend the means of intercommunication between the different parts of the country.

SURVEY OF THE ILLINOIS RIVER.

A detailed report, setting forth the results of the resurvey of this river during the fall of 1866, was made on the 15th of February, 1867, but, as our operations were confined to that part of the river lying below La Salle, it was thought

necessary to extend the survey to Lake Michigan, before absolutely fixing upon the plans of improvement, and estimating, finally, the cost of carrying them into effect. I therefore recommend a detailed and exhaustive survey of the country lying between La Salle and Chicago, and of a low-water survey of the river from La Salle to its mouth. Congress acted upon this recommendation, and made the necessary provision for defraying the expense that would thereby be incurred. Accordingly, I was informed by engineer order, dated May 8, 1867, that \$20,000 of the appropriation "for surveys of western and northwestern rivers" would be set apart for this purpose. I had estimated that this sum would be sufficient, but, having been directed to pay the salary of William Gooding, United States civil engineer, out of it, the sum actually available for the payment of the parties in the field will not exceed \$17,000. The survey extending over 400 miles, and requiring about five months for its completion, and three months for the preparation of plans and reports, the funds thus decreased will prove inadequate, by about \$5,000.

On the 13th of May I received the order just mentioned, from the engineer department, organizing a board of engineers, to consist of myself and Mr. William Gooding, United States civil engineers, for the purpose of "conducting surveys and examinations, and preparing plans and estimates for a system of navigation by the way of the Illinois river, between the Mississippi and Lake Michigan, adapted to military, naval and commercial purposes, in accordance with the act of March 2, 1867." In pursuance of these instructions, after conferring fully with Mr. Gooding, I assigned my assistant, Colonel James Worrall, civil engineer, to the immediate charge of the surveys, directing him to organize his parties, and begin the work at Chicago, at as early a date as the season would

allow.

On the 19th of June, the weather having been unfavorable up to that time, he began the survey of the main line with one party, under the immediate control of Civil Engineer Assistant George B. Griffin, at Bridgeport, on the south branch of Chicago harbor, taking careful and accurate notes of the present canal, the summit, the Des Plaines valley, and all alternate lines. This party has completed its work thoroughly as far as Ottawa, on the Illinois river, and by the 15th instant will reach La Salle.

Early in July a second party was organized under the control of Civil Engineer Assistant George A. Keefer, with instructions to survey and examine the Calumet and the Kankakee, together with the country lying between the latter and the lake, for the primary purpose of ascertaining whether or not a feasible

line for the improvement could be found in that direction.

This party has already completed the survey of a line from a point near Lemont, called "the sag," (Saganaska creek,) following the Calumet feeder of the Illinois and Michigan canal to Blue island, and thence across the divide to Momence, on the Kankakee. From Momence a reconnoissance of the Kankakee, extending some thirty miles into Indiana, was made, during which soundings and topographical notes were taken, but the swampy nature of the valley above Momence prevented the use of instruments for a detailed survey. Mr. Keefer's party has just finished the survey of the Kankakee from Momence to its junction with the Des Plaines, and will probably spend the balance of the month on the lower part of the Fox. As soon as that part of the work is disposed of he will be sent to the lower Illinois with instructions to make a careful examination and survey of the localities likely to be selected for dams and locks, and to assist in completing the hydrographical and topographical survey of the river.

Late in August the Illinois river having reached the lowest stage known for several years, a third party was organized under the control of Civil Engineer

^{*} During the reconnoissance Colonel Worrall received a severe injury in his right hip and thigh, by falling from the gang plank of the little steamer used by the exploring party, and since then has been confined to his bed.

Assistant L. L. Nichols, aided by Civil Engineer Assistant R. E. McMath, for the purpose of surveying the bed of the river, and particularly the sand-bars below La Salle.

Before getting this work fairly begun Mr. Nichols was summoned to New York to give testimony before a committee of the legislature, and Brevet Lieutenant Colonel H. A. Ulffers, civil assistant, was sent from this office to carry forward the important investigations connected with this part of the survey. His party has already reached Negro creek, and is making satisfactory pregress.

Careful and detailed instructions have from time to time been issued from this office, with the concurrence of Mr. Gooding, covering every point of interest and importance connected with the survey, and it is believed that we shall be able to report fully by the 1st of January in regard to its results, and also to submit detailed maps, plans and estimates of the improvement based thereupon. During the months of July Mr. Gooding and myself, accompanied by one assistant, made a careful examination of the country between Lake Michigan and La Salle, including the valleys of the Des Plaines, Kankakee and Illinois rivers, and also a part of the Fox, which, taken in connection with the instrumental survey, will enable us to decide definitely upon all the details of the plan, location and character of the proposed "system of navigation." As soon as the survey of the lower river has advanced sufficiently we shall make a like examination of the valley from La Salle to Grafton. So far as we can judge now, the results of this year's operations will confirm the facts developed by the survey of last fall and approve the general plan of improvement based thereupon, as recommended in my report of February 15, 1867.

The survey of 1866 cost \$5,857 36, and up to the date of this report the field operations of this season have cost \$9,730 39, and it is estimated that about \$15,000 will be required for its entire completion. Of this sum there is an unexpended balance of \$10,269 61 yet available, and it is hoped that \$5,000 more from the appropriation "for surveys and examinations of western and northwestern rivers" may be allotted to this work, and thus remove the ne-

cessity for any further appropriation.

The region traversed by this survey lies in several collection districts of Illinois, whose designations are not known to this office; the nearest port of entry is Chicago, at which \$547,558 80 were collected on customs during the last fiscal year.

RECOMMENDATIONS.

Before leaving this part of my report it may not be improper to call the attention of the department again to the fact that the legislature of Illinois during its last session passed an act looking to the improvement of the Illinois river and the enlargement of the Illinois and Michigan canal, provided proper assistance can be obtained from the general government. While this is a work of such vast importance to the people of Illinois as to fully justify them in undertaking it, even without assistance from other sources, I am fully persuaded that the commercial, military and naval interests of the entire country call for its commencement and completion under the auspices and by the assistance of the United States.

For this reason I take the liberty of recommending the passage of an act providing for the commencement of the work as early in the spring of 1868 as the season will permit, and in accordance with the plans to be submitted hereafter, when the survey now in progress shall have been finished and the results and estimates made known. It is safe to say in this connection that these estimates will fall within the limits of those submitted in my report of February 15, and that enough is known of the probable cost and the whole improvement to warrant the action that I suggest, particularly since all the details which may be required will be prepared for the consideration of Congress before action can be taken.

SURVEY OF THE ROCK RIVER IN WISCONSIN AND ILLINOIS.

This survey was provided for by act of Congress approved June 23. 1866, and was made under my supervision by Civil Engineer Assistant James Worrall, assisted by Civil Engineer Assistant William F. Shunk, during the fall of 1866. It was begun at Fond du Lac, Wisconsin, on the 1st of September, and finished at Rock Island, Illinois, on the 1st of December. Carefully prepared maps, profiles, and plans for a system of canal and slack-water navigation of the greatest dimensions that Lake Horicon, the summit level, could be made to supply, have already been submitted. Of this navigation, 167_{100}^{42} miles can be provided for by building twenty-two dams and locks along the Rock river; the remainder, 117_{100}^{58} miles, will have to be obtained by the construction of independent canal and locks.

From the south end of Lake Winnebago to Green Bay good navigation is already provided, and is susceptible of enlargement to any desired capacity by

the enlargement of the Fox river improvement.

The locks recommended for the Rock river improvement were fixed at a length of 200 feet between the mitre-sills, and a width of thirty feet. The width of the independent canal was fixed at eighty-eight feet on the water surface in deep cutting, and ninety-eight feet in shallow cutting. The depth of water throughout the entire system was calculated at seven feet. The entire cost of this work is estimated at \$14,738,370.

Plans were submitted for a smaller canal and slack water, giving a depth of

five feet only, and estimated to cost \$5,252,013.

This system of navigation is now called for by the local interests of Wisconsin, Illinois, and a part of Iowa. Its national importance is inferior to that of the Illinois river only by the difference of capacity of development. Should Congress think it advisable to extend the national aid to this line of communication between the lakes and the Mississippi, it should insist upon the adoption of the most commodious plan of improvement, for reasons fully set forth in my detailed report on this subject.

The surveys, maps, and plans were made at a cost of \$9,874 99, and were paid for out of the appropriation "for the surveys of western and northwestern rivers." Having been completed, no further appropriations are required. The line of the survey traverses several collection districts in Illinois and Wisconsin,

the designation of which are not known at this office.

In addition to the documents previously mentioned in this report, I transmit herewith a list of the civil engineers at present engaged on the works under my

charge

For all other details, not sufficiently set forth herein, the department is referred to my correspondence, returns, statements, and reports now on file in the bureau.

I am, general, very respectfully, your obedient servant,

J H. WILSON,

Lieut. Col. Thirty-fifth Inf., But. Maj. Gen. U. S. A.

Brigadier General A. A. Humphreys, Chief of Engineers, U. S. A., Washington, D. C.

E 1.

United States Engineer Office,
Des Moines and Rock Island Rapids Improvement,
And Illinois and Rock River Surveys,

Davenport, Iowa, January 1, 1867.

GENERAL: I have the honor to submit the following report and recommendations in regard to "the improvement of the Des Moines and Rock Island rapids

of the Mississippi river."

Having been assigned by the War Department to the superintendence of these works, August 3, 1866, I proceeded to Washington city as soon as possible after the completion of the duties upon which I was then engaged in connection with the defences at Delaware bay and river. While at Washington, making arrangements to secure copies of such maps and reports concerning the improvement to which I had been assigned as might be on file in the engineer bureau, I received the following instructions:

Engineer Department, Washington, August 14, 1866.

GENERAL: In obedience to Special Orders No. 379, Adjutant General's Office, August 3, 1866, you will proceed to Keokuk, Iowa, to superintend the improvement of the Des Moines and Rock Island rapids of the Mississippi, and the survey of the Rock river in the States of Illinois and Wisconsin, with its connection with Lake Winnebago.

In addition to the above duties you are hereby assigned to the survey of

the Illinois river from its mouth to La Salle.

You will accordingly make the necessary arrangements for the prosecution of the above duties, which it is very desirable should be commenced at the earliest practicable period, that the bureau may place before Congress at its next session the result of your examinations and plans of improvement.

You are authorized to employ as many civil assistants as you may require, at such compensation as will command suitable qualifications, to be subject to the approval of the department, with the mileage allowed to officers while trav-

elling on duty.

Very respectfully, your obedient servant,

A. A. HUMPHREYS,

Chief of Engineers, Brig. and But. Maj. Gen. U. S. Army.

Brevet Major General J. H. WILSON, U. S. A.,

Corps of Engineers, Washington, D. C.

Engineer Department, Washington, August 15, 1866.

General: The amounts appropriated for the improvement of the Des Moines and Rock Island rapids, in the Mississippi river, by act of Congress approved June 23, 1866, for repairs, &c., of certain public works, are in accordance with the report of the Bureau of Engineers of the 1st of March, 1866, (a copy of which has been sent to you, founded upon the estimates of the board of engineers of August 26, 1854.)

The efforts of the department have been heretofore directed to the removal of the obstructions in the rapids, with a view of securing a continued low-water channel of two hundred feet width, and four feet depth, throughout. Your attention is now directed particularly to this subject, and, in resuming operations at these localities, a careful examination or survey of those portions that have

been improved should be made to ascertain if any modification of the plan shall

be deemed necessary and proper.

The main object is to promote the interest of commerce by additional facilities to overcome the existing difficulties upon the rapids, under the conditions of the above act, until a plan shall have been adopted to secure a safe and convenient navigation to the naval and commercial vessels of the United States. The latter subject is, also, placed with you for investigation, and an early report is desired from you, with a plan accompanied by the necessary drawings, and estimates of cost matured after complete examination and surveys of the localities.

The surveys of Rock and Illinois rivers will also require your attention, and your report and plan of improvements, accompanied by estimates, should be submitted as soon as it is possible to obtain the necessary data therefor.

Your operations will be strictly conformable with the provisions of the act referred to, respecting contracts and the information to be incorporated in your report.

Copies of such reports and drawings of the works as you may need, and now

on the records of the bureau, will be furnished.

Very respectfully, your obedient servant,

A. A. HUMPHREYS,

Chief of Engineers, Brig. and Bv't. Maj. Gen. U. S. Army.

Brevet Major General J. H. WILSON, U. S. A.,

Washington, D. C.

In addition to the foregoing instructions, I conferred freely with the Chief Engineer, and his assistant, Brevet Colonel I. C. Woodruff, in regard to the scope of the duties to which I was assigned, and, in accordance with their views, made my arrangements for a thorough examination of both rapids, and for the exhaustive consideration of all the plans of improvement which had been suggested, or might present themselves during the survey.

Brevet Lieutenant Colonel P. C. Hains, captain of engineers, was directed to repot to me, to assist in the investigations, and the sum of \$28,500, out of the general appropriation for "the survey and examination of western and northwestern rivers," was placed to my credit for the purpose of defraying the expense

of the various surveys under my charge.

It was estimated that \$10,000 would be required for the survey of the rapidal Having proceeded to Keokuk, Iowa, and organized the field parties for the survey of the Rock and Illinois rivers, I directed Colonel Hains to take post at Davenport and assume personal charge of the Rock Island rapids survey.

He was instructed "to examine and survey accurately the places on the rapids where boats experience difficulty and encounter danger from shoal water, swift currents, and tortuous channels, so that an accurate estimate may be made of the amount of excavation necessary to render navigation safe and expeditious; to locate correctly the Rock Island railroad bridge on his maps, ascertain the direction of the currents through its bays, state its general influence upon the free navigation of the stream by all kinds of craft, to obtain all the data within reach in regard to the cost of rock excavation under water, by the various practicable means, to note the condition and influence of the dams at Moline, and ascertain whether or not they might be continued for mill purposes without injury to the navigation of the open river."

He was also directed to carefully survey the river valley on both sides, with accurate instruments, theodolite, level, and chain, along the rapids for five or six miles above and below, so as to show on his maps both longitudinal and cross sections of shores and water; to make careful observations of the velocity of the current at various points on the rapids, as well as above and below them;

and, in short, to obtain every kind of information likely to throw light upon the various plans of improvement which might be presented for discussion. In order to complete the work assigned him at the earliest possible day, Colonel Hains was authorized to employ as many civil engineer assistants as might be necessary.

The report of Colonel Hains and that of the board of engineers convened

at his request are submitted herewith as a part of my own report.

The survey for additional information at the Des Moines rapids was intrusted to H. A. Ulffers, civil engineer, brevet lieutenant colonel volunteers, assisted by Ernst F. Hoffman, civil engineer, late major and additional aide-de-camp, United States army, under similar instructions to those given to Colonel Hains. The instruments furnished by the engineer department and used in these surveys were of the best quality, and every precaution was taken to secure extreme accuracy. Upon an examination of the surveys made by Lee and Warren, it was thought necessary to make entirely new surveys, not only to verify the maps projected from the old data, but to extend the information sufficiently to allow a thorough discussion of the entire subject. Lee's map, made in 1837, was found to be of no use, except to give a general idea of the topography and hydrography of the localities; while Warren's, although sufficiently accurate, has been made to illustrate simply the projects of excavating a channel in the bed of the river.

Before proceeding to the discussion of the various plans of improvement which I have thought entitled to consideration, I desire to state that as it is required the entire fall till the beginning of the severe weather of winter to finish the field work of the surveys, it has been found impracticable to submit with this report minute and detailed maps and specifications of the plans of improvement herein recommended, but they will be prepared as soon as the necessary time can be devoted to that purpose. The general plans, with maps to illustrate them, as well as the estimates of cost, have been made with great care and sufficiently in detail to give a correct idea of the localities and the works adapted and to them, as well as what it will cost to carry them into execution speedily and economically.

THE DES MOINES RAPIDS.

The Des Moines, or lower, rapids are situated near the mouth of the Des Moines river, and extend from Keokuk to Montrose, a distance of about eleven miles. During the low water season they interpose a serious, and at times an

absolutely impassable barrier to steamboat navigation.

The upper Mississippi, from St. Paul to the mouth of the Missouri, has, during the lapse of more recent geological periods, worn for itself a valley varying from one to fifteen miles in width, and sunken below the general level of the prairies on either side from one hundred and fifty to three hundred feet. In many places and for much of the distance the valley is cut through a strata of rock, varying in thickness, hardness, and mineral characteristics. Without discussing the agencies by which this erosion, so disproportionate to the present powers of the river, has been effected, the Des Moines rapids present evidence strongly confirmatory of changes in the river itself. General Humphreys, in his report upon the hydraulics of the Mississippi river, suggests that it was formerly a clear water river like Niagara, fed by a fresh water lake or lakes of great extent, occupying a large portion of what is now prairie land of Illinois and Iowa, and that its transformation "from a clear into a muddy stream may have been the result of changes which have taken place in its basin." In support of this theory he refers to the passage of the river through the northeastern extremity of the Ozark mountains at "Grand Tower," below St. Louis, where the water has cut down through beds of rock upwards of three hundred feet thick, and probably

drained the lake just mentioned. Another instance, developed by the survey and examination of my assistant. Colonel Ulffers, amounting almost to a proof, can be found at the head of the Des Moines rapids. Just below Montrose the rocky bluffs recede at right angles from the river, bearing gradually to the northward, and enclosing a considerable extent of country above and to the westward, which was once evidently the bed of an extensive lake, whose outlet was at the rapids, and whose surface was about one hundred and five feet above the present low water level. This lake basin is an extensive level plain, intersected by a network of sloughs, its lower part subject to annual overflows and covered with a heavy growth of willow, maple, hickory, haw, and vines. The terraces around it are well developed and of an unvarying height. Its upper end and, in part, its western side are united by beds of loam, rising one hundred and fifty feet above low water at Fort Madison, and forming the entire river bluff at this place. How far this loam deposit extends above and into the interior of Iowa has not yet been determined, and therefore what may have been the extent of the lake itself in these directions cannot be ascertained.

As before stated, the outlet of this lake was at the present head of the lower rapids. The waters stored up there have, by their ceaseless action for ages, assisted by ice and other geological agencies, gradually eroded for themselves a channel at least a mile wide, nearly two hundred feet below the general level of the prairies, and extending through limestone rock to the mouth of the Des Moines river and beyond. This erosive action, though productive of such remarkable results, has not yet been carried sufficiently far to render the river through this part of its bed available and safe at all times for the purpose of navigation. From Fort Madison to Montrose the river is about 2,500 feet wide and sufficiently deep, but on the rapids its bed of limestone rock, which, by some unknown cause, seems to have been hardened to a greater degree than the corresponding stratum above and below the rapids, has resisted the action of the water, while its sides have given way. The result is that this mass of rock remains there acting exactly as an artificial dam, whose upper surface slopes about twenty-two feet in eleven miles, and conforms very nearly to the plane of stratification of the rocks through which the channel is cut. The bluffs extend along the banks of the river throughout the length of the rapids, presenting a rock escarpment at the present high water mark, with a sloping gravel beach to low water, and also another escarpment of rocks at one hundred and five feet above the present water level, having likewise a sloping beach at its foot. The exposed ledges are formed of different strata in different localities. At some places they are brecciated limestone, (near Montrose,) in others magnesian limestone, (above Larey's creek,) and in others the coal measure sandstone, (below Price's creek;) but notwithstanding the varying hardness of these strata, they have all been cut through equally by the river in its progress from the upper beach just mentioned down to the one at the present low water level on the rapids. About sixty feet of these bluffs, however, consist of the grade bed lying between the two beaches, and made up mostly of an accumulation of clay and marls, easily washed away. The river, in forcing its way through these beds of stubborn materials, must therefore have gradually receded from the foot of the rapids, like the Niagara is doing now, until it reached its present condition. Undoubtedly the process of smoothing its path is still going on, although in an imperceptible degree. In places where the bluffs recede from the bank at the mouth of creeks emptying into the river, there are two "terraces" besides the present river bottom, respectively twenty-five and seventy feet above present

The level part of the town of Nauvoo, at the head of the rapids, opposite Montrose, is built upon the twenty-five foot terrace, which is likewise prominent around the edges of the plains between the latter place and Fort Madison. The seventy foot terrace is most prominently developed near Sandusky.

The total length of the Des Moines rapids, measured along the Iowa shore from Montrose (old Fort Des Moines) to the St. Louis Packet Company's landing at Keokuk, (station D to station V,) is 10.92 miles; on the Illinois side, from Nauvoo to the Hamilton ferry landing, (station 23 to station 84,) is 12.20 miles, or a mean of 11.56 miles.

The total fall in this distance on the Iowa side is 22.046 feet; on the Illinois side, 21.933 feet; or a mean of 21.989 feet—sensibly 22 feet. The difference of the two lines of levels, being only .113 of an inch, may be regarded as inap-

preciable.

The mean width of the river is about 4,500 feet on the rapids, or almost 2,000 feet wider than it is above and below; its mean depth, calculated from upwards of 2,000 low water soundings, is 2.4 feet, though there are large areas where the depth is much less; its average area of cross sections is 17,550 square feet, at ordinary low water; its mean surface velocity is 2.88 feet per second, and its mean velocity deduced therefrom is 2.304 feet per second. From these data the discharge has been calculated, and is found to be 40,435 cubic feet per second.† For extreme low water 1.5 foot must be subtracted from the average depth, which will give 24,883 cubic feet per second.

The bed of these rapids, throughout its entire length, has reached a stratum of "cherty limestone," belonging to the Keokuk group of the carboniferous series. The nature of this rock, consisting as it does of thin beds of limestone interstratified with at least an equal amount of brittle chert or limestone, precludes the idea that the different reefs, or heads of chains, might be occasioned, as formerly supposed, by a succession of strata which in their outcrops would form a barrier across the river. The deeper parts of the rapids are all caused by erosion, originating partly from the strength of the current, aided by the presence of numerous granite boulders, and partly by masses of ice which are annually swept down by the spring freshets with tremendous force.

In the various soundings carried across the river no trace was found of pools or places where the area of the cross section would be sensibly greater than on the heads of the chains. In other words, there is no proper chain crossing the river at any place, nor is there anything like a true pool, the deep water being found more properly in fissures and pockets. The river bottom is a broad, smooth rock, seamed by a narrow, crooked channel, or in some places several of them, alternately widening and narrowing, shoaling and deepening; nowhere good navigation, but more difficult in some places than in others. The rapids are, therefore, not broken and noisy; but, the descent being gradual, the water flows over its bed in a broad, smooth, unbroken sheet, with nothing but the faintest ripples on its surface to indicate the dangerous places. The casual observer would not suspect the presence of the rapids unless he were notified of their locality beforehand. From these facts it may be readily inferred that boats would not undertake their passage at night, even if the channel were deep and well defined.

The worst parts of these rapids are called, by steamboat and river men, chains, of which there are five principal ones, known as Lower, English, Lamallee's, Spanish, and Upper, respectively. From the Lower to Spanish chain, inclusive, the channel used by steamboats is crooked, shallow, and exceedingly difficult of navigation, requiring, as General Warren states in his report of April 6, 1854, to be excavated "almost continuously to the landing at Nashville, a distance of seven and a half miles." For this distance the fall is about eighteen feet, and the average slope 2.4 feet per mile. From Nashville to the Upper chain the channel is straight and deep, and the fall only four feet, or not more than a foot per mile.

^{*} Reduced to the low water of 1864.

^{*}This discharge was calculated with a mean depth of 3.9 feet, the average depth at the time the velocity was observed.

The Lower chain extends from Keokuk to the mouth of Price's creek. The channel is 3.5 feet to eight feet deep, but it is very swift and crooked, and therefore intersected by surface and under currents, particularly in that part of it known as Sucker chute and Omega patch. The greater part of the last appropriation was expended here, improving the channel materially, but the fragments of the rock blown out were piled alongside of the channel, injudiciously it appears, as the ice has already carried away the greater part of the piles and redeposited the fragments of rock in the channel.

The English chain extends from Montebello to Waggoner's warehouse. The channel running near the Illinois shore in this reach is comparatively straight, from four to seven feet deep, and from fifty to two hundred feet wide, and presents no particular difficulties to navigation except in times of high wind.

The head of Lamallee's chain is opposite the mouth of Larey's creek; the channel is from 4.5 to 10 feet deep, and an average of one hundred and twenty feet wide; but it is crooked, intricate and dangerous, owing to the presence of many shoal patches of rock, the breaks of which are not distinguishable in rough weather.

Spanish chain extends from Judge Ballinger's place to within a half mile of Nashville; the channel is from three to thirteen feet deep, and from twenty-five to three hundred feet wide, and, although sufficiently difficult of navigation, has

been somewhat improved by excavation.

The Upper chain extends but a short distance below Montrose island, and is made by a broad flat bed of rock extending entirely across the river, upon which the water in dry seasons is not over two feet, and frequently not a foot deep. The channel is a channel only in name, being nothing more than a slight depression in the rock about three feet deep and fifty feet wide in the narrowest An accurate idea of the geological and other physical features of the entire rapids may be obtained by an inspection of the maps, sections and profiles submitted herewith. The difficulty of navigation, it may be observed on all the chains, lies not so much in the shallowness of the channel or thread of the current as in its unevenness of bottom, insufficient width, tortuous direction, and great velocity. The influence of these features is exaggerated by cross-surface and under currents, and by east and west winds. From fear of the rocks boats are compelled to move slowly, and are therefore more difficult to guide, and no matter how skilful the pilot may be his boat will be carried forward in the direction of the original impulse for some distance before it can be made to obey the helm; this is particularly the case in windy weather and while going down stream. From these causes I am of the opinion that, had a careful record been kept of the boats grounded on the rapids, it would probably show that as many had struck upon the rocks near the pools as on the chains themselves. If this view of the case is correct, no amount of excavation would entirely remedy the difficulty.

The greatest fall on the rapids is on the Lower chain, where it amounts to 1.472 feet in 1,000 feet, or 7.77 feet per mile; the velocity of the current is consequently greater here than at any other place, though it was not accurately measured on account of running ice. The greatest observed velocity was at the head of English chain, where it was found to be 4.35 feet per second, or 3.02

miles per hour.

The maximum range of the water surface at the head of the rapids could not be determined with exactness for the lack of well-defined high water marks, but from the best that could be obtained it is found to be about 12.65 feet. General Warren, in his report of April 6, 1854, gives it at 11.75 feet. The average range will probably not exceed 9 feet. At Waggoner's, Nashville and Keokuk the difference between extreme high and low water was accurately determined, and found to be 15.88 feet and 19.28 feet respectively.

In 1865 and 1866 navigation opened about the 1st of March, and closed about the 1st of December, giving 275 days, which may be considered somewhat

above the average for the boating season. By an examination of the register kept at the Keokuk Indicator by the Upper Mississippi Pilots' Association it appears that the water on the Lower chain was less than four feet for fifty days during 1865, the least depth, 2.3 feet, having been reached on the 20th of June; but this was quite an unusual season, the lowest water generally occurring about the middle of September and continuing much longer. During 1866 the same register shows ninety-two days, or one-third the entire season, when the water was less than four feet.

The least available depth recorded was two feet, and occurred on the 4th or 5th of October. From the 28th of September to the 2d of December the water ranged between two feet and 3.3 feet on the Lower chain. This was about an average year. The driest season known was that of 1864, when the river, early in September, reached a stage about ten inches lower than just mentioned. The plane of this low water has been taken as the plane of reference for all the sound-

ings made by the parties under my direction.

During the extreme low water season navigation for steamboats along the rapids is entirely suspended, and their cargoes are transferred either by rail or lighters, at an extra cost of about one dollar per ton and an average cost of about \$500 per day to the steamboats themselves while discharging and taking on freight, or about \$1,000 per trip. By a register, kept at the Rock Island bridge, it is ascertained that during the period of seven years ending on the 1st day of December, 1866, 2,412 steamboats and 1,363 barges have passed up and down the river. This would be an average of 344 steamboats and 175 barges each way, but the last year shows a large increase in the number of boats of all classes, 677 steamboats having passed up, and 671 having gone down. As many as 600 of these boats, probably, carried their cargoes to the lower rapids; and as we have just seen these rapids had less than four feet water upon them for ninety-two days, and were almost entirely impassable for sixty-four days, it is safe to calculate that the steamboats alone engaged in this commerce were taxed to the amount of \$250,000 for extra labor, in order to transship their freight at the lower rapids,* while the owners of the freight were compelled to pay one dollar per ton in addition for lighterage and railroad charges. What might have been the influence of the rapids in deflecting commerce from its natural channel, or in repressing it altogether, must remain for the present a matter of conjecture, though it is not difficult to demonstrate that the products which have gone to the railroad, in order to reach a market, have cost the producers twenty-five per cent. of their value more than would have been a fair rate for freight by good water transportation. From these facts there can be but little doubt that the Des Moines rapids alone, in their present state, cost the people of the upper Mississippi valley, at the least possible calculation, half a million of dollars annually. they are a barrier to nearly 40,000 miles of internal navigation, the equivalent of double that many miles of internal coast bordering upon the Mississippi and its navigable tributaries, it is not difficult to see that their improvement is a matter of national import, in which all sections of the country are equally inter-Tabulated statements of the amount of commerce and navigation which would be benefited thereby, as well as the amount of revenue collected at the nearest ports, are submitted herewith, under the head of general remarks.

In carrying out the instructions of the engineer department, heretofore recited, I have endeavored to consult the interests of commerce in its broadest extent, but have listened attentively to every suggestion in regard to the plan and necessity of improvement, whether made by persons locally or generally interested. But before discussing the various plans which have claimed my attention, I desire to state that all the experience of the past goes to show that no

^{*} See the certificate of Mr. Griffith, secretary of the Northern Line Packet Company, submitted herewith.

plan can be made entirely effective which is based upon temporary expedients, or depends for its execution upon successive appropriations of small sums by Congress, as has been the custom heretofore. Something like \$400,000, exclusive of the last appropriation, have been already appropriated for both rapids, \$335,000 of which have been expended upon the lower rapids, for which amount not to exceed 25,000 cubic yards of stone have been excavated. As this money was expended when labor was comparatively cheap, it ought to have been sufficient, with a judicious plan of improvement, to do nearly the entire work required. Notwithstanding the work done has benefited the channel, I regard the expenditure as almost thrown away, most of it having been consumed in experiments and preparations.

After a feasible plan has been devised, and proper estimates have been made, the necessary money should be appropriated at once, so as to secure the means by which the work may be carried forward vigorously and economically till the

improvement is finished.

The plan of excavating the channel is for a variety of reasons exceedingly difficult to execute at these rapids, either by blasting under water or by the use of coffer dams.

In order to enlarge the channel to 200 feet width and four feet depth, at extreme low water, it will be necessary, according to General Warren's report, (making allowance for the ten inches additional depth, not provided for by him, and rendered necessary by the low water of 1864,) to excavate about 150,000 cubic yards of stone. According to the data recently obtained, it will require the excavation of 177,519 82 cubic yards, which, at an average of fifteen dollars per yard, will cost \$2,662,797 30.

Should this channel be completed it will not accomplish all that is required, for in addition to the dangers consequent upon cross-currents, it is only indicated by the faintest ripple mark of the running water, and could not, therefore, be used either during the night, in the fog, or during unfavorable winds, and a special pilot would be required at nearly all times. Hence one-half the time of

extreme low water the river would still be impassable at the rapids.

Other difficulties would also continue to exist; the fall of eighteen feet in seven and one-half miles, with an increased current, would have to be overcome at a great expense of cost and time by the ascending boats, and the navigation for descending boats would not be shorn of its dangers. In addition to this, as the water is nowhere deep, and as the excavation has to be carried lower than actually required, the tendency is to draw the water from the shores and above, and proportionately transfer the shallow places. From these considerations I do not hesitate to recommend the abandonment of this plan at the lower rapids, and the adoption of another, which will secure safe, easy, and economical navigation at any time of night or day, and in all kinds of weather, for every species of craft plying upon the river, either above or below the rapids. Nothing less than this will fully subserve the interests of commerce or national defence, and therefore nothing else should be adopted. The fullest and most efficient means to overcome the difficulty must ultimately be the most economical, although they might involve the expenditure of a large sum of money. But when those means are found but a little more expensive, as shown herein, as well as much more certain to accomplish the desired result, they should be adopted at once and without further argument.

Several other plans, such as the construction of a continuous dam with locks across the entire stream, and various modifications of wing dams, sluices, and "chutes" for narrowing and deepening the thread of the current, have been suggested and carefully considered, but have all been rejected as involving too many elements of uncertainty to warrant their application to a river of such magnitude as the Mississippi. They are, without exception, more or less experimental in their character, and concerned with elements in the problem quite in-

determinate in their value and imfluence, and while some of them might materially ameliorate the present difficulties, the chances in their favor are too uncertain, and their probable cost too much a matter of speculation, to authorize their adoption.

So far as the necessities of commerce are concerned the river has already been

sufficiently experimented upon; effective plans are now required.

In view of these facts I have respectfully to recommend the construction of an independent navigation canal along the Iowa shore from a point near the present site of the Keokuk Indicator, at the city of Keokuk, to a point just below the village of Nashville; the balance of the distance to be overcome by using the natural channel, which from here to the Upper chain is found to contain sufficient water for all purposes. At the Upper chain a through cut 200 feet wide, six feet deep, and 2,400 feet long, will be necessary. A careful calculation shows that 54,882.29 cubic yards will have to be excavated, and this can most surely and economically be done by the use of coffer-dams at the low-water season.

The estimated dimensions of the canal are as follows: length 7.6 miles; width on the water surface three hundred feet, and depth at the lowest stage six feet. The stage of water here referred to is that of 1864, which was ten inches lower than any other season recorded and about fifteen inches below that of 1866, which may be taken as an average. The canal would therefore pass boats drawing full six feet, and have a sufficient depth in addition for perfect safety during seasons of average low water. It should run the entire distance, except at one or two low points along the shore in the bed of the river. The river embankment for strength and economy should be constructed of earth, and securely covered inside and out with a well made riprap of broken stone, so as to render it entirely safe against the running ice and freshets. It will require two lift-locks and one guard-lock; the lift-lock to be three hundred and fifty-six feet between the mitre-sills, eighty feet wide beween tops of chamber walls, (seventy-eight feet at the water-surface,) and to lift respectively eight and ten and one-third feet.

The lower lock should also be furnished with a set of guard-gates for security against floods, and the guard-lock proper at the head of the canal should be so arranged as to admit of being used as a lift-lock whenever required, which would be after the water in the river at the head of the canal had raised (three or four feet) above the extreme low-water mark. This construction will admit of nine feet draught, the utmost likely ever to be required for purposes of navigation, either by vessels of commerce or war, and will allow the canal to be used at all ordinary stages of the river, so that ascending boats may avail themselves of the slack water of the canal to overcome the increased currents of high water in the open river. The saving in fuel and time would alone pay a handsome revenue upon the entire cost of the improvement.

It will be observed that in this canal a greater depth at extreme low water is provided for than is required by the present ruling depth of the river above and below; but as the work is intended to be permanent, it should be constructed so as to meet all possible contingencies of the national defence and river improve-

ments in the future.

It is believed that before the expiration of fifty years the average ruling lowwater depth of the Mississippi from St. Louis to St. Paul may be increased to six or seven feet, in which event no changes will be necessary in the proposed canal at the lower rapids. This system of navigation, providing in the fullest manner for the improvement of the rapids, will cost \$3,390,000. Should it be deemed inadmissible or unnecessary by Congress to provide, in the dimensions of the proposed canal, for the ultimate development of the river above and below, the cost may be reduced to \$2,731,722 96, by reducing the depth of water in the channel and through-cut to five feet instead of six, so as to give good navigation for boats drawing four-feet water. This may be ultimately diminished by the sum of \$34,155, should it be found unnecessary to construct the stone pier or

wing-dam estimated for at the foot of Montrose island.

The through-cut along the channel at the Upper chain involves the use of coffer-dams, and although they are quite expensive in their application, it is believed that they are more likely than any other means to result successfully; but as the work is to be done by contract, the contractor should be permitted to use his own discretion in the selection of the plan, under sufficient bond and surety, for the expeditious excavation of the new channel to the dimensions specified in this report. Estimates of cost and a general plan of the proposed canal, prepared under my directions by my assistant, Mr. D. C. Jenné, a civil engineer of sound judgment and enlarged experience in the construction of canals, together with a map of the rapids and a part of the river above and below, are submitted herewith.

Detailed maps, plans, and specifications will be prepared and forwarded to the engineer department as soon as the more pressing duties with which I am charged

will permit.

The following extracts from the report of Mr. D. C. Jenné, embodying sound opinions confirmatory of my judgment, will give all the additional information necessary for a complete understanding of the proposed plan of improvement, viz:

"Your special instructions to me were to make a detailed estimate for a canal three hundred feet wide and six feet deep at low water, (low water of 1864,) with locks three hundred and fifty feet by eighty feet, extending from the deep water at Nashville to the deep water at Keokuk. After a personal examination of the route, I was enabled very carefully to locate on the map which Colonel Ulffers had prepared a centre line of canal, which is generally about one hundred and fifty feet from the shore, except for a distance of 1.44 miles, where it runs on the low table-land adjoining the river. A profile of the surface line of low and high water, and of the ground and the bed of the river, showing both excavation and embankment, has also been prepared. At the head of the canal is located a guard-lock with walls twenty-one and two-thirds feet high, or two feet above highwater mark. This will be used as a lift-lock when the water in the river is more than three feet above low water. It will have a favorable location independent of the river, and the cost of bailing and draining will be comparatively small.

"A lock of eight-feet lift is located at a point about five and six-tenths miles from the upper end and two miles from Keokuk. This lock also has a location

independent of the river, and not expensive for bailing and draining.

"At the lower end a lock of 10\frac{1}{3} feet lift is located entirely in the river, and its construction will require an expensive coffer-dam, and make the item of bailing and draining very heavy. Its location is such as to make six feet of water on the mitre-sill at low water. The walls will be 28\frac{2}{3} feet high, which will carry them two feet above high water. In the centre is placed an extra set of gates, which reverse in closing to keep high water out of the canal.

"The river bank of the canal for the entire length is to be raised four feet above the high water of 1851, and is to be 20 feet wide on the top, with an outside slope of 1½ to 1, and an inside slope of 1½ to 1, with a heavy rip-rap wall on the outside, and a higher one on the inside and over the top. The height of

the bank will vary from 18 to 30 feet above the bed of the river-

"A large amount of rock excavation will be necessary at the guard-lock, and for one mile below, in order to obtain six feet depth in low water in the river; also at the middle lock and for a half mile below.

"On the flats from four to eight feet of excavation are earth, and the balance rock. The excavated material will generally be used in the embankment and rip-rap walls, and can be hauled from the pit directly to the point required, and

thus save the expense of borrowing the materials from other points to form the

"The character of the earth in the adjoining hills or banks is very favorable for forming solid and water-tight banks, but for a considerable part of the distance the rocks run up so high on the hills that it will be expensive obtaining the same, and for this reason a high price has been used in the estimate.

"There is probably no stone on this side of the river that will answer for face stone for the locks, but the backing and vertical wall stone can mostly be obtained here, and perhaps a large portion of the excavated rock can be used for

this purpose.

"On the Illinois or east side of the river, within a reasonable distance of the work, are located splendid quarries of magnesian and other limestone, which will answer every purpose for face and dimension stones, and which can be delivered on the ice during the winter, or in boats during the summer.

- "The magnitude of this work is such that the actual cost per mile must far exceed the general cost of most of the canals in this country. The enlarged Erie, with a prism of 56 feet width of bottom, 70 feet surface, and seven feet depth of water, with double locks 110 by 18 feet, has cost about \$91,000 The proposed canal will have a cross section of prism about four times as large as the Erie, with locks three times as wide. The banks will be almost entirely built in the river, with a heavy guard bank to protect against high water, while the Erie canal has comparatively a small amount of this kind of work."
- "I would recommend that the Upper chain be improved by excavating, on the west side of Montrose island, a channel 200 feet wide and six feet deep at low water; and for the purpose of preventing the water from spreading out toward the Nauvoo or east side of the river, for at least half the length of the channel, that a stone pier, extending down from the foot of said island for 1,000 feet, be built parallel with the channel. The excavation can be done by constructing coffer-dams, say three in number, which shall embrace an area of 1,000 by 230 feet each, pumping out the water and excavating the lock by the ordinary process of drilling and blasting.

"If it be necessary to contract the water still further, the excavated material

can be used to form a wing dam from the channel to the west shore.

"From all the information in my possession, it seems that a depth of five feet is all that can be obtained in the river above, until improvements shall be made to increase the same. If the demands of commerce shall ever call for this increased depth, it will seem to have been a great oversight not to have made the improvement of the rapids for six feet depth of water, even though the cost had thereby been greatly augmented. Therefore, in view of what the commerce of the river may hereafter demand, I would recommend that the channel be made six feet deep at low water. This is undoubtedly all that will ever be required, inasmuch as no greater depth can probably be obtained in the river above.

"If a dam were built across the river at Nashville, raising the water four feet, the most of the rock excavation in the canal and a very large part of that in the channel at the Upper chain would be avoided, and the cost of the whole work materially reduced. Such a dam would, however, prevent boats from navigating the river, and compel them to use the canal even when the water in the river is of sufficient depth; it is therefore inadmissible.

"The annexed estimates for the canal are based on 300 feet width and six feet depth, but it will be seen that by reducing the width to 200 feet where the excavation of rock and earth occurs, a saving of \$280,000 can be made. This reduction would occur in detached portions, where the canal leaves the river, and in my opinion would answer every purpose. If it should afterwards be considered necessary, the additional 100 feet could be excavated during the

suspension of navigation.

"In order to obtain materials for embankment and to locate a good line for the canal, it will be necessary to alter in several places the line of the public highway, and of the Keokuk and Fort Madison railroad. The total length of each which requires alteration is about three miles, and the cost is embraced in the annexed estimates.

"There are several small streams which will have to be taken in, and as the river guard bank must be carried very much above the surface of the water in the canal, no waste-weirs can be constructed at those points. The most important of these streams is Price's creek. If they in times of freshet bring in too much water, it will have to be passed off in sluices which must be constructed around the lift-locks.

"The cost of the sluices is provided for in the estimates."

TIME REQUIRED TO CONSTRUCT THE WORK.

"In the construction of the canal, the first season should be occupied in excavating a part of the canal, putting in the bottom of the outside of the rip-rap wall for the entire length, and raising it from ten to fifteen feet high. This can be done and allow the high water of the next spring to overflow it and do no harm. During the second year the excavation should be continued, the earth for embankment put in and the outside rip-rap wall completed. The third season the excavation for the canal should be finished, the inside rip-rap wall put in, and the remainder of the bank and walls completed. For the locks, the first season should be occupied in procuring, preparing, and delivering materials at the works. The second season, the coffer-dams should be put in, the lock pits excavated, the foundations prepared, and a portion of the masonry laid. During the third year, the masonry, gates, embankment, and all the other work should be completed. In this manner, if the work is commenced in the ensuing summer, the whole can be completed in the fall of 1869, or in about two and a half years.

"The channel and piers at the Upper chain can be finished by the fall of 1868, or in one and a half year.

COST OF MAINTENANCE.

"The general average cost for the repairs of the enlarged Erie canal, for the last five years, has been about \$1,000 per mile per year, including all items of repairs, superintendence, and lock-tending. This canal is about 350 miles long, and has fifty-seven double locks and fifteen single locks.

"It is my opinion that the maintenance of the proposed canal and locks cannot exceed \$2,000 per mile per year for the next ten years, and that for the first five years it will not be over \$1,000 per mile per year, unless some unforeseen accident shall occur. At this rate the cost of repairs of the whole canal for ten years will be \$15,200 per year.

"If the demands of commerce shall ever require a double set of locks, the proposed ones can be so located on one side of the centre as to leave sufficient room for the construction of other locks by their side without in any manner disturb-

ing the outside river embankment wall.

"In the prism of the canal and in the locks, provision is made for a depth of nine feet when there is a rise of three feet in the river, in order that boats of

eight feet draught may pass through the canal.

"I have prepared a plan for the lower lock, of the dimensions and form heretofore described, the general details of which, with the exception of the reversed gates, will apply to the other lift-lock and guard-lock. It is supposed that the foundation of all the locks will be on rock of such a character as to obviate the

necessity of using timber.

"The gates are to be of wood, properly arched on the upper side and strengthened, with wrought-iron braces or hog-chains on the lower side. This, it is believed, will answer the purpose designed, and the expense will be much less than that of iron gates. It is proposed to hang the gates on the suspension plan which has been successfully applied to the locks on the St. Mary's canal, between Lakes Huron and Superior.

" Detailed estimates of the cost of all the work are hereunto annexed, of which the following is a

SUMMARY.

"For a canal three hundred feet wide and six feet deep, with a chan- six feet deep.—	nel at Mont-
Cost of canal, embankment, and walls	\$1,717,480
Cost of lift-lock	671, 265
Cost of middle-lock	244, 910
Cost of guard-lock	242, 822
Total cost canal and locks	2, 576, 477
Cost of channel and pier at Montrose	619, 155
Total	3, 195, 632
Add contingencies and engineering	194, 368
Total cost	3, 390, 000
"For a canal three hundred feet wide in embankment and two hundrin excavation, and six feet deep, with a channel at Montrose six	ed feet wide
"For a canal three hundred feet wide in embankment and two hundr in excavation, and six feet deep, with a channel at Montrose six	ed feet wide feet deep—
"For a canal three hundred feet wide in embankment and two hunds in excavation, and six feet deep, with a channel at Montrose six Cost of canal embankment and walls	red feet wide feet deep— \$1, 454, 680
"For a canal three hundred feet wide in embankment and two hunds in excavation, and six feet deep, with a channel at Montrose six Cost of canal embankment and walls	red feet wide feet deep— \$1, 454, 680 371, 265
"For a canal three hundred feet wide in embankment and two hunds in excavation, and six feet deep, with a channel at Montrose six Cost of canal embankment and walls	red feet wide feet deep— \$1, 454, 680 371, 265 244, 910 242, 822
"For a canal three hundred feet wide in embankment and two hunds in excavation, and six feet deep, with a channel at Montrose six Cost of canal embankment and walls	red feet wide feet deep— \$1, 454, 680 371, 265 244, 910 242, 822
"For a canal three hundred feet wide in embankment and two hundr in excavation, and six feet deep, with a channel at Montrose six Cost of canal embankment and walls	red feet wide feet deep— \$1, 454, 680 371, 265 244, 910 242, 822 2, 313, 677 619, 155
"For a canal three hundred feet wide in embankment and two hundr in excavation, and six feet deep, with a channel at Montrose six Cost of canal embankment and walls	red feet wide feet deep— \$1, 454, 680 371, 265 244, 910 242, 822 2, 313, 677
"For a canal three hundred feet wide in embankment and two hundr in excavation, and six feet deep, with a channel at Montrose six Cost of canal embankment and walls	red feet wide feet deep— \$1, 454, 680 371, 265 244, 910 242, 822 2, 313, 677 619, 155

"For a canal 300 feet wide and five feet deep, with a ch feet deep—	annel at l	Montrose five
Total for a canal six feet deep and locks, as above \$2 Deduct difference in cost for five feet	, 576, 477 220, 000	
Cost of canal and locks	619, 1 <i>5</i> 5 157, 500	\$ 2, 356, 477
Cost of channel and pier.		461, 655
Total	•••••	2, 818, 132 171, 868
Total cost	•••••	2, 990, 000
"For a canal 300 feet wide in embankment, 200 feet is feet deep, with a channel at Montrose five feet deep— Total for canal six feet deep and locks, as above \$2,	313, 677	tion, and five
feet deep, with a channel at Montrose five feet deep-	313, 677	tion, and five
feet deep, with a channel at Montrose five feet deep— Total for canal six feet deep and locks, as above \$2, Deduct difference in cost for five feet Cost of canal and locks	313, 677 220, 000	
feet deep, with a channel at Montrose five feet deep— Total for canal six feet deep and locks, as above \$2, Deduct difference in cost for five feet Cost of canal and locks	313, 677 220, 000 619, 155 157, 500	
feet deep, with a channel at Montrose five feet deep— Total for canal six feet deep and locks, as above \$2, Deduct difference in cost for five feet Cost of canal and locks	313, 677 220, 000 619, 155 157, 500	\$ ≥, 093, 677

From the foregoing it will be seen that provision has been made in the estimate for the reduction of the canal, where excavation is necessary, to 200 feet in width, should it be deemed advisable; but this is not recommended, since the full width of 300 feet will be required for the meeting of boats and their barges ascending and descending. As the delays are more apt to occur at the locks than elsewhere, the full width should be preserved especially in their vicinity. If the canal for boats of four-feet draught is adopted, its width of 300 feet should be preserved throughout.

THE ROCK ISLAND RAPIDS.

The Rock Island or upper rapids extend from Le Claire to Davenport, a distance of 14.26 miles, measured on the Iowa shore, with a total fall of 21.46 feet. The mean width of the Mississippi in this distance is about 2,500 feet varying from 1,500 feet (at Port Byron) to 3,960 feet (below Campbell's island.) The area of cross-section varies from 6,829 square feet (on Moline chain) to 21,093 square feet (at the foot of St. Louis chain.) The greatest velocity is on Moline chain, being 5.0545 feet per second, or about 3½ miles per hour, at low water. The least velocity is found opposite Hampton, being not greater than 1½ mile per hour.

The great difference, in hydrographic features, between the Rock Island and the Des Moines rapids must find its explanation in the geological structure of the country. At the Des Moines rapids the river runs over an inclined stratum

of a single rock formation, the cherty limestones of the Keokuk series, the dip of which is equal to the present inclination of the water surface. At the Rock Island rapids the geological conditions are entirely different. Here we find, at the head of the rapids, near Le Claire, a magnesian limestone of upper silurianage, (Niagara group.) about 50 feet thick, dipping to the southward more rapidly than the surface of the water, its top layers disappearing below water near Hampton. Succeeding this, in regular sequence, we find below Hampton limestones of the Devonian age, (commonly referred to as the Hampton group.) the successive ledges of which crop out on both river banks, and occasionally in

the river bed, forming reefs or chains.

The consequences of this difference in geological structure are at once apparent in a comparison of the profiles of the two rapids. While in the Des Moines rapids we find a uniform depth and width throughout their entire length, varying only in depth and width of comparatively insignificant fissures or channels, we have in the Rock Island rapids some reefs or chains obstructing navigation for a short distance each, and separated by deep intervals or pools from six to thirty feet in depth. Of the fourteen miles between the head and foot of the rapids, nearly eleven miles afford good navigation in the lowest stages, the obstructed portion covering a distance of only a little more than three miles. At the Rock Island rapids the total descent is about the same as at the Des Moines rapids; but in the latter case it is mostly found in the lower seven miles, whilst in the former it is nearly equally distributed over their entire fourteen miles.

The average length of time when the water is less than four feet on the Moline, the shoalest of the chains, is about ninety days, more than four feet about 180

days, or two-thirds of the average boating season.

From these circumstances, and the reasons set forth in the report of my assistant, Brevet Colonel P. C. Hains, embodied herein, I have the honor to recommend that the navigation of these rapids be improved by excavating the natural channel, so as to give a width of 200 feet and a navigable depth of four feet, at extreme low water, the plane of reference being the low water of 1864. The disparity of cost between this plan and that of a canal leaves no doubt in my mind as to which should be adopted, notwithstanding the fact that the slack-water of the canal would be a great advantage in itself to the boats navigating it.

It is believed that the plan recommended can be executed at these rapids without any extraordinary difficulty; and as the river does not average over 2,500 feet in width, the channel, when completed, will conform to the natural direction of the main current, will be free from hurtful cross-currents, and will not be difficult to follow during the prevalence of unfavorable winds. A few buoys, or pyramids of stone, properly placed, will enable the boats to navigate it during the night. From this it will be seen that the proposed excavated channel promises all that is required. When finished it will be good for all time, will require no attendance or repairs, and will not interfere with any other plan, should the future improvement of the river render a further improvement necessary.

The amount of commerce and navigation interested in the improvement of these rapids is essentially the same as that for the lower rapids, and hence for information on this point I respectfully call attention to the "statement" pre-

viously mentioned.

The details of the plan of improvement of the upper rapids are given in the following report of Brevet Lieutenant Colonel Hains:

DAVENPORT, IOWA, December 20, 1866.

Sir: In obedience to your orders of the 5th of October, 1866, directing me "to proceed to Davenport, Iowa, for the purpose of making a detailed survey and examination of the Rock Island rapids of the Mississippi river," and of

your letter containing more detailed instructions of the same date, I have the honor to submit the following report, with the accompanying drawings:

I left Keokuk, Iowa, on the 6th of October, 1866, arriving in Davenport Iowa, the next day, and immediately commenced organizing my parties, and in order that the work might be pushed forward with the greatest despatch consistent with the importance of the work, and inasmuch as the season for work would be of short duration, I put into the field all the force that could work to advantage.

The hydrographic party was placed under the immediate charge of Mr. J.E. Abbott, civil engineer. Their work included all that related to the topography of the bed of the river, and other information concerning the flow of the water

over the rapids.

A larger party was placed under the charge of Mr. W. D. Clark, civil engineer, with a view of making an accurate survey of the valley on both sides of the river, showing the meandering of the shores, and gathering all other information necessary for the investigation of the several projects for the improvement of the navigation on the upper rapids. Lines of levels were run on both shores, from a point about four miles below Rock Island to a point about four miles above Le Claire; perpendicular offsets connecting with the main line at various distances from 50 to 500 feet apart, according to the changes in the general feature of the shore, were run.

The accompanying maps show the work that has been accomplished, better

than any explanation.

As the time allowed us would not warrant an entire resurvey of the bed of the river, and, moreover, as General Warren's map, wherever tested, proved to be sufficiently accurate, I caused Mr. Abbott's party to restrict themselves, at first, more particularly to a thorough examination of the bottom on the chains, in order to get the most accurate possible data for estimating the amount of rock excavation necessary to make a channel of 200 feet width and four feet depth in low water. A favorable season has, however, enabled us to accomplish more

hydrographic work than could reasonably have been anticipated.

The upper or Rock Island rapids begin at a point near the lower end of Rock Island, and extend 14 $^{26}_{00}$ miles up the river to a point near the lower end of the town of Le Claire. The bed of the river throughout this entire distance consists of a hard surface of limestone rock, worn in many places into deep furrows by the long continued action of the water and the material washed along the bottom. This rock crops out along the shores, and is generally found stratified in thin layers; the lower strata in the bed of the river appear to be harder and of different thicknesses, from four inches to two feet and upwards. There are also a number of large erratic boulders of granite to be met with, but these, as a general thing, do not present serious obstructions, but in some cases, as at Campbell's chain, they rather serve as guide marks for pilots, who would protest against their removal on that account, unless replaced by other equally permanent marks.

The only difficulty in the way of navigating the rapids consists in passing over the chains, of these there are seven, viz: the Upper or Smith's chain, Sycamore, St. Louis, Campbell's, Duck Creek, Moline, and Lower chains. At these places the rocky bed of the river projects out from each shore like a bar, the projecting points sometimes overlapping each other, leaving only a narrow, tortuous channel between them, and in some instances extending like a dam or rocky bar entirely across the river. Between the chains, throughout almost the entire distance, is a wide and navigable channel, with plenty of water for boats that navigate the upper Mississippi, and at such places the velocity of the current is much less than on the chains.

Between the head and foot of the rapids, a distance of a little more than four-

teen miles, nearly eleven miles are good navigation in the lowest stages, the obstructed portion covering a distance of only a little more than three miles.

The channel pursued by steamboats, and that followed by rafts, are indicated on the accompanying map. The centre line of the steamboat channel is also

shown in profile.

By referring to the map it will be seen that the steamboat channel, beginning at the head of the rapids, runs in close to the Iowa shore, with plenty of water until it strikes the Upper chain, generally called by pilots Smith's chain; here the channel is narrow, crooked, and the current swift, having a velocity of more than three miles per hour. A large reef or rocky bar known as Asprey patch stands in the middle of what would otherwise be a wide channel. This chain is not considered, however, as difficult or dangerous as most of the others.

Passing Smith's chain, the channel inclines gradually toward the Illinois shore, until it comes to Sycamore chain, which is conceded to be the most difficult place to pass on the whole rapids. Here the rocky ledges project out from each shore, leaving between them only a narrow and crooked water-way. The current being swift and the turns short, boats in passing are exposed to strong cross currents, which tend to sweep them on the lower ledge; besides, in one of the sharp bends a deep pocket has been cut, and a large amount of water runs through it, which by its action tends to draw boats into it, where they sometimes become fastened, and to extricate them involves a loss of much time, and is a labor of great difficulty.

The difficulties at Sycamore chain are not the result of a want of sufficient depth of water, for by referring to the profile it will be seen that there is a good depth in the channel, but they arise from its narrowness and crookedness, together

with the strong cross current that sweeps over it.

After passing Sycamore chain the channel runs close to the Illinois shore, passing inside of Crab island, where it becomes very narrow, and then inclines towards the Iowa shore, until, at St. Louis rocks, it reaches a point about midway between the Illinois shore and Fulton's island. Passing the St. Louis rocks it again inclines toward the Illinois shore, until it reaches St. Louis chain, where the channel becomes narrow again, but boats that pass the chains above or below this seldom experience great difficulty here. Below this chain the channel opens out gradually into a stretch of three miles, perfectly navigable at all times. In front of Hampton the current becomes quite sluggish.

Opposite the head of Campbell's island the channel crosses Campbell's chain, which is not only crooked and exposed to cross currents, but the rocky ledge extends entirely across the river. In the channel pursued by steamboats across this chain the water is not much deeper than on either side of it. The slough

behind Campbell's island is not used for navigation.

After passing Campbell's chain, with the exception of the rocks near Winnebago island, which are somewhat of an obstruction, the channel is wide and easily navigated until it comes to Duck Creek chain, nearly three miles below. Here it is crooked and narrow, so much so as frequently to necessitate the use of auchors at low water for the purpose of working boats through. This is another difficult chain to pass.

Below Duck creek the channel widens out again, giving good navigation, with the exception of one narrow place for about two miles, when it comes to Moline chain. Here again the ledge of rock extends entirely across the river and forms in low water an impassable barrier to boats drawing more than thirty inches. The water passes over this chain at a mean surface velocity of 3.878 feet per second at low water, and a maximum velocity of 5.0545 feet per second, as determined by actual observations with floats.

The dams at Moline and Little Rock island cut off a large body of water that would otherwise flow out of the main channel, and the universal testimony of the pilots establishes the fact that they have raised the water on this chain some ten

inches. It is generally conceded that the navigation has been materially benefited in low water, but the increased volume of water has no doubt increased the velocity of the current also. During the low stages, however, when the velocity of the current is less than at high water, this increase is of little account in com-

parison with the advantages of getting the increased depth.

From Moline chain the channel widens out again, becomes deep, inclining towards the Iowa shore, and is perfectly navigable for the largest boats on the upper Mississippi until it comes to the Lower chain. The channel here is very crooked, but the current is not so swift as on some of the other chains, and consequently not so difficult to pass. This chain is about a half mile above the Chicago and Rock Island railroad bridge, and no more natural obstructions present themselves below this point in the ordinary low stages.

From the above statement of facts it is evident that these rapids are a serious obstruction to navigation; and though a comparatively small sum of money, judiciously expended, would do much to relieve the pressing demands of commerce, they have stood and still stand a barrier to the free and full development of the resources of the great Mississippi valley. True, the States bordering on the river have shown unexampled advances in population, wealth, and prosperity,

but it has been in spite of the natural obstacles in their way.

I have no data on which to base an estimate of the loss occasioned by steam-boats being delayed on the rapids; they frequently remain fastened on the rocks for days at a time; during the present season one boat, the "Little Giant," was delayed on the Sycamore chain for ten days. Besides the loss occasioned by delay, there is the danger of loss of life as well as the loss of boat and cargo. All these are arguments that call for improvement in the navigation.

It does not devolve upon me to enter into a discussion of the benefits to be derived from the successful execution of a judicious plan of improvement. It must be evident to all that it has now become an absolute necessity, and some

plan must at once be carried out.

The average length of the boating season is about 260 days. During the winter.

as a matter of course, navigation is closed by the ice.

When the river is open for navigation, about one-third of the whole time is rendered dangerous by the shoalness of the water on the rapids, and sometimes impassable for boats drawing more than two feet. In the year 1864 the water was lower than had been known before in many years, attaining its lowest point September 2.

From the record of the stages of water kept at the Chicago and Rock Island railroad bridge, it is found that the greatest range between high and low water during the last seven years is fifteen feet and nine and one-half inches, being the high water of 1862 and the low water of 1864, the mean range during the

same period being less than twelve feet.

The range between the highest floods and lowest water at other points along the rapids, from the best authority that could be obtained, are at Valley City, opposite Hampton, thirteen feet and eight inches, and at Le Claire twelve feet which, if correct, shows a diminution of only three feet and nine inches in the

fall at high water as compared with that at low.

The average width of the river on the rapids is about one-half mile. At Le Claire it is only fifteen hundred feet in one place, but widens out above and below. Below the rapids the river is wider than on them, as may be seen from the map. A line of levels, from the head to the foot of the rapids, shows a fall of 21.46 feet in a distance of about fourteen miles, or an average fall of 1.53 feet per mile in low water.

The greatest fall is on Moline and Sycamore chains, as may be seen by re-

ferring to the accompanying profile.

The area of a cross section at the head of the rapids, where the river is only

1,650 feet wide, is 30,220 square feet; at a point near Sycamore chain 12,408

square feet; at Moline chain 6,829 square feet.

Careful experiments were made on the velocity of the current in order to determine the amount of discharge over the rapids in ordinary low water. For this purpose the stations were taken, and a number of velocities between them, at different distances from the shore, determined by floats; a mean of these was taken as the surface velocity.

The mean area of the two cross sections at the stations was taken as the area of the cross section, and by applying this to D'Aubisson's formula for the approximate discharge of a river, it was found to be 36,456 cubic feet per second. The approximate discharge behind Campbell's island was also determined in the same manner, and was found to be 10,276 cubic feet per second.

With the exception of the places where the bluffs approach close to the river,

the banks are usually steep and rocky.

In your letter of instructions I am required to "locate the Chicago and Rock Island railroad bridge on my maps, and ascertain the direction of the current through its bays, and state its general influence on the free navigation of the stream by every kind of craft." The bridge is accurately located on the map of the rapids herewith transmitted, and also on an enlarged scale on sheet No. 2. A series of experiments with floats sunk in the water two feet below the surface were made, and the lines taken by them in floating down the stream carefully fixed by frequent observations with three theodolites.

The course of each is indicated on the map, and by an inspection of it it will be seen that the turn-table pier makes an angle of about sixteen degrees with the direction taken by the floats. This was in low water. The angle of inci-

dence increases as the water rises.

Having obtained data sufficient to enable me to state its general influence on the free navigation of the stream, I found that the subject had been thoroughly investigated by more able engineers in 1859. I refer to the board of topographical engineers convened by an order of the topographical bureau, March 1, 1859, and consisting of Captains Humphreys, Meade, and Franklin, of the corps of topographical engineers. From their report I extract the following, which shows very clearly the influence it has on the free navigation of the river. I need only add that my investigations are confirmatory of their conclusions.

"The Chicago and Rock Island railroad bridge is thrown from the island of Rock Island to the city of Davenport, Iowa, and it is supported by two stone abutments on the shores, and six stone piers. The spans (five in number) are 250 feet broad, the draw-span being at the water level (9½ feet stage) 117 and

112 feet respectively. The whole length of the bridge is 1,535 feet.

"The piers, except those of the draw, are thirty-five feet long, and seven feet broad at top, and fifty-three feet long, and eleven feet broad at bottom. The two small draw piers thirty-eight feet long, and ten feet broad at the top, and fifty-four feet long, and fourteen feet broad at bottom. The turn-table pier, including the guard pier and starling, is 350 feet long, 40\frac{1}{3} feet broad at the top, and 386 feet long, and 45 feet broad at the bottom."

The following is the decision of the board:

"1st. That the railroad bridge which crosses the Mississippi river between Rock Island, in the State of Illinois, and Davenport, in the State of Iowa, is not constructed according to correct principles, reference being had to the interests of navigation.

"2d. 'The piers of the said bridge are not of the best form, and that there was no practical difficulty in constructing them of the proper form. With the exception of the turn-table pier, the board is of the opinion that the defective form

of the piers is a matter of no material importance.

"3d. The only pier longer than is necessary is the turn-table pier. This pier, in the opinion of the board, should have been constructed no larger than was absolutely necessary to sustain the truss when the draw is open, and protect it from injury from passing boats. It might have been constructed with a length of 295 feet, affording ample support and protection, and being actually 355 feet in length. The difference (sixty-five feet) is unnecessary, and, in the opinion of the board, pernicious. The effect of making it longer than was absolutely necessary is to contract the water way, increase the velocity, narrow the draw-passage, and present more surface for boats to strike against, thus increasing the difficulty of their passage through the draw. In a pier of this size the starling is of importance, and the upper faces of the piers should have been curved surfaces.

"4th. The piers are not placed parallel with the current, but at angles varying from 26 degrees to 14 degrees 30 minutes. The effect of this obliquity is to treble the obstruction to the flow of the water, and, consequently, to affect the increase of velocity in the same ratio. Another consequence is, that the passages of steamboats and rafts through the draw, and between the piers, are rendered much more difficult and hazardous; furthermore, the draw on the Iowa side is rendered useless by the formation of an eddy therein.

6th. "The eddy on the Iowa side of the turn-table pier, as nearly as could be estimated, is about 100 feet wide at the foot of the pier, and the turbulence, or boiling of the water, extends about 500 feet below. This eddy is constantly varying in its position and dimensions. The effect on the passage of boats ascending and descending is undoubtedly to render them more difficult, on account of the care required to avoid getting one part of the boat in it, when another part of it is in the current of the draw.

"8th. The bridge is badly located, and, in consequence of this bad location, is a greater obstruction to the passage of steamboats and rafts than would have been necessary, had the location been good. Any site in the vicinity below Rock Island, out of the rapid current, would have been better. The board, having this point in view, examined the line of the ferry between Rock Island city and Davenport, and found there would be no practical difficulty to the erection of a bridge at this site, or near it, which, if constructed upon proper principles, would be of no material obstruction to navigation."

Although there is no evidence of any specific plan having been proposed for the improvement of these rapids, except that of improving the natural channel itself by excavation, so as to give four feet depth in low water, and a passage-way 200 feet in width, several plans have been proposed in a general way for overcoming the difficulties.

It has been proposed to erect dams across the river, with locks for steamboats and chutes for rafts and flatboats, making a slack-water navigation in the river. A careful inspection of the map of the rapids will show that in order to carry out such a plan it will require at least two locks and three dams to avoid overflowing the bottom lands between the river and the bluffs; one dam and lock should be located just above the town of Hampton; the next lock near the city of Rock Island and the Illinois shore, and one dam at the head and another at the foot of Rock island, between it and the Iowa shore. But without making a detailed estimate of the cost of such a plan, it seems to me altogether inadmissible for the following reasons, viz:

1. No craft of any kind, however large or small, could pass up stream without going through the locks. The smallest skiff and largest steamboat would alike be subjected to this inconvenience at all times.

2. It would obstruct the navigation of the river during the time when no difficulties are experienced on the rapids. For five or six months in the year,

or about two-thirds of the navigable season, the river is high enough not to require an improvement, and in order to overcome the natural obstructions during a period of about three months, we would be substituting an artificial one during the other six.

In planning an improvement for the rapids of the Mississippi river, one point must not be lost sight of, viz: the improvement must not of itself become an obstruction, and no plan should be adopted for the improvement of navigation in low water that would be prejudicial to its present state in high water. On this point I am not alone, and respectfully call your attention to the report of the board of engineers, convened at my request and in obedience to your orders, a copy of which is herewith appended.

PLAN OF IMPROVEMENT BY MEANS OF AN INDEPENDENT LATERAL CANAL ON BITHER SIDE OF THE RIVER.

I have no hesitation in saying that in my opinion this plan is far preferable to the foregoing, no matter on which side the canal should be located. Though it has some objections, it recommends itself from the fact that it in no way interferes with the river as it is, and will be navigable at all times, night or day,

windy or foggy weather.

I have not made any estimate of the cost of constructing a canal on either side of the river: first, on account of the limited time at my disposal; and second, because I think the time has not yet come when it is necessary to resort to a canal as a means of overcoming the difficulties of navigation on these rapids. It is certain, however, that a canal of the dimensions necessary to accommodate the commerce of this river cannot be built on either side for less than about two and a half million to three and a half million dollars.

The third plan, viz., by improving the natural channel of the river, will recommend itself to all on account of the obvious advantages of having an improvement of this kind free from all tolls and sources of delay. I have already

described the difficulties in the way of navigation on the rapids.

Between the chains there are long reaches of navigable river ranging in depth from six feet upwards, the difficulties being confined to the chains almost entirely, a few boulders and patches of rock, which can easily be removed, occurring here and there between them.

As I have already remarked, I think the time has not yet come when a canal is necessary in order to overcome the difficulties of navigation on the rapids, nor is it difficult to see that even a canal improvement would fail to confer all the benefits on commerce that a perfect improvement of the river itself would.

I have conversed with a number of persons interested in the navigation of the upper Mississippi, and they all admit that four feet of water on the rapids is all that is required. In fact, in low stages, boats cannot carry more than from three to four feet above and below, and hence more than four feet on the rapids is unnecessary. As this depth can be had more economically and quickly by the plan of improving the channel than in any other way, and, moreover, as it gives all the facilities that the present commerce demands, I am of the opinion

that this is the plan that should be adopted.

If, however, at any future time the navigation of the river should be improved, so as to produce more than four feet above and below the rapids in low water, and consequently to require more than four feet on the rapids, I would recommend the adoption of some other plan, as the amount of excavation in that case (supposing the river was improved so as to secure six feet) would, probably, be so great, as to render its cost far more than the construction of a canal. But let me add, that whatever may be the ultimate demands of commerce, or the ultimate plan that may be adopted to satisfy those demands, this improvement will in nowise be thrown away. It will always be useful, and should, in my opinion, be made, even though a canal were to be built to-morrow.

There are many boats navigating the upper Mississippi that never require more than four feet, and for such this improvement will at all times be ample; besides, it is probable that when more than four feet has been obtained in low water on the bars above and below the rapids, the commerce of this river will be beyond the capacity of any one canal to accommodate.

There are many advantages connected with the channel improvement that do not exist in connection with other plans, and these must all have weight in

deciding this question.

It will be free. It will not require an annual appropriation from the general government to keep it in repair, and when completed will be permanent. It will not interfere with a canal improvement in case that should at some future time become advisable or necessary, and, moreover, the benefits arising from this plan will be felt as the work progresses. Every patch of rock removed is that much benefit to navigation, whereas a canal improvement can be of no benefit until completed.

The latter, however, has one advantage, viz., in affording safe navigation in

the darkest and stormiest nights.

The principal objection urged against the application of the channel improve-

ment at these rapids is without substantial foundation.

It is feared that by deepening the channel to four feet in low water and widening it to two hundred feet, will increase the capacity of the water-way to such an extent as to draw off from the pools above such an amount of water as to develop new dangers where these heretofore did not exist. But by a careful examination one can scarcely fail to see that this objection is groundless.

At the Upper or Smith's chain, besides removing a few patches of rock that are a source of difficulty, there will be a few points of the projecting ledges cut off as indicated on the accompanying map. The excavated material can be deposited below the last cut in order to check the velocity of the current, and make it conform more to the direction of the channel at this turn. Certainly this can have no prejudicial effects; the channel is already deep enough, the difficulty being in making the sharp turns.

At Sycamore the cutting will be almost entirely from the projecting ledges, and the material can be used in closing the deep pocket that runs off towards Mechanic's Rock, which produces a strong cross-current and carries off an im-

mense volume of water from the main channel.

The amount of water cut off from this pocket will more than compensate for the increased capacity of the new channel.

The other cuts between this chain and Campbell's are so slight as to render

their consideration unnecessary.

At Campbell's chain there will be a long cut, and the capacity of the new channel here will doubtless be increased, but the current is not rapid, and by closing the slough behind Campbell's island, (which, as I have already remarked, is not used for navigation,) with the excavated material a much larger volume of water can be thrown into it than its increased capacity can carry off.

I should have mentioned that by closing this slough behind Filton's island some benefit would doubtless be felt in the raft channel and on St. Louis chain.

At Duck Creek chain we cannot compensate for the increased capacity of the new channel by closing up sloughs in its vicinity, but we can accomplish the same end by depositing the excavated material in the river in the form of a dam, and thus throw into it any amount of water that may be necessary.

At Moline chain the slough behind Rock island has already been practically closed by the Moline dam, and according to the testimony of pilots the depth of

the water in the chain has been increased at least ten inches.

There is no doubt but that the removal of a portion of this reef, as it extends entirely across this river, will affect in a slight degree the level of the water for a short distance above, and may necessitate the removal of a few rocks. But

the upper edge of the reef where the channel crosses it slopes off into deep water, as may be seen by referring to the accompanying profile of the channel.*

By means of a dam made of the excavated material, sufficient water can be thrown into the new channel to more than compensate for its increased capacity

of discharge.

The water-way across this reef will be increased in cross sectional area some three hundred square feet, but the material taken out would make, if desired, a dam of loose stone four feet high that would extend entirely across the river. The excavated material can, in each and every case, be disposed of during the operation of the work for closing lateral channels to compensate for the extra quantity of water that would otherwise be drawn off. The places at which the excavation is recommended are shown on the map of the rapids transmitted herewith, and the estimated amount at each place in the annexed tabular statement.

Believing that the plan of improving the natural channel of the river is the one that should at the present time be carried out at these rapids, and also that the practicability of getting four feet water in the lowest stages for a channel of two hundred feet in width is beyond question, I desired to have my views strengthened if they were correct, or their fallacy demonstrated if not; for this purpose I requested that a board be convened, consisting of civil engineers in the government service, for the purpose of considering some of the proposed plans of improvement.

The report of the board I enclose herewith.

In regard to the condition and influence of the dams at Rock island, and as to whether or not they can be continued for mill purposes, without injury to navigation: The dams are located on the map accompanying this report; their influence on navigation has been noticed. These are without a benefit to navigation on Moline chain. Their removal is not necessary, but, on the contrary, they had better remain. I would suggest, however, that in low stages they should not be permitted to consume more water than passes through them under existing circumstances. In high stages it makes no material difference.

I have carefully considered the means by which the excavation in the channel can be effected in the most economical and at the same time expeditious manner. Rock excavation in running water is a difficult undertaking in any case. A comparison with the cost of excavation at the lower rapids by Lee and Floyd can scarcely be instituted as a basis for the same operation here. In the cases referred to the larger portion of the money was expended in preparing machinery, and when the work was well under way the appropriations ceased. The consequence, as might be expected, was, a large sum of money expended and but little work accomplished.

In estimating the cost of excavation at these rapids I have endeavored to ascertain as nearly the actual sum required as possible, and have taken into consideration the delays occasioned in removing working parties from one point to another, the difficulties of the position, and the character of the rock to be excavated I unhesitatingly recommend the use of coffer-dams at all points where the amount to be excavated will warrant its expenses, as the most satis-

factory and certain in its results of any plan yet proposed.

These points are indicated in the tabular statement above referred to.

The following figures represent the plan, elevation, and section of a cofferdam, on which the estimates for coffer-dams are based. It consists of two rows of two-and-a-half-inch iron rods driven into holes drilled in the rock, the distance between the rows and the rods themselves being each five feet; they are

^{*} In making this cut, the slope of the water surface will be lessened, and the area of the cross section increased. These are variable elements that enter into the problem of discharge, and would probably neutralize each other, so that the velocity and discharge would remain sensibly what they were before.

braced and tied by diagonal iron bars, as indicated in the plan. The interior and exterior rows of sheeting piles consist each of two rows of two-inch plank inserted between the wale or string-pieces, and breaking joints with each other; the number of string-pieces may be increased or reduced, as the case seems to require. The holes into which the main piles are driven should be one and a half foot deep and drilled somewhat smaller than the piles themselves, the latter being driven into them. Between the piles there is five feet of puddling. To give additional stability to the dams they should be securely braced from the inside whenever necessary.

Statement exhibiting the amount of rock to be excavated in order to make the present channel two hundred feet wide and four feet deep in low water, and the estimated cost of excavating at the various places.

•	0				
Locality.	Number of cubic yards to be excavated.	Cost of excavation per cubic yard.	Length of coffer-dam to be used.	Cost of coffer-dam per linear foot, including construction, pumping,	Total cost of excava-
	- 	<u> </u>		<u> </u>	<u></u>
SMITH'S CHAIN.		1 .	Feet.		
Point A	. 20	\$10 00			\$200 00
<u>B</u>	755	10 00			7, 550 00
<u>c</u>	1, 357	10 00			13, 570 00
D	246 80	10 00 10 00			2,460 00 800 01
Ospreyt's patch*	1,970	10 00			35, 460 00
Point F	652	10 00			6 520 00
G †	2, 277	4 50	1, 100	\$25 00	6, 520 00 37, 746 00
H	296	10 00			2, 960 00
Total	7, 653				107, 266 50
			İ		
SYCAMORE CHAIN,	ł			ļ	
Point A	955	10 00			9, 550 00
В	1, 630	12 00			19, 560 00
<u>c</u>	160	10 00		<u> </u>	1,600 00
D	212	14 00			2,968 00
E ‡	5, 416	4 00 14 00	1, 500	25 00	59, 164 00 14, 518 00
G	1,037 1,008	14 00	• • • • • • • • • • • • • • • • • • • •		14, 112 00
Н	1,006	12 00		• • • • • • • • • • • •	10, 632 00
χ	1, 632	5 00	300	25 00	15, 660 00
j*	338	17 00			5, 746 00
K*	109	19 00			2,071 00
Total	13, 383		•••••		155, 581 (0
CRAB ISLAND.					
Patch A	00	15 00			1 047 00
B	83 1,000	15 00			1, 245 00 15, 000 00
Point C	200	10 00		••••••	3,000,00
Ď	502	10 00			2, 000 00 5, 020 00
Total	1, 785			[-	23, 265 (0
1000					<u> </u>
ST. LOUIS CHAIN.					
Point A	211	10 00		1	2, 110 0
		10.00			6 MSR) (A)
В	689	10 00		••••••	6, 890 W
		10 00			9,000 00
B	689	10 00			
Total	689				9,000 00
B Total CAMPBELL'S CHAIN. Boulder A	900	20 00		:	9,000 00
B Total CAMPBELL'S CHAIN. Boulder A	900 3 637	20 00 15 00		-	9,000 00
B	900 900 3 637 5	20 00 15 00 20 00		:	9, 000 00 60 00 9, 535 00 100 (\)
B Total CAMPBELL'S CHAIN. Boulder A Centre patch B Boulder C	900 900 3 637 5 4	20 00 15 00		=	9, 000 00 60 00 9, 555 00 100 (a) 80 00
B	900 900 3 637 5	20 00 15 00 20 00	3, 000	25 00	9, 000 00 60 00 9, 535 00 100 (\)

^{*} Very difficult to work at.

[†] Coffer-dam can be used to advantage.

[!] Coffer-dam to be used.

Statement exhibiting the amount of rock to be excavated, &c.—Continued.

Locality.	Number of cubic yards to be excavated.	Cost of excavation per cubic yard.	Length of coffer-dam to be used.	Cost of coffer-dam per linear foot, including construction, pump- ing, &c.	Total cost of excava-
Campbell's Chain—Continued.			Feet.		
Point G	60 15	\$15 00 20 00			\$900 00 300 00
I	36	12 00			432 00
J	260	15 00		•••••	3, 900 00
Total	7, 967				118, 115 00
FROM CAMPBELL'S TODUCK CREEK.					
Centre patch A	118	12 00			1, 416 00 492 00
Patch B	41 55	12 00 · 15 00			825 00
D	47	15 00			705 00
Boulder F	64 5	15 00 20 00			960 00 100 00
Patch G	35	12 00			420 00
н	18	15 00			270 00 396 00
I	33 55	12 00 12 00	• • • • • • • • • • • • • • • • • • • •		660 00
J		12 00	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
Total	471				6, 244 00
DUCK CREEK CHAIN.		•			
Point A	569	10 00			5, 690 00
В	592 5, 258	10 00	1, 100	\$25 CO	5, 920 00 53, 790 00
Through-cut C*	3, 238 1, 178	5 00 10 00	1, 100	\$33 60	11,780 00
E	40	12 00		1	480 00
F	2, 180	10 00			21,800 00
Total	9, 817				99, 460 00
MOLINE CHAIN.		1	 		
Patch A	200	12 00			2, 400 00
Through-cut B	12, 215	4 00	3, 000	25 00	123, 860 00
Patch D	118	14 00			1,652 00
<u>F</u>	15 233	20 00	¹. 	ļ. 	300 00 3,728 00
F	233	16 00			3, 126 00
Total	12, 781				131, 940 00
LOWER CHAIN.]	1		
Point A	1, 347	10 00) !	13, 470 00
В	380	10 00			3,800 00
C	948	10 00			9,480 00
Patch D	8 5	20 00 20 00			160 00 100 00
E	6	20 00			120 00
		1	I	1	

^{*} Coffer-dam to be used.

From the foregoing we obtain the following as the estimated cost to improve the natural channel of the rapids:

Smith's chain	7, 653	cubic y	ards	\$107, 266	50
Sycamore chain	13, 383	"		155, 581	00
Crab island	1,785	**		23, 265	00
St. Louis chain	900	"		9,000	00
Campbell's chain	7, 967	44		118, 115	00
Campbell's chain to Duck creek	471	"		6, 244	00
Duck Creek chain			• • • • • •	99, 460	00

Moline chain	12, 781 2, 694	cubic yar	ds	\$131,940 27,130	00 00
Add for contingencies 20 per cent				678, 001 135, 600	
Total	• • • • • •			813,601	80

Average cost of excavation, \$14 16 per cubic yard.

It may appear at the first glance that the percentage which I have added for contingencies in excavating is rather large, but after a careful consideration of the subject I can only say I am convinced that it is not. Experience has taught me that in excavating rock at the lower rapids the cost is materially affected by the more or less favorable seasons for working; and, moreover, the rock being in strata of various thicknesses, if in deepening a part to four feet we should come to a stratum two feet thick, the entire stratum must be taken out. The estimate I consider as small as can reasonably be expected to execute a work of such magnitude; at the same time I am of the opinion that the work is of such importance as would justify the expenditure of treble that amount.

I will only add that I consider promptitude in engaging on this work, and energy in prosecuting it, of vital importance. If it is to be done at all it should be done promptly. Every moment lost is a loss to commerce and to the country.

Estimate of the amount that can be advantageously expended in the fiscal year ending June 30, 1867, \$100,000; amount that can be advantageously expended in the fiscal year ending June 30, 1868, \$500,000.

This estimate is based on the fact that economy requires that the most important portions of the work, as that of Sycamore and Moline chains, for instance, when once begun should be pushed forward to completion at once, so as not to necessitate the expense of coffer-dams twice in the same place. The balance to be appropriated and used in removing those points of the least difficulty and during the succeeding low-water season.

Very respectfully, your obedient servant,

PETER C. HAINS,

Captain of Engineers, Brevet Lieut. Colonel U. S. A.

Brevet Major General J. H. WILSON, U. S. A.,

Lieut. Colonel 35th Infantry, in charge of the

Des Moines and Rock Island rapids improvement.

Proceedings of a board of engineers convened at the United States engineers' office, at Davenport, Iowa, on the 19th day of December, 1866, in obedience to the accompanying order.

U. S. ENGINBERS' OFFICE,
DES MOINES AND ROCK ISLAND RAPIDS IMPROVEMENT
AND ILLINOIS AND ROCK RIVER SURVEYS,
Davenport, Iowa, December 19, 1866.

At the request of Brevet Lieutenant Colonel P. C. Hains, captain of engineers, in charge of the Rock Island rapids survey, a board of engineers will convene at this office, at 7 p. m. to-day, for the purpose of considering and recommending a plan of improvement for the above-mentioned rapids.

Detail for the board.—1. Brevet Lieutenant Colonel P. C. Hains, captain of engineers; 2. Mr. James Worrall, civil engineer, assistant; 3. Mr. D. C. Jenné, civil engineer, assistant; 4. Mr. W. F. Shunk, civil engineer, assistant,

who will act as recorder.

J. H. WILSON,

The board met in pursuance of the above order, and having carefully investigated the advantages and disadvantages of the various plans proposed for the improvement of the Rock Island rapids, recommend the following:

1. That the present steamboat channel be enlarged, by excavation, to a minimum width of 200 feet, and a navigable depth of four feet at the time of low water, which is somewhat greater than the ruling depth in the river north and south of the rapids during that season.

This recommendation is made in view of the economical execution of the

work and the present demands of commerce.

Should the ultimate interests of commerce demand a greater depth than four feet, as it would in case a greater depth could be obtained above and below these rapids, the plan of an independent canal and locks without dams would then best promote that interest, the practical result of such a plan being beyond question.

- 2. That the excavated material be so deposited in the river bed as to check cross-currents and confine the volume of water as far as practicable to the new channel.
- 3. That coffer-dams be used in carrying on the work, more particularly, however, at Moline and Campbell's chains. From the best information we can obtain they can be put in by the first of September, the period of low water beginning about this time, leaving at least three and a half months for taking out the material; but as the work is to be done by contract, the mode of operation may be left, to a great extent, to the contractor.

4. That the work should not be entered upon until an appropriation be made which will cover its estimated cost, experience having shown that small consecutive appropriations, in such cases, augment the ultimate expense beyond all reasonable calculation and are virtually thrown away in mere preparation.

5. That the improvement which has been proposed by means of locks and dams across the river is inadmissible. It would erect an obstruction permanent throughout the year, whereas at high and ordinary stages no hindrance to navigation now exists.

PETER C. HAINS,
Captain Engineers, Brevet Lieut. Col. U. S. A.
JAMES WORRALL,
Civil Engineer.
DANIEL C. JENNE,
Civil Engineer.
W. F. SHUNK,
Civil Engineer.

GENERAL REMARKS UPON THE NECESSITY OF IMPROVING THE UPPER AND LOW-BR RAPIDS, BASED UPON STATISTICAL AND COMMERCIAL DATA OBTAINED FROM OFFICIAL DOCUMENTS AND OTHER SOURCES.

The five States, Illinois, Missouri, Wisconsin, Iowa, and Minnesota, bordering on the Mississippi river are agricultural States, the great staples of which consist of wheat, corn, beef, and pork. They annually furnish from one-third to one-half of all the produce grown in the United States, viz: Wheat, 66,105,786 bushels, or about one-half the entire crop; corn, 244,986,768 bushels, or about one-half the entire crop; cattle, 2,526,979 head, or about one-third the entire crop; hogs, 4,896,506 head, or about one-third of the entire crop; or, in value, \$677,560,204. Add to this the value of the mining, manufacturing, and mechanical products of these States, valued at \$204,150,000, and we have a total of \$881,710,204. In the years 1861-'62-'63 the average yearly tonnage of all American ves-

sels engaged in trans-oceanic commerce, and entering the ports of the United States, was 2,564,252 tons, and the average tonnage of all the vessels of the various countries engaged in oceanic commerce, and entering the ports of the United States, was 5.341,867 tons. Now the three staples contributed by the five upper Mississippi river States just mentioned, to our exports, were equivalent to 1,315,000 tons annually. They therefore not only contributed one-third in value to our entire exports, but gave employment upon the ocean to more than one-half of our entire American tonnage, which was equivalent to one-fourth of all the tonnage of all our trans-oceanic commerce.

New York city is the controlling market for the States, and the value of the surplus products of the different sections are practically regulated by the market value of those products in New York. Accordingly, we usually find the price of wheat, corn, or pork at any particular place to be the respective price of those articles at that time in New York, less the cost of transportation thither. It is

the cost of transportation that absorbs the profit on the produce.

There are but two convenient routes of water transportation to New York, one from St. Paul by way of the great lakes and the New York canals, the other by way of St. Louis and New Orleans. The latter route is materially affected by the upper and lower rapids. Losses by detention and accidents on the lower rapids alone are estimated at \$500,000 yearly, and to cover extra hazard by these obstructions an additional insurance of thirty to fifty per cent. of the usual rates is asked.

If it were not for these obstructions, transportation by way of the Mississippi would be decidedly the cheaper, as appears from the following table:

The shipment of a bushel of wheat from St. Paul to New York costs:

Via Chicago:

Via Onicago.	
Freight from St. Paul to Chicago	\$ 0 32
Transfer at Chicago	2
Freight to Buffalo, New York	10
Transfer at Buffalo	2
Freight from Buffalo to New York	24
	70
Via St. Louis with the rapids improved:	=
Freight from St Paul to St. Louis	\$0 15
Transfer at St. Louis	3
Freight from St. Louis to New Orleans	124
Transfer at New Orleans	2
Freight from New Orleans to New York	20
Extra insurance	2
	55
	99

being a saving of at least fifteen cents per bushel in favor of the latter route, and giving to the produce the benefit of it, as well as a choice of markets.

The upper Mississippi valley in particular finds in the Mississippi river its only natural outlet for commerce, and, in spite of the present obstructions, employs a large amount of tonnage, as shown hereafter; and as this river is the only highway which is perfectly free to all classes of commerce and every species of craft, its improvement is a matter of interest to every citizen who lives in the region bordering upon it or its tributaries.

From official statistics it appears that of all the flour, wheat, corn, oats, barley, hay, hemp, tobacco, and pork (valued at \$235,873,878) which found a market at St. Louis during the year 1865, nearly seven-tenths were received from the upper Mississippi States, by the river, and only one-sixth came from other sources.

Mr. Howard, the collector of that port, states officially that \$780,706 97 were collected by him for duties on imports during the fiscal year ending Sep-

tember 30, 1866.

It is further stated by Mr. Egbert Dodge in his report to the board of directors of the Union Merchants' Exchange, St. Louis, that important diversions from the river are made at La Crosse, Prairie du Chien, Dunleith, and Rock Island, amounting in the shipments of flour, corn, barley, oats, hay, and cattle, to the east by rail, to eight-tenths of the entire surplus of the region; while the shipment by the river south, comprises only one-fifth. The chief reasons for this deflection are the existing obstructions at Davenport and Keokuk, though it cannot be denied that other important laws of trade are exerting their influence in the same directions.

The means of transportation, as steamboats, barges, &c., are corresponding in

extent to the considerable trade of the upper Mississippi valley.

From a statement prepared by Mr. James F. Griffith, secretary of the Northern Line Packet Company, we find that in December, 1865, there were 910 steamers, with an aggregate of 216,067.83 registered tonnage, and valued at \$24,556,600, on the Mississippi river and its tributaries.

Plying on the upper Mississippi alone there were 304 steamers, with an aggregate registered capacity of 96,296.86 tons and valued at \$10,556,600.

From this it will be seen that the upper Mississippi gives employment to onethird of all the steamers on the Mississippi and its tributaries, with nearly one-

half of all the tonnage.

This statement is not far from the truth, as the collector of the port of St. Louis alone certifies to 93,607.33 tons in steamers, and 13,055.69 tons in flats, barges, &c., or a total of 106,663.02 tons registered during the fiscal year ending September 30, 1866.

The number of barges, lighters, and similar craft used as auxiliaries is very large, but the enrolling of such being a new though advisable regulation, the

compilation of the same was not carried out.

The importance of the upper Mississippi is likewise shown by the following data:

In 1865 there were 3,823 arrivals of steamers in St. Louis, with a capacity of 1,229,826 tons, of which the upper Mississippi valley contributed 827, with a tonnage of 265,467 tons, or three-tenths of the whole; and of 2,953 departures, there

were 811, or three-tenths, for the upper Mississippi ports.

In 1866 the arrival of steamers at St. Louis was 2,972, with a tonnage of 1,227,078 tons, of which the upper Mississippi contributed 917 arrivals, with a tonnage of 377,804 tons, or four-tenths of all arrivals. The total of departures from St. Louis was 3,066, of which 943, or three-tenths, were for the upper Mississippi valley.

In 1866 no less than 677 steamers and 389 barges, going up stream, and 671 steamers and 365 barges, going down stream, passed through the Rock Island

railroad bridge.

Besides all this there is about 400,000,000 feet of lumber annually which finds its way to market on the Mississippi. Not less than two per cent. is added to the price of lumber on account of dangers and delays in passing the obstructions

at the rapids.

Contemplating the immense trade of the upper Mississippi valley, through its natural channel, the Mississippi river, we must come to the conclusion that the obstructions at Keokuk and Davenport are the great drawback of the growing States bordering on the river, and that the damaging influence of those rapids must necessarily increase with the constantly increasing population.

To represent the necessity of the proposed improvement in dollars and cents is, from the nature of the problem, simply impossible, but it is believed that sufficient statistics are embodied herein to show that the work is national and not

local or sectional. A simple calculation will show that the improvement of the rapids of the Mississippi will decrease the cost of transportation, by increasing competition between the carriers; thus adding greatly to the wealth of the entire northwest, and removing an oppressive tax from the eastern consumers of its products. Every farmer, manufacturer, and mechanic who depends upon the Mississippi river for the transportation of his produce to market, or who consumes a single article of foreign growth or manufacture, will get more for what he sells and give less for what he buys, when the navigation of the river shall have been made safe, easy, and economical. This being true, the people of the eastern States are nearly as deeply concerned in these improvements as those of the western. The Mississippi river is a great national highway, which cannot become the exclusive property of any chartered company, and therefore it must forever remain free to all classes of commerce and all species of craft.

The dangerous places of the sea-coast are marked by light-houses, and provided with harbors of refuge at the national expense, for the benefit of commerce not exclusively our own. It seems, then, no more than justice that the general government should authorize and provide the means for the improvement of a river in which our own people are exclusively interested, and in the navigation of which nearly every material interest of the entire country is intimately con-

cerned.

I am, general, very respectfully, your obedient servant,

J. H. WILSON, Lieut. Colonel 35th Infantry, Brt. Maj. Gen. U. S. A.

Brevet Major General A. A. HUMPHREYS, Chief Engineer U. S. Army, Washington, D. C.

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Estimate of the cost of constructing 7.6 miles of canal from Nashville to Keokuk, for the improvement of the Des Moines rapids.

For a canal 300 feet wide and six feet deep, including the necessary protection	n walls:
	\$3 , 000
Grubbing and clearing	45,000
304,000 cubic yards excavation of rock, at \$2	608,000
437,000 cubic yards excavation of earth, at 40 cents	174,800
674,000 cubic yards embankment, at 50 cents	337,000
400,000 cubic yards embankment hauled from excavation, at 25 cents	100,000
10,000 cubic yards lining, at 60 cents	6,000
50,000 cubic yards puddling, at 25 cents	12,500
143,000 cubic yards loose stones and riprap wall, at \$1 50	214,500
200,000 cubic yards riprap wall, made from rock excavation, at 75 cents	150,000
1,000 cubic yards slope and pavement wall, at \$2	2,000
5,000 cubic yards vertical wall laid in hydraulic cement, at \$5	25,000
3,170 cubic yards vertical wall laid dry, at \$1	12,680
3 miles changing line of railroad, at \$8,000	24,000
3 miles changing line of public road, at \$1,000	3, 000
Total	1 717 480
10001	1,777,400
For a canal 300 feet wide in embankment, 200 feet wide in excavation, and si	ix feet deep:
Grubbing and clearing	\$3,000
14 miles bailing and draining, including coffer dams, at \$30,000	45,000
200,000 cubic yards excavation of rock, at \$2	400,000
300,000 cubic yards excavation of fock, at 40 cents	120,000
	337,000
674,000 cubic yards embankment, at 50 cents	
400,000 cubic yards embankment hauled from excavation, at 25 cents	100,000
10,000 cubic yards lining, at 60 cents	6,000
50,000 cubic yards puddling earth, at 25 cents	
,	12, 500

244,910

143,000 cubic yards loose stone and riprap wall, at \$1 50	\$214,500
200,000 cubic yards riprap wall made from excavation, at 75 cents	150,000
1,000 cubic yards slope and pavement wall, at \$2	2,000
5,000 cubic yards vertical wall laid in hydraulic cement, at \$5	25,000
3,170 cubic yards vertical wall laid dry, at \$4	12,680
3 miles changing line of railroad, at \$8,000	24, 000
3 miles changing line of public road, at \$1,000	3,000
Total	1,454,680
=	
Felimate of the cost of constructing the longs look of 101 feet lift	for the im-
Estimate of the cost of constructing the lower lock, of 10\frac{1}{3} fect lift,	joi the tim-
provement of the Des Moines rapids.	
Grubbing and clearing	\$100
Grubbing and clearing	40,000
10, 860 cubic yards of excavation of rock, at \$3	32,580
1,000 cubic yards of excavation of earth, at 60 cents	600
60,000 cubic yards of embankment, at 50 cents	30,000
5,000 cubic yards lining, at 75 cents.	3,750
10,000 cubic yards puddling earth, at 30 cents	3,000
1,000 cubic yards slope wall and pavement, at \$2	2,000
1,000 cubic yards loose stone, at \$1 50	1,500 30,000
550 cubic yards vertical wall laid dry, at \$4 50	2, 475
13,256 cubic yards masonry in lock walls, at \$13	172, 328
500 cubic vards concrete masonry at \$6.	3,000
500 cubic yards concrete masonry, at \$6	12,720
60,000 feet, board measure, white pine lumber and plank, at \$60 per M	3,600
109,000 pounds wrought iron, at 18 cents. 42,000 pounds cast iron, at 12 cents. 5,000 pounds spikes and nails, at 12 cents.	19,638
42,000 pounds cast iron, at 12 cents	5,064
5,000 pounds spikes and nails, at 12 cents	600
160 lineal feet snubbing posts, at \$1	160
Sulphur and sand cement for irons let into masonry	500
Painting upper part of lock gates	150
Painting upper part of lock gates 130 lineal feet superstructure for draw bridge, at \$50. Fixtures for opening and closing gates.	6,500
-	1,000
Total	371, 265
=	
Estimate of the cost of constructing middle lock, 8 feet lift, for the in	nprovement
of the Des Moines rapids.	
Grubbing and clearing	\$3 00
Bailing and draining, including expenses of coffer dams	10,000
19.300 cubic yards excavation of rock, at \$2.50	48, 250
19,200 cubic yards excavation of earth, at 50 cents	9,600
12,000 cubic yards embankment, at 50 cents	6,000
4,000 cubic yards lining, at 75 cents	3,000
8,000 cubic yards puddling earth, at 30 cents	2,400
NU cubic yards slope wall and pavement, at \$2	1,000 1,200
800 cubic yards loose stone, at \$1.50	15,000
300 cubic yards vertical wall, laid dry, at \$4 50.	1,350
8,652 cubic yards masonry in lock walls, at \$13	112, 476
300 cubic yards concrete masonry, at \$6	1,800
64,000 feet, board measure, white oak lumber and plank, at \$100 per M	6,400
42,000 feet, board measure, white pine, at \$60 per M	2,520
64,300 pounds wrought iron, at 18 cents	11,574
27,500 pounds cast iron, at 12 cents.	3, 300
4,000 pounds spikes and nails, at 12 cents	480
160 lineal feet snubbing posts, at \$1	160
Sulphur and sand cement for irous let into masoury	500
Painting upper parts of gates	100 6 500
Fixtures for opening and closing gates.	6, 500 1, 000

Estimate of the cost of constructing a guard-lock at the head of the canal, for the improvement of Des Moines rapids.

Grubbing and clearing	\$ 300
Grubbing and clearing	13,500
19,300 cubic yards excavation of rock, at \$2 50	48, 250
19,200 cubic yards excavation of earth, at 50 cents	9,600
12,000 cubic yards of embankment, at 50 cents	
4 000 cubic yards of lining at 75 cents.	. 3.000
8,000 cubic yards of puddling earth, at 30 cents	2,400
800 cubic yards of slope wall and pavement, at \$2	1,600
1,000 cubic yards of loose stone, at \$1 50.	1,500
1,500 cubic yards of vertical wall laid in hydraulic cement, at \$6	9,000
400 cubic yards of vertical wall, laid dry, at \$4 50	1,800
8,586 cubic yards of masonry in lock walls, at \$13	. 111,618
400 cubic yards of concrete masonry, at \$6	2,400
400 cubic yards of concrete masonry, at \$6	7,400
9,000 feet, board measure, of white pine timber at \$60 per M	540
68,300 pounds of wrought iron, at 18 cents	12,294
24.000 pounds of cast iron, at 12 cents	
4,000 pounds of spikes and nails, at 12 cents	450
160 lineal feet snubbing posts, at \$1	160
160 lineal feet snubbing posts, at \$1	500
Painting upper parts of gates	100
130 lineal feet superstructure for drawbridge, at 50 cents	6, 500
Fixtures for opening and closing gates	1,000
Total	242, 822
Upper chain, near Montrose island, for the improvement of I rapids.	
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing	
rapids. Channel 200 feet wide and 6 feet deep:	
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams. 65,000 cubic yards of rock excavation, at \$7.	\$130,000 00 455,000 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing	\$130,000 00 455,000 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams	\$130,000 00 455,000 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams	\$130,000 00 455,000 00 . 585,000 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams	\$130,000 00 455,000 00 . 585,000 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams	\$130,000 00 455,000 00 . 585,000 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams	\$130,000 00 455,000 00 . 585,000 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams. 65,000 cubic yards of rock excavation, at \$7 Total for channel	\$130,000 00 455,000 00 . 585,000 00 \$10,000 00 12,080 00 7,000 00 1,740 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams. 65,000 cubic yards of rock excavation, at \$7 Total for channel	\$130,000 00 455,000 00 585,000 00 \$10,000 00 12,080 00 7,000 00 1,740 00 2,001 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams	\$130,000 00 455,000 00 . 585,000 00 \$10,000 00 12,080 00 7,000 00 1,740 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams. 65,000 cubic yards of rock excavation, at \$7. Total for channel Construction of pier from foot of island: Bailing and draining. 1,208 cubic yards of masonry in pier, at \$10 350 cubic yards coping to pier, at \$20. 11,600 pounds wrought iron dowels and clamps, at 15 cents. 667 putting dowel clamps into masonry, at \$3 667 putting dowel clamps into masonry, at \$2	\$130,000 00 455,000 00 . 585,000 00 \$10,000 00 12,080 00 7,000 00 1,740 00 2,001 00 1,334 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams. 65,000 cubic yards of rock excavation, at \$7 Total for channel	\$130,000 00 455,000 00 . 585,000 00 \$10,000 00 12,080 00 7,000 00 1,740 00 2,001 00 1,334 00
rapids. Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams. 65,000 cubic yards of rock excavation, at \$7. Total for channel Construction of pier from foot of island: Bailing and draining. 1,208 cubic yards of masonry in pier, at \$10 350 cubic yards coping to pier, at \$20. 11,600 pounds wrought iron dowels and clamps, at 15 cents. 667 putting dowel clamps into masonry, at \$3 667 putting dowel clamps into masonry, at \$2	\$130,000 00 455,000 00 585,000 00 \$10,000 00 12,080 00 7,000 00 1,740 00 2,001 00 1,334 00
Channel 200 feet wide and 6 feet deep: Bailing and draining, including the expenses of constructing and removing coffer dams. 65,000 cubic yards of rock excavation, at \$7. Total for channel. Construction of pier from foot of island: Bailing and draining. 1,208 cubic yards of masonry in pier, at \$10 350 cubic yards coping to pier, at \$20. 11,600 pounds wrought iron dowels and clamps, at 15 cents. 667 putting dowel clamps into masonry, at \$3 667 putting dowel clamps into masonry, at \$2 Total for channel and pier	\$130,000 00 455,000 00 585,000 00 \$10,000 00 12,080 00 7,000 00 1,740 00 2,001 00 1,334 00
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B.

GROLOGY OF THE DRS MOINES RAPIDS.

The rocks exposed along the Des Moines rapids consist of seven distinct groups, which, although subject to considerable local changes, are easily distinguished from each other. Six of them unquestionably belong to the carboniferous or mountain limestone series; the seventh or highest, I think, can be proven to belong to the coal measures.

I. Cherty limestone, forty feet or more thick, is exposed all along the rapids on both shores, and forms the bed of the river. It consists of thin beds of hard gray limestone interstratified with bands of chert. It is worthless for any pur-

pose except for forming riprap embaukments.

II. The Keokuk limestone proper, twenty feet thick. It should be included in limestone I, as it presents all its characteristics, only some of the layers are locally developed into considerable thickness and furnish valuable building stone. It has been extensively quarried near Nauvoo and Keokuk. The following is a description of the "temple quarry," near Nauvoo, where this group attains its

fullest development.

The lowest bed, resting directly on the cherty limestone, is a solid layer of two feet four inches thick, very hard and compact, light gray, semi-crystalline, without any chert or spur, and contains but few fossils. This is a very durable and beautiful rock. Separated from this by two feet four inches of shaly limestone and marl, is another good bed of limestone three feet two inches thick, in two layers, similar to the lower one, but rather inferior on account of a greater number of fossils, especially orthisimas, which tend to laminate the rock. Above this, separated by two feet of shaly limestone, is a bed of dark gray limestone, four feet two inches thick, in five layers, very hard, regularly bedded, and well adapted for all purposes where no considerable thickness is required. It would make splendid caps and sills.

Similar quarries have been opened at and near Keokuk, and it would be easy to point out localities between the two places where the same quality of rock could be obtained. But none of these layers are permanent, they thin out or change into shaly and chert limestone, and can only be relied on for a limited space. In the Keokuk quarries the finest blocks are frequently spoiled by cav-

ities containing calcite and beautiful crystals of zincblende.

III. This is the well-known "geode bed" about fifty feet thick, consisting of marls, clays, and irregular patches of thin bedded buff limestone. It will furnish a very good, abundant, and easily accessible material for embankments, but is otherwise perfectly useless.

IV. There are seventeen feet of arenaceous rock, changing from ferruginous or micaceous sandstone to arenaceous limestone, in some instances of an oolitic structure. It decomposes readily, for which reason it is observed in but few

localities. Quite worthless for any economical purpose.

- V. Magnesian limestone, ten to twelve feet thick, usually heavily bedded, of rather coarse texture, dark buff, and not so pleasing in appearance as that of Joliet; but it is a most excellent building rock, obtainable in any required dimensions. One detached block, solid without fissures, measured twelve by ten feet and six feet high. It breaks in square blocks, requiring but little trimming. Many years ago this rock was quarried, above Larvey's creek, for the Illinois internal improvements, and thousands of tons of the finest dimension rock are now lying, already quarried, along the slopes of the bluff. It retains the sharpest edges and toolmarks, and is in every way the best rock for heavy buildings that I saw in the west.
 - VI. Brecciated limestone sixteen feet thick. This is a very variable bed, usu-

ally consisting of a very pure, fine grained, blue limestone, broken up into small angular masses and cemented together again with fragments of chalcedony and other pebbles. In some places it forms a bed of fine grained limestone upwards of a foot thick, but not very valuable for building purposes. It furnishes, how-

ever, a very good material for lime.

VII. Sandstone, heavily and irregularly bedded, fifteen feet thick, varying from a coarse, hard, ferruginous stone to a very fine, friable, and perfectly pure white sand. It overlies the brecciated limestone unconformably; contains stigmaria and charred impressions of other coal plants; is in some places overlaid by fireclay, slates, and coal, and is, therefore, entirely distinct from either the ferruginous sandstone of St. Louis, or the lower sandstone of the Chester beds. I think it clearly belongs to the coal measures. Some portions of this rock would furnish very good building material, but owing to its irregular bedding and varying texture it would hardly pay to quarry it.

In a few isolated places clay-shoals, slates, and coal have been found above the sandstone, but as they furnish no material for building and are of no import-

ance any way, I paid but little attention to them.

H. A. ULFFERS, Civil Engineer Assistant.

C.

I, Thomas H. Griffiths, secretary of the Northern Line Packet Company. a company owning a large amount of tonnage, consisting of steamboats, barges, and lighters, exclusively engaged in the transportation of freight and passengers from St. Louis, Missouri, to St. Paul, Minnesota, and intermediate points between said cities on the Mississippi river, do hereby certify, that at low stages of the water on the upper Mississippi river, and also at what is called medium stages of the water on said river, that the steamboats owned by said Northern Line Packet Company have to lighten the freight that they carry at the lower or Des Moines rapids into lighters used for that purpose exclusively, and also to lighten into freight cars of the Keokuk, Mt. Pleasant and Muscatine railroad, from Keokuk to Montrose, on the up stream trips, and from Montrose to Keokuk, on the down stream trips, being a distance of about twelve miles, Keokuk being at the foot of said rapids, and Montrose being immediately at the head, and that the cost of said lightering is considerable; and also that the steamboats with their full crews are necessarily detained there while transporting their freight into lighters and railroad cars, and after passing the rapids in transferring the same back to the steamboats, causing the company thereby a heavy expense.

That the deponent's position of secretary of said Northern Line Packet Company gives him free access to all the accounts of the said steamboats so employed, and it is his duty to examine the accounts of the steamboats so employed, and from examinations of said accounts, and comparing and computing the same, it is the deponent's opinion, to the best of his knowledge and belief, that the said expenses actually incurred, caused by the impediment to navigation at the Des Moines rapids, per trip, for each steamboat, will not average less than one thousand dollars per trip for the entire season of navigation, meaning when not obstructed by ice, counting the extra labor paid, actual expenses paid, for transporting the freight across or around the said rapids, and a reasonable estimate of the expenses of the steamboats while engaged in transferring the freight, and that this estimate is based on the expenses there for the past two years. That during the year 1866 the boats owned by the company made their trips, as per list, viz: Steamboat Minnesota, 21 trips; steamboat Muscatine, 20 trips; steamboat Sucker State, 21 trips; steamboat Hawkeye State, 14 trips; steamboat

Davenport, 22 trips; steamboat Burlington, 19 trips; steamboat Pembina, 20 trips; steamboat Canada, 18 trips; steamboat Savannah. 8 trips; steamboat Reserve, 17 trips; steamboat Petrcl, 16 trips; steamboat Little Giant, 9 trips; steamboat Bill Henderson, 3 trips; steamboat America, 3 trips; making in all 211 trips, costing on an average of \$211,000 for the season of 1866. The company owns and maintains for this exclusive purpose and benefit a steam tow-boat or tug and seven lighters, for use at the rapids. They also own thirty-seven barges, which are used in transporting freight, as before mentioned, between St. Louis and St. Paul, and intermediate points.

That deponent has served as secretary of the Northern Line Packet Company for seven years, and previous to which he was a practical navigator, as master of a steamboat for twenty-three years, and from his long experience in the freighting business, and acquaintance with this particular trade, is satisfied that the commerce of said river will increase rapidly, and should the rapids be improved, as contemplated, that the amount would be doubled in a year, or in two years, at the furthest, after said improvement is made, owing to the more rapid and certain transportation of freight, and that it would be reduced very largely in cost.

It is difficult so state the exact delay there is to our boats, but in my opinion it will average, during the season, a delay to each boat, each trip, about two days, not having exact data to make the estimate; but this is nearly correct, if

not quite so.

I would further state that the Northern Line Packet Company own in steamboats, barges and lighters, engaged in transportation on the upper Mississippi river, in round numbers, about ten thousand tons.

THOMAS H. GRIFFITHS, Secretary Northern Line Packet Company.

STATE OF MISSOURI, County of St. Louis:

On this 8th day of January, A. D. 1867, before me, the undersigned, notary public, came Thomas H. Griffiths, secretary of the Northern Line Packet Company, who states on his oath that the within affidavit subscribed by him in my presence he believes to be true.

[SBAL.]

C. HEQUEMBOURG,

Notary Public.

True copy:

J. H. WILSON.

Lieut. Col. 35th Infantry and Brevet Major General U. S. A.

E 2.

Plan for the improvement of the Des Moines rapids of the Mississippi river, by means of a lateral canal along the Iowa shore; unanimously adopted by the Board of Engineers, May 13, 1867.

United States Engineer's Office, Davenport, Iowa, May 13, 1867.

General: The Board of Engineers convened by Engineer Orders Nos. 18 and 20, March 22 and March 26, 1867, having met at Keokuk, Iowa, on the 15th of April, and adjourned to this place on the 3th of April, beg leave to submit the following conclusions and recommendations in regard to the improvement of Des Moines rapids of the Mississippi river:

1. After a careful examination and consideration of all the plans, profiles and river soundings, and of the calculations and views of the engineer and his assistants, an inspection of the ground on both sides of the river, from a point above

Montrose and Nauvoo to a point opposite the mouth of the Des Moines river and the town of Warsaw, and a study of all the plans proposed, it is the unanimous opinion of this board that the general plan of a canal and locks, submitted to the engineer department by Brevet Major General J. H. Wilson, dated January 1, 1867, and approved by Brevet Major General A. A. Humphreys, Chief of Engineers United States army, in his report of February 5, 1867, to the Secretary of War, is the best for permanently improving the navigation of the Des Moines rapids of the Mississippi river, namely, "a canal along the Iowa shore from a point near the present site of the Keokuk indicator at the city of Keokuk, to a point just below the village of Nashville," seven and six-tenths miles long, with such modifications in detail as are hereinafter specified.

2. That taking the gauge of extreme low water at or near Montrose as a guide, the canal bottom at the upper end of the proposed canal at Nashville and the level of the tops of the mitre-sills at the guard-lock should be fixed at five feet below such low water at the guard-lock, and that the lower mitre-sill of the outlet lock at Keokuk be fixed at five feet below the low water of 1864, at that point, as referred to in the alternate suggested in General Wilson's That the canal and the middle and lower locks be arranged for eight feet depth of water when the river rises above extreme low water three That the head of the guard-locks be built two feet above the high water of 1851, as shown in the plan submitted by General Wilson, and that the chamber and lower gate be built high enough to pass boats through the lock when the river at that point may be twelve feet above low water. That the size of the lock chambers shall remain as given in General Wilson's report, namely, 350 feet between the quoins and 80 feet wide at the top. That the thickness of the lock walls, as planned, is abundant for any pressure that can come against them; and it is suggested that, in the final arrangements of the details, the engineer in charge should be authorized to make such reduction in thickness, for economy, as he may deem safe. That the reverse gates proposed at the outlet lock are unnecessary, and may be economically dispensed with.

3. That the final width of the canal be established at not less than 300 feet throughout, both in excavation and embankment; but in excavation at the time of construction it may be reduced to 250 feet, without material disadvantage. That the top of the embankment be fixed at two feet above the flood of 1851, at ten feet width, with side slopes not less than 1½ and not more than 1½ base to 1 vertical, as the engineer in charge of the work may determine. That, if it is found economical in the progress of the work, the thickness of the ripraps may be reduced from the thickness suggested in the plan submitted by General Wilson, provided that it be not less than two feet, and that the ripraps, or slope wall on the canal side of the embankment, be raised only to the height of twelve feet above canal bottom, or four feet above the highest water in the canal; and that the residue of the inside slope and the top of the embankment, being above all floods, be sowed with grass, sodded, or covered with a thin coating of stone, as the engineer in charge may determine.

4. In the report of General Wilson, respecting the improvement of the channel above Nashville, provision is made for the excavation of a considerable quantity of rock under water. It is recommended that the engineer in charge be authorized carefully to investigate this part of the stream, with a view of an artificial increase of the depth of the water above the guard-lock, and with a view, also, of dispensing with such portion of said rock excavation under water as may be found practicable—the final arrangement to be determined by him, sub-

ject to the approval of the engineer department.

5. If the general plan of the canal, with modifications as herein recommended, should meet the approval of the Secretary of War, it is respectfully suggested that all the details and matters not specifically fixed in this report may be left

to be arranged by the engineer in charge upon survey, reports, &c., approved

by the engineer department.

6. Estimate for a canal 300 feet wide in embankment, 250 feet wide in excavation, with two lift-locks and one guard-lock, with embankments ten feet wide on top; locks 350 feet long from quoin to quoin, 80 feet wide on top, between Keokuk and Nashville, seven and six-tenths miles.

Grubbing and clearing	\$3,000 45,000
125,000 cubic yards rock excavation, at \$2	250,000
330,000 cubic yards earth excavation, at 40 cents	132,000
200,000 cubic yards embankment hauled from excavation, at 25	102,000
cents	50,000
1,069,000 cubic yards earth embankments, at 50 cents	534, 500
40,000 cubic yards puddling, at 25 cents	10,000
120,000 cubic yards ripraps, made from rock excavation, at 75	10,000
cents	90,000
150,000 cubic yards ripraps, at \$1 50	225, 000
190,000 cubic yatus ripiaps, at \$1 00	
	1, 339, 500
Lower lock	346, 407
Middle lock	222, 552
Guard-lock	220, 464
	2, 128, 923
Three miles changing line of railroad	
Three miles changing line of public road	3 000
Three mines changing fine of public toad	3,000
Total for 7 to miles	2, 155, 923
Add for work between Nashville and Montrose, to make a channel	
300 feet wide and 5 feet deep	4 75, 6 53
m	
Total	2, 631, 576

The above sum is believed to be ample to cover contingencies and complete

a perfect navigation over the whole length of the rapids.

7. That the work necessary to carry into effect the plan herein recommended should be put under contract between Keokuk and Nashville without delay, and pushed vigorously to completion, so that the improvement of the rapids may be

made available for commerce and navigation as soon as possible.

8. The important question submitted to the board has occupied their undivided attention for several weeks, during which numerous points of interest have been discussed and many views presented and considered. Some time must elapse before a complete report can be drawn up embodying the views and opinions of the board upon the several plans which have been considered; meanwhile pressing business demands the immediate personal attention of the members of the board at their respective headquarters. Under these circumstances it is the opinion of the board that they will best subserve the public interest by adjourning, to meet again at this place, on the call of the president of the board, for the purpose of adopting the final report of proceedings. During this interval it is understood that General Wilson, the member in charge of the work, will cause to be prepared a map on a small scale for convenient reference, showing the ground, river bed, and such lines as will aid in exhibiting and explaining the plans of improvement which have been investigated, with the location of the canal on the Iowa side, the profiles representing the canal levels and locks, the excavations, and the top of the embankments two feet above high water of 1851; also showing the line of low water of 1864, and the line of high water of April, 1867—being a copy, on a reduced scale, of the maps, charts, and profiles already prepared by General Wilson, showing the modification indicated in the proceedings of the board.

All of which is respectfully submitted.

J. N. MACOMB,

Colonel Engineers, Brevet Colonel U. S. A. J. H. WILSON,

Lieutenant Col. 35th Infantry, Brev. Maj Gen'l U. S. A. G. K. WARREN,

Major Engineers, Brevet Major General U.S. A. W. MILNOR ROBERTS,

Superintending Engineer Ohio river Imp., Civil Engineer. PETER C. HAINS,

Captain Engineers, Brev. Lt. Col. U.S. A., Recorder.

Major General A. A. HUMPHREYS,

Chief of Engineers, Washington, D. C.

E 3.

DAVENPORT, IOWA, July 20, 1867.

GENERAL: The undersigned, pursuant to your instructions, met at Keokuk, Iowa, on the 16th of April, 1867, for the purpose of submitting a plan of improvement for the Des Moines rapids of the Mississippi river, under the following orders of the engineer department:

[Engineer Orders No. 18.]

Engineer Department, Washington, March 22, 1867.

In accordance with the following provision of the "act making appropriations for the repair, preservation and completion of certain public works, heretofore commenced under the authority of law, and for other purposes," approved March 2, 1867, viz: For improving navigation on the Mississippi river at Des Moines, or lower rapids, according to such plan as the Secretary of War shall, on the report of a board of engineers, approve, five hundred thousand dollars, (\$500,000,) and, by authority of the Secretary of War, a board is hereby constituted for the purposes herein specified, as follows:

Brevet Brigadier General T. J. Cram, colonel engineers; Brevet Colonel J. N. Macomb, lieutenant colonel engineers; Brevet Major General J. H. Wilson, lieutenant colonel 35th infantry; Brevet Major General G. K. Warren, major engineers: W. Milnor Roberts, civil engineer Ohio river improvement.

engineers; W. Milnor Roberts, civil engineer Ohio river improvement.

The recorder of the board will be Brevet Lieutenant Colonel P. C. Hains.

captain of engineers.

The board will assemble at Keckuk, Iowa, on the first Monday of April, 1867, or as soon thereafter as practicable.

The subject of the improvement of the Mississippi river at the Des Moires

rapids is committed to the board without restrictions.

Upon the call of the board Brevet Major General J. H. Wilson, the officer in charge of the surveys and examination of the Des Moines rapids, will lay before it all the information relating thereto that has been collected by him, embracing reports, plans, estimates, maps, &c.

Should further examinations or surveys be necessary in the opinion of the board in order to facilitate the investigations, it is authorized to have them made,

provided they are not of an extended character; the necessity for such as are of that nature will be reported here before being executed.

Authority is given to the board to collect all information that in its judgment

may be necessary to the proper determination of the questions involved.

The result of the proceedings of the board will be transmitted to this department at as early a day as practicable, in order that it may be submitted to the Secretary of War for his decision.

A. A. HUMPHREYS,

Brig. Gen. and Chief Engineer, Maj. Gen. Volunteers.

[Engineer Orders No. 20.]

Engineer Department, Washington, March 26, 1867.

The board of engineers constituted by Engineer Order No. 18, dated March 22, 1867, is modified so far as prescribes the day of assembling.

The day of the meeting of the board is postponed until Monday, April 15,

1867.

A. A. HUMPHREYS, Brig. Gen. and Chief of Engineers, Maj. Gen. Vols.

[Telegram.]

WASHINGTON, D. C., April 17, 1867.

Colonel J. N. MACOMB, United States Engineers:

The board will proceed with the business for which it was convened upon the arrival of General Warren, not waiting for General Cram.

A. A. HUMPHREYS, Chief of Engineers.

On the arrival of General Warren the board immediately organized and entered upon the duties assigned to it. A personal examination of both shores of the river, from Nauvoo to the mouth of the Des Moines river, was made by the board. The board was also supplied with the detailed maps made by General Wilson last fall, which furnish all the information necessary for a thorough investigation of all the plans of improvement that have been proposed, consequently the board did not find it necessary to make any further surveys, and having considered the character of the facilities and accommodations required by commerce, have the honor to submit the following report:

In the order of the engineer department, the "subject of the improvement of the navigation of the Mississippi river at the Des Moines rapids is committed

to it without restriction."

The board, in view of the latitude given it by the department, has carefully and thoroughly investigated every plan of improvement that has been proposed, or could be applied, to remove the present difficulties, and every effort has been made to arrive at none other than correct conclusions. The conclusions of the board have already been made known in their preliminary report of the 13th of May.

The considerations that come up on the investigation of the subject, and upon which the board is called upon to make and express its views, will, for conve-

nience, be arranged in the following order:

First. Description of the rapids, comparison of the surveys made at different times, and the relation between the stages of water at different places on the rapids and above and below.

Second. The dimensions that any improvement should have in order to fully accommodate the requirements of navigation; and in case locks are used, their plan, size, and general arrangement.

Third. Plan of improvement of the natural channel of the river by widening

and deepening it. Estimate of its probable cost.

Fourth. Plan of improvements by means of dams entirely across the river, with locks for steamboats, and chutes for rafts and flatboats. Estimate of its probable cost.

Fifth. Plan and position of an improvement proposed by Mr. G. Edmunds, jr.,

of Illinois. Estimate of its probable cost.

Sixth. Plan and position of an improvement by means of an excavated shore channel on the Iowa side from Montrose to Keokuk. Estimate of its probable cost; also, of a similar improvement on the Illinois side, from Nauvoo to Hamilton.

Seventh. Plan and position of a canal improvement on the Iowa side. Esti-

mate of its probable cost.

Eighth. Plan and position of a canal improvement on the Illinois side. Esti-

mate of .its probable cost.

Ninth. Comparative advantages and disadvantages of the plan of a canal on the Iowa side with that of an excavated side channel on the Iowa side, and also with the canal on the Illinois side. Comparative cost of each, and the opinion of the board as to which is the preferable plan.

I.—Description of the rapids, comparison of the surveys made at different times, and the relation between the stages of the water at different places on the rapids, and on the bars above and below.

The following extracts, descriptive of the rapids, are taken from General Wilson's report:

"The Des Moines or lower rapids are situated near the mouth of the Des Moines river, and extend from Keokuk to Montrose, a distance of about eleven miles. During the low water season they interpose a serious and at times an absolutely impassable barrier to steamboat navigation. The upper Mississippi, from St. Paul to the mouth of the Missouri, has, during the lapse of the more recent geological periods, worn for itself a valley, varying from one to fifteen miles in width and sunk below the general level of the prairies on either side, from one hundred and fifty to three hundred feet. In many places, and for much of the distance, this valley is cut through strata of rock, varying in thickness, hardness, and mineral characteristics. Without discussing the agencies by which this erosion, so disproportionate to the present powers of the river, has been effected, the Des Moines rapids present evidence strongly confirmatory of changes in the character of the river itself.

"General Humphreys, in his report upon the hydraulics of the Mississippi river, suggests that it was formerly a clear-water river like the Niagara, fed by a fresh-water lake or lakes of great extent, occupying a large portion of what is now the prairie land of Illinois and Iowa, and that its transformation from a clear into a muddy stream may have been the result of changes which have taken place in its basin. In support of this theory, he refers to the passage of the river through the northeastern extremity of the Ozark mountains at Grand Tower, be'ow St. Louis, where the water has cut down through beds of rock upwards of three hundred feet thick, and probably drained the lake just men-

tioned.

"Another instance, developed by the survey and examination of my assistant, Colonel Ulffers, amounting almost to a proof, can be found at the head of the Des Moines rapids. Just below Montrose the rocky bluffs recede at right angles from the river, bearing gradually to the northward, enclosing a considerable extent of country above and to the westward, which was once evidently the bed

of an extensive lake, whose outlet was at the rapids and whose surface was about 105 feet above the present low water level.

"This lake basin is an extensive level plain, intersected by a network of sloughs, its lower part subject to annual overflows, and covered with a heavy growth of willow, maple, hickory, haw, and vines. The terraces around it are well developed and of an unvarying height; its upper end, and in part its western side, are limited by beds of loam rising 150 feet above low water at Fort Madison, and forming the entire river bluff at this place. How far this loam deposit extends above and into the interior of Iowa has not been determined, and therefore what may have been the extent of the lake in these directions cannot be asserted.

"As before stated, the outlet of this lake was at the present head of the lower rapids. The water stored up there by their ceaseless action for ages, assisted by ice and other geological agencies, gradually eroded for themselves a channel at least a mile wide, nearly 200 feet below the level of the prairies, and extending through limestone rock to the mouth of the Des Moines river and beyond. This erosive action, though productive of such remarkable results, has not been carried sufficiently far to render the river, through this part of its bed, available and safe at all times for the purposes of navigation. From Fort Madison to Montrose, the river is about 2,500 feet wide, and sufficiently deep; but in the rapids its bed of limestone rock, which by some unknown cause seems to have been hardened to a greater degree than the corresponding stratum above and below the rapids, has resisted the action of the water, while its sides have given The result is that this mass of rock remains there, acting exactly as an artificial dam whose upper surface slopes about twenty-two feet in eleven miles, and conforms very nearly to the plane of stratification of the rock through which the channel is cut. The bluffs extend along the banks of the river throughout the length of the rapids, presenting a rock escarpment at the present high-water mark, with a sloping gravel beach to low water, and also another escarpment of rock at 105 feet above the present water level, having, likewise, a sloping beach The exposed ledges are formed of different strata in the different lo-At some places they are brecciated limestone, (near Montrosc,) in others magnesian limestone, (above Larey's creek,) and in others the coal measure sandstone, (below Price's Creek;) but notwithstanding the varying hardness of the strata, they have all been cut through equally by the river in its progress from the upper beach, just mentioned, down to the one at the present low-water level About sixty feet of these bluffs, however, consist of the geode on the rapids. bed lying between the two beaches and made up mostly of an accumulation of clay and marl, easily washed away. The river, forcing its way through these beds of stubborn material, must, therefore, have gradually receded from the foot of the rapids, like the Niagara is now doing, until it reached its present condition; undoubtedly the process of smoothing its path is still going on, although in an imperceptible degree. The places where the bluffs recede from the bank at the mouth of creeks emptying into the river, there are two terraces beside the present river bottom, respectively twenty-five and seventy-five feet above present low water.

"The level part of the town of Nauvoo, at the head of the rapids opposite Montrose, is built upon a twenty-five foot terrace, which is likewise prominent around the edge of the plain, between the latter place and Fort Madison. The seventy-five foot terrace is most prominently developed near Sandusky.

"The total depth of Des Moines rapids, measure along the Iowa shore, from Montrose (old Fort Des Moines) to the St. Louis Packet Company's landing, at Keokuk, (station D to V,) is 10.92 miles; on the Illinois side, from Nauvoo to the Hamilton ferry landing, (station 23 to 84,) is 12.20 miles, or a mean of 11.56 miles. The bed of these rapids, throughout its entire length, has reached a stratum of cherty limestone, belonging to the Keokuk group of the carboniferous

series. The nature of this rock, consisting as it does of thin beds of limestone, interspersed with at least an equal amount of brittle chert or horn stone, precludes the idea that the different 'reefs' or 'heads of chains' might be occasioned as formerly supposed, by a succession of strata, which, in their outcrop, would form a barrier across the river. The deeper parts are all caused by erosion, originating partly from the strength of the current, aided by the presence of numerous granite boulders, and partly by masses of ice which are anually swept down by the spring freshets with tremendous force.

"In the various soundings carried across the river, no trace was found of 'pools' or places where the area of the cross-section would be sensibly greater than on the 'heads of the chains;' in other words, there is no proper chain crossing the river at any point, nor is there anything like a true 'pool,' the deep

water being found more properly in pockets and fissures.

"The river bottom is a broad, smooth rock, seamed by a narrow, crooked channel, or in some places several of them, alternatively widening and narrowing, shoaling and deepening, nowhere good navigation, but more difficult in some places than in others. The rapids are therefore not broken and noisy, but, the descent being gradual, the water flows over its bed in a broad, smooth, unbroken sheet, with nothing but the faintest ripples on the surface to indicate the dangerous places. The casual observer would not suspect the presence of the rapids unless he were notified of their locality beforehand.

"From these facts it may readily be inferred that bouts would not undertake their passage at night, even if the channel were deep and well defined.

"The worst parts of these rapids are called by steamboat and river men 'chains,' of which there are five principal ones, known as Lower, English, Lamil-

lies, Spanish, and Upper, respectively.

"From the lower to Spanish chain, inclusive, the channel used by steamboats is crooked, shallow, and exceedingly difficult of navigation, requiring, as General Warren states in his report of April 6, 1854, to be excavated almost continuously to the landing at Nashville, a distance of seven and a half miles. For this distance the fall is about 18 feet, and the average slope 2.4 per mile. From Nashville to the upper chain the channel is straight and deep, and the fall only four feet, or not more than a foot per mile.

"The lower chain extends from Keokuk to the mouth of Price's creek. The channel is 3.5 to 8 feet deep, but it is very swift and crooked, and therefore intersected by surface and under currents, particularly on that part of it known

as 'Sucker's Chute' and 'Omega Patch.'

"The greater part of the last appropriation was expended here, improving the channel materially, but the fragments of the rock blown out were piled alongside of the channel, injudiciously, it appears, as the ice has already carried away the greater part of the piles and redeposited the fragments of rock in the channel.

"The English chain extends from Montebello to Waggoner's warehouse. The channel, running near the Illinois shore, in this reach is comparatively straight, from 4 to 7 feet deep, and from 50 to 200 feet wide, and presents no

particular difficulties to navigation except in times of high wind.

"The head of Lamillies chain is opposite the mouth of Larey's creek. The channel is from 4.5 to 10 feet deep, and an average of 120 feet wide, but it is crooked, intricate, and dangerous, owing to the presence of many shoal patches or rocks, the peaks of which are not distinguishable in rough weather.

"Spanish chain extends from Judge Ballinger's place to within half a mile of Nashville; the channel is from 3 to 13 feet deep, and from 25 to 300 feet wide, and although sufficiently difficult of navigation, has been somewhat improved by excavation.

"The upper chain extends out a short distance below Montrose island, and is made by a broad flat bed of rock extending entirely across the river, upon which the water in dry seasons is not more than 2 feet deep, and frequently not a foot deep. The channel is only a channel in name, having nothing more than a slight depression in the rock, about 3 feet deep and 50 feet wide in the narrowest

place.

"The difficulty of navigation, it may be observed, on all the chains lies not so much in the shallowness of the channel or thread of the current as in its unevenness of bottom, insufficient width, tortuous direction, and great velocity. The influence of these features is exaggerated by cross-surface and under currents, and by east and west winds. From fear of the rocks, boats are compelled to move slowly, and therefore more difficult to guide, and no matter how skilful the pilot may be, his boat will be carried forward in the direction of the original impulse for some distance before it can be made to obey the helm. This is particularly the case in windy weather, and while going down stream. The greatest fall on the rapids is on the lower chain, where it amounts to 1.472 feet in 1,000 feet or 7.77 feet per mile. The velocity is consequently greater here than at any other place.

"In 1865 and 1866 navigation opened about the first of March, and closed about the first of December, giving 275 days, which may be considered some-

what above the average for the boating season.

"By an examination kept at the Keokuk indicator by the Upper Mississippi Pilots' Association, it appears that the water on the lower chain was less than four (4) feet for fifty days during 1865, the least depth, 2.3 feet, having been reached on the 20th of June; but this was quite an unusual season, the lowest water generally occurring about the middle of September and continuing much

longer.

"During 1866 the same register shows ninety-two days, or one-third the entire season, when the water was less than four feet. The least available depth recorded was two feet, and occurred on the fourth or fifth of October. From the 28th of September to the 2d of December the water ranged between two feet and 3.3 feet on the lower chain. This was about an average year. The dryest season known was that of 1864, when the river, early in September, reached a stage about ten inches lower than that just mentioned.

"During the extreme low-water season navigation for steamboats along the rapids is entirely suspended; and their cargoes are transferred either by rail or lighters, at an extra cost of about one dollar per ton, and an average cost of about five hundred dollars per day to the steamboats themselves while discharg-

ing and taking on freight."

The board do not feel called upon to enter into an argument upon the benefits to be derived from an improvement of the navigation at the Des Moines rapids, nor the necessities that demand such improvement; that it is desirable, even necessary, must be patent to all, and Congress has already recognized this fact

by making large appropriations for that purpose.

An examination of the printed reports of Lieutenant R. E. Lee of his survey in 1836, of Lieutenant G. K. Warren of his survey in 1853, and of General J. H. Wilson of his survey in 1866, shows some seeming discrepancies in the report of the maximum fall from the head to the foot of the rapids, and in the ranges of the surface between extreme low water and extreme high water at the head and foot of the obstructions.

Thus Lieutenant Lee reports the maximum fall at low water to be 24 feet; Lieutenant Warren reports it to be 21 feet; and General Wilson's survey,

"sensibly 22 feet."

The range from low water to high water at the head of the rapids is placed by Lieutenant Warren at 11\frac{3}{4} feet; by General Wilson at 12.65 feet. The same quantities at the foot of the rapids are given by Lieutenant Warren at 21 feet; by General Wilson at 19.28 feet.

Some of this discrepancy may be explained by the low water of 1864, which

is reported as the lowest known, and assumed by General Wilson as the lowwater plane. This would increase Lieutenant Warren's range at the head of the rapids, and his total of the fall from head to foot nearly to that reported by General Wilson; but it should also increase his range of surface at foot of the rapids, previously exceeding General Wilson's by 13 foot. It should also apparently increase the slope determined by Lieutenant Lee, which before gave a fall two feet greater than that determined by General Wilson.

The second most obvious source of disagreement is the uncertainty of ascertaining the extreme high and low-water marks, because of their rare occurrence, the long interval of years between the three surveys, and the different conditions under which the high and low-water stages of a river result, which different conditions nowhere make themselves more felt than at the head and foot of

a rapids or dam.

To these varying conditions the Des Moines rapids are peculiarly subjected, though their considerable length in comparison with the maximum fall makes the effect much less apparent than at the falls of the Ohio at Louisville. Then, too, the broad and low bottom lands of the Mississippi above and below the rapids tend to diminish the effect of local freshets, for, serving as a reservoir for the waters, they diminish the rise of floods, and the time required for their drainage prolongs the medium and shortens the lowest stages.

Flowing, as the Mississippi does, from north to south, its spring, summer, and autumn floods follow, in its different latitudes, the changes of the season, so that a rise on the rapids may come from its extreme sources, or from the regions

intermediate, or from the local streams.

And it happens that the principal of these latter, the Des Moines river, enters the Mississippi below the rapids themselves. We have thus produced the variations known as head rises and back-water rises, and, of course, all the combinations which they can produce by uniting in their various magnitudes.

We have, besides, the effect of wind, which, blowing up the stream at low water, diminishes the flow at the head of the rapids, while it banks up at the foot, diminishing the fall in a two-fold degree, and, when blowing in the opposite direction, causing a rise on the upper chain and a fall on the lower chain.

These sources of fluctuation would seem ample to account for all the discrepancies noted. Indeed, these discrepancies do not, it may be feared, indicate the extent of the liability to err from conclusions based on a single season's work.

As the data derived from these three surveys are of the greatest importance in determining the question of improvement, it is evident that a most exhaustive treatment of the information possessed is required.

Before proceeding with this, however, a third source of discrepancy, if exist-

ing, must be eliminated, namely, errors in the levelling itself.

COMPARISON OF LEVELS.

Lieutenant R. E. Lee says of his survey :

"A line of levels was run on the Wisconsin (now Iowa) side, and the mean-

ders of each shore determined by a compass.

"The distance from the head to the foot of the ran

"The distance from the head to the foot of the rapids is 11.005 miles, and the entire fall at the time of making the survey (autumn of 1836) was 24.015 feet.

"The height of the river could only be compared with extreme low water, by the relative quantity on the shoalest parts, and which was ten or twelve inches more than is said can be found at its lowest stages."

This is all the information of this levelling at present available, and there is

no connection with his bench-marks.

The level notes taken by Lieutenant Warren were on the right bank, the same bank as Lieutenant Lee's. They were made on November 15 and 16.

1853, the water showing no change during the time, and being at its lowest

stage for that year.

Lieutenant Warren was led to conclude it was within six inches of the lowest The level notes were taken by himself personally and with care, but admits that there being but a single line run, errors may possibly have been committed. The only one of his bench-marks on the right bank now recognizable is that of "the door-sill of Chittenden's (now Iowa Hotel) hall door," in Keokuk, which is on his datum line 41.896. There is also a bench-mark for the grade of the streets of Keokuk, and on the plans of that city has a reference 21.74, being that number of feet above the surface of the water when the levels for establishing the grades were run by Mr. Guy Wells in 1849.

General Wilson's survey comprised lines of levels on both banks of the Mississippi, which gave, according to his report, a difference of fall from head to foot, differing only .113 of a foot, and may be regarded as in perfect accordance.

Owing to shortness of the season a direct transfer of the levels across the . stream by the instruments was not effected, a matter of difficulty with so wide

a river intervening.

The recorded observations made by the pilots on their gauges at the head and foot of the rapids, furnish, however, a tolerably good means of transferring these levels, by assuming the water of the river to be level across the stream at opposite points of the pool above the rapids, and again below them.

At Montrose, October 16, 1866, right-bank levels gave the water a reference

34.658.

At Nauvoo, October 18, 1866, left-bank levels gave the water a reference 36.436; the Montrose gauge during the time being at a stand, and reading one foot, shows that the two datum lines were 1,778 feet apart, and that the zero of

the Montrose gauge was at a reference to right-bank levels 33.658.

Below Keokuk, November 12, 1866, the right-bank levels gave the water a reference 11.767, and below Keokuk, November 12, 1866, the left bank levels gave the water a reference 13.993; thus showing a difference here, in the two datum lines, of 2.226 feet, and differing only 2.226 - 1.778 = .458 foot from what it was at the head, which is as close agreement as the method of comparison could exact. General Wilson's levels, therefore, contain all the test for accuracy that can be applied. In any comparison of his level notes with others, his datum line on the right bank will be the common one to which all will be reduced, and two feet will be subtracted from his left-bank references to bring them to this reference.

The level of the door-sill of Chittenden's hall door, on General Wilson's survey, is 36.734; on Lieutenant Warren's (as before stated) 41.896; hence 5.162 feet subtracted from Lieutenant Warren's reference reduced them to

General Wilson's.

The comparison of the total fall on the rapids shows a close agreement in amount between Lieutenant Warren and General Wilson, but Lieutenant Warren's low-water plane of 1853 is, both at the head and foot of the rapids, nearly two feet lower on the common datum line than General Wilson's of 1866.

Other differences exist which can only be satisfactorily accounted for by a different level of the water itself, or a difference in the recorded remarks of high

water and low water, or by errors in levelling.

The first discrepancy is in the high-water mark of 1851. Lieutenant Warren's was obtained from the Phœnix Hotel in 1853; General Wilson's from Brown's warehouse in 1866. The first is using the common reference 30.881,

and the second 30.001, a difference of $\frac{723}{100}$ feet.

The Phœnix hotel is now gone, but the two locations were nearly the same so that the difference can only be accounted for by an error in the marking of

one place or the other.

This was a most remarkable high water, especially at the foot of the rapids.

Waggoner, an old pilot living midway of the rapids, says "the highest ever known," exceeding, at his place, that of 1828 by two feet. This, he says, was due to the back water of the Des Moines river, (at the time in excessive flood,) for at Montrose the water only reached the height it did in 1828.

Again, Lieutenant Warren's low water of 1853, where it crossed the lowest reef of the rapids, has nearly the same reference as General Wilson's for 1866. But in the succeeding 2,300 feet below, Lieutenant Warren found the river surface to fall (November 15, 1853) 4 5-10ths feet, while on November 12, 1866,

General Wilson's survey found a fall of only 2 45-100ths feet.

The slope of Lieutenant Warren's low water passes through the reading (1) one foot on the Keokuk pilots' gauge, so that the low water of 1864, which is the zero of this gauge, should have, opposite Brown's warehouse, a reference 9.160, which taken from Lieutenant Warren's reference for high water of 1851, viz, 30.881, makes the extreme range at foot of rapids to be 21.721 feet.

Lieutenant Warren's survey makes the fall of the river on the lower chain,

in 1853, to be 8.34 feet.

General Wilson's survey, November 12, 1866, makes it only 6 78-100ths feet. Lieutenant Lee's tables, in his printed reports, makes his fall only 5.8 feet, which is the more noticeable, as he makes the total fall on the rapids greater than either of the others. An examination and comparison of his table shows that it will not accord with General Wilson's or Lieutenant Warren's levels below Nashville, making the fall much greater than they do on Lamillies and English chains, but less than on the Lower chain.

It would therefore seem safe, in view of all the facts, to adopt, as the highest known water at the foot of the rapids, Lieutenant Warren's for 1851, and the lowest as the low water of 1864, having the respective reference of 30.881 and 9.160; the small space over which Lieutenant Warren's levels extended in making these references to the bench-mark now existing, rendering it improba-

ble that he committed any material error.

Proceeding now in our examination to the head of the rapids, more unaccountable differences are found to exist between Lieutenant Warren's and General Wilson's levels. The former found, at the time he was opposite Mechanic's Rock, (November 15, 1853,) that it was 2 feet out of water.

Captain Holliday says "that in the low water of 1864, it was out 40 inches;"

hence it must have been 16 inches lower in 1864 than 1853.

This agrees mainly with the result deduced before at the foot of the rapids. But the zero of the Montrose pilots gauge was placed at the level of the low water of 1864, which zero, referred to General Wilson's survey, has the reference 33.95; and Lieutenant Warren's low water, November 15, 1853, at Montrose has, on the same datum, a reference 32.7; whereas it should have had in order to be a foot above low water of 1864) a reference 34.95.

The conclusion from this is that there is an error of $2\frac{1}{4}$ feet in the levels between the head and foot of the rapids. Either Lieutenant Warren makes the fall at low water $2\frac{1}{4}$ feet too little, or General Wilson's survey makes it $2\frac{1}{4}$

feet too great.

The test already applied to General Wilson's double line of levels, places the grounds for confidence in their favor. On this conclusion we have the reference of low water of 1864 at Montrose, as given by General Wilson's survey, 33.95. Taking now the lowest water reference at Keokuk, viz., 9.160, we have for the maximum fall or lockage to be overcome 23.790 feet, which differs from Lieutenant Lee's total fall by being $\frac{1}{4}$ foot less.

Lieutenant Warren's high water of 1851, at Montrose, referred to General Wilson's datum, has a reference 46.25; and as low water of 1864 has reference

33.95, we have maximum range at Montrose 12.30 feet.

The difference of high water of 1851 at Montrose and Keokuk is 46.250—30.881=15.369 feet.

Lieutenant Lee's survey makes the low water fall from Montrose to Nashville 3.65 feet.

Lieutenant Warren's survey makes the low water fall from Montrose to Nashville 3.3 feet.

General Wilson's survey makes the low water fall from Montrose to Nashville 4.00 feet. The mean of all is 3.65 feet.

In any plan of improvement the result most unfavorable to it should be taken. General Wilson's high water of 1851, at Waggoner's, has a reference 34.000. This place is about 37,500 feet from head of rapids, and 22,500 feet from the foot of the rapids. High water at head is 46.25; at foot, 30.881. High water fall from head of the rapids at Waggoner's, 12.25; from Waggoner's to foot of the rapids, 3.129; making very apparent the effect of back water, which Waggoner has stated.

Before considering the effect of a rise on different parts of the rapids, and of the different kinds of a rise; a description will be given of the so-called indicators, established at the head and foot of the rapids by the Pilots' association.

These consist of excellently constructed wells on the river bank, extending down below the lowest probable water levels at the head and foot of the rapids, and communicating freely with the river. A copper drum is made which floats in this well, and supports a graduated rod, which slides as the water rises and falls through guides on a frame which keeps the rod vertical. The first graduation is in feet and inches, and gauges simply the rise and fall of the river at the place. The others were designed to show the corresponding number of feet of rise on the different chains, and at the other end of the rapids. Over these wells and gauges neat houses were built which protect the indicators from injury and observation from without.

At the Keokuk indicator a book is kept in which each pilot records the reading of the gauge at Montrose and Keokuk, at the time he passed the rapids, as also the draught of his vessel, wind, ice, &c. The zero of each gauge was placed at the level of the low water of 1864, and the record has been kept since

the opening of the liver in 1865.

As gauge records, they answer admirably, and are the most excellent ones on the Mississippi; but as indicators of the stages on the rapids intermediate, the graduated scale must, in the nature of the case, be very unreliable, especially the one at the foot of the rapids, owing to the influence of back water from the Des Moines river, and also from high south winds in the low stages.

Being politely furnished with the record book kept at the Keokuk indicator for examination, the observations have been plotted so that one co-ordinate shall be the stand of the gauge at Montrose, and the other the corresponding fall

from head to foot of the rapids.

The two zeros being the low water of 1864, have, according to General Wilson's levels, a distance apart vertically of 23\frac{3}{2} feet, (which is the low water fall.) The curve then begins with the co-ordinates, zero, and 24\frac{3}{2} at low water; at high water of 1851 they would be as hereinbefore deduced, gauge reading 12.3 and fall 15.369. These two points include the highest and lowest points of the curve.

The highest observation recorded by the pilots was eight feet at Montrose; this gave a fall 19.4 feet on rapids; the next highest, 7½ feet; this gave in one instance a fall of 17 feet, and another of 19.4 feet, thus showing a difference of fall 2, 6 feet, not due to the amount of water passing at Montrose. For it must be considered that the stand of the Montrose gauge is mainly dependent upon the quantity of water passing, and is not, like the Keokuk one, affected materially by back water.

At a stand of six feet the observations show variations in fall from 18.5 feet

to 201 feet.

At a stand of five feet the observations show variations in fall from 19.5 feet

At a stand of four feet the observations show variations in fall from 20 to 22

At a stand of three feet the observations show variations in fall from 21.6 feet to 22.4 feet.

At a stand of two feet the observations show variations in fall from 21.3 feet to 22.5 feet.

At a stand of one foot the observations show variations in fall from 22.3 feet to 23.2 feet.

At a stand of six inches the observations show variations in fall from 22.5 feet to 23.5 feet.

In one instance in June, 1865, when the Montrose gauge stood at 1.9, the fall was only 19.6 feet on the rapids. If there was no error in the observation, this would indicate a back-water rise from the Des Moines at the low stage of about three and a half feet on the Lower chain.

The effect of wind up or down the stream probably makes the majority of the fluctuations in the total fall, and appears to amount to about a foot in medium stages. The observations are not complete enough to decide exactly what amount is due to the wind.

The above includes all the practical information to be derived from the before-

mentioned plot, and it is not thought necessary to introduce it here.

The best observations we have on the relative rise at different parts of the rapids are those made by General Wilson during the rise in March and April,

These were taken at the respective readings on the Montrose gauge 2.3, 5.5. 7, and 7.5, and they show that the influence of back water is sensibly felt as far up as Waggoner's, the foot of Lamillies chain, and above that the water does not back, for it so happened that these observations were made during a heavy rise in the Des Moines river.

The plane of high-water in 1851 shows back water up to Waggoner's, but above there soon becomes parallel with the observed plane of April 23, the highest

water of the spring of 1867.

We are therefore somewhat authorized to conclude that a rise at Montrose will always produce an equal rise as low down as the foot of Lamillies chain, and that below that point nearly all the fluctuations of back-water and wind are confined. The reading of the Montrose gauge can then be taken for the stage of the river for at least the upper half of the rapids.

VOLUME OF THE MISSISSIPPI.

Observations made at Keokuk for discharge on the 27th of April, 1867, when the river stood on the gauge 13.35, gave the volume at 195,000 cubic feet per second.

The highest water was April 24, gauge reading 15.3. Assuming the velocity not to be greatly different, as it was much affected by back-water from the Des Moines river, we have the high-water discharge of April 24, 1867, 215,000 cubic feet per second; applying the same reasoning to the high water of 1851, (which. however, is very uncertain as the railroad embankment on the Illinois side opposite Keokuk was not built at that time, though the back-water of the Des Moines acted much in the same way,) we would have the maximum high-water discharge of the Mississippi at 265,000 cubic feet per second.

The lowest water discharge is approximated to in the following manner: Measurements made by General Warren, October 23, 1866, at Burlington, gave a discharge of 36,100 cubic feet per second. This was an average low-water year. The gauge at Montrose, October 23, stood at one (1) foot, and making this deduction at Burlington, and supposing the mean velocity to not change, we should get the extreme low-water discharge of 1864 at 31,913 cubic feet per second. This is probably in excess, but it would be safe to take it at 30,000 cubic feet per second.

General Wilson in his report says the average area of cross section is 17,550 feet at ordinary low water; mean surface velocity is 2.88 feet per second; and its mean velocity deduced therefrom is 2.304 feet. From these data the discharge

has been calculated, and is found to be 40,435 cubic feet per second.

Measurements made of the discharge of the Des Moines river, April 29, 1867, ascertained it to be about 35,000 cubic feet per second. The Des Moines was then 2½ feet below high water of April 24, 1867, on which day the discharge was not less than 42,000 cubic feet per second. The high-water discharge of 1851 in the Des Moines could not have been less than 55,000 cubic feet per second, its height then being 7½ feet above the level on April 24, 1867.

At the time of our observations on the Mississippi, April 27, 1867, a rise of one foot corresponded to an increased discharge 10,000 cubic feet per second; consequently the volume of the Des Moines at its floods would be able to secure

the Mississippi perhaps five feet above what it would be without it.

An examination of the maps will show that no great rise can occur in the Des Moines valley without a corresponding one in the Mississippi. The Des Moines runs through the whole length of Iowa, and its valley embraces about 12,600 square miles. The area of the valleys of the other tributaries of the Mississippi in the same latitude amounts to about 40,000 square miles, and the country above would probably contribute as much more water contemporaneous with it, so that the Des Moines can never form more than one-sixth part of any considerable flood in the Mississippi.

SEDIMENT.

There is a decided rise in the river produced by the spring floods in the streams in the latitude of the Des Moines, and the water is much more filled with sediment at the rapids at such times than it is at any other. Such fact was very observable to us while at Keokuk, about the 20th of April, but before we were prepared to measure the amount the rise came from the rivers further north and restored it to its usual purity. Out of two quarts of water taken from the Mississippi on the 25th of April careful filtering and weighing obtained but $3\frac{1}{2}$ grains of sediment, or about $\frac{1}{10000}$ of the weight. This is the character of the river during very much of the season, so that there would be but little deposition of mud from it in a canal.

The greatest amount of filling up which a stream produces is well known to arise from the material swept along near the bottom, and the amount of it is very difficult to measure. But the observed fact that the upper Mississippi throws out no material amount of sand on its present bottom lands, which are submerged at high water six to twelve feet, and which rise about ten feet above the low water, gives assurance that walls or banks of canals would not be much affected by the material drifted along the bottom, if carried up as high as the

average of the bottom-land banks when submerged at high stages.

The following table exhibits the number of days at which the water stood above the different-numbered feet on the gauges at Montrose and Keokuk, by actual observation. 'The zeroes of these gauges are at the low water of 1864, at which time there was about one foot of water on the rapids. The rise given on the Montrose gauge extends quite uniformly down the rapids as far as the foot of the Lamillies chain; below this it often and generally rises faster, owing

to the back-water and such influences, the greatest amount of which is measured at the Keokuk gauge:

	Above 15 feet.	Above 14 feet.	Above 13 feet.	Above 12 feet.	Above 11 feet.	Above 10 feet.	Above 9 feet.	Above 8 feet.	Above 7 feet.	Above 6 feet.	Above 5 feet.	Above 4 feet.	Above 3 feet.	Above 2 feet.	Above 1 foot.	Below 0 and 1
MONTROSE GAUGE. Number of days March 5 to December 8, 1865 Number of days March 16 to Decem		ļ. .			ļ 	0	0	0	3	16	43	80	146	196	274	0
Number of days April 1 to April	• • • •	ļ		- -		2	6	9	14	!	43	56	1	197	257	6
27, 1267				••••					8	12	16	21	26	28	28	
Number of days March 5 to December 3, 1865	••••				12	19	49	64		l		1			274	0
comber 3, 1866 Number of days April 1 to April 28, 1867	····	3 5	10 13	14	20 16	28 18	42 20	46 25	27	37	98 45	151 59	1 96 59		253	0

The following table, from observations kept at the Rock Island bridge, gives the duration of different stages there. The zero is the low water of 1864:

	Above 15 feet.	Above 14 feet.	Above 13 feet.	Above 12 feet.	ove 11 feet.	Above 10 feet.	Above 9 feet.	Above 8 feet.	ove 7 feet.	Above 6 feet.	bove 5 feet.	Above 4 feet.	ove 3 feet.	ove 2 feet.	bove 1 foot.	ģ
	₹	ΨP	₹	₹	Abo	₽₽ 	₹ 	Ap.	Above	₽	_°₹	₹	₽ po.	Abo	₹	Š
8 60 861			. <u></u> .		7	18	32	47	60	112	135	197	262	300	364	•
862	8	7 13	13 18	21 24	44 37	63 58	78 92	102 126	149 159	166 175	182 210	233	303 287	347 343	265 360	
863 864*	•••	••••	••••	. .	••••	••••		13 2	30 4	59	85 35	127 77	188 167	214 1 181	322 207	
865'	••••		::::		i	23	31	44	56	85	126	221	315	361	365	::
866 867	6	9	17	26 6	32 24	40 36	50 60	62 87	79 124	91 131	119 May	164 11, 186	275 7.	348	345	1

* From July 1 to July 30 no observation.

The maximum range of the Mississippi at Davenport, according to Lieuten ant Warren's report of his survey in 1853, is 23 feet, and at the head of these rapids 13 feet. During the time the Rock Island bridge gauge has been kept, the highest water observed was May, 1862, viz.; fifteen feet two inches, which would thus appear to have been at this point seven feet ten inches below the range of highest floods, such as that of 1851.

It is difficult to deduce from these observations at the head and foot of the rapids the corresponding rise of the river at the places between the rapids, or above and below them. That it is somewhat more rapid at the foot is apparent from the levelling made by General Warren's parties in October, 1866, at Clinton and at Burlington. These show the extreme ranges at Clinton from lowest to highest water is about 1886 feet, at Burlington 18.75 feet, and at Quincy 20.31 feet.

The following statement is taken from Lieutenant Warren's report as to the relative effect of rises on the rapids and on the bars above and below, and is the best that can now be given. Supposing that the natural channel was deepened so as to give a low-water depth of four feet, we would then have—

With 3 feet on the bars, 4 feet over the rapids.

With 4 feet on the bars, 4 feet 3 inches over the rapids.

With 5 feet on the bars, 4 feet 7 inches over the rapids.

With 6 feet on the bars, 5 feet over the rapids.

With 7 feet on the bars, 5 feet 8 inches over the rapids.

With 8 feet on the bars, 6 feet 6 inches over the rapids.

From this it is evident it would be required to increase the improved navigation to a low-water depth over the rapids of at least five feet, in order to accommodate the passage of boats, when six feet navigation would be afforded.

II.

Before proceeding to the discussion of the different plans of improvement, the board will state their opinions in reference to the dimensions that any improvement should have, in order to fully accommodate the requirements of

First. If it be an improvement without locks.

The least width at any place should not be less than 300 feet. The depth should not be less than five feet in extreme low water, in order that boats may pass over the rapids with six feet, when they can carry six feet over the bars.

The ordinary low water is about five feet on the bars.

It will be seen by referring to the table taken from Lieutenant Warren's report, which was prepared with great care, that when there is five feet on the bars, (supposing the rapids improve to four feet in extreme low water,) there would be only four feet seven inches on the rapids; and hence a boat drawing the maximum draught up as far as the foot of the rapids would still be unable to pass over them. The ordinary low water, that is, the average depth from about the 1st of July till the 1st of December, will give about five feet on the bars, and boats will invariably load to that depth, if they can get freight and pass over the bars in safety, even though they rub along the bottom; but they cannot be allowed to rub over the rocky bottom of the rapids—they ought to have at least six inches to spare. This would require five feet six inches on the rapids when there are five feet on the bars, and the improvement will then give five feet depth on the rapids in extreme low water.

This the board regard as the least depth that can be given, and make an improvement that will adequately accommodate the requirements of commerce.

Second. If it be an improvement involving the use of locks.

The same depth, viz., five feet in extreme low water, must be had on the mitresills of the locks, and the width of the water-way should not be less than 250 feet in excavation, and 300 feet in all other cases, when it could be had without

a great increase in the cost of the work.

If it be a canal improvement, taking the pilots' gauge at Montrose as a guide for extreme low water at that place, the canal bottom at the upper end, and the level of the top of the mitre-sills of the guard-lock, should be fixed at five feet below extreme low water. The lower mitre-sill of the lower or outlet lock should be fixed at the same distance—five feet below extreme low water (viz., that of 1864) at that place. The lockage to be effected by means of two lift locks, using the guard-lock as a lift lock, after the water at the head of the rapids rises four feet above the extreme low water.

Third. Width of the lock chambers should be eighty feet, in order that the largest boats navigating the Mississippi or its tributaries may pass through it.

According to the information in the possession of the board, this width of lock chamber will pass any steamer on the Mississippi, with probably only one or two exceptions, and will allow an increase in beam of the majority of steamers that now havigate the upper Mississippi.

It is believed that the time is not far distant when eighty feet width of lock chamber will be as little as will lock through ordinary steamers on the upper Mississippi, though it may now seem large. Eighty feet is the width of lock chamber adopted for the canal around the falls of the Ohio; less than that should not be adopted here.

Fourth. Each lock should have two pairs of gates.

Fifth. Length of lock should be 350 feet between the quoins.

Sixth. The locks should be filled by means of valve or slide gates in the main gates, and culverts and openings passing at right angles through one of its side walls—the number and size of the culverts to be sufficient to fill the lock in as short a time as possible consistent with the safety of the masonry and boats. The locks to be emptied in like manner through culverts in the opposite side wall, and valve gates in the lower gates. The details of construction and mode of operation should be left to be determined by the engineer in charge of the work.

Seventh. That the middle and lower locks should be built high enough to maintain eight feet depth of water in the levels, so that boats of a maximum draught of seven feet may pass over the mitre-sills with one foot to spare. The guard-lock should be built high enough to lock boats through when the gauge

at Montrose shall indicate twelve feet above the low water of 1864.

Eighth. The bottom of the canal should slope about one and one-half inch to the mile, when rock excavation on the bottom occurs, in order that a more rapid current may be given to the water at those places, should it be necessary to wash out sediment.

Ninth. The width of the canal embankment should be not less than ten feet on top, including the riprap covering, the slopes of the embankment should be made with one and a half base to one vertical on both sides, and the average thickness of the riprap covering, on the river side, need not exceed two and a half feet, and on the canal side two feet, and on top one and a half feet.

The general range of the top of the embankment should be made not less than two feet above the range of extreme high water. In excavating, however, a large amount of rock will be at the disposal of the engineer, and can be advantageously used in increasing the thickness of the riprap covering of the embankment if deemed necessary. The guard-lock walls to be two feet above the extreme high water, the other lock walls to be high enough to maintain eight feet depth in the levels.

In case any other than a canal plan should be adopted requiring locks to overcome the fall, they should have the same general dimensions and the same general arrangement of their several parts, so far as may be applicable to the case.

III.—Plans of improvement discussed—Plan of improving the natural channel of the river by excavating the rock.

This plan was first recommended by Lieutenant Lee of the engineers in 1837, and some work was accomplished during the next two succeeding years under his direction.

The plan was further partially carried out under the direction of J. G. Floyd. United States agent, some twenty years later. About \$350,000 have been spent, and as nearly as can be ascertained about 25,000 cubic yards of rock excavated.

It is conceded by pilots, notwithstanding the many assertions to the contrary that the navigation on Lamillies and Lower chains has been somewhat benefited, but in no degree commensurate with the amount of money expended, when the price of labor and materials at that time is taken into consideration.

General Wilson, in his report, speaking of this plan of improvement, says:

"The plan of excavating the channel is for a variety of reasons exceedingly difficult to execute at these rapids, either by blasting under water or by the use of coffer dams. In order to enlarge the channel to 200 feet wide and four feet depth in extreme low water, (low water of 1864,)

* * * according to data recently obtained, it will require the excavation of 176,519 cubic yards of rock, which, at an average of \$15 per yard, will cost \$2,662,797.

"Should this channel be completed it will not accomplish all that is required,

for, in addition to the dangers consequent upon cross-currents, it is only indicated by the faintest ripple marks of the running water, and could not, therefore, be used either at night or in fogs, or during unfavorable winds, and a special pilot would be required at nearly all times. Hence, over half the time of extreme low water, the river would still be impassable at the rapids. Other difficulties would also continue to exist. The fall of eighteen feet in seven and a half miles with an increased current would have to be overcome at a great expense in money and time by ascending boats, and the navigation for descending boats would not be shorn of its dangers. In addition to this the water is nowhere deep, and, as the excavation has to be carried lower than is actually required, the tendency is to draw the water from the shores and above and to proportionably transfer the shallow places."

The board concur with General Wilson in the opinion expressed by him that the channel improved to 200 feet in width and four feet depth in extreme low water is not an adequate improvement, and that it would not fully accommodate the requirements of commerce, even supposing there was no difficulty experienced from its crookedness and narrowness, no dangers to be encountered, and no difficulties in navigating the channel at night, and in windy or foggy

weather.

It will be seen by referring to the table of stages on the rapids and on the bars prepared by Lieutenant Warren, that with the rapids improved to four feet depth in low water it will only give five feet on the rapids when there is six feet on the bars above and below. This would necessitate the loading of boats a foot lighter to pass the rapids than would be requisite on the bars, or would necessitate a transfer of the freight by lighters or otherwise. Boats frequently load to six feet or more when the stage of the water will allow it, and from this improvement such boats would derive little benefit.

From this it will be seen that a channel improvement ought not to be less than five feet deep in low water, for though it might materially ameliorate the present condition of the rapids to make a channel four feet deep, it would be only a partial remedy of the difficulties. Again, a crooked channel 200 feet wide, exposed in very many places to cross-currents, with a velocity of from three to six miles per hour, is inadequate to afford the facilities that the growing commerce of this great river demands. Many of the same difficulties that are

now experienced would still be encountered.

The board is of opinion that a channel at these rapids less than 300 feet wide and five feet deep in low water, following the natural channel, would fail to be

such an improvement as is required.

The following estimates, based on the surveys made last fall by General Wilson, will show the amounts of rock excavation and estimated cost of making this sort of channel improvement:

A channel 200 feet wide and four feet deep in extreme low water, is estimated

to require the excavation of 176,519 cubic yards of rock.

The board have estimated for a channel 200 feet wide and five feet deep, 318,562 cubic yards; a channel 300 feet wide and four feet deep, 291,869 cubic yards; a channel 300 feet wide and five feet deep, 503,435 cubic yards.

General Wilson estimates that this work will cost \$15 per yard, and this is

probably a fair price at the present price of materials and labor.

Supposing that in the case of the channel 300 feet wide by five feet deep, on account of the increased amount, it can be done for \$10 per yard, and in the other two cases at \$11 and \$12 50 per yard respectively—we will then have for the estimated cost of the channel improvements as follows:

200 feet wide by 4 deep, at \$15 per yard	\$2,647,785
300 feet wide by 4 deep, at \$12 50 per yard	3,648,362
200 feet wide by 5 deep, at \$11 per yard	3,504,182
3000 feet wide by 5 deep, at \$10 per yard	5,034,350

IV.—Plan of dams entirely across the river, with locks for steamboats and chutes for rafts and flat boats, creating a slack-water navigation in the river.

As this plan has been advocated by several engineers, and as there are no insuperable difficulties in executing it, the board will describe what they conceive to be the best location for such a plan of improvement at the Des Moines rapids.

It will require at least three dams to overcome the fall from the head to the

foot of the rapids.

In the lowest stages there is a fall of about 22 feet on the rapids. In extreme low water, the first dam must raise the water on the upper chain 4.2 feet; 0.8 feet being the shoalest depth in the channel on that chain, this will increase the fall from the head to the foot of the rapids to 26.2 feet, which should be divided into three lifts of 8.73 each. The upper dam might be built across the river near station "A" of General Wilson's survey, the second near station "O," and the third or lower dam near the site of the "Keokuk Packet Landing."

The locks should all be on the Iowa side.

The first dam would be 12.9 feet higher than the surface of extreme low water at that place; the middle dam 11.5 feet above low water at that place; and the lower dam 8.7 feet above low water at the Keokuk indicator; or, what is the same thing, the middle and lower dam should each be high enough to raise the water at the foot of the one preceding it to such a height that boats reaching the rapids from above or below could always find plenty of water to pass through the chutes or locks.

For the purpose of estimating the cost of this plan, the dams are assumed to be built of heavy crib ten feet from centre to centre, filled with stone, the upper slopes with $2\frac{1}{4}$ feet base to one foot perpendicular, the lower slope $4\frac{3}{4}$ to one.

The timbers of the crib should be about one foot square, notched two inches and bolted; the upper slope should be covered with two courses of plank, two and four inches thick respectively; the lower slope with timbers one foot square.

Each dam should be provided with at least one chute, at the most favorable place for navigation. At this place the dam, instead of being built up to the full height, should be finished off at the height of the upper end of the chute for a width of 320 feet.

At a distance of 150 feet on each side of the axis of the chute a crib should be built, high enough to be beyond overflow in the highest stages of the river, ten feet thick, covered with timbers, planked on the channel face; each chute should be at least 150 feet in length—a greater length preferable.

A section of a dam and chute that might be applied in carrying out this plan of improvement at the Des Moines rapids is shown on the accompanying maps. The locks should agree, both as regards the dimensions and general arrangement, with those already prescribed.

The following is an estimate of the probable cost of such a plan of improve-

ment:

UPPER DAM, SIXTEEN FEET HIGH AND 4,500 FEET LONG.

681,667 lineal feet twelve-inch timber, at forty cents 855,450 feet, board measure, oak and pine covering and sheet		80
piling, at \$45 per thousand	38, 495	25
80,497 cubic yards stone filling, at \$1 50	120,745	50
31,364 cubic yards gravelling, at fifty cents	15, 682	00
411,280 pounds bolts and spikes, at fifteen cents	61,692	00
Two abutments, 1,480 cubic yards masonry, at \$10 per yard	14,800	00
One chute	22,000	00

5,000 cubic yards excavation, at \$1 per cubic yard	\$5 , 000 2 , 40 0	
	553, 4 81	55
Add ten per cent. for contingencies	55, 348	
Total estimated cost of upper dam and one chute	608, 829	
MIDDLE DAM, 141 FRET HIGH, 4,800 FRET LONG.		
652,800 lineal feet twelve-inch square timber for cribs, notched and		
bolted, at forty cents		00
993,600 feet, board measure, oak and pine covering and sheet		
piling, at \$45 per thousand	44, 712	00
81,600 cubic yards stone filling, at \$1 50	122, 400	
28.800 cubic yards gravelling, at fifty cents	14, 400	00
393,600 pounds bolts and spikes, &c., at fifteen cents	59, 040	00
Two abutments, 1,400 cubic yards masonry, at \$10	14, 000	
5,200 cubic yards excavation, at \$1	5, 200	
8,500 cubic yards embankment, at thirty cents	2, 550	
One chute	22,000	00
	545, 422	00
Add ten per cent. for contingencies	54, 542	
and the por source source outling outlines.		
Total estimated cost of middle dam and chute	599, 964	20
LOWER DAM, $13\frac{1}{2}$ FERT HIGH, 4,200 FERT LONG, AND EMBA	NKMBNT.	
504,000 lineal feet twelve-inch timber, square, for cribs, notched		
and bolted, at forty cents	\$ 201, 600	00
piling, at \$45 per thousand	35, 910	00
67,200 cubic yards stone filling, at \$1 50	100, 800	
23,100 cubic yards gravelling, at fifty cents	11, 550	00
336,000 pounds bolts spikes, &c., at fifteen cents	50, 4 00	00
1,300 cubic yards, two abutments, masonry, at \$10	13, 000	00
One chute	22, 00 0	
11,000 cubic yards excavation, at forty cents	4, 400	
58,000 cubic yards excavation, at \$1	5,800	
23,700 cubic yards embankment, at thirty cents	7, 110	00
	452, 570	00
Add ten per cent. for contingencies	45, 257	
Lower dam complete, with chute		-
<u>-</u>		=
RECAPITULATION.	A 000 000	~ ^
Estimated cost of upper dam	\$ 608, 829	
Estimated cost of middle dam	599, 964 497, 827	
Estimated cost of three dams	1, 706, 620	90
the canal plan	763, 471	00
Total cost of this plan	2, 460, 091	90 ==

The foregoing estimate is made for the purpose of showing how this plan of improvement will compare with others that have been or will be referred to in regard to cost; the plan is one that has frequently been advocated on account of its supposed cheapness.

The board are of opinion that a weaker dam than the one they have based

their estimate upon would not answer in a great river like the Mississippi.

Some improvement could be made in this dam, particularly in regard to the length of the chutes, which would give more facilities for commerce, but would add to the cost.

This plan possesses no advantages over the plan of the canal that will presently be described; on the contrary, the advantages are all on the other side.

It is an important fact, which should not be overlooked, that the Mississippi river has, for the greater portion of the year, a good navigation over the rapids. During two-thirds of an ordinary season no improvement to the navigation is necessary or required.

The board hold that this fact should be prominently kept in view, and whatever plan is adopted, it should be one that will not interfere with the navigation

in high stages.

No improvement should be constructed at this place for low-water stages that

would itself become an obstruction in high water.

It is true that in the plan of locks and dams a sluice navigation might be made in the dams, which would pass rafts, &c., down stream tolerably well in low water, and which steamboats could ascend in medium high stages, but in order to make a good ascending navigation in moderate stages of the water, much longer chutes would be necessary than those described herein and estimated for.

The cost of maintaining the locks of a canal would be about the same as locks

in this plan.

The cost of maintaining the dams, however, would be much greater than that for maintaining the canal, they being much more subject to injury from ice. Well built dams might stand for years without material repairs, but it would not be proper to assume that such would be the case, and therefore an estimate for superintendence and repair is an important item, and as the money for this purpose, as the laws now are, must in all cases come by direct appropriation, delay might some time cause the destruction of the entire work. If we assume that in the course of fifty years a dam would cost as much in superintendence and repairs as the original cost, (and this is deemed by the board a fair estimate,) that would be two per cent. per annum on the entire cost; and as the dams are estimated to cost \$1,706,620 90, this would give \$34,132 42 annually, and allowing \$15,000 for the locks, (the same as in the canal system,) would make the annual cost of maintaining this system of improvement \$59,132, to be made by direct appropriations from the treasury.

A law of Congress prohibits the levying of a tax on the produce, &c., passing

through the locks.

While the board do not entertain a doubt as to the practicability of carrying out this plan, the fact of the volume of water passing over the rapids is so great as to require no improvement in the navigation for the greater part of the year; and in view of the fact that the adoption of this plan will destroy that navigation, and as the cost is no argument in its favor; and as it has no advantage not possessed by the canal improvement referred to, but on the contrary some disadvantages, the board cannot recommend this plan for adoption. It is accordingly rejected.

V.—Plan and position of an improvement proposed by G. Edmunds, jr., of Illinois.

The following plan has been advocated by certain parties in Illinois, and is

thus described by G. Edmunds, jr.:

"A dam should be erected from the island at the Upper chain to the Illinois side, and a wall should be built from the west end of the dam down parallel with the Iowa shore, about one (1) mile, thereby deepening the water on the Upper chain, and continuing the water in a regular channel until it falls into deep water below the chain. By this means, a sluice five (5) feet deep on the chain could be produced with a fall within its length of about three and a half (31) feet per mile—a foot less than the present fall on the Lower chain, and, consequently, giving much less velocity of current. By this mode, all obstructions presented by the Upper chain will be overcome, and the navigation made complete to the head of Spanish chain, a distance of between four and five miles. . The dam could be constructed by cribs filled with rock, or with brush and stone, say three feet deep, 4,200 feet long, and twenty-four feet wide, containing 11,200 yards, at an expense of \$11,200. The wall to consist of earth covered with riprap three feet deep; total, fortyfive feet at the base, nine feet on top, and eighteen feet deep on an average, at an expense of \$52,800.

"At the head of the dam there must be an ice-breaker of heavy masonry—about 100 yards—at an expense of \$2,000, making a total expense of improv-

ing the Upper chain of \$101,200.

"Continuing the same plan of improvement, commence on the Illinois side of the river, and put in a similar wall from the deep water at the head of Spanish chain, at a distance of say 500 or 600 feet from the east bank, and east of the present steamboat channel to deep water below English chain, a distance of about four miles, in which the river falls about nine and a half feet. By straightening the projection on the east bank of the river, this would present a sluice of four miles in length, in which, by narrowing the sluice at the rate of about ten feet per mile as you go down, an even and regular current may be had at any desired depth.

"Construct a dam from the west bank of the river, near Nashville, to the head of this wall, raising the water three feet, will give a depth of five feet in the sluice, forming an inclined plane of four miles, in which there will be a fall of twelve and a half feet, being at all times less fall per mile and less velocity

than on the Lower chain at present.

"The expense of this work will not exceed \$355,200, calculated on the same basis as that proposed on the Upper chain. This would perfect the navigation

to the head of the Lower chain.

* * "Construct a dam from the west bank of the river to Fille Rock chute, at the head of the Lower chain; also, from the east shore to Sucker chute, the ends of the dams approaching within about 600 or 800 feet of each other, and constructing two walls like those herein before described, nearly parallel, but approaching each other at the rate of twenty-five feet each per mile until deep water is reached below the chain; then continuing one-fourth of a mile further, diverging at the same rates, forming a chute or sluice 600 to 800 feet wide, and about one and a fourth to one and a half miles long. The dams, walls, and heads of walls constructed as described for Upper chain. This would leave both Sucker and Fille Rock chutes within the sluice, and by raising the dam at the head of the chain three feet, would give about five and a half feet in the Lower chain, with a fall in the sluice, say one and a half miles in length, of about seven and a half feet—less than double the fall now overcome in one-half mile, being only five feet to the mile. * * The dams would cost about \$12,000; the heads of the walls about \$4,000, and the walls themselves (total

length three miles) \$264,000, making a total expense on the Lower chain of \$280,000, and a total for the improvement of the entire rapids \$736,400."

The board will proceed to point out grave errors into which Mr. Edmunds has

fallen.

In his estimate of the cost of the entire improvement it would appear that his plan has economy in its favor; but this is only an appearance, not substantiated by facts. In the first place, he has estimated \$11,200 for the cost of a dam from Montrose island to the Illinois shore made of cribs filled with stone.

A stone and brush dam that he refers to is altogether out of the question on such a river as the Mississippi, and would be swept away in one season by the

ice.

The dam he proposes will contain 11,200 cubic yards, at the rate of one dollar per yard. It is hardly necessary to compare an estimate at such rates with those upon which other works herein referred to are based.

The board is of the opinion that at any point on the rapids nothing but the most substantial structure should be placed—a dam of loose stone and crib-work

should be certainly bolted to the rocks.

The proposed dam for the Upper chain, three feet high, is not sufficient to raise the water 4.2 feet in the sluice, which rise is necessary in order to make it five feet deep in extreme low water. The dams should not be less than 4.2 feet high.

If such a plan should be adopted for passing the Upper chain as is proposed by Mr. Edmunds, the board would suggest that a dam less strong than the one represented in section on the map herewith should not be allowed at this place

nor any other on those rapids.

Again, in regard to the wall that should extend from Montrose island down the river parallel to the Iowa shore. This embankment will be subject to as great abrasion as a similar wall for a canal, and should not be of less dimensions. This the board have already established at maximum width on top of ten feet. Mr. Edmunds makes the average height of the embankment eighteen feet; this is nearly correct, but he omits to notice the fact that the greater part of Montrose island is under water in high stages, and consequently an embankment will have to be built along the whole length of the island.

In order to make five feet depth in the sluices in extreme low water, the water must be raised 4.2 feet, or else rock must be excavated from the channel. As this improvement is designed to obviate this difficulty, the depth must be had

by raising the water surface.

There is already a fall on this chain of one foot to the mile; adding the increased level to this, we would have a fall in the sluice of over five feet per mile, by no means affording good navigation for even the most powerful steamers.

The same observations in regard to the dam at the Upper chain will apply to the dam he proposes near Spanish chain. Mr. Edmunds speaks of straightening up the projections of the east bank along the line of the Illinois shore in the second sluice, but makes no estimate of its cost. This will be found to be no inconsiderable item, for it can only be done by the excavation of a considerable amount of rock.

Again, he proposed to keep the water in the sluice about three feet above the water outside, but no arrangement is made at the lower end for dropping into deep water. (The sluice is supposed to end when it comes to the deep-water channel.) If the embankment is extended a mile further down, widening out gradually so as to distribute this fall over a distance of a mile, there would then be at the lower end of the sluice a current, to be overcome by ascending steamboats, due to the fall of five feet per mile, or the head of water in the sluice, in addition to the regular fall in the river at that place, which is about two feet. If this embankment is not run down in this manner it would be impossible for the most powerful steamboats to ascend the sluice at all. No estimate is made of this very necessary extension of the embankment.

In describing his plan he says: "Raising the water three feet will give five feet depth in the sluice, forming an inclined plane of four miles, in which there will be a fall of 124 feet."

But if there is only one incline in the sluice, and that begins three feet above the natural level at the head, and gradually falls off to nothing at the end, there will be a large amount of rock excavation necessary in the sluice to make the five feet depth throughout the lower half of it. A full head of three feet must be maintained in the sluice throughout, else there will not be five feet depth at the lower end.

He says: "This plan would perfect the navigation to the head of the lower chain." In a few lines thereafter, he speaks of building a dam to "Fille Rock

chute at the head of the Lower chain."

It should be observed, if he regards this as the head of the Lower chain, he has failed to make an improvement of one of the worst places in the rapids, viz: Montebello crossing, about half a mile above Fille Rock chute. There is a fall between these points of over two feet, and the fact that he says "the dam at the head of the sluice would give about $5\frac{1}{2}$ feet on the Lower chain," is evidence that he does not regard Montebello crossing as a part of the Lower chain, because a dam three feet high could not, located where he proposes it, raise the water at Montebello crossing $3\frac{1}{2}$ feet, which it would have to do. In order to get three feet more water in Montebello crossing than there is at present, the dams should be moved further up the river, or raised higher, and to do either would materially increase the cost of the embankment walls of the sluice.

Then, again, the latter are estimated on the same basis as the embankment

which he proposes to run from the lower end of Montrose island.

It has been shown in this report that the range between the surfaces of extreme high and lower water at the foot of the rapids is 21.721 feet, and hence his sluice walls, the tops of which are only eighteen feet above the bottom of the river, would be overflowed in even ordinary high water.

In order to be properly protected from this danger the embankment should not be less than 23.721 feet above the surface of extreme low water. The average depth from the head to the foot of the proposed sluice is not less than four and a half feet, (the lower part being in deep water,) and hence will require an embankment about twenty-eight feet instead of eighteen feet high.

With these corrections made, and for the purpose of instituting a fair comparison between this and the other plans of improvement, the board have made

the following estimate of its probable cost:

ESTIMATE FOR DAM FROM MONTROSE ISLAND TO THE ILLINOIS SHORE FOR EACH ONE HUNDRED FEET IN LENGTH.

23,301 feet board measure heavy timber for cribs, at \$50 per M.	\$1, 165 05
104 cubic yards loose stone, at \$1 50	156 00
90 cubic yards gravelling, at \$1	90 00
664 pounds bolts, spikes, &c., at 15 cents	99 60
Total for one hundred feet	1,510 65
4,200 feet in length	\$ 63, 447 30
214 cubic yards masonry abutments, at \$10	2, 140 00
Coffer dams and bailing	25,000 00
Preparing foundation	2,500 00
Total	93, 087 30
Contingencies and engineering, 15 per cent	13, 962 78
Total for upper dam	107, 050 08
·	

2.244 cubic yards riprap covering, at \$2	97,000 cubic yards of embankment, one mile long, from Mon-	A .o.
10,009 cubic yards embankment on Montrose island, at 50 cents. 7,022 cubic yards riprap on Montrose island, at \$2	trose island, at 50 cents	\$48, 500 00
7,022 cubic yards riprap on Montrose island, at \$2	32,244 cubic yards riprap covering, at \$2	
Contingencies and engineering, at 10 per cent. 134, 036 50 13, 403 65 147, 440 15 Total for dam and sluice. \$254, 490 23 SECOND DAM. 5 miles sluice wall, average height 22 feet. \$94, 007 60 743,110 cubic yards earth embankment, at 50 cents. 371, 555 00 181,865 cubic yards riprap, at \$1 50 272, 797 50 107 cubic yards masonry at head of the wall, at \$10 1,070 00 Straightening the projections on the east side of the river in the sluice, 298, 129 cubic yards of earth paid for as embankment, 37,037 cubic yards rock, at \$2 74,074 00 Contingencies and engineering, 10 per cent. 813, 504 10 Total for second dam and sluice. 894, 854 51 LOWER DAM AND SLUICE. Dams. \$81, 407 36 3 miles of embankment; 747,411 cubic yards earth, at 50 cents. 373, 705 50 124,371 cubic yards riprap, at \$2 248, 742 00 Head walls: 285 cubic yards masonry, at \$10 2,850 00 Contingencies and engineering, 10 per cent. 706, 704 86 70,670 48 Lower dam and sluice. 777, 375 34 RECAPITULATION. Upper dam and sluice. \$254, 490 23 Middle dam and sluice. 894, 854 51 Lower dam and sluice. 894, 854 51 Lower dam and sluice. 894, 854 51 Lower dam and sluice. 894, 854 51 Total for second dam and sluice. 894, 854 51 RECAPITULATION.	10,009 cubic yards embankment on Montrose island, at 50 cents.	
Total for dam and sluice \$254, 490 23	7,022 cubic yards riprap on Montrose island, at \$2	14,044 00
### Total for dam and sluice. ### SECOND DAM. SECOND DAM.		134,036 50
### Total for dam and sluice ### SRCOND DAM. SRCOND DAM.	Contingencies and engineering, at 10 per cent	13, 403 65
### SECOND DAM. 5 miles sluice wall, average height 22 feet		147, 440 15
### SECOND DAM. 5 miles sluice wall, average height 22 feet	Total for dam and sluice	
5 miles sluice wall, average height 22 feet		
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181,865 cubic yards riprap, at \$1 50		
1, 070 00 Straightening the projections on the east side of the river in the sluice, 298, 129 cubic yards of earth paid for as embankment, 37,037 cubic yards rock, at \$2	743,110 cubic yards earth embankment, at 50 cents	371,555 00
Straightening the projections on the east side of the river in the sluice, 298, 129 cubic yards of earth paid for as embankment, 37,037 cubic yards rock, at \$2	181,865 cubic yards riprap, at \$1 50	
Sluice, 298.129 cubic yards of earth paid for as embankment, 37,037 cubic yards rock, at \$2	107 cubic yards masonry at head of the wall, at \$10	1,070 00
Sluice, 298.129 cubic yards of earth paid for as embankment, 37,037 cubic yards rock, at \$2	Straightening the projections on the east side of the river in the	
Contingencies and engineering, 10 per cent	sluice, 298,129 cubic yards of earth paid for as embankment,	
Contingencies and engineering, 10 per cent	37,037 cubic yards rock, at \$2	74,074 00
Contingencies and engineering, 10 per cent		813, 504 10
LOWER DAM AND SLUICE. Dams.: \$81, 407 36 3 miles of embaukment; 747,411 cubic yards earth, at 50 cents. 373, 705 50 124,371 cubic yards riprap, at \$2 248, 742 00 Head walls: 285 cubic yards masonry, at \$10 2, 850 00 Contingencies and engineering, 10 per cent. 706, 704 86 70, 670 48 Lower dam and sluice. 777, 375 34 RECAPITULATION. Upper dam and sluice \$254, 490 23 Middle dam and sluice 894, 854 51 Lower dam and sluice 777, 375 34	Contingencies and engineering, 10 per cent	
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Samiles of embankment; 747,411 cubic yards earth, at 50 cents. 373, 705 50 124,371 cubic yards riprap, at \$2 248,742 00 Head walls: 285 cubic yards masonry, at \$10 2,850 00	LOWER DAM AND SLUICE.	
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124,371 cubic yards riprap, at \$2 248,742 00 Head walls: 285 cubic yards masonry, at \$10 2,850 00 To6, 704 86 70,670 48 Lower dam and sluice 777,375 34 RECAPITULATION. Upper dam and sluice \$254,490 23 Middle dam and sluice 894,854 51 Lower dam and sluice 777,375 34		
Head walls : 285 cubic yards masonry, at \$10		
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### RECAPITULATION. Upper dam and sluice ### \$254, 490 23 Middle dam and sluice ### 894, 854 51 Lower dam and sluice ### 777, 375 34	Contingencies and engineering, 10 per cent	
### RECAPITULATION. Upper dam and sluice ### \$254, 490 23 Middle dam and sluice ### 894, 854 51 Lower dam and sluice ### 777, 375 34	Lower dam and aluica	777 375 34
Upper dam and sluice \$254, 490 23 Middle dam and sluice 894, 854 51 Lower dam and sluice 777, 375 34	LOWGI GAM SHUNG	
Middle dam and sluice 894, 854 51 Lower dam and sluice 777, 375 34	RECAPITULATION.	
Middle dam and sluice 894, 854 51 Lower dam and sluice 777, 375 34	Upper dam and sluice	\$ 254, 490 23
Lower dam and sluice		894, 854 51
Total for improvement		777, 375 34
	Total for improvement	1, 926, 720 08

From this estimate it will be seen that the saving of money in carrying this plan into execution is not as great as it would appear from Mr. Edmunds's estimate.

The board entertain the opinion that, constructed according to the plan of Mr. Edmunds, with the modifications that have been indicated as necessary, it would still fail to produce the result claimed for it, and would consequently not be an adequate improvement. There are too many elements of uncertainty in

the plan to warrant the board in recommending it. It is experimental in its character, and would materially exaggerate one of the principal sources of difficulty of these rapids by making the fall several feet greater than it already is. In short, whatever may be the merits of the plan, if it has any, they have not been presented by Mr. Edmunds with sufficient accuracy and regard to detail to warrant the board in giving it further consideration; it is therefore rejected.

VI.—Plan and position of an improvement by means of an excavated shore channel with an exterior embankment on the Iowa side. Estimate of its probable cost; also of a similar improvement on the Illinois side.

Lieutenant Warren, in his report of his survey in 1853, says: "Captain Shreve, in his report in 1836, urgently recommends that these rapids be improved by excavating a channel ninety feet wide and five feet deep along the lows shore from Keokuk to Nashville, and through the Upper chain, near the foot of Montrose island." He states, "by pursuing this plan, the navigator will have the shore for his guide, and cannot miss the channel in any stage of water; consequently it will not be necessary to excavate a channel more than ninety feet wide, which width can be more easily navigated than a channel 300 feet wide, following the meanderings of the natural channel that now exists between the reefs."

Although this plan is practicable, the board do not concur with Captain Shreve in the opinion that ninety feet width is all that would be required. The fact is, that two ordinary sized steamboats that now navigate the upper Mississippi could not pass each other at all in such a channel, and even a large one could barely thread it. A channel of that kind, with the dimensions proposed by Captain Shreve, is out of the question at the present day.

The board will now describe a plan that could be carried out, and would make

a practicable channel, following the ideas of Captain Shreve.

Excavate a channel near the shore 300 feet wide, and with the excavated material and other material that could be obtained from the shore by straightening and smoothing its sides, construct an exterior embankment ten feet wide on top, with slopes of 1½ base to 1 perpendicular, with two feet thickness of riprap on both slopes. The embankment need not necessarily be as high as would be required in a canal, but high enough to be safe from submergence until the main river outside should be navigable. This channel could be run at all times when the river should be low enough to require it.

A channel like this could also be made on the Illinois side, extending from Edmunds's warehouse to Hamilton, but there would be a considerable increase of

embankment wall and rock excavation.

The main objection to such a work on that side, however, would be the river channel entrances, being on the Iowa side; it would necessitate the crossing of the river twice, and one of those would be at "Nashville crossing," a difficult place even in an ordinary low water. This plan, therefore, will not be further considered.

If the channel is made continuous from Montrose to Keokuk, to a similar improvement on the Illinois side, from Nauvoo to Hamilton, both in respect to cost,

location, and all other advantages, the Iowa side is again preferable.

Such a channel, 300 feet wide and six feet deep, would require the excavation of 3,556,543 cubic yards of material. The proportion of this classed as earth is 437,000 cubic yards, which, deducted from the total, leaves 3,119,543 cubic yards of rock excavation.

By throwing the channel further into the river the total quantity of excavation would be somewhat reduced, but would be almost entirely rock under water. It is evident, however, from an inspection of the profile and river soundings, that to obtain one regular inclined channel 300 feet wide, and six feet deep in low

water, would involve an excavation in rock about five feet in depth for a distance of about ten miles.

By throwing it into two inclines the quantity of rock excavation would be reduced to 2,595,555 cubic yards. By means of a temporary coffer-dam or embankment thrown across from the shore to the head of the proposed line of embankment, and the raising of said embankment a few feet above low water, keeping out the river, the excavation inside could be rendered comparatively dry, and most of the rock excavation could be made in the ordinary manner by drilling and blasting, except at and near the lower end, where it would have to be done by the use of coffer-dams. All the stone required for the embankment and riprap on its slopes and top would be obtained from the bed of the channel. The top of the embankment should be two feet above extreme high water, and have the same dimensions as the canal embankment hereinafter to be described.

By making the channel 5 feet deep and 200 feet wide the estimated cost will be \$3,317.874.

By making the channel 4 feet deep and 300 feet wide the estimated cost will be \$3,503,910.

By making the channel 200 feet wide and 4 feet deep the estimated cost will be \$2,752,933.

A comparison of side channel improvement just described, with that of the open river channel, in reference to cost, will show the following differences in this respect:

From the above comparison it will be observed that in every case except the first, the advantage in regard to cost is in favor of the side channel improvement.

The first and second of the above are not, however, regarded by the board as adequate improvements on account of the want of sufficient depth. They are both accordingly rejected.

The great cost of the latter plans, 300 feet wide and five feet deep, both of

the natural and side channels, will cause their rejection.

The other plan of a side channel 200 feet wide and five feet deep, and of the natural channel improved to the same width and depth, shows a difference in cost of \$186,308 in favor of the former.

Let us now compare them in other respects:

First. In regard to the time of execution.

It is the opinion of the board that the channel along the shore could be constructed in two years; provided, an amount of money equal to the estimated cost should be placed at the disposal of the superintending engineer. The work would be less liable to interruption from floods or ice, and would not interfere

with the navigation of the rapids during its construction.

In the improvement of the river channel the time at which it could be completed is extremely uncertain; the work would necessarily have to be suspended during the greater portion of the year, on account of floods and ice, and would be liable to interruption by sudden rises in the river during the progress of the work. This would be particularly the case if the work should be done with coffer-dams. If it should be done by chiselling and dredging during the low-water season, it would be necessary to suspend work a short time for the passage of each boat and raft.

The contingencies that might arise to increase the cost are much more numerous in the natural channel improvement than in the other. The latter will give for a width of 200 feet excellent navigation, free from all cross-currents, a well de-

fined outline and comparatively straight course.

The natural channel improved to 200 feet in width will remain still tortuous, be exposed to the action of cross-currents, and its outlines would not be well defined. In the side channel the slope of the water surface would be more even throughout, allowing the passage of boats through it that would not have sufficient power to cross the Lower chain in low water. The average current would be about the same in both cases, but in the side channel it would be at no place much more than the average, where, as in the natural channel, it would be very much greater in some places and less in others. The side channel could be navigated in all kinds of weather, by night or by day. In the other, navigation would often have to be suspended at night, in fogs, and during storms.

The difficulties of navigation at night might be somewhat ameliorated, however, by the establishment of floating lights; this would probably become necessary, but they would be of no advantage in fogs and would require continual

superintendence and replacing when carried away by rafts or ice.

The advantages, it will seem, are all in favor of the side channel improvement; consequently an improvement of the natural channel of the river cannot be recommended by the board.

VII.—Plan and position of a canal improvement on the Iowa side. Estimate of its probable cost.

The following is a description of a canal recommended by General Wilson and taken from his report:

"Construct an independent navigation canal along the Iowa shore, from a point near the present site of the 'Keokuk indicator,' at the city of Keokuk, to a point just below the village of Nashville; the balance of the distance to be overcome by using the natural channel, which from here to the Upper chain is

found to contain sufficient water for all purposes. At the Upper chain a through cut two hundred feet wide, six feet deep, and 2.400 feet long, will be necessary.

"A careful calculation shows that 54,882.29 cubic yards will have to be

excavated.

"The estimated dimensions of the canal are as follows: length, 7.6 miles, width on the water surface, 300 feet; and depth at the lowest stage, six feet, (with an alternate of five feet.) The stage of water here referred to is that of 1864, which was ten inches lower than any other season recorded, and about fifteen inches lower than any other season recorded, and about fifteen

inches below that of 1866, which may be taken as an average.

"The canal would therefore pass boats drawing full six feet, and have a sufficient depth in addition for perfect safety during seasons of average low water. It should run the entire distance except at one or two low points along the shore in the bed of the river; the river embankment, for strength and economy, should be constructed of earth, and securely covered inside and out with a well-made riprap of broken stone, so as to render it entirely safe against the running ice and freshets. It will require two lift-locks and one guard-lock; the lift-locks to be three hundred and fifty feet between the mitre-sills, eighty feet wide between the tops of chamber walls, (seventy-eight feet at the water surface.) and to lift respectively eight and ten and one-third feet. The lower locks should also be furnished with a set of guard-gates for security against floods, and the guard-lock proper, at the head of the canal, should be so arranged as to admit of being used as a lift-lock whenever required, which would be after the water in the river at the head of the canal had raised three feet above the extreme low-water mark.

"This construction will admit of nine feet draught, the utmost likely ever to be required for purposes of navigation, either by vessels of commerce or war, and will allow the canal to be used at all ordinary stages of the river so that ascending boats may avail themselves of the slack-water of the canal to overcome the increased currents of high water in the open river.

"It will be observed that in this canal a greater depth at extreme low water is provided for than is required by the present ruling depth of the river above and below; but as the work is intended to be permanent, it should be constructed so as to meet all possible contingencies of national defence and river improvement in the future.

* * This system of navigation, providing in the fullest manner for the improvement of the rapids, will cost \$3,390,000.

"Should it be deemed inadmissible or unnecessary by Congress to provide, in the dimensions of the proposed canal, for the ultimate development of the river, above and below, the cost may be reduced to \$2,731,722 96, by reducing the depth of water in the channel and throughout to five feet instead of six, so as to give good navigation for boats drawing four feet water. This may be ultimately diminished by the sum of \$34,155 should it be found unnecessary to construct the stone pier or wing-dam estimated for at the foot of Montrose island.

"At the head of the canal is located a guard-lock, with walls 21 feet highor two feet above high-water mark. This will be used as a lift-lock when the water in the river is more than three feet above low water. It will have a favorable location, independent of the river, and the cost of bailing and draining will be comparatively small.

"A lock of eight feet lift is located at a point about 5.6 miles from the upper end, and two miles from Keokuk. This lock also has a location independent of

the river, and not expensive for bailing and draining.

"At the lower end a lock of ten and one-third feet lift is located entirely in

"Its location is such as to make six feet of water on the mitre-sill at low water. The walls will be twenty-eight and two-thirds feet high, which will

carry them two feet above high water. In the centre is placed an extra set of

gates, which reverse in closing, to keep high water out of the canal.

"The river bank of the canal for the entire length is to be raised four feet above the high water of 1851, and is to be twenty feet wide on the top, with an outside slope of one and a balf to one, and an inside slope of one and a quarter to one, with a heavy riprap wall on the outside, and a lighter one on the inside and over the top. The height of the bank will vary from eighteen to thirty feet above the bed of the river.

"A large amount of rock excavation will be necessary at the guard-lock, and for one mile below, in order to obtain six feet depth in low water in the river;

also at the middle lock and for a half mile below.

"On the flats from four to eight feet of excavation are earth, and the balance rock. The excavated material will generally be used in the embankment and riprap walls, and can be hauled from the pit directly to the point required, and thus save the expense of borrowing the material from other points to form the bank.

"The character of the earth in the adjoining hills or banks is very favorable for forming solid and water-tight banks, but for a considerable part of the distance the rock runs so high on the hills that it will be expensive obtaining the same, and for this reason a high price has been used in the estimate.

"There is probably no stone on this side of the river that will answer for face stone for the locks, but for the backing and vertical wall stone can mostly be obtained here, and perhaps a large portion of the excavated rock can be used

for this purpose.

"On the Illinois or east side of the river, within a reasonable distance of the work, are located splendid quarries of magnesium and other limestone, and which can be delivered on the ice during the winter, or in boats during the summer.

* * * * *

"If a dam were built across the river at Nashville, raising the water four feet, the most of the rock excavation in the canal, and a very large part of that in the channel at the Upper chain, would be avoided, and the cost of the whole work materially reduced. Such a dam would, however, prevent boats from navigating the river, and compel them to use the canal even when the water in

the river is of sufficient depth; it is therefore inadmissible.

"The annexed estimates for the canal are based on three hundred feet width and six feet depth; but it will be seen that by reducing the width to two hundred feet, where the excavation of earth and rock occurs, a saving of two hundred and eighty thousand dollars can be made. This reduction would occur in detached portions, where the canal leaves the river, and, in my opinion, would answer every purpose. If it should afterwards be considered necessary, the additional one hundred feet could be excavated during the suspension of navigation.

"In order to obtain the materials for embankment, and to locate a good line for the canal, it would be necessary to alter in several places the line of the public highway, and of the Keokuk and Fort Madison railroad. The total length of each which requires alteration is about three miles, and the cost is

embraced in the annexed estimates.

"There are several small streams which will have to be taken in, and as the river guard bank must be carried very much above the surface of the water in the canal, no waste-weirs can be constructed at these points. The most important of these streams is Price's creek.

"If they in time of freshets bring too much water, it will have to be passed off in sluices, which must be constructed around the lift-locks.

"The cost of these sluices is provided for in these estimates.

"TIME REQUIRED TO CONSTRUCT THE WORK.

"In the construction of the canal, the first season should be occupied in excavating a part of the canal, putting in the bottom of the outside of the riprap wall for the entire length, and raising it from ten to fifteen feet high. This can be done, and allow the high water of the next spring to overflow it and do no During the second year the excavation should be continued, the earth for embankment put in, and the outside riprap wall completed. The third season the excavation for the canal should be finished, the inside riprap wall put in, and the remainder of the bank and walls completed.

"For the locks, the first season should be occupied in procuring, preparing, and delivering materials at the work. The second season the coffer-dams should be put in, the lock-pits excavated, the foundations prepared, and portions of the masonry laid. During the third year the masonry, gates, embankment, and

all the other work should be completed.

"In this manner, if the work is commenced in the ensuing summer, the whole can be completed in the fall of 1869, or in about two and a half years.

"COST OF MAINTENANCE.

"If the demands of commerce shall ever require a double set of locks, the proposed ones can be so located on one side of the centre as to have sufficient room for the construction of other locks by their side, without in any manner disturbing the outside river embankment wall.

"In the prism of the canal, and in the locks, provision is made for a depth of nine feet, when there is a rise of three feet in the river, in order that boats of

eight feet draught may pass through the canal.

"The gates are to be of wood, properly arched on the upper side, and strengthened with wrought-iron braces or hog chains on the lower side. This, it is believed, will answer the purpose designed, and the expense will be much less than that of iron gates. It is proposed to hang the gates on the suspension plan, which has been successfully applied to the locks on the St. Mary's canal, between Lakes Huron and Superior.

"Detailed estimates of the cost of all the works are hereto annexed, of which the following is a summary:

"For a canal 300 feet wide and six feet deep, with a channel at feet deep-	Montrose six
Cost of canal embankment and walls Cost of lift-locks Cost of middle lock Cost of guard lock	\$1, 717, 480 371, 265 244, 910 242, 822
Total cost of canal and lock	2, 576, 477 619, 155
'Total	3, 195, 632 194, 368
Total cost	3, 390, 000
WT	and

"For a canal 300 feet wide in embankment, and 200 feet in ex	Cavation, and
six feet deep, with a channel at Montrose six feet deep-	
Cost of canal embankment and walls	\$1, 454, 680
Cost of lower lift-lock	371, 265

Cost of middle lift-lock	\$244, 910 242, 822
Total cost of canal and locks	2, 313, 677 619, 155
TotalAdd contingencies and engineering	2, 932, 832 177, 168
Total	3, 110, 000
"For a canal 300 feet wide and five feet deep, with a channel at feet deep—	Montrose five
Total for a canal six feet deep, and locks as above Deduct difference in cost for five feet	\$2, 576, 477 220, 000
Cost of canal and locks	2, 356, 477
Cost of channel and pier	461, 655
Total	2, 818, 132 171, 868
Total cost	2, 990, 000
"For a canal 300 feet wide in embankment, 200 feet in excava feet deep, with a channel at Montrose five feet deep—	ation, and five
Total for a canal six feet deep, and locks as above Deduct difference in cost for five feet	\$2, 313, 677 220, 000
Cost of canal and locks	2, 093, 677
Cost of channel and pier	461, 655
Total	2, 555, 332 154, 668
Total cost	2, 710, 000"

The board have already, in their report of April 13, 1867, made known to the Chief of Engineers their views in regard to the plan and location of the improvement that they consider the best adapted to accommodate the demands of commerce, and which will give all, or at least the most important advantages of any plan that has been considered, and with considerably less expense than most of them. That plan, it will be remembered, is essentially the same as that recommended by General Wilson, with certain modifications, which will lessen the cost without changing the character of the improvement.

The following is a detailed estimate of the cost of this plan:

Estimate of the cost of constructing 7.6 miles of canal, from Nashville to Keokuk, for the improvement of the Des Moines rapids of the Mississippi river.

provement of the Des Moines rapids of the Mississippi river.	
For a canal 300 feet wide in embankment and 250 feet in excavation, and 5 fe	eet:
	\$3, 000 60
Grubbing and clearing	45,000 (0)
161, 440 cubic yards excavation of rock, at \$2	322, 880 00
315, 870 cubic yards excavation of earth, at 40 cents	126, 348 00
651, 500 cubic yards embankment, at 50 cents	325,750 (II)
203, 500 cubic yards hauled from excavation, at 25 cents	50,875 (0)
10.000 cubic yards lining, at 60 cents	6,000 00
50,000 cubic yards puddling, at 25 cents	12,500 00
88, 000 cubic yards loose stone in riprap wall, at \$1 50	132,000 00
80,000 cubic yards stone hauled from excavation, 75 cents	60,000 00
1,000 cubic yards slope and pavement wall, at \$2	2,000 00
5,000 cubic yards vertical wall in cement, at \$5	25,00 0 00
3,000 cubic yards vertical wall laid dry, at \$4	12,000 00
3 miles changing lines of public roads, at \$1,000	24, 000 00 3, 00 0 00
·	1, 150, 353 (0)
Estimate of the cost of constructing lower lock of $10\frac{1}{4}$ feet lift with walls 23 fe improvement of the Des Moines.	et high, for the
Grubbing and clearing	\$10 0 (0)
	40,000 00
Bailing and draining	32, 580 (N
1,000 cubic yards excavation of earth, at 60 cents	600 00
45,000 cubic yards embankment, at 50 cents	22, 500 00
4,000 cubic yards lining, at 75 cents	3,000 00
8,000 cubic yards puddling, at 30 cents	2,400 00
1,000 cubic yards slope and pavement walls, at \$2	2,000 00
900 cubic yards loose stone, at \$1.50	1,350 00
4, 650 cubic yards vertical in hydraulic cement, at \$6	27,900 00
450 cubic yards vertical laid dry, \$4 50	2,025 00
	129, 67 5 (€) 3, 00 0 (€)
500 cubic yards masonry in lock walls, at \$6	7, 330 (0)
50,000 feet white pine, at 60 cents	3,000 00
72,000 pounds wrought iron, at 18 cents	12,760 00
28, 450 pounds cast iron, at 12 cents	3, 414 00
4,500 pounds spikes and nails, at 12 cents	540 00
160 lineal feet snubbing posts, sulphur, &c., at \$1	160 00
Sand cement for irons let into masonry	500 (x)
Painting upper part of gates	10 0 00
Painting upper part of gates	6, 500 (0)
Fixtures for opening valves	1,000 00
House for lock tender	1,500 00
	304, 134 (1)
Estimate of the cost of constructing middle lock of 8 feet lift for the improven Moines rapids.	nent of the Des
Grubbing and clearing	\$300 (ii)
Bailing and draining	10,000 (0
17, 100 cubic yards excavation of rock, at \$2 50	42,750 (0)
19, 200 cubic yards excavation of earth, at 50 cents	9,600 (0)
12,000 cubic yards embankment, at 50 cents	6,000 (0
4,000 cubic yards lining, at 75 cents	3,000 (11)
8,000 cubic yards puddling, at 30 cents	2,400 00
500 cubic yards slope wall and pavement, at \$2	1,000 GO
800 cubic yards loose stone, at \$1 50.	1, 200 (4)
2,500 cubic yards vertical wall in hydraulic cement, at \$6	15,000 (0
300 cubic yards vertical wall laid dry, at \$4 50	1,350 00

461,655 00

8 100 auhia warda masanur in look walle at \$12		\$100 ATC	00
8, 192 cubic yards masonry in lock walls, at \$13		\$106,476	
300 cubic yards cement masonry, at \$6		1,800	
42,000 feet white pine timber and plank, at \$60	J	6,400	00
61 200 nounds reported from at 10 cents	••••	2,520	
64,300 pounds wrought iron, at 18 cents	• • • • • • • • • • • • • • • • • • • •	11,574	
27,500 pounds cast iron, at 12 cents.		3, 300	
4,000 pounds spikes and nails, at 12 cents	• • • • • • • • • • • • • • • • • • • •	480	
160 lineal feet snubbing posts, at \$1. Sulphur and sand cement for irons let into masonry		160	
Sulphur and sand cement for irons let into masonry		500	
Painting upper part of gates		100	
130 lineal feet superstructure for drawbridge, at \$50	• • • • • • • • • • • • • • • • • • • •	6, 500	
Fixtures for opening and closing gates		1,000	00
House for lock tender		1,500	00
		\$234,930	00
,	•		_
Estimate of cost of constructing guard-lock of 9 feet lift, with 5 feet extra walls, 18' 8", with head of lock 20' 8" high, for the in rapids.	feet depth of w sprovement of t	ater, and th he Des Moi	ree nes
O-bling and alassian		# 200	ΔΔ.
Grubbing and clearing		\$3 00	
Bailing and draining	•••••	13, 500	
19, 200 cubic yards excavation of earth, at 50 cents	• • • • • • • • • • • • • • • • • • • •	9,600	
17, 100 cubic yards excavation of rock, at \$250		42,750	
11,500 cubic yards embankment, at 50 cents		5,750	
4,000 cubic yards lining, at 75 cents		3,000	
8,000 cubic yards puddling, at 30 cents		2, 400	
8,000 cubic yards puddling, at 30 cents		1,600	00
950 cubic yards loose stone, at \$1 50		1, 425	00
1,500 cubic yards vertical wall in hydraulic cement, at \$6		9,000	00
400 cubic yards vertical wall laid dry, at \$4 50		1,800	00
7,598 cubic yards masonry, at \$13		98,774	00
400 cubic yards concrete masonry, at \$6		2,400	00
73,000 feet board measure white-oak timber and plank, at \$100.		7,300	
9,000 feet white pine timber and plank, at \$60		540	
62,000 pounds wrought iron, at 18 cents		11, 160	
23, 900 pounds cast iron, at 12 cents	4	2 868	
4,000 pounds spikes and nails, at 12 cents		480	
160 lineal feet snubbing posts, at \$1		160	
Sulphur and sand cement		500	
Painting unner part of crates		100	
Painting upper part of gates 130 lineal feet superstructure for drawbridge, at \$50	•••••	6,500	
Fixtures for anening and aloging gates	• • • • • • • • • • • • • • • • • • • •	1,000	
Fixtures for opening and closing gates House for lock tender		1,500	
110tise for fock tellder		1,500	
		224, 407	00
Estimate of the cost of excavating a channel and constructing a	nier at the Enn	er chain n	
Montrose island, for the improvement of the Des M	loines rapids.		
Expensions a channel 900 feet mide and E feet do-			
Excavating a channel 200 feet wide and 5 feet deep:	<u>.</u>		
Bailing and draining, including coffer-dams	\$130,000 00		
42,500 cubic yards excavation of rock, at \$7	297, 500 00	_	
		\$427,500	00
Construction view from fact of toland			
Constructing pier from foot of island:			
Bailing and draining	\$ 10,000 00		
1,208 cubic yards masonry in pier, at \$10	12,080 00		
330 cubic yards coping in pier, at \$20	7,000 00		
11,000 pounds wrought-iron dowels and clamps, at 15 cents	1,740 00		
067 putting dowel bolts into masonry, at \$3	2,001 00		
667 putting clamps into masonry, at \$2	1,334 00		
Total cost of channel and pier		34, 155	00
			_

SUMMARY OF ESTIMATES.

Cost of canal	\$304, 134 00 234, 930 00	\$1, 150, 353	00
Cost of guard-lock	224, 407 00		
o		763, 471	00
Cost of channel and pier at Montrose		461, 655	00
		2, 375, 479	00
Contingencies and engineering			
Total		2, 530, 000	00

VIII.—Plan and position of a canal improvement on the Illinois side of the river. Estimate of its probable cost.

It has been proposed to make a canal on the Illinois side of the river, of similar dimensions to the one recommended by the board. As this plan has many advocates in the State of Illinois, the board will describe it and make an estimate of its cost.

Commencing on the Iowa side at Montrose, an improvement should be made for passing the "Upper chain," after which there is good navigation as far as Nashville. Whatever may be the plan adopted for the canal improvement on the Iowa side, the same will be applicable for the one on the Illinois side. The board will therefore assume that they are identical, and the estimate cost of this part will be the same in both cases. From Nashville following the river channel, cross over to the Illinois side, and commence a canal in the deep water near Edmunds's warehouse, by excavating through the point of land at that place, and then building an embankment parallel to the Illinois shore and about 300 feet from it, ending in deep water just below the Hamilton ferry dike. An interior embankment to be built from the bluffs at Cheney's creek to the same point where should be located the lower or outlet lock. The canal to be provided with two lift-locks and one guard-lock, the middle-lock about two miles above the lower one, the guard-lock at the head.

All the details as to size and general arrangement of the locks and the size and structure of the embankment, to be the same as in the case of the canal recommended by the board.

The steamboat channel near Nashville strikes directly across from the Iowa to the Illinois side, and in extreme low water affords less than two and one-half feet depth. This difficulty could only be obviated in two ways, either by throwing a dam across the river to the Iowa shore from the head of the canal, or by the excavation of the rock from the bed of the river.

Though the former plan would doubtless be the most economical, the fact that it would destroy the navigation in the river, and force every craft into the canal in all stages, must prevent its application; and a channel should be cut from the deep water on the Illinois side to the deep water channel on the Iowa side, at least 300 feet wide and five feet deep in extreme low water.

The following is the estimated cost of this plan of improvement:

For a canal 300 feet wide in embankment, and 250 feet in excavation, and five feet deep, extending from Edmunds's warehouse to the Hamilton ferry dike, with an excavated channel through "Upper chain" and a pier from Montrose island about one mile long.

Grubbing and clearing	\$3 , 000 (#)
14 miles of Dailing and Graining, at \$30,000	45, (iii) (h)
243,000 cubic yards excavation of rock, at \$2	486, 000 (1)

200 000 autic contraction of contract at 40 contract.	A10.2 000	00
320,000 cubic yards excavation of earth, at 40 cents	\$128,000	
731,000 cubic yards embankment, at 50 cents	365,000	
731,000 cubic yards embankment, at 50 cents	51,250	
12,000 cubic yards lining, at 60 cents	7,200	00
52,000 cubic yards puddling, at 25 cents	13,000	00
107.000 cubic yards loose stone in riprap wall, at \$1 50	160,500	00
80,000 cubic yards loose stone in riprap wall, hauled from excavation, at	,	
75 cents	60,000	00
1,000 cubic yards slope and pavement wall, at \$2	2,000	
5,000 cubic yards vertical wall in cement, at \$5	25,000	
2,000 cubic yards vertical wall in content, at 60		
3,000 cubic yards vertical wall laid dry, at \$4	12,000	
Amount	1,358,450	
2 looks including look postions (the same or for the same) or the Torse side)	262 421	00
3 locks, including lock sections, (the same as for the canal on the Iowa side)	763, 471	
Channel and pier at Upper chain, (the same as for the canal on the Iowa side)	461,655	
53,333 cubic yards rock excavation at Nashville crossing, at \$10	533, 333	00
Contingencies and engineering	154, 521	00
Total	3, 271, 430	00
	-,	

Another plan of canal on the Illinois side has been considered by the board, viz: To commence at the Nauvoo steamboat landing, with an embankment in the river; thence continue the line of the canal along the low ground near the water's edge, and excavate the prism until a point below "Joe Smith's Mormon Hotel" is reached. From here continue the line by building an embankment in the river to the point near Edmunds's warehouse, where the canal just described begins, and the rest will be the same as the latter. This plan will require much more rock excavation, more embankment, higher lock-walls and gates, as four more feet lift will have to be added. This plan has no advantages not possessed by the other, and as it requires about four miles more of canal construction, would involve a great increase in cost. In fact, it has positive disadvantages, and will, therefore, not be further noticed.

IX.—Comparative advantages and disadvantages of the plan of a canal on the lows side, with that of an excavated side channel, and of the canal on the Illinois side.

The board having described the various plans proposed for improving the navigation of the Des Moines rapids, some of which have for reasons given been rejected, will now compare the plan they have recommended with that of the excavated side channel, and then with that of the canal on the Illinois side of the river, both in regard to advantages and cost.

The plan of the canal from Nashville to Keokuk as recommended by the board, in connection with an excavated channel through the Upper chain, and a low dam from Montrose island to the Illinois shore, would, it is believed, afford all the advantages to commerce and navigation which could be obtained at any reasonable cost, or by means of any improvement yet suggested, and would be subject to no disadvantages that would be obviated by any other plan:

First. It would overcome certainly and entirely all the difficulties of navigating the rapids. There is no doubt in regard to the practical result that will be accomplished by its execution.

Second. The canal will give five feet navigation in extreme low water, and its depth in all stages of the river will be sufficient to pass any boats that the bars in the river would permit to reach it; and if at any future time the river should be improved, the cost of increasing the depth to even six feet would not be nearly as great, with this plan, as with that of the excavated side channel.

Third. It is the shortest line from the head to the foot of the rapids. The river channel above the rapids runs close to the Iowa shore near Montrose, and the lower lock of the canal will be located in the deep water at the upper end of Keokuk, affording a good landing place for boats waiting to enter the lock.

Fourth. It would not in the slightest degree interfere with or obstruct the navigation of the river, either in high or low water, so that boats and rafts preferring to do so may always pass outside of the canal; the only necessity that will compel the use of the canal will be the want of sufficient depth of water in

the open river, or inability to stem the rapid current.

Fifth. The water in the canal being slack, boats may ascend almost as quickly as they descend; the current will no longer be an obstacle. At the present time it takes a loaded boat of considerable power five hours to ascend from Keokuk to Montrose in low water, and almost as much in high water. Allowing two hours for descending in low water, (they have to run very carefully,) will make seven hours for the round trip in low water. The same boats in the canal will make in descending, running time 1½ hours, detention at the locks one hour, making 2½ hours. The ascending time will be about 2½ hours. Total for the round trip 4½ hours. Different steamers will give somewhat different results, but it is believed actual experience will show this to be nearly correct. A towing steamer, with heavily loaded barges going up, would be proportionally longer on the rapids.

Sixth. It will give good navigation for all crafts on the river. It can be run with equal facility, up or down, in all kinds of weather, by night as well as by

day, and will be perfectly safe at all times.

Seventh. It is believed that the single set of locks will be sufficient for some years to accommodate the commerce of this river, but they can be so located that at any future time, in case it should become desirable or necessary, another set of locks can be placed beside the first. A double set of locks and guard-locks could, without difficulty, pass four boats per hour, either up or down, and double that number if half were going in one direction and half in the other.

Eighth. The canal will afford a capacious, convenient, and safe harbor for steamboats or naval vessels during the winter. This is merely an incidental advantage, but one that is entitled to some consideration on account of the pressing want of such a harbor for boats navigating the upper Mississippi.

Ninth. It would afford admirable sites for dry-docks, which could be con-

structed near the middle locks at comparatively little cost.

The only disadvantage of the canal, as compared with the shore channel, that the board are able to discover, is the annual cost of repairs and superintendence. There would be little else than the lock-gates to be kept in repair. It has been estimated that repairs and superintendence will cost \$15,000 per annum, which is less than one-third the interest at six per cent. on the difference of cost of the two plans. The embankment and lock-walls may be considered as permanent structures.

The side channel will to some extent interfere and obstruct the navigation of the river. The embankment being built up to Montrose island, and the channel of the river being between the island and the Iowa shore, it will be necessary for all rafts or boats, as soon as the water attains a stage low enough to prevent their passing the Upper chain, between the Illinois shore and Montrose island, to go through the side channel. In other words, the natural channel of the river is obstructed as soon as the Upper chain becomes an obstruction, and boats would be compelled to pass through the side channel, whether they desired to do so or not. If they should wish to land on the Illinois side, between the head and foot of the rapids, they could do it only by following the old route on the Illinois side, all the way—an impossibility with some steamers in low water. The side channel, being only 200 feet wide, will not afford as good navigation as the canal, which will for the most part be 300 feet wide.

It would not overcome all the difficulties of navigation, for one of those difficulties consists in a very rapid current which ascending boats have to stem. It

would still be the case with this improvement.

It will require a longer time for a boat to ascend in the side channel than in the canal, but less to descend. The difference for the round trip would be greatly in favor of the canal.

The side channel will not afford a safe, commodious, and capacious harbor for

boats in the winter, and will not afford advantageous sites for dry docks.

Without reference to the cost of the two plans, the board are of the opinion that the canal plan is preferable to the proposed side channel.

The estimated cost of the canal, 300 feet wide in embankment and

five feet deep, is

3, 317, 874

Difference in favor of the canal is

787, 874

From this it will be seen that that the canal plan has the additional advantage of economy. The plan of the proposed side channel cannot therefore be recommended by the board.

Comparison of the plan proposed by the board with the plan of a canal on the Illinois side, beginning at Edmunds's warehouse and ending at Hamilton, with the natural channel improved at the Upper chain, in the same manner in each case.

The board at their second meeting had laid before them by General Wilson several communications on the subject of the improvement of the rapids, two of which were written by Mr. G. Edmunds, jr., of Illinois; one addressed to "General Warren, and other topographical engineers," which had previously been received and considered, and bearing the date of April 15, 1867; the other addressed to the Secretary of War, dated Sonora, Illinois, May 31, 1867.

The purport of these communications is to show the benefits to be obtained by locating a canal improvement on the Illinois side of the river, instead of on the Iowa side, "having reference to cost of construction, advantages to navigation, and the general improvement of the country." In regard to the location

of the work on the Illinois side, Mr. Edmunds says:

"I have come to the conclusion that the work proposed by the government, to wit, the construction of a canal for the purpose of navigation, can be done on this side (meaning the Illinois side) of the river at very much less expense than on the other side, and the advantages to navigation would be the same, and hydraulic power would be created which will not be excelled in this country.

"The cost of construction will, in my opinion, be at least \$500,000 less on this side of the river than on the other. The construction of the work will not in the least interfere with the navigation of the present channel of the river, with keel or steamboat channel, during its construction, or after its completion.

* * * *

"The wall to be built in the river would be on the west side of French channel and east of the present steamboat channel, and there would be sufficient depth of water to enter the canal with, from the pool above, without any excavation of rock whatever.

"A temporary dam can be thrown from the shore to the outer wall of the canal, and the rock and earth transported by rail from the quarries and clay banks to the wall; and as the wall progresses down the river, would exclude the water from the canal, so that any unevenness of its bottom from rock or projecting points could be as easily removed as upon the shore.

"The dimension rock for the locks could be transported directly from the quarries, immediately upon the bank of the river, to the locks without handling,

while, if the canal is constructed on the Iowa side, the dimension rock must all be transported from this side, at largely more expense than required for the work on this side. The locks would cost no more on this side than on the other, the great difference being in the expense of transportation and the facilities of handling. There would probably be no necessity of excavation below the middle lock to enable boats drawing five feet water to pass.

"The length of the canal will be substantially the same on either side. between seven and a quarter and seven and a half miles. * * * To enter the canal at Nashville will require a large amount of rock excavation, while none will be required on this side. The construction of the canal on this side will not interfere in any degree with any other public work, while its construction on the other side will require considerable outlay in changing the Keokuk and

Mount Pleasant railroad.

"The canal on this side could as easily be five hundred as three hundred feet in width, and would necessarily furnish additional facilities for navigation, passing and repassing of boats, &c. It would furnish an inexhaustible supply of water for hydraulic purposes, which would not interfere with the usefulness of the canal for purposes of navigation. * * * The construction of this canal presents precisely this position. If made in Iowa, navigation is to be benefited. If in Illinois, navigation is to be equally benefited, and the great manufacturing interests of the country advanced. * * * Should it be found that the work can be constructed on this side at the same expense, and will be of equal advantage to navigation, the canal should be located here."

In his letter to the Hon. Secretary of War, Mr. Edmunds says: "There is now a company chartered in this State for the purpose of doing the work; that company will at any time undertake to do the work on this (Illinois) side of the river for \$250,000 less than it can be done in Iowa, and give ample security to the government for performance on their part, and to produce the desired depth

of water in the canal."

The board have given their close attention to the project of a canal on the Illinois side of the river, and, having carefully examined the maps and profiles of General Wilson's survey, have made an estimate of the probable cost of the work; (this estimate has already been given.) This canal and the one recommended by the board are assumed to have the same dimensions in every respect, and to give the same depth.

The arguments of Mr. Edmunds in favor of the location of the work on the

Illinois side will now be considered.

The board are unanimously of the opinion that Mr. Edmunds errs when he says the work can be done at less expense on the Illinois side than on the Iowa side. The estimated cost of the two works has already been given. A comparison of those estimates will show a difference of \$741,430 in favor of the Iowa side. He asserts that it will cost less by so many dollars, but he does not give sufficient data by which others may test the accuracy of his conclusions.

The board would respectfully call the attention of the Chief of Engineers to

their estimates for comparison in regard to cost.

Again he errs when he says the advantages to navigation would be the same in both cases.

The fact that the canal on the Illinois side necessitates the crossing of the river twice to pass the rapids is sufficient evidence of itself that it has not the same advantages as the canal on the Iowa side. But when it is considered that one crossing has to be made near Nashville, where, during the low water of 1864, as has been already stated, there was only little more than two feet depth, what then would be the use of five feet in the canal?

It should be remembered that it will become necessary to cut a channel in the river bed of at least the same width and depth of the canal, and in a direction almost perpendicular to the thread of the current. Such a channel would be not only dangerous of dark nights, but in stormy and foggy weather would be almost impassable.

The board are of the opinion that the advantages to navigation of the two plans are by no means the same in both cases, and that the one on the Iowa side

is preferable in all respects.

The board agree with Mr. Edmunds in regard to the fact that a great hydraulic power can be developed equal in both cases; that the Illinois side is preferable to the Iowa side for its development, the low ground in the vicinity of the lower lock affording ample room for the erection of mills for using this hydraulic power.

The board have no hesitancy in saying, if the object of this improvement is to develop a great water power, that the Illinois side is the one that should be selected for that purpose; but if this is the primary object for their consideration, or if it even enters as an important element in the solution of the problem,

the board have mistaken the object of their mission.

In the order convening the board the subject of the improvement of the navigation "has been committed to it." Nothing is said about the development of water power.

The questions before the board are of a national character, and the work proposed a national work; the development of local interests has, therefore, not

been considered.

Mr. Edmunds states that "the construction of this canal presents precisely this position: if made in Iowa, navigation only is to be benefited; if in Illinois, navigation is to be equally benefited, and the great manufacturing interests of the country materially advanced."

This statement does not present the case correctly.

1st. It does not follow that "if made in Iowa, navigation only is to be benefited." If the government should think proper to connect hydraulic power with the canal, it can be done on the Iowa side, although not so advantageously or cheaply to the private parties who might be interested, yet with the same general results to the government and the country, only that the works would be in Iowa instead of Illinois.

2d. Navigation is not to be equally benefited by the location of the canal on either side, for the reason that a better and more convenient navigation for the public will be obtained by its construction on the Iowa side, and because two unnecessary crossings of the river will thereby be saved to the craft navigating

the stream for all time.

He errs again when he says the construction of the work will not in the least interfere with the navigation of the present channel of the river. By referring to the map of the rapids submitted herewith it will be seen that the steamboat channel from Waggoner's warehouse to "Montebello crossing" runs close to the Illinois shore. If the canal should be made three hundred feet wide a part of the exterior embankment must be built in the channel itself, and, consequently, will make it narrower, and hence it will interfere with the present steamboat channel. But he says, in another part of his communication, as one of the advantages in favor of the Illinois side, that "the canal can as easily be five hundred feet wide as three hundred feet." If, then, the canal is made five hundred feet wide, it will throw the embankment beyond the present steamboat channel, taking it into the canal, thereby destroying entirely the navigation of the steamboat channel in low water. The fact is, a canal five hundred feet wide on the Iowa side would interfere less with the navigation of the river than one of the same width on the Illinois side, both during the process of construction and after completion.

The manner of building the embankment down stream, with a temporary dam at the head, so as to exclude the water after a certain distance has been attained,

as proposed by him, is equally practicable on the Iowa side.

The statement made in Mr. Edmunds's paper referring to the supposed quarry advantages on the Illinois side conveys an erroneous impression, for if it even be assumed that all the stone for the locks could only be procured from the quarries of Mr. Edmunds on that side, the stone would still have to be boated from the quarries to the locks on either side of the river.

The difference in cost of transporting stone from the said quarries on the Illinois side could only be such as would appertain to the necessity of obliquely crossing the river, instead of running only up or down the stream, which could

not be a material item.

It is known, however, that there is a quarry of stone of similar general character to that in Mr. Edmunds's quarry, on Price's creek, on the Iowa side, which may furnish good material for the locks.

He states that there would probably be no necessity for excavation below the middle lock. This will depend altogether on the location and lift of the locks.

The Illinois side has no advantage over the Iowa side in this respect.

The length of the canal on each side is very nearly the same, but the difference is in favor of the Iowa side, being a little more than one thousand feet shorter.

There will be some rock excavation necessary in order to enter the canal at Nashville, but it will be an insignificant matter in comparison with excavating a channel at "Nashville crossing," in order to reach the head of the canal on the Illinois side.

The construction of the work on the Iowa side, it is true, will necessitate the changing of the location of the Keokuk and Mount Pleasant railroad and the public road in several places, for short distances, making, in all, about three miles. This item, however, is included in the estimates.

The supply of water for hydraulic purposes, if that is an important consider-

ation, will be no greater on the Illinois side than on the Iowa side.

The proposition of Judge Edmunds, to build the work on the Illinois side for less than it can be done on the Iowa side, is no argument in its favor. If the work is to be let in that manner, viz., to the parties who will build it the cheapest, choosing their own location, there will doubtless be found those on the Iowa side who will offer quite as favorable terms to the government.

The items that make the difference in cost are, the greater length of the canal on the Illinois side by 1,056 feet; the extra interior embankment from Cheney's creek to the outlet lock; the embankment along part of the canal being in deep water, and the excavation of a channel in the river at Nashville crossing.

For the foregoing reasons, the board are unanimous in their opinion that the Iowa side is preferable to the Illinois side for the location of a canal designed for the improvement of the navigation of the Mississippi river at the Des Moines rapids, "having reference to expense of construction and advantages to navigation." How far either would have the advantage in regard to the improvement of the country has not been investigated by the board. The subject, as presented to them, was understood to be the improvement of the navigation of the river.

LIVERMORE'S IMPROVED CHUTE.

The plan and model of an improved chute for rivers, and as a substitute for canal locks, together with a model of the same, having been submitted to this board, accompanied with the able report and description of Alonzo Livermore, civil engineer, its inventor and patentee, we have examined with care. The plan and model were further explained by Mr. Roberts, one of the board, who had previously examined them in company with Mr. Livermore.

The primary object of this invention is to secure a slack-water and chute navigation by the expenditure of a greatly reduced quantity of water, compared with the quantity required to maintain a navigation through a plain, unob-

structed chute, by the introduction of cross bottom walls and side walls. with lips at intervals, so arranged as to divide the fall into small lifts, and create reaction and retardation of the whole current.

It is not deemed necessary in this place to enter into an elaborate description of the plan or to embody the calculations, views, and conclusions of the inventor. The plan appears to have merit, and may, perhaps, be made practically available elsewhere, especially on streams having a limited supply of water; but its application cannot be recommended for the improvement of the Des Moines

rapids.

If a plain, open chute, in preference to locks, should be deemed advisable at these rapids, it is believed that there is always water enough in this part of the Mississippi to maintain navigation in such a chute more than equal to the depth on the shoals above and below the rapids; and while the plan of Mr. Livermore might work well in practice on a comparatively small scale or in a narrow channel, it contains elements which have not yet been accurately determined for a very wide channel. It has not yet been tested on a large scale, although it appears to answer in the model.

BRUNOT'S IMPROVED FLOAT GATE FOR SLUICES IN DAMS, ETC.

A working model of this gate was exhibited to the board, and its mode of operation shown by Mr. Roberts, one of the board. It is designed especially to act as a movable dam, as a regular and convenient sluice-gate on a large scale. It consists of a hallow water-tight gate, of wood or iron, held by a hinge extending across the sluice-way or opening in a dam, with side-valve gates above and below the gate for admitting and discharging water from the upper level into a recess under the gate, which, when the upper valve is open and the lower valve closed, has the pressure due to the difference of level. The hollow gate having considerable flotative power, correspondent with its dimensions, remains up in an inclined position, forming a dam, over which the water may flow. is desired to lower the gate to its horizontal position, water is let into the gate itself by another valve, which immediately descends by its own weight, leaving a clear sluice-way for the water; for the sluice in a dam can be so arranged as to be in part self-acting, by means of an open vertical pipe on one end in the abutment, having its mouth level with the height of water at which it may be When the pool rises to that height the water desired to allow the gate to fall. flows down the pipe and fills the gate, which then falls to its horizontal position, when vessels, rafts, &c., may pass over it.

When the stream falls the gate is raised by being emptied, the water running into the lower level, and the emptying valve is then closed, when the gate immediately rises to its inclined position and again becomes a dam, over which

the water may flow without depressing it.

This gate works well in the model, and it may be worth a trial on a large

Now that the construction of very large locks for ships and steamboat canals is becoming more general, it is desirable that the best plans for the construction and working of large gates should be adopted, and it might be economical for the government to institute a thorough investigation of the merits of different methods of making and operating very large locks or sluice-gates.

It is not deemed strictly within the province of the board to offer any formal recommendation on this subject, but it seems proper thus briefly to bring it to

the notice of the department for consideration.

For locks eighty feet wide, having lifts of ten or twelve feet, the ordinary mitre gates would be about forty-three feet long by twenty-four to twenty-six feet high, forming a large structure, which should be strong enough to sustain the pressure of twelve feet head on a width of over forty feet. These may be built of either wood or iron-wooden gates being much cheaper in first cost. It is possible that hollow water-tight wooden or iron mitre gates may be advantageously substituted for the solid gate, and that the ordinary plan of filling

and emptying large locks may be advisable.

In case of the construction of large locks at the Des Moines rapids it might serve a useful purpose to authorize the engineer in charge to thoroughly investigate this matter by experiment, if necessary, on some sufficient scale, to obtain good practical results. The cost of this would be quite trifling and insignificant compared with the possible advantage which might accrue to the public.

As there is always a large surplus of water for canal purposes, it appears to be a favorable place for testing plans for rapidly filling and emptying large locks by the power of the water itself. It is possible that a safe, convenient, and reliable arrangement may be devised which would dispense with half of the men

needed to work large gates on the old plan.

The following is a tabular statement of the estimated cost of each plan of

improvement herein discussed:

1st. Improvement of the natural channel by excavation:	
200 feet wide and 4 feet deep	\$2,647,785
300 feet wide and 4 feet deep	3, 648, 362
200 feet wide and 5 feet deep	3, 504, 182
300 feet wide and 5 feet deep	5, 034, 350
2d. The plan of dams entirely across the river, locks and chutes	2, 460, 091
3d. Plan proposed by Mr. Edmunds, viz: a sluice navigation	•
entirely	1, 926, 720
4th. Plan of an excavated side channel on the Iowa side:	
200 feet wide and 4 feet deep	2, 752, 933
300 feet wide and 4 feet deep	3, 503, 910
200 feet wide and 5 feet deep	3, 317, 874
300 feet wide and 5 feet deep	4,661,910
	_

5th. Plan of canal on the Iowa side from Nashville to Keokuk, 300 feet wide in embankment, and 250 feet wide in excavation, and 5 feet deep, in connection with channel and pier at Upper chain, \$2,530,000.

6th. Plan of a canal on the Illinois side from Edmunds's warehouse to the Hamilton Ferry dike, 300 feet wide in embankment, and 250 feet wide in excavation, and 5 feet deep, in connection with channels and pier at Upper chain, and excavated channel at Nashville crossing, \$3,271,430.

A general plan and profile of the rapids, embracing a plan of the canal and locks proposed under the direction of Brevet Major General J. H. Wilson, are

submitted herewith to the Chief of Engineers.

The board having already submitted their conclusions, based upon the facts set forth herein, append hereto the same, for the purpose of completing this record as follows:

The board of engineers, convened by Engineer Orders Nos. 18 and 20, March 22, and March 26, 1867, having met at Keokuk, Iowa, on the 15th of April, and adjourned to this place on the 30th of April, beg leave to submit the following conclusions and recommendations in regard to the improvement of the

Des Moines rapids of the Mississppi river:

1st. After a careful examination and consideration of all the plans, profiles, and river soundings, and of the calculations and views of the engineer and his assistant, an inspection of the ground on both sides of the river from a point above Montrose and Nauvoo to a point opposite the mouth of the Des Moines river, in the town of Warsaw, and a study of all the plans proposed, it is the unanimous opinion of this board that the general plan of a canal and locks submitted to the engineer department by Brevet Major General J. H. Wilson, dated January 1, 1867, and approved by Brevet Major General A. A. Humphreys, Chief of Engineers United States army, in his report of February 5.

1867, to the Secretary of War, is the best for permanently improving the navigation of the Des Moines rapids of the Mississippi river, namely, a canal along the Iowa shore from a point near the present site of the Keokuk indicator, at the city of Keokuk, to a point just below the village of Nashville, seven and six-

tenths miles long.

2d. That, taking the gauge of extreme low water at or near Montrose as a guide, the canal bottom at the upper end of the proposed canal at Nashville and the level of the top of the mitre sills at the guard-lock should be fixed at five feet below such low water at the guard-lock, and that the lower mitre sill of the outlet lock at Keokuk be fixed at five feet below the low water of 1864 at that point, as referred to in the alternate suggested in General Wilson's report.

That the canal, and the middle and lower locks, be arranged for eight feet

depth of water when the river rises three feet above extreme low water.

That the size of the lock chambers shall remain as given in General Wilson's

report, namely, 350 feet between the quoins and 80 feet wide at top.

That the thickness of the lock walls as planned is abundant for any pressure that can come against them; and it is suggested that, in the final arrangement of the details, the engineer in charge should be authorized to make such reduction in thickness for economy as he may deem safe.

That the reverse gates proposed at the outlet lock are unnecessary, and may

be economically dispensed with.

3d. That the final width of the canal be established at not less than 300 feet throughout, both in excavation and embankment, but in excavation at the time of construction it may be reduced to 250 feet without material disadvantage.

That the top of the embankment be fixed at two feet above the flow of 1851, at ten feet width, with side slopes not less than one and one-fourth and not more than one and a half base to one vertical, as the judgment of the engi-

neer may determine.

That, if it be found economical in the progress of the work, the thickness of the ripraps may be reduced from the thickness suggested in the plan submitted by General Wilson, provided that it be not less than two feet, and that the ripraps or slope wall on the canal side of the embankment be raised only to the height of twelve feet above canal bottom, or four feet above the highest water in the canal, and that the residue of the inside slope and the top of the embankment, being above all floods, be sowed with grass or sodded, or covered with a thin coating of stone, as the engineer in charge may determine.

4th. In the report of General Wilson respecting the improvement of channel above Nashville, provision is made for the excavation of a considerable quantity

of rock under water.

It is recommended that the engineer in charge be authorized carefully to investigate this part of the stream with a view to an artificial increase of the depth of water above the guard-lock, and with a view, also, of dispensing with such portion of said rock excavation under water as may be found practicable, the final arrangement to be determined by him, subject to the approval of the engineer department.

5th. If the general plan of the canal, with modification as herein recommended, should meet the approval of the Secretary of War, it is respectfully suggested that all the details and matters not specifically fixed in this report may be left to be arranged by the engineer in charge upon surveys, reports,

&c., approved by the engineer department.

[Preliminary estimate omitted. For detailed estimate see p. 304.]

That the work necessary to carry into effect the plan herein recommended should be put under contract between Keckuk and Nashville without delay, and pushed vigorously to completion, so that the improvements of the rapids may be made available for commerce and navigation as soon as possible.

The important question submitted to the board has occupied their undivided attention for several weeks, during which numerous points of interest have been

discussed, and many views presented and considered.

Some time must elapse before a complete report can be drawn up embodying the views and opinions of the board upon the several plans which have been considered. Meanwhile, pressing business demands the immediate personal attention of the members of the board at their respective headquarters.

Under these circumstances, it is the opinion of the board that they will best subserve the public interest by adjourning, to meet again at this place on the call of the president of the board, for the purpose of adopting the final report of

proceedings.

During this interval it is understood that General Wilson, the member in charge of the works, will cause to be prepared a map on a small scale, for convenient reference, showing the ground, river bed, and such lines as will aid in exhibiting and explaining the plans of improvement which have been investigated, with the location of the canal on the Iowa side, the profiles representing the canal levels and locks, the excavation and the top of the embankment two feet above high water of 1851, also showing the line of low water of 1864 and the line of high water of April, 1867, being a copy on a reduced scale of the maps, charts, and profiles already prepared by General Wilson, showing the modifications indicated in the proceedings of the board.

J. N. MACOMB, Colonel Engineers, But. Col. U. S. A., President of the Board.

Colonel Engineers, Bet. Col. U. S. A., Prendent of the Board.

J. H. WILSON,

Lieutenant Colonel Thirty-fifth Inf., Bvt. Maj. Gen. U. S. A. G. K. WARREN,

Bvt. Maj. Gen. U. S. A., Major Engineers. W. MILNOR ROBERTS,

Supt. Engr. Ohio River Improvement, U. S. Civil Engineers.
PETER C. HAINS,
Captain Engineers, Bvt. Lt. Col. U. S. A., Recorder.

ABSTRACT OF PROPOSALS

FOR THE

IMPROVEMENT OF THE DES MOINES RAPIDS

OF THE

MISSISSIPPI RIVER.

23 w----Vol. ii

E 4.—Abstract of proposals received at the United States engineer's office at sippi river, at a letting

		Prop	osal No. 1.	Prop	oosal No. 2.	Propo	sal No. 3.
Items.	Quantitles	a J. J. Dull, Harris- burg, Pa.		Charles H. Sherrill, New York city.		Wm. Rankin & Co., Rochester, N. Y.	
		Price.	Amount.	Price.	Amount.	Price.	Amount.
	Cubic yds.						
Grubbing and clearing Bailing and draining		••••••	\$5, 000 5, 000		\$15, 000 00 30, 000 00		82, ≈10 46 (0)
Excavation of earth	315, 000	\$0 50	157, 500	\$0 43	135, 450 00	\$9 38	119,700
Excavation of rock	161,440	3 75	605, 400	2 23	360, 011 20	1 95	314. 4 -
Loose stone in riprap wall	88,000	1 20	105, 600	2 10	184, 800 00	1 55	136, 44
Embankment	651, 500	60	390, 900	47	306, 205 00	50	325, 73
Lining	10,000	60 22	6,000 11,000	58 27	5, 800 00 13, 500 00	65 33	6, 500
Puddling Slope-wall	50, 000 500	22	11,000	21	13, 300 00	2 00	18,50
Vertical wall in cement	1,000					5 00	5, (1)
Vertical wall laid dry						3 90	1,930
Concrete mesoner	900					6 25	1, 23
Changing line of railroad	Miles, 3	20,000	60,000	2,500	7, 500 00	7,800	23, 400
Changing line of railroad	Rods, 960	· • • • • • • • • • • • • • • • • • • •	3,000	8 00	7,680 00	4 00	3, 840
Total			1, 349, 400		1, 065, 946 20	!	1, 006, 88
		Propo	sal No. 10.	Proposal No. 11.		Propos	al No. 12
Items.	Quantities	trobe, Swan, Pa; P Ebens Geo. M	Fenlon, La- Pa; Robert Pittsburg, hilip Collins, burg, Pa.; lurray, Dav- t, Iowa.	Robert sburg, Collins, Pa.; Rochester, Pa. Collins, Da Iowa.		ll.; E. A.	
		Price.	Amount.	Price.	Amount.	Price.	Amount.
	0.12						
	Cubic yds.			l 1			
Grubbing and clearing	Cuoic yas.		\$2,750 40,000		\$10,000 00 60,000 00	\$25,000	\$2 , 500 37, 500
Grubbing and clearing Bailing and draining		8 0 R5	40, 000	\$0.50	60,000 00	per mile	37, 500
Grubbing and clearing Bailing and draining Excavation of earth Excavation of rock	315, 000	\$0 85 3 00	40, 000	\$0.50 4.00	60,000 00	\$25, 000 per mile \$0 48 2 30	37, 500 151, 241
Grubbing and clearing Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall	315, 000 161, 440 88, 000	3 00 1 25	40, 000 267, 750 484, 320 110, 000	4 00 1 50	60, 000 00 157, 500 00 645, 760 00 132, 000 00	per mile \$0 48 2 30 1 80	37, 500 151, 244 371, 312 158, 434
Grubbing and clearing Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall Embankment	315, 000 161, 440 88, 000 651, 000	3 00 1 25 60	40, 000 267, 750 484, 320 110, 000 390, 900	4 00 1 50 70	60, 000 00 157, 500 00 645, 760 00 132, 000 00 465, 050 00	\$0 48 2 30 1 80 50	37, 500 151, 240 371, 312 158, 440 325, 750
Grubbing and clearing Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall Embankment	315, 000 161, 440 88, 000 651, 000	3 00 1 25 60 50	40, 000 267, 750 484, 320 110, 000 390, 900 5, 000	4 00 1 50 70 1 50	60, 000 00 157, 500 00 645, 760 00 132, 000 00 465, 050 00 15, 000 00	90 48 2 30 1 80 50 55	37, 500 151, 200 371, 312 158, 400 325, 750 5, 500
Grubbing and clearing Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall Embankment	315, 000 161, 440 88, 000 651, 000 10, 000 50, 000	3 00 1 25 60 50 40	40,000 267,750 484,320 110,000 390,900 5,000 20,000	4 00 1 50 70 1 50 50	60, 000 00 157, 500 00 645, 760 00 132, 000 00 465, 050 00 15, 000 00 25, 000 00	90 48 2 30 1 80 50 55 20	37, 500 151, 240 371, 312 158, 440 325, 750 5, 500 10, 000
Grubbing and clearing Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall Embankment Lining Puddling Slope-wall	315, 000 161, 440 88, 000 651, 000 10, 000 50, 000	3 00 1 25 60 50 40 7 00	40,000 267,750 484,320 110,000 390,900 5,000 20,000 3,500	4 00 1 50 70 1 50 50 5 00	60, 000 00 157, 500 00 645, 760 00 132, 000 00 465, 050 00 15, 000 00 25, 000 00 2, 500 00	per mile \$0 48 2 30 1 80 50 55 20 2 25	37, 500 151, 240 371, 312 158, 440 325, 750 5, 500 10, 000 1, 125
Frubbing and clearing. Bailing and draining. Excavation of earth Excavation of rock. Loose stone in riprap wall. Embankment Lining. Puddling. Slope-wall Vertical wall in cement.	315, 000 161, 440 88, 000 651, 000 10, 000 50, 000	3 00 1 25 60 50 40	40,000 267,750 484,320 110,000 390,900 5,000 20,000 3,500 10,000	4 00 1 50 70 1 50 50	60, 000 00 157, 500 00 645, 760 00 132, 000 00 465, 050 00 15, 000 00 2, 500 00 7, 000 00	per mile \$0 48 2 30 1 80 50 55 20 2 25 8 00	37, 500 151, 200 371, 312 158, 450 325, 750 5, 500 10, 000 1, 125 8, 000
Grubbing and clearing. Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall. Embankment Lining Puddling Slope-wall Vertical wall in cement Vertical wall dry laid Concrete masonry	315, 000 161, 440 88, 000 651, 000 10, 000 500 1, 000 500 200	3 00 1 25 60 50 40 7 00 10 00	40,000 267,750 484,320 110,000 390,900 5,000 20,000 3,500	4 00 1 50 70 1 50 50 5 00 7 00	60, 000 00 157, 500 00 645, 760 00 132, 000 00 465, 050 00 15, 000 00 25, 000 00 2, 500 00	per mile \$0 48 2 30 1 80 50 55 20 2 25	37, 500 151, 200 371, 312 158, 400 325, 750 5, 500 10, 100 1, 125 8, 000 3, 000
Grubbing and clearing Bailing and draining Excavation of earth Excavation of rock. Loose stone in riprap wall. Embankment. Lining Puddling Slope-wall Vertical wall in cement Vertical wall in cement Concrete masonry Changing line of railroad Changing line of public road.	315, 000 161, 440 88, 000 10, 000 50, 000 50, 000 1, 000 500 200 Miles, 3	3 00 1 25 60 50 40 7 00 10 00 7 00	40,000 267,750 484,320 110,000 390,900 5,000 20,000 3,500 10,000 3,500	4 00 1 50 70 1 50 50 5 00 7 00 5 00	60, 000 00 157, 500 00 645, 760 00 132, 000 00 465, 050 00 15, 000 00 25, 000 00 2, 500 00 7, 000 00 2, 500 00	per mile \$0 48 2 30 1 80 50 55 20 2 25 8 00 6 00	37, 500 151, 244 371, 312 158, 434

a Guarantee not in duplicate. b For earth hauled more than 700 feet, one-half of one cent per yard for each 100 feet in excess. For all rock material hauled more than 700 feet, three-quarters of one cent per cubic yard for each 100 feet \approx delivered in excess. To work at all stages of water not exceeding four feet above low-water mark of 1864, or at any stage when the work is all out of water. c Not in duplicate.

Davenport, Iowa, for the improvement of the Des Moines rapids of the Missisheld September 4, 1867.

	Mal No. 4.	Propo	eal No. 5.	Propos	al No. 6.	Propo	osal No. 7.	Propo	sal No. 9.
	G. Case & alton, N. Y.		in Smith & ordova, Ill.		Freeland, E. ith, Millers- 'a.	c John S. Carper wa, W	Wolf, S. D. ater, Ottum- is.		olan & Co., ikee, Wis.
Price.	Amount	Price.	Amount.	Price.	Amount	Price.	Amount.	Price.	Amount.
	* 2000		A4 P00		00 500	Acres,			41 700
•••••	\$3, 000 10, 000	\$32, 500 per mile	\$4, 500 48, 750		\$2, 500 45, 000	50	\$50,000	\$25,000 per mile	\$1,700 37,250
\$0 60	189, 000	\$0 48	151, 200	\$ 0 50	157, 500	\$0 45	141,750	\$0 35	110, 250
2 50	403, 600	2 50	403 600	2 20	355, 168	3 90	629, 616	1 75	282 520
2 00	176, 000	1 50	132,000	2 20	193, 600	3 50	308,000	1 45	127, 600
35	228, 025	65	423, 475	65	423, 475	84	547, 260	55	358 325
1 00	10, 000	80	8,000	90	9, 000	90	9,000	21	2, 100
25	12, 500	32	. 16,000	1 50	75, 000	1 75	87, 500	[- <i></i>]	
3 00	1, 500	5 00	2, 500	5 00	2, 500	4 50	2, 250	1 69	845
6 50	6, 500	12 00	12,000	9 00	9,000	12 50	12,500	4 87	4,870
5 00	2,500	8 00	4,000	8 00	4, 000	8 00	4,000	3 25	1,625
6 00 3,000	1, 200 9, 000	12 00 1,000	2, 400 3, 000	6 00 8,000	1, 200	10 00 800 00	2,000	5 00	1,000 3,600
4 00	3, 840	4 00	3, 840	5 00	24, 000 4, 800	1 00	2,400 960	1,200 2 00	2, 700
	1, 056, 665	 	1, 215, 265		1, 306, 743		1, 797, 236		934, 385
Propo	eal No. 13.	Propo	sal No. 14.	Propo	sal No. 15.	Propo	osal No. 16, 		Reynolds,
J. V. C Lanma Karna,	M. Lanman, riswell, E. J. an, William Wm. Behm,		ell & Clin- bany, N. Y.		der Graham, water, Wis.		. Parkins & adison, Wis.	Thos. L. E. Thoms	Saulpaugh, Saulpaugh, as J. Buford,
J. V. C Lanma	riswell, E. J. m. William Wm. Behm,							Thos. L. E. Thoms	Saulpaugh, Saulpaugh,
J. V. C Lanma Karna,	riswell, E. J. m. William Wm. Behm,							Thos. L. E. Thoms	Saulpaugh, Saulpaugh, as J. Buford,
J. V. C Lanma Karna, Readin	riswell, E. J. an, William Wm. Behm, ag, Pa.	ton, All	bany, N. Y.	Whitev	Amount.	Price.	Amount.	Thos. L. E. Thoms Rock I	Saulpaugh, Saulpaugh, as J. Buford, sland, Ill. Amount.
J. V. C Lanma Karna, Readin	riswell, E. J. un, William Wm. Behm, ng, Pa. Amount.	Price.	Amount.	Whitev	water, Wis.	Price. Pr. mile, \$400	Amount.	Thos. L. E. Thoms Rock I	Saulpaugh, Saulpaugh, Saulpaugh, sa J. Buford, sland, III. Amount.
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J. V. C Lanma Karna, Readin Price.	riswell, E. J. m., William Wm. Behm, ng, Pa. Amount. \$3,500 22,000 185,850	Price.	Amount. \$500 37, 500	Price.	Amount, \$35,000 50,000 220,500	Price. Pr. mile, \$400	Amount.	Thos. L. E. Thoms Rock I	Saulpaugh, Saulpaugh, J. Buford, aland, Ill. Amount. \$2,000 37,500 141,750
J. V. C Lanma Karna, Readin Price.	riswell, E. J. an, William Wm. Behm, g, Pa. Amount. \$3,500 22,000 185,850 532,752	Price. \$25,000 permile. \$0 70	Amount. \$500 37, 500 220, 500 807, 200	Price.	Amount. \$35,000 50,000 220,500 298,664	Price. Pr. mile, \$400 \$33,000 per mile. \$0 60 1 90	Amount.	Thos. L. E. Thoms Rock I Price. \$25,000 per mile. \$0 45 2 25	Saulpaugh, Saulpaugh, J. Buford, aland, Ill. Amount. \$2,000 37,500 141,750
J. V. C Lanma Karna, Readin Price. \$0 59 3 30 2 00 25	riswell, E. J. an, William Wm. Behm, g, Pa. Amount. \$3, 500 22, 000 185, 850 532, 752 176, 000 102, 875	\$25,000 \$25,000 5 00 5 00 98	\$500 37, 500 220, 500 807, 200 440, 000 638, 470	Price.	Amount. \$35,000 50,000 220,506 176,000	Price. Pr. mile, \$400 \$33,000 per mile. \$0 60	Amount.	Thos. L. E. Thoms Rock I Price. \$25,000 per mile. \$0 45	Saulpaugh, Saulpaugh, J. Buford, aland, Ill. Amount. \$2,000 37,500 141,750
J. V. C Lanma Karna, Readin Price. \$0 59 3 30 2 00 25 62	riswell, E. J. an, William Wm. Behm, ag, Pa. Amount. \$3,500 22,000 185,850 532,752 176,000 162,875 6,200	\$25,000 permile. \$0 70 5 00 5 00 98 1 25	\$500 37, 500 220, 500 807, 200 440, 000 638, 470 12, 500	Price. \$0 70 1 85 2 00 85 90	Amount. \$35,000 50,000 220,500 298,664 176,000 553,775 9,000	Price. Pr. mile, \$400 \$33,000 per mile. \$0 60 1 90 1 85 52 1 00	Amount. \$3,000 49,500 189,600 306,736 162,800 338,780 10,000	Thos. L. E. Thoms Rock 1 Price. \$25,000 per mile. \$0 45 2 25 1 00 48 60	Baulpaugh, Saulpaugh, Saulpaugh, J. Buford, aland, Ill. Amount. \$2,000 37,500 141,750 363,240 88,000 312,730 6,000
J. V. C Lanma Karna, Readin Price. \$0 59 3 30 2 00 25 62 35	riswell, E. J. an, William Wm. Behm, Wg, Pa. Amount. \$3,500 22,000 185,850 532,752 176,000 162,875 6,200 17,500	\$25,000 permile. \$0 70 5 00 98 1 25 60	\$500 37, 500 220, 500 807, 200 400, 000 638, 470 12, 500 30, 000	Price. \$0 70 1 85 9 00 85 9 00 30	Amount. \$35,000 50,000 220,500 298,664 176,000 553,775 9,000	Price. Pr. mile, \$400 \$33,000 per mile. \$0 60 1 95 52 1 00 1 55	Amount. \$3,000 49,500 169,000 306,736 162,800 338,780 10,000 75,000	Thos. L. E. Thoms Rock 1 Price. \$25,000 permile. \$0 45 2 25 1 00 48 60 20	Saulpaugh, Saulpaugh,
J. V. C Lanma Karna, Readin Price. \$0 59 3 30 2 00 25 625 4 00	riswell, E. J. an, William Wm. Behm, g, Pa. Amount. \$3,500 22,000 185,850 532,752 176,000 162,875 6,200 17,500 2,000	\$25,000 permile. \$0.00 1 98 1 25 60 4 00 4 00	\$500 37, 500 220, 500 807, 200 440, 000 12, 500 30, 000 2, 000	#0 70 1 85 90 90 300 2 50	**Amount.** \$35,000 50,000 220,500 220,664 176,000 53,775 9,000 15,000	Price. Pr. mile, \$400 \$33,000 per mile. \$0 60 1 90 1 85 52 1 00 1 50 1 50 1 90	Amount. \$3,000 49,500 189,000 306,736 162,800 338,780 10,000 75,000	Thos. L. E. Thoms Rock 1 Price. \$25,000 per mile. \$0 45 2 25 1 00 48 60 20 1 50	8aulpaugh, Saulpaugh,
J. V. C Lanma Karna, Readin Price. \$0 59 3 30 2 00 25 62 35 4 00 9 00	riswell, E. J. an, William Wm. Behm, Wg, Pa. Amount. \$3,500 22,000 185,850 532,752 176,000 162,875 6,200 17,500 2,000 9,000	\$25,000 \$27,000 \$25,000 \$0,70 \$0,70 \$0,98 \$1,25 \$0,40 \$0,600	\$500 37, 500 220, 500 807, 200 400, 000 638, 470 12, 500 30, 000 2, 000 6, 000	#0 70 1 85 90 30 2 50 6 00	**************************************	Price. Pr. mile, \$400 \$33,000 permile, \$0 60 1 50 1 50 1 90 7 70	Amount. \$3,000 49,500 189,600 306,736 162,800 338,780 10,000 75,000 950 7,000	Thos. L. E. Thoms Rock 1 Price. \$25,000 permile. \$0 45 2 25 1 00 48 60 20 1 50 6 00	Saulpaugh, Saulpaugh,
J. V. C Lanma Karna, Readin Price. \$0 59 3 30 2 20 25 62 4 00 9 00 6 00	riswell, E. J. an, William Wm. Behm, g, Pa. Amount. \$3, 500 22, 000 185, 850 532, 752 176, 000 102, 875 6, 200 17, 500 2, 000 9, 000 9, 000 3, 000	Price. \$25,000 permile. \$0 70 5 00 98 1 25 60 4 00 4 00	Amount. \$500 37, 500 807, 200 440, 000 30, 000 32, 000 6, 000 2, 000	\$0 70 1 85 2 00 2 50 6 00 5 00	**Amount.** **35,000 50,000 220,500 229,664 176,000 553,775 9,000 15,000 01,250 6,000 2,500	Price. Pr. mile, \$400 (\$33,000 per mile. \$0 60 1 90 1 85 52 1 00 1 500 1 500 1 500 5 50 1 500 7 00 5 50	\$3,000 49,500 189,000 306,736 162,800 75,000 7,000 7,000 2,750	Thos. L. E. Thoms Rock 1 Price. \$25,000 per mile. \$0 45 2 25 1 00 48 60 20 0 1 50 6 00 5 00 5 00	Saulpaugh, Saulpaugh,
J. V. C Lanma Karna, Readin Price. \$0 59 3 30 2 00 25 62 4 00 9 00 6 00 10 00	riswell, E. J. an, William Wm. Behm, wg, Pa. Amount. \$3,500 22,000 185,850 532,752 176,000 102,875 6,200 17,500 2,000 3,000 3,000 3,000 2,000	\$25,000 permile. \$0.70 5.500 9.860 4.00 4.00 7.00 7.00 9.00 9.00 9.00 9.00 9.00 9	\$500 37, 500 220, 500 807, 200 440, 000 638, 470 30, 000 2, 000 6, 000 2, 000 1, 400	Price. \$0 70 1 85 90 30 2 50 6 00 5 00 10 00	**Amount.** \$35, 000 50, 000 220, 500 229, 664 176, 000 553, 775 9, 000 12, 500 2, 500 2, 500 2, 500	Price. Pr. mile, \$400 (\$3,000 per mile. \$0 60 1 90 1 85 52 1 00 1 50 1 90 1 90 1 90 1 90 1 90 1 90	Amount. \$3,000 49,500 189,600 306,736 162,800 338,780 10,000 75,000 2,750 2,400	Thos. L. E., Thoms Rock 1 Price. \$25,000 per mile. \$0 45 2 23 1 50 6 00 5 00 5 00	Saulpaugh, Saulpaugh,
J. V. C Lanma Karna, Readin Price. \$0.59 3.30 2.00 25.62 35.4 9.00 6.00 10.00 1,200	riswell, E. J. an, William Wm. Behm, g, Pa. Amount. \$3, 500 22, 000 185, 850 532, 752 176, 000 102, 875 6, 200 17, 500 2, 000 9, 000 9, 000 3, 000	Price. \$25,000 permile. \$0 70 5 00 98 1 25 60 4 00 4 00	Amount. \$500 37, 500 807, 200 440, 000 30, 000 32, 000 6, 000 2, 000	\$0 70 1 85 2 00 2 50 6 00 5 00	**Amount.** **35,000 50,000 220,500 229,664 176,000 553,775 9,000 15,000 01,250 6,000 2,500	Price. Pr. mile, \$400 (\$33,000 per mile. \$0 60 1 90 1 85 52 1 00 1 500 1 500 1 500 5 50 1 500 7 00 5 50	\$3,000 49,500 189,000 306,736 162,800 75,000 7,000 7,000 2,750	Price. \$25,000 permile. \$245,000 permile. \$045 \$245 \$25,000 permile. \$045 \$25,000 permile. \$045 \$25,000 permile. \$150 600 200 1,500	Saulpaugh, Saulpaugh,

d If borrowed material is to be furnished, \$10,000 in addition.

The above bid for embankment is with the understanding that land for borrow-pits will be furnished within a reasonable distance, or will furnish land at an addition of ten cents per cubic yard for the embankment.

f Ten per cent, will be added to the embankment if I furnish the materials. (Not in duplicate.) g Will furnish the borrowed material for an addition of two cents per cubic yard.

Abstract of proposals for the improvement of the Des Moines rapids of

1						
		Pro	posal No. 18.	Propo	William Budd, W. II. Decker, J. M. Ault. S. K. Concanon, Thos. M. Hacket, Adolph Knipper, St. Louis, Missouri.	
Items.	Quantities.		Henegen & S. Vernon, Ohio.	on, 8. K. Co M. Hac Knipper,		
		Price	Amount	Price.	Amount.	
	Cubic yards.					
rubbing and clearing				00	\$1,50	
ailing and draining			2, 5	i00	43,00	
xcavation of earth	315, 000	\$0	22 102 0	50 \$0 65	204.7	
Excavation of rock	164, 440	1		44 2 25	363, 24	
cose stone in riprap wall	88,000	î	35 118,8	2 25	198, 0	
mbankment	651,000	· -	37 241,0	055 60	390 9	
dningdning	10,000		40 4,0	000 1 50	15, 00	
uddling	50,000		20 10,0		25,0	
lope-wall	500			000 8 00 250 10 00	10,0	
Vertical wall in cement	1,000 500			250 600	3.0	
Concrete masonry	200			050 8 00	1.0	
hanging line of railroad	Miles 3	500	00 1,	500	8.5	
Changing line of public road	Rods, 960	2	25 2,		4,00	
Total			711,	659	1, 272, 49	
				Propos	al No. 24.	
Items.			Quantities.	Wm. W. W		
Items.			Quantities.	Wm. W. W	right, Genev	
			Quantities. Cubic yards.	Wm. W. W	right, Genev York. A mount	
3rubbing and clearing			•	Wm. W. W	right, Genev York. Amount.	
3rubbing and clearing		• • • • • • • • • • • • • • • • • • •	•	Wm. W. W New Price.	Amount.	
Frubbing and clearing		• • • • • • • • • • • • • • • • • • •	Cubic yards. 315, 000 161, 440	Wm. W. W New Price. \$0 62 3 32	right, Genev York. A mount. \$3.00 90.00 195.35 535.50	
Prubbing and clearing Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall			Cubic yards. 315,000 161,440 88,000	Wm. W. W New Price. \$0 62 3 32 2 00	### Amount. ### \$3.00 90.00 195.36 535.00 176.00	
Brubbing and clearing			Cubic yards. 315, 000 161, 440 88, 000 651, 500	Wm. W. W New Price. \$0 62 3 32 2 00 22	right, Genev York. A mount. \$3.00 90.10 195.3 535.9 176.11	
Frubbing and clearing Bailing and draining Exervation of earth Exervation of rock Loose stone in riprap wall Embankment	•••••••		Cubic yards. 315,000 161,440 88,000 651,500	Wm. W. W New Price. \$0 62 3 32 2 00 22 60	right, Genev York. Amount. \$3,00 90,00 195,33 535,94 176,18 143,3	
Frubbing and clearing sailing and draining Szcavation of earth Excavation of rock Loose stone in riprap wall Smbankment Lining Puddling			Cubic yards. 315,000 161,440 88,000 651,500 10,000 50,000	Wm. W. W New Price. \$0 62 3 32 2 00 22	### Amount. \$3.00 \$0.00 195.33 535.00 175.30 17.50	
Grubbing and clearing Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall Embankment Lining Paddling Slope-wall			Cubic yards. 315, 000 161, 440 88, 000 651, 500 10, 000 50, 000	Wm. W. W New Price. \$0 62 3 32 2 00 22 60 335	### A mount. ##	
Brubbing and clearing Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall Embankment Lining Proddling Slope wall Vertical wall in cement			Cubic yards. 315, 000 161, 440 88, 000 651, 500 10, 000 50, 000 5, 000 1, 000	Wm. W. W New Price. \$0 62 3 32 2 00 22 60 35 4 50 9 00 7 00	right, Genev York. A mount. \$3.00 90.00 195.33 535.90 176.03 163.05 2.20 9.00 3.30	
Grubbing and clearing Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall Embankment Lining Puddling Bloose wall Vertical wall in cement Concrete masonry			Cubic yards. 315, 000 161, 440 88, 000 651, 500 10, 000 50, 000 1, 000 500 2, 000 900	Wm. W. W New Price. \$0 62 3 32 2 00 22 60 35 4 50 9 00 7 00 10 50	right, Genev York. A mount. \$3.00 99.00 195.38 143.3 6.0 17.5 2.2 9.00 3.3 2.1	
Grubbing and clearing Bailing and draining Excavation of earth Excavation of rock Loose stone in riprap wall Embankment Lining Puddling Slope-wall Vertical wall in cement Vertical wall in cement Concrete masonry Changing line of railroad Changing line of railroad			Cubic yards. 315, 000 161, 440 88, 000 651, 500 10, 000 50, 000 5, 000 1, 000	Wm. W. W New Price. \$0 62 3 32 2 00 22 60 35 4 50 9 00 7 00	right, Genev York.	

I certify that the foregoing is a true abstract of the original bids received by me for this work.

United States Engineer's Office, Davenport, Iowa, September 10, 1867.

the Mississippi river, at a letting held September 4, 1867-Continued.

Proposal No. 20.		Proposa	l No. 21.	Propo	Proposal No. 22.		al No. 23.
Wm. Armst Philadel	trong & Co., phia, Pa.		ott, Farming- (owa.		ward, Chicago, inois.		corse & Co., ago, Ill.
Price.	Amount,	Price.	Amount	Price.	Amount	Price.	Amount.
Per rod.						Per mile.	
\$4 00 25, 000	\$ 9, 60 0 37, 50 0	\$30,000	\$3, 000 45, 000		\$1,500 39,000	\$400 19, 200	\$3, 000 28, 800
per mile.	110.050	per mile.	141 850	\$0 47	140.000	per mile.	004 750
\$0 35 1 95	110, 250 314, 808	\$0 45 2 95	141, 750 476, 248	2 55	148, 050 411, 672	\$0.65 2.75	204, 750 435, 888
1 49	131, 120	1 75	154, 000	1 50	132,000	1 45	127, 600
45	293, 175	55	358, 325	58	377, 870	40 1	260, 000
65	6, 500	75	7, 500	58	5, 800	95	9, 500
20	10, 000	25	12, 500	25	12, 500	24	12,000
2 25 1 25	1, 125 1, 250	3 00	1, 500 10, 000	4 50 7 00	2, 250 7, 000	2 90 6 90	1, 450
1 00	500	6 00	3,000	6 75	3, 375	5 90	6, 900 2, 950
2 50	500	12 00	2, 400	7 50	1,500	7 90	1, 580
3,000	9,000	4,000	12, 000	1,700	5, 100	Rod, 20 00	19, 200
4 00	3, 840	3 15	3, 024	3 25	3, 120	3 00	2, 880
	929, 168		1, 230, 247		1, 150, 737		1, 117, 098
Proposi	al No. 25.	Proposal No. 26.		Proposal No. 27.		Proposal No. 28.	
				l .		ł	
	Wm. Irvin, ort, Ill.	John Ross, Miss		Peter D. Nev	Tobio, Utica, v York.		e & Co., Chi- Illinois.
				Peter D. Nev	Tobio, Utica, v York.		
Price.	Amount.	Migs	Amount.	Nev	Amount.	cago,	Amount.
Preep	Amount	Migs	Amount. \$1,000 28,000	Nev	Amount.	Price.	Amount.
Price,	Amount,	Price.	Amount. \$1,000 26,000 179,550	Price.	Amount. \$100 30,000 78,750	Price.	Amount. \$1,000 00 33,666 00 166,950 00
Price,	Amount	Price.	\$1,000 28,000 179,550 548,896	Price.	*York. Amount. \$100 30,000 78,750 322,880	Price.	Amount. \$1,000 00 33,666 00 166,950 00 406,828 80
Price.	Amount. \$3,500 45,000 141,750 314,808 132,000	Price.	\$1,000 28,000 179,550 548,896 132,000	Price.	*York. Amount. \$100 30,000 78,750 322,880 352,000	Price. \$0 53 2 52 1 40	\$1,000 00 33,666 00 166,950 00 406,828 80 123,200 00
Price.	Amount. \$3,500 45,000 141,750 314,808 132,000 325,750	Price.	Amount. \$1,000 26,000 179,550 548,896 132,000 436,505	Price. \$0.25 2.00 4.00	*York. Amount. \$100 30,000 78,750 322,880 332,000 162,875	Price. \$0 53 2 52 1 40 70	#1,000 00 33,666 00 166,950 00 406,828 80 123,200 00 456,000 00
Price.	Amount. \$3,500 45,000 141,750 314,808 132,000	Price. 90 57 3 40 1 50 67 1 00 26	\$1,000 28,000 179,550 548,896 132,000 436,505 10,000	Price.	*York. Amount. \$100 30,000 78,750 322,880 352,000	Price. \$0 53 2 52 1 40 70 75 25	\$1,000 00 33,666 00 166,950 00 406,828 80 123,200 00 456,050 00 7,500 00
Price, \$0.45 1.95 1.50 60	Amount	Price. \$0.57 3.40 1.50 67 1.00 26 4.00	Amount. \$1,000 26,000 179,550 548,896 132,000 436,505 10,000 13,000 2,000	Price. \$0 25 \$2 00 4 00 25 40 10 00 6 00	*York. Amount. \$100 30,000 78,730 322,880 332,000 162,875 4,000 5,000 3,000	Price. \$0.53 2.52 1.40 70 75 25 4.00	\$1,000 00 33,668 00 166,950 00 426,828 80 123,200 00 456,050 00 7,500 00 12,500 00 2,000 00
Price, \$0 45 1 95 1 50 60	Amount	Price. \$0.57 3.40 1.50 67 1.00 26 4.00 6.00	\$1,000 26,000 179,550 548,896 132,000 436,505 10,000 2,000 6,000	Price. \$0 25 2 00 4 00 25 40 10 00 6 00 9 50	*York. Amount. \$100 30,000 78,750 322,880 352,000 162,875 4,000 5,000 3,000 9,500	Price. \$0 53 2 59 1 40 70 75 25 4 00 6 50	\$1,000 00 33,666 00 166,950 00 406,828 30 123,200 00 456,050 00 7,500 00 2,000 00 6,500 00
Price, \$0 45 1 95 1 50 60	Amount	Price. \$0.57 3.40 1.50 67 1.00 26 4.00 6.00 5.00	Amount. \$1,000 28,000 179,550 548,896 132,000 436,505 10,000 13,000 6,000 2,500	Price. \$0.25 2.00 4.00 9.50 6.00 9.50 6.00	*York. Amount. \$100 30,000 78,750 322,880 322,000 5,000 5,000 9,500 9,500 3,000 9,500	Price. \$0.53 2.52 1.40 75 4.00 6.50 6.00	\$1,000 00 33,666 00 166,950 00 406,828 80 122,200 00 7,500 00 12,500 00 2,000 00 6,500 00 3,000 00
Price. Price. \$0.45 1.95 1.50 50 60 30	Amount.	Price. 90 57 3 40 1 50 67 1 00 96 4 00 5 00 6 00 6 00	\$1,000 22,000 179,550 548,896 132,000 436,505 10,000 13,000 2,500 2,500 1,200	Price. \$0 25 2 00 4 00 25 40 10 00 6 00 9 50 6 00 10 00	*York. Amount. \$100 30,000 78,750 322,880 352,000 162,875 4,000 5,000 9,500 3,000 9,500 3,000 2,000	Price. \$0 53 2 52 1 40 70 75 25 4 00 6 50 6 00 7 00	\$1,000 00 33,666 00 166,950 00 406,828 80 123,200 00 456,050 00 2,000 00 6,500 00 3,000 00 1,400 00
Price, \$0 45 1 95 1 50 60	Amount	Price. 90 57 3 40 1 50 67 1 00 96 4 00 5 00 6 00 6 00	Amount. \$1,000 28,000 179,550 548,896 132,000 436,505 10,000 13,000 6,000 2,500	Price. \$0.25 2.00 4.00 9.50 6.00 9.50 6.00	*York. Amount. \$100 30,000 78,750 322,880 322,000 5,000 5,000 9,500 9,500 3,000 9,500	Price. \$0.53 2.52 1.40 75 4.00 6.50 6.00	\$1,000 00 33,666 00 166,950 00 456,650 00 7,500 00 12,500 00 6,500 00 3,000 00

a The above bid is for the whole work.

J. H. WILSON, Lient. Colonel 35th Infantry, Brevet Major General U. S. A.

E 5.

ANNUAL REPORT OF COLONEL P. C. HAINS ON THE IMPROVEMENT OF THE ROCK ISLAND RAPIDS OF THE MISSISSIPPI RIVER.

> DAVENPORT, IOWA, September 10, 1867.

GENERAL: In obedience to your verbal orders, I have the honor to submit the following report of operations connected with the improvement of the navi-

gation of the Mississippi river at Rock Island rapids:

It will be remembered that, during the last season, when the survey was made for determining the best place of improvement at these rapids, the limited time at our disposal necessarily prevented our making as thorough an examination of the bed of the river as was desired. I have thus far this season endeavored to leave nothing wanting in the way of useful information touching the plan of improvement that was adopted. For this purpose it became necessary to add many more soundings to the number taken last year, and accordingly a party has been organized and is now engaged on this work under my direction.

In the early part of May tide-gauges were established at various points along the rapids and connected with our levels, for the purpose of ascertaining accurately the relation between the depths of water on the different chains, and the slope of the water surface from the head to the foot of the rapids for all stages of the river. Observations are made twice a day at each gauge, viz: at 6 a. m. and 6 p. m., and the result recorded. The records up to the present time include periods when the river was both rising and falling. These records have not yet been plotted, however.

Though the river here has not been excessively high, or, in other words, though there has not been what is properly termed a flood, the average height for the season has been thus far much greater that usual. During last month (August) the river was much higher than it has been known to be at that season for many

years.

I enclose herewith a tracing of the curves, showing the oscillations of the river at the Chicago and Rock Island railroad bridge, near the foot of the rapids. The curves run through the whole period of the last seven years, with the exception of a small portion of 1864, during the time the bridge was destroyed. They are plotted by taking one co-ordinate for time, and the other for the stage of the river above an assumed zero. This was established at what was supposed to be the lowest known stage, viz: that of September 2, 1864. This is also the zero of the gauge of the bridge. It will be seen, however, that in November of that year the river fell two inches below that point. The curves are started at a common origin and made of different colors, so that they can all be seen at

one time and comparisons easily made.

Some very instructing features connected with the rise and fall of the water in the river may be noticed by examining this diagram, which will serve as a tolerably good guide to predict what may be the probable stage of the water at certain periods with something like accuracy, thereby affording the means of judging what would be the probable length of time that work on the improvement of the navigation could be prosecuted. For instance, it will be seen that during the months of February and March the river is almost invariably subject to great and sudden rises, sometimes as much as five or six feet in twenty-four hours. They do not, however, last long, being generally caused by what are termed "ice gorges," which occur nearly every season on the breaking up of the ice in the river and piling up below the rapids like a dam, causing for the time a partial stoppage of the flow of water. These "gorges" sometimes occur in the winter, when the river becomes filled with floating ice and is retarded or driven on either shore by a strong adverse wind. Every year, except 1861, it

will be observed there was a rise, more or less great, that occurred about the first of May. In 1861 this rise came later than usual, but it will be seen that it always comes. This is what is commonly called the "June rise," due to the thawing of the ice and snow in the region about the sources of the river. At this point it would doubtless be more properly termed the "May rise." In 1864 this was not very great; but it will be remembered this was one of the most remarkably dry seasons known in many years.

The small variations as shown by these curves, either increasing or decreasing the height, are generally due to the effects of wind and rain. The river here is also somewhat affected by the stage of Rock river, which empties into the Mississippi about four miles below. Variations due to wind or rain are not generally of long duration or very great extent, seldom exceeding a few inches.

The low stages commence generally about the 1st of July; the river then falls gradually somewhat lower and continues low until about the middle of

December.

With this information before us, it is no very difficult matter to estimate with tolerable accuracy the length of time that can be generally relied on for work

on the improvement of the navigation.

The present season has been one of unusual high water, and has greatly retarded operations on the work, the short working season that now remains rendering the use of coffer-dams for excavating the rock a hazardous undertaking. Notwithstanding this, timbers have been framed and every preparation made to commence a coffer-dam on "Duck creek chain" at once. This will be used on the next reef below the uppermost one of this chain. At the latter two chisels have been put in operation. They are like those formerly used by Major Floyd with apparent success on the Lower rapids, but it is found that the rock at this place is extremely hard, and very slow progress is made. A fair opportunity to test their ability has not yet been afforded, however. They have been at work only two or three days, and during this time much delay has been caused by breaking and readjusting parts of the machinery, which were made too light for the enormously heavy chisels, weighing nearly 8,000 pounds. The first part of the work is necessarily more difficult owing to the trouble of getting a proper face upon the ledge required to be removed.

The channel at "Duck creek" is one of the most tortuous and difficult of navigation of any on these rapids. The improvement that is to be made this season will give a good straight channel throughout almost the entire length of the "chain." As soon as the work at "Duck creek" is accomplished, I would propose that our efforts be next directed to Sycamore and Campbell's chains.

About 8,000 cubic yards should be taken out from Sycamore and Crab island, about 1,500 yards from Campbell's chain, and the balance of the present appropriation at Smith's chain. In my report of last year I estimated the amount required to complete the work at \$813,601 80. I have no changes to make in that estimate. The amount that can be profitably expended during the next fiscal year will be determined by circumstances. There should be at least \$500,000 at the disposal of the engineer, so that in case of a favorable season, the greater part of the entire work could be finished promptly.

Very respectfully, your obedient servant,

PETER C. HAINS,

Captain Engineers, Brevet Lieutenant Colonel, U. S. A.

Brevet Major General J. H. WILSON,

Lieutenant Colonel 35th Infantry.

E 6.

Abstract of proposals for removing the rock and other obstructions from the bed of the Mississippi river, at the Rock Island rapids, at a letting held in the United States engineer's office, Davenport, Iowa, June 5, 1867.

			<u> </u>
No.	Names of bidders.	Conditions.	Price per cubic y'd
1	E. R. Blackwell, New York, N. Y	2,000 cubic yards per month, at . 3,000do do do do	\$12 2 13 00 15 00
2	Jason C. Osgood, Troy, N. Y Damon Wells, Chittenango, N. Y	5,000dodo To remove all the rock at the rate of. To do the work by means of chis-	17 50 15 0
	Damon Wells, Chinemango, 11. 1	elling, and to remove at least 1,000 cubic yards per month	
		during 1867, when the river is not more than 4 feet above the low water of 1864, or ice in the	
		river to prevent working, and during all the working months of 1868, 2, 000 cubic yards per month until the whole amount	
		of the present appropriation is expended.	
3	John F. Hosch, Mohawk, N. Y	To remove all the rock agree- ably to the terms of advertise- ment, at the rate of.	17 75
4	W. Hunkins, Galena, Ill.; H. M. Man- deville, St. Louis, Mo.; Richard Smith, St. Louis, Mo.	To remove all the rock agreea- bly to the terms of the adver- tisement, at the rate of.	*14 00
5	Charles G. Case & Co., Fulton, N. Y. †.	To remove the rock at Smith's chain, for.	‡14 O
		To remove the rock at Sycamore chain, for.	13 00
		To remove the rock at Crab island, for.	13 00
		To remove the rock at St. Louis chain, for.	13 00
		To remove the rock at Camp- bell's chain, for.	15 00
		To remove the rock from Campbell's chain to Duck Creek chain, for.	13 00
	•	To remove the rock at Duck Creek chain, for.	10 00
		To remove the rock at Lower chain, for.	10 00
		To remove the rock at Moline chain, for.	10 00
6	William Irvin, Freeport, Ill.; Loda- wick Stanton, Freeport, Ill.	To remove all the rock agreeably to the terms of the advertise- ment, at the rate of.	16 (9)
7	George Murray, Davenport, Iowa	To remove the obstructions at the rate of.	18 95

^{*}This proposal was not in duplicate.
† Contract awarded June 12, signed June 28.
† This bid being at an average price of \$12 71 per cubic yard less than any other, the parties being recommended by parties known to this office as responsible and thoroughly competent to perform what they undertake, and the contemplated contract requiring the work to be done at such point or points as may be designated by the engineer in charge of the work, it is to the interest of the government that they should be declared the successful bidders and be required to enter into contract without delay. ders and be required to enter into contract without delay.

Abstract of proposals for removing rock, &c.—Continued.

No.	Names of bidders.	Conditions.	Price per cubic y'd.
	George H. French, Davenport, Iowa; James E. Abbott, Davenport, Iowa.	To build all coffer-dams to resist a rise equal to 4 feet above the low water of 1864, and to do all other excavation, except that done by coffer-dams at a stage of water not exceeding 3 feet above the low water of 1864.	
8	Elisha Reynolds, Rock Island, Ill.; Thomas Saulpaugh, Rock Island, Ill.; Louis E. Saulpaugh, Rock Island, Ill.; Thomas J. Buford, Rock Island, Ill.	To remove the rock agreeably to the terms of the advertisement, at the rate of.	*\$12 80

^{*}This bid is seventy-one cents per cubic yard higher than the average of Case & Co. If the work should be done from the upper end, the appropriation would go further in the hands of Reynolds, Saulpaugh, Saulpaugh & Buford, than in those of Case & Co., but the interest of the government, under the circumstances, require that the work should begin at the lower end, and that Case & Co. should have it.

I certify that the foregoing is a true abstract of the original bids put in by the bidders for this work.

J. H. WILSON,

Lieutenant Colonel 35th Infantry, Brevet Major General U.S. A. United States Engineer's Office,

Davenport, Iowa, Scptember 10, 1867.

E 7.

Abstract of contracts for the improvement of the Mississippi river at the Rock Island ravids.

No.	Name of contractors.	For what purpose.	Price per cubic y'd
1	Charles G. Case, F. D. Van Wanenew, Ful- ton, N. Y.		13 0 13 0 13 0 15 0 13 0 10 0

REMARKS.—The prices for excavating and removing the rock are to be in full compensation for all coffer-dams, bailing, machinery, boats, and materials of every sort required, and all labor necessary to complete, in all respects, the work as provided for in the contract, and to remove at least 5,000 cubic yards per month, unless prevented by some cause beyond their control.

J. H. WILSON,

Lieutenant Colonel 35th Infantry, Brevet Major General U. S. A.

United States Engineer's Office, Davenport, Iowa, September 10, 1867.

APPENDIX F.

NEW ORLEANS, LOUISIANA, September 12, 1867.

GENERAL: In compliance with engineer department circular No. 11, dated June 10, 1867, I have the honor to submit the following report of progress of all works of river and harbor improvements and surveys in my charge since November 15, 1866, the date of my last annual report:

IMPROVEMENT OF THE MOUTH OF THE MISSISSIPPI RIVER.

The contractor, Mr. Horace Tyler, who, according to contract, was to have formed a channel across the Southwest Pass bar, of 18 feet depth and 200 feet width, by the 23d of January, 1867, and maintain the same three months, failed to complete his dredge-boat, the Wiggins, before the latter part of March, 1867. He commenced work at the bar March 19, 1867. The time for completing the formation of the channel was, on his application, repeatedly extended till the latter part of May, 1867, when it appearing that, owing to the inadequate and imperfect character of his boat and machinery, he was likely to accomplish no results, his contract was annulled. In view of the additional appropriation of \$200,000, which was to become available on the 1st of July, 1867, I deemed it important to ascertain definitely which of the several passes was at the present period most susceptible of improvement by dredging or excavating. For this purpose authority was obtained from the engineer department to make surveys of Southwest Pass and Pass à Loutre, and the Coast Survey office kindly ordered two of its parties to report to me. The surveys were made during the spring months of the year, and consisted mainly in ascertaining the soundings, the extent of the bars along the mid-channel line, the positions and degree of straightness of the latter, and the character of the surfaces and sub-strata of the bars. Since July 3, 1867, I have been engaged in perfecting models, drawings, and specifications of a dredgeboat, authorized by a joint resolution of Congress, approved March 29, 1867.

On the 26th ultimo I commenced advertising for proposals for constructing and delivering the same. By the terms of the advertisements, the proposals were to be opened on the 10th instant, and the contract executed on the 14th instant. The hour of opening proposals has by further notice been postponed to 12 m to-morrow. The following information relates to points specially referred to in

the circular:

1. As a result of the survey, Pass à Loutre was selected for improvement under the appropriation for the current fiscal year, and the unexpended balance of the one for last year, amounting together to about \$273,000. The plan adopted is that of excavating and stirring or harrowing up the minute alluvial material, forming mainly the bars by deposit from the heavily-laden waters of the river, by means of double-ender dredge-boats fitted with an excavating screw 14 feet in diameter, of four blades, at one end, (similar and similarly mounted to the ordinary screw-propeller,) turning by means of a double engine at the rate of sixty revolutions per minute, and reaching two feet below the under side of the keel; and an auxiliary scraper or harrow at either end. The light, and in many cases almost semi-fluid material thus again brought into a disseminated and floating condition will be gradually carried off to deep soundings by the current of the river and the tide of the gulf.

2. Owing to the character of the condition under which the bars are formed and maintained, this work is not susceptible of "entire and permanent completion." The water of the river is heavily and constantly laden with sediment, with an increasing tendency to rapid deposit on the bars the instant the channel depths are increased. After the completion of the two dredge-boats required for the work, an annual expenditure of \$100,000 will be required for the constant maintenance of a 20-foot channel; of which \$75,000 is estimated for removing

and working expenses, and \$25,000 for repairs, buoys, &c.

3. During the next fiscal year \$375,000 can be profitably expended upon the work, and is absolutely essential to its success, of which amount \$100,000 is estimated for running and working expenses, repairs, &c., and \$275,000 for construction and delivery of the second of the two required dredge-boats. least two dredge-boats are required for constant maintenance of a channel, since a machine of their character is liable at any time to become disabled to the extent of requiring a cessation of work for repairs, and must be docked at least once a year for general repairs and overhauling.

4 and 5. This work is located in the collection district, and near the port

of New Orleans.

6. The amount of revenue collected at the port of New Orleans during the last fiscal year is five million three hundred and eighty-eight thousand three hundred dollars and sixty cents, (\$5,388,300 60.)

7. During the year 3,953 vessels, with a tonnage amounting to 2,290,461 tons, were cleared from the port of New Orleans. The freights on flour and grain from St. Louis to New York via New Orleans, and from St. Louis to New York by rail, are about as follows, viz:

St. Louis to New York via New Orleans.	St. Louis to New York by rail.		
Flour	Flour		

For further information and remarks in this connection reference is respectfully made to my annual report dated November 14, 1866.

8, 9, and 10. No proposals have been considered nor contracts executed since my report of last year.

SURVEY OF GALVESTON HARBOR WITH A VIEW OF FORMING PLANS FOR ITS PRESERVATION AND IMPROVEMENT.

Great difficulty was found in securing the services of experts competent for this survey, and some delay had occurred in efforts to this end, when, at the solicitation of the engineer department, the Coast Survey Office kindly ordered one of its parties, under command of Mr. Nes, to report to me for the execution

In May last I ordered Lieutenant W. S. Stanton, corps of engineers, one of my assistants, to proceed to Galveston and direct the survey. The assistant of the Coast Survey did not report with his party until the 31st of that month. From the 1st until the 6th of June the party was engaged in repairing the surveying vessels and in taking in supplies. On the 6th they commenced taking soundings and making current observations in that part of the harbor west of the meridian through the eastern extremity of Bird Key; they were occupied upon this work until the 20th.

During this time 22,859 soundings, distributed over an area of fifty-two (52) square miles, were taken. Current observations were made at the points where the surveying vessel was anchored. On the 21st preparations were commenced for the survey of Red Fish bar, but were discontinued on the 24th, and the survey of the shore line of the harbor begun.

From the 25th of June to the 6th of July the party was engaged on the survey of the shore line of Pelican island and Bolivar Point, in making current observations in Galveston and Bolivar channels, and in ascertaining the character

of the sub-strata of Pclican Spit.

Sickness having broken out among the officers and men, and the weather being very unfavorable for the prosecution of the work, the party was discharged on the 6th.

The charts, exhibiting the results of the survey as far as it had progressed,

are now preparing at the Coast Survey Office in Washington.

As soon as a party can be had after the subsidence of the yellow fever the field work of the survey will be resumed. I anticipate that in the course of the ensuing winter I shall be able to complete the survey and prepare and submit a plan for preserving and improving the harbor.

I have in hand funds probably sufficient to complete the survey, from the

appropriation for examinations and surveys on the Atlantic coast.

SURVEY OF PASS AND BAYOU MANCHAC AND AMITE RIVER, LOUISIANA, WITH A VIEW TO DETERMINING THE FEASIBILITY OF FIRST-CLASS STEAMBOAT NAVIGATION THROUGH THEM BETWEEN THE MISSISSIPPI RIVER AND LAKE PONT-CHARTRAIN.

The field work of this survey was begun and completed during the months of May and June last under the superintendency of Lieutenant J. K. Hezlep, corps of engineers, one of my assistants, since deceased The report and accompanying maps and plans are in course of preparation, and will, doubtless, be in readiness to be submitted early in December next. No funds in addition to the amount derived from the appropriation for examinations and surveys of western and northwestern rivers now in hand are required.

Very respectfully, your obedient servant,

M. D. McALESTER,

Brevet Major General A. A. Humphreys,

Chief of Engineers, Washington, D. C.

F 1.

United States Engineer Office, New Orleans, March 29, 1867.

GENERAL: I have the honor to acknowledge the receipt of engineer department letter of the 23d instant, notifying me of an additional appropriation of \$200,000 for improving the mouth of the Mississippi, and asking information as to the success and prospects of success obtained by the present contractor, Mr.

Horace Tyler, &c.

After considerable delay, due to repeated accidents to the machine during informal trials at the wharf, the ferry boat Wiggins, containing the machine designed for reducing the bar, started for the mouth of the river on the 19th instant, commanded by Captain E. L. Brady, and accompanied by Lieutenant D. W. Payne, corps of engineers, and myself. The pumps for feeding the boat's boilers got out of order several times on the way down, and the buckets of her driving wheel were found shifting, owing to the greatly increased depth to which she was settled in the water by the introduction of the dredging machine. A day at Fort Jackson was, therefore, taken for repairs, and another after arrival at the telegraph wharf at Southwest Pass. It became apparent on the way down that the Wiggins had not sufficient power to permit of her being handled easily, and to enable her to hold the conical screws in contact with any desired points of the bar, either with or against the current. Captain Brady therefore decided not to attempt work on the bar until the contractor should send a tug to assist the Wiggins.

On the 23d, however, the machine was taken to the flats, near Pilot Town, in Southwest Pass, about a mile and a half above the bar, for trial. The first trial was in six feet water, hard bottom, the Wiggins being tied to a wharf.

Whenever the conical screws came in contact with the bottom they tore it up without difficulty, and in the course of twenty to thirty minutes the water under and in immediate vicinity of the screws (their position was shifted laterally but five or six feet) was deepened from one foot to two and a half feet. For the second trial the Wiggins was cast off into eighteen feet water where the current was comparatively slight, her screws lowered to that depth, and set to work, the boat being placed and held in position by means of her own propelling power; bottom soft. The boat could no more than hold her own against the current, but she shifted position gradually in shore to fifteen feet water, the screws still turning about ten revolutions per minute, and easily clearing their way in the three feet of soft mud. The hoisting and lowering apparatus worked well, the work being done by one of the engines provided for turning the screws. The screws, with the frame in which they are mounted, were hoisted or lowered through a distance of twelve feet in eight minutes.

The conical screws are twenty feet long, five feet in diameter at their basis, and are placed so that their points come together in front of the boat's cutwater, and their bases are separated from each other so as to measure about twenty feet "from out to out." They are mounted so that their axes are horizontal, the salient angle they form being foremost. Their flanges are twelve feet wide at

the base of the cones, diminishing to six inches at the points.

These trials convince me that the screws can remove whatever sedimentary portions of the bar they may be brought in actual contact with, whether they be hard or soft. The limited surfaces of the flanges, and the small velocity with which they strike the water, can insure very little, if any, work, however, (except when the bar is very soft,) from the "wash," and the effort to throw up the particles, disturbed from their beds, well into the current will doubtless be slight. It will be noted that all the dimensions adopted for the conical screws are reduced one-half (about) from those prescribed by Mr. Bishop, their inventor.

With the knowledge I have, I can as yet predict neither the success nor nonsuccess of the contractor. The doubtful questions are: Can the machine be handled, even with the help of a tug, with sufficient facility and precision to perform the required work? To what extent will operations be hindered by accidents to which the character both of the boat and machine render them peculiarly liable? There is, moreover, the liability to shipwreck. (See my letter of 2d

instant.)

I returned to this city on the 25th instant, leaving Lieutenant Payne on the spct, with orders to report to me without delay any incident relating to the success or prospect of success of the contractor's efforts. Anything of this character transpiring I will report immediately to the department. A tug, to serve constantly as tender and tow-boat for the Wiggins, started for the bar this morning.

The last extension granted the contractor expired on the 10th instant. I have decided to take no further action as to extension, awaiting the determination of

the question of the contractor's success.

As to the boat designed especially for work on the tide-water bars of the Mississippi, proposed in my letter of the 2d instant, a further description of her may not be out of place at the present time. She should be a "double-ender," provided with two strong and powerfully driven screw propellers, one at each end, driven by separate engines, and with water-tight compartments or tanks, such that when empty the vessel would draw sixteen feet, (a depth of channel always possible across the bars,) and when full of water, say twenty-four feet, (a depth of channel as great as will probably be called for for many years to come.) When drawing twenty-four feet her upper or spar deck should be not more than two feet above the water, in order to bring the point of attachment of any device it might be found necessary to add in aid of the action of the propellers as near the working point on the bar as possible. The total depth of the vessel from the spar deck to the bottom of the keel would, therefore, be twenty-six feet, (about.) Her spar deck should be nearly flat, like a monitor's,

and perfectly clear of all obstructions from both ends to near "midships," to facilitate the addition at any time of any device found necessary. have a single pilot-house midships, about or under which any structures required for quarters and other purposes could be placed. Her length and beam would be determined by the space and flotation required for her water compartments, boilers, and machinery, coal, &c. The minimum draft being sixteen feet, the screw propellers should describe circles of about that diameter. of blades allowed the propellers would be a matter for consultation with experts, but it is likely that four blades would be preferable. Their ends should be shaped so as most readily to cut away compacted mud. I apprehend no difficulty in mounting and shaping the rudder-posts and rudders so as to be fully able to stand any impact against sedimentary deposits of any degree of hardness, and so as not to interfere essentially with the action of the screws in cutting away such deposits. The forward rudder would, of course, be always made fast in a position parallel with the keel, (like that of a New York ferry-boat,) to be released when the engines should be reversed. The rudders would, of course, have a different shape from an ordinary ship's rudder, in order to avert damage by impact.

The vessel should be very staunch and strong.

Such is a brief outline of my plan for a dredge-boat. I would not feel competent to elaborate the details and working plans without consultation with expert ship-builders.

The screw propeller embodies whatever is valuable in the principle of Bishop's conical screws, and moreover exceeds them in efficiency to the extent of the powerful wash it exerts in immediate contact with the bar, both to displace the particles composing it and to throw them well up into the current.

On witnessing the performance of Bishop's cones, I determined that their efficiency would be as great as it is at present were the cones cut off within five feet of their bases and placed side by side, nearly in contact, in front of the bow of the boat. As at present arranged, they have a ridge between them.

I have no doubt whatever of the ability of a vessel (one or more) like the one I have described to produce and maintain the requisite depth of channel everywhere when the bar is smooth and free from mud lumps, with her screws unaided, and very little doubt of her ability to reduce even the mud lumps. One thing is certain, and that is, if the screws are sufficiently strong, and turned by engines sufficiently powerful, they will necessarily cut away whatever compacted mud they come in contact with.

During my passage up the river on the screw steamer Monterey, (25th instant,) one of the blades of her screw struck the limb of a drifting tree, (the limb being perfectly sound and eight inches in diameter,) and cut it short off as if done with a single blow of an axe, producing no injury to the propeller screw, the ship, or the machinery. The engine was not even stopped. If necessary, however, to cut away the mud lumps any device thought to be efficient for that purpose can at any time be added. Indeed, it may in any event be a question whether the plan of blasting the mud lumps, so successfully applied by Colonel Long, (see his reports,) is not the best one. The experience of Mr. Tyler, under the present contract, in regard to mud lumps may be valuable.

I am authentically informed that the propeller Matanzas, reaching the bar about eighteen months since with a greater draught than was then admissible, worked her way through, stern foremost, by means of her propeller screws, the depth of mud worked through being estimated at two to two and a half feet. Of course the ship drew more water aft than forward. *

[&]quot;Several other instances of ships making their way through the muddy deposit, stern foremost, by means of their own screws, have been indirectly reported to me.

The plan of adjusting the draught of vessels by filling and emptying their water compartments provided for that purpose, has been frequently applied successfully to vessels of war-

A dredge-boat such as the one proposed could, I presume, be constructed within three months after the completion of the working plans, at ship-yards possessing extensive resources like some existing at New York, Philadelphia, and Wilmington. Requisite skill and facilities are not to be found here nor on this coast.

I would respectfully call the attention of the department again to my remarks on the subject of width of channel, thorough and ample buoying of the same, and the necessity of the constant presence of a person properly authorized to control ship and tow-boat masters and pilots, and regulate the passage of vessels

through the channel, (see my letter of 21st ultimo.)

The buoys should be ranged along either side of the channel at intervals of fifty to one hundred yards, so as to serve at night on lines say 200 feet apart, equidistant from the mid-channel line. The work of dredging could then proceed for a width of channel of 120 to 150 feet, and vessels pass through without interference with the buoys. The buoys should be placed under direction of the officer in charge of the improvement; the buoys being furnished by the light-house bureau, or purchased by himself. At present there are but five buoys in the Southwest Pass, one of which marks a wreck and another a mud lump. Buoys are placed under direction of the light-house inspector when their necessity is claimed by the pilot. As a matter of course the pilots will have no more buoys placed than are absolutely necessary to indicate the channel to themselves, in order that no vessel may enter the channel without their services.

I regard the proposition of stationing a person at the channel, with absolute control of it, as of the first importance, not second even to the project of deepening the channel itself. Should Pass à Loutre be finally selected for improvement, there will be scarcely any opposition to obtaining and maintaining absolute

control, Southwest Pass being the one at present principally used.

In my letter of the 21st ultimo, above referred to, I stated that I believed the pilot commissioners were appointed by the governor of the State. I have since learned authentically that they are elected by the pilots themselves, the persons elected being themselves bar pilots, the governor merely going through the form of confirming the election.

As strange as it may appear, when the importance of the interests involved are considered, the pilot commissioners, once elected, are subject to no authority

whatever.

Very respectfully, your obedient servant,

M. D. McALESTER,

Brevet Brigadier General of Engineers.
Brevet Major General A. A. HUMPHREYS,
Chief of Engineers, Washington, D. C.

F 2.

United States Engineer Office, New Orleans, La., June 7, 1867.

General: In accordance with instructions contained in engineer department letter of the 13th ultimo, I transmit herewith a plan, with specifications, of a dredge-boat which, in my opinion, will accomplish the best results in opening channels across the bars at the Mississippi river mouths. I have been unable to describe more specifically many important parts of the boat in consequence of the absence in this vicinity of any person sufficiently expert in ship-building, marine architecture, and marine machinery, to be of any aid in consultation. Should the department approve the plan, I would respectfully suggest, if deemed expedient, that I be authorized to visit some point on the Atlantic seaboard, where access can be

had to persons expert in planning mercantile or United States naval, sea-going steamers, with a view to fixing definitely upon the several parts of the dredge-boat, so that they shall be compatible with each other, and with maximum efficiency; and to drawing up specifications corresponding thereto, to be used in the advertisements for proposals, and in the draft of the contract for building the dredge-boat. Otherwise I would respectfully suggest that working plans and specifications, based on the outline description and specifications herewith submitted, might be advertised for, offering a premium for the one adopted.

As stated in my previous communication on the subject, I feel confident that the dredge-boat I propose will be competent to deepen and maintain the required channel by means of the propellers alone. But with a view to rendering ultimate success certain, I have so devised that the boat can at any time receive additional machinery thought competent or necessary either to accomplish or hasten the desired results. In furtherance of the view, and also for the following reasons, I have made full provision (see specifications and sketches) for the addition, at will, of scrapers to act on the bars, in conjunction with the propellers:

1. The adaptation of the boat (apertures, hawser-holes, &c., in the hull) will occasion a very insignificant outlay while she is yet in the stocks; and the scraper apparatus (see rough sketch herewith, illustrating its character) can be fabricated in this city, and attached at any time while the boat is afloat, by means of simple appliances, it being only necessary to moor her in still water.

2. We know that the plan of scraping is, to a certain extent at least, efficient by actual trial during a period of at least seven months of the season during which the bars constitute the greatest obstruction to navigation, (January to August, 1860; see certificates of C. H. Fuller, supervisor, &c., and approvals of Colonel Long, topographical engineers, United States agent, pp. 205 to 210. Executive Document No. 5, thirty-sixth Congress, second session, House of Representatives.) The scrapers I propose are equally efficient with the one applied by Colonel Long, (see p. 162 of document just referred to,) are much

simpler in their construction and application, and far less unwieldy.

3. In passing up stream across the bar, the resistance of the current is such that the boat may be made to pass over the bottom slowly enough to obtain the maximum excavating effect of the propeller on the bar, without, at the same time, preventing efficient steering, or the boat being kept parallel with the current, (in this case it is the up-stream propeller that does the work both of propulsion and excavation.) But in passing down stream, across the bar, it is desirable also to get the effect of the propellers in excavating it; and as the current is three feet to four feet per second, in order to obtain "steering headway" the boat must pass so rapidly over the bottom as partially to neutralize that excavating effect. But the scrapers (which are attached to the up-stream end of the boat only, and arranged to work down stream, the work of propulsion being in this case done by the down-stream propellers) obviate this defect, aid to hold the boat parallel to the current, and the propeller to its work of excavating. Moreover, in moving down stream, there will be a surplus of power which can, better than not, be utilized for working scrapers. As before stated, however, any other preferable device which may occur or be presented may, at any time, be attached when the necessity becomes apparent.

In devising the dredge-boat, I have constantly kept in view the important consideration of ample power combined with thorough staunchness; fully impressed with the idea that no cheap device, however ingenious, could be invented, such that the immense deposits obstructing the bar channels could be disposed of as if by something like magic; and fully believing that, in order to accomplish the object, a certain sum of money must be expended, a certain amount of

steam generated, and a certain quantity of actual work done.

I am, mereover, still of the opinion (see my letter on this subject, June 25, 1866) that continuity of work is a consideration of great importance, whatever

plan may be used, in order that the deposits, once displaced or floated up, may be floated or washed beyond the limits of the bar before being redeposited. Hence the "double-ender," obviating the loss of time occasioned in turning a "single-ender," at each change of direction across the bar.

I am aware that the cost of the boat will be considerable; but that it can be built and a channel of twenty feet maintained for a year within the amounts now appropriated, I have no doubt, and the boat will be available for operations

under subsequent appropriations.

It may be proper to note that some time last February I intimated my plan of combining the ordinary ship's propeller with the adjustment of a boat's draught of water by means of water tanks to Captain E. L. Brady, Mr. Tyler's agent, in command of the Wiggins, and in charge of the excavating operations on the bar under his contract. In the latter part of March following, while at the bar, witnessing the operations of the Wiggins, I described my plan more fully in his hearing. About the middle of last month he showed me in my office drawings of a device which he had conceived, depending upon my plan, as I had described it in his hearing; the application of the plan differing considerably, however, from the one I submit herewith. I have just been informed that he has applied for a patent in the premises. Whether or not his patent, should he obtain it, will cover the principle involved, that of using propellers in connection with adjustment of draughts, as stated, for the purpose of excavating under running water, I am unable to decide. It might be worth while to refer to the Commissioner of Patents on the subject. Although I had no intention of patening myself any device involving the principle named, or the principle itself, (were such a thing possible,) I certainly did not authorize Captain Brady to do so.

The surveys of Southwest Pass and Pass à Loutre by the coast survey parties designated for that purpose under my direction have been completed, and full reports, illustrated by charts, submitted to me. From these I have caused deductions to be made, a statement of which is herewith respectfully submitted. I hese show, as I had anticipated, that Pass à Loutre possesses decided advantages for improvement over the other pass; although owing to the rapidity of the current and the imperfect character of the appliances available for the purpose, the investigation of the character of the sub-strata of the bars was not as thorough as was desired. Moreover, as hitherto remarked, the absence at Pass à Loutre of the throng of vessels at some seasons obstructing Southwest Pass,

constitutes a consideration of vast importance in favor of the former.

I therefore recommend, unreservedly, that Pass à Loutre be selected for future operations.

Very respectfully, your obedient servant.

M. D. McALESTER,

Brevet Brigadier General, Major of Engineers.

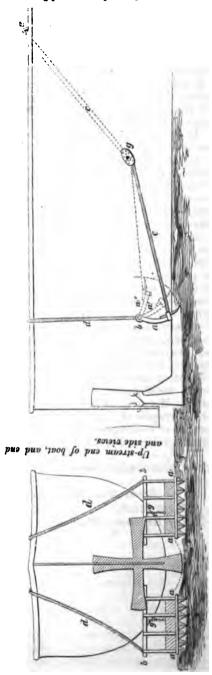
Major General A. A. HUMPHREYS,

Chief of Engineers, Washington, D. C.

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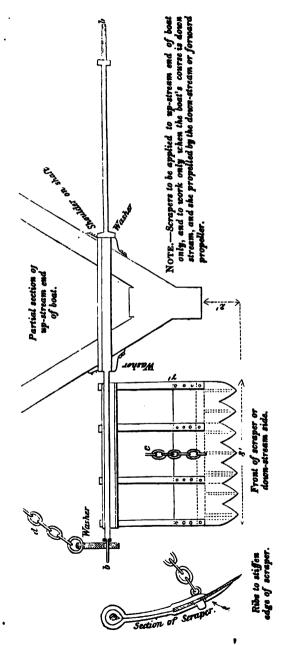


Improvement of the mouth of the Mississippi river.—Dredge-boat.—Proposed application of scrapers.

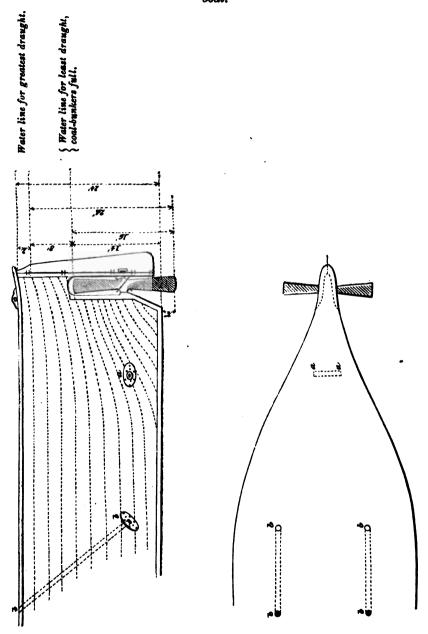


a, scrapers in position for deepest cutting; a', position for less depth of cutting; a'', position of scrapers raised clear of bottom passing up stream across bar; b,

cast-iron shaft for scrapers; c, chains holding scrapers in position; d, chains attached to washers on ends of shaft, holding scrapers and shaft against any tendency to motion in direction of axis of shaft; c, windlasses on deck for holding and adjusting scraper-chains; g, hawser-holes through which scraper-chains pass.



Improvement of the mouth of the Mississippi river.—Plan of proposed dredgeboat.



The dredge-boat to be a "double-ender" of like shape and construction at both ends, provided with two strongly built four-bladed propellers, one at each end, on separate shafts, powerfully driven by separate engines, or sets of engines, and with compartments or tanks, such that when they are empty and the coalbunkers full the boat will draw fourteen feet, and when full twenty-two feet.

The distance between the spar deck and bottom of the keel to be about 24 feet, so that the former will be about two feet above the water-line at deepest

draught.

The extremities of the propeller blades to describe circles of 16 feet diameter; their shaft centres to be six feet above the bottom of the keel, and directly over it, so that the ends of the blades will pass two feet below the bottom of the keel, and at minimum draught (coal-bunkers full) tangent to the water surface.

Length and beam to depend mainly upon the space and flotation required for boilers, engines, pumps, and other machinery, coal storage, quarters for crew, and the consideration of a good model. The beam can probably be safely assumed at 30 feet, and a well-proportioned length would probably be at least 150 feet.

SPECIFICATIONS.

Hull.—Of wood, constructed with as much strength throughout as is possessed by first-class Atlantic steamers, and, moreover, to be re-enforced at both ends in the most practicable manner to the extent of affording complete security against injury from shocks occasioned by the propeller blades striking the tough clay incrustations covering portions of the Mississippi river bars. Dead-wood to be built in at the stems, so as to increase their stability and steadiness against shocks. The model to be so formed as to give the least practicable resistance, consistent with strength and other primary considerations, to progress through the water, and to speed. The bottom to be covered with yellow metal to the water-line at 18 feet draught. Cut-waters to be inclined near the keel, so as to clear well the space about the propellers at that point.

Spar deck.—To be flat as possible consistent with adequate drainage; surrounded with a substantial railing, and as clear as possible of obstructions, especially from the stems to distances of 40 or 45 feet towards midships.

Propellers and shafts.—These to be, say, twenty-five per cent. heavier and stronger than would be necessary for ordinary sea-going steamers. The pitch of the blades to be that which is most approved for speed and efficiency in water. The ends of the blades, and the edges for a distance of two feet from the ends, to be given well-defined edges, the angles of which being, say, 25 degrees. Shafts to be confined by means of bearings of unusually great numbers and strength, (to avoid as much as possible vibrations caused by impact of the propeller blades against hard or tough substances,) and to be provided with shoulders of sufficient number and strength to prevent the possibility of any longitudinal motion by the unusual strain in that direction to which they will be subjected.

Rudders and rudder-posts.—All these to be wrought iron. Posts to extend to within, say, two feet of the bottom of the keel; to be well secured to the ends of the propeller shafts, resting against these and held there firmly by means of bolts or journals in prolongation of the axis of the shafts, the journals passing through the posts and taking keys or heads on the outside. So much of the posts as interfere with unshipping the propellers to be movable. Rudders to admit of being unshipped, of course. Rudders to be so arranged as to be fixed

at will in positions parallel to the keel.

Water tanks for regulating draught.—These to be arranged in sets running fore and aft; each set to be equally distributed on both sides of the keel, to have the areas of its cross-sections perpendicular to keel, proportional to the areas of the corresponding cross-sections of the hull from stem to stem, and to be subdivided in such a manner as to confine the contained water and prevent its following any motion of the boat, for the purpose of obviating as far as practicable any tendency to "shift cargo." The tanks or compartments of each set to be connected by apertures; these to be fitted with convenient devices for firmly closing and opening them at will. Each tank to have a manhole, admitting of the re-

moval of sand and mud (precipitated by the contained water) when pumped dry. The tanks to be filled by means of scuttles placed just below water-line for minimum draught when the coal-bunkers are empty, and emptied through scuttles placed just above water-line for maximum draught, by means of pumps driven by donkey engines.

The capacity of these scuttles and pumps to be sufficient to fill all the tanks

within, say, thirty minutes, and to empty them in a like space of time.

All scuttles to be fitted with convenient devices for securely closing them, and those admitting water to the tanks to be also fitted with substantial strainers.

Propeller engines.—Each propeller to be driven by a power, say, twenty-five per cent. greater than would be required for a like propeller in an ordinary sea-going steamer. This to be accomplished by means of two cylinders placed base to base, with their axes in the same right line perpendicular to the shaft, the common base of the two cylinders being directly over the prolongation of the shaft; each piston-rod being connected with the crank-pin by means of two short connecting rods and a rock-shaft with two arms strongly made and mounted, an arrangement adopted generally on sea-going monitors, giving great simplicity and compactness while obviating all dead points.

Boilers.—The two sets of engines driving the two propellers should be supplied by steam from separate boilers, or sets of boilers, so arranged, however, as

to be all connected together, and disconnected at will.

The several donkey engines ought also to have separate boilers.

Furnaces. These to be adapted for burning soft coal.

Storage for coal —Sufficient coal storage to be provided to supply the furnaces of all the engines on board one hundred consecutive hours of uninterrupted running.

Anchors and cables.—Three anchors, each capable of holding the boat against the Mississippi river current, and five chain cables of weight and strength to correspond to the anchors, of the length usually provided for sea-going ships.

Masts and sails.—One short mast rigged for a fore-and-aft sail, with sail and rigging all complete; and the gaff fitted completely for hoisting coal on board.

Quarters.—To be provided below spar deck for thirty men, and a house, say fifty feet long by eight feet wide, running fore and aft amidships on the spar deck, to be fitted up for an office and quarters for six persons, (one officer, ship's captain, mate, purser, and two steersmen.)

Pilot-house.—One pilot-house amidships on top of officers' quarters.

Pumps.—An ample supply of steam and hand pumps for putting out accidental fires, washing decks and other necessary purposes, with full supply of hose to lead to every part of the boat; steam pumps to be worked by steam from the main boilers.

Arrangement for attaching two scrapers, one each side the keel at either end of the boat, at will.—At a point near each stem, say twelve feet distant from it, and five feet above bottom of keel, a cylindrical tube of boiler iron five inches interior diameter (a, see sketch) to be inserted, running from out to out of the hull, athwart ships or at right angles to the keel, and projecting far enough on either side to take a large and strong wrought iron washer which is to be accurately fitted and strongly bolted to the ship's side, and to present a shoulder three inches wide around the aperture of the tube, the plane surface of the shoulder being perpendicular to its axis

At points at least twenty-four feet from these tubes or apertures (measured parallel to the keel and towards midships,) and eight feet (measured horizontally) from the middle of the keel, hawser-holes to be inserted (four in all) through the boat's sides, connecting with the spar deck by means of strong cast iron tubes extending up obliquely towards midships, large enough to admit short-

linked three-fourth inch chains, (b, see sketch.)

Should the iron in contact with the yellow metal sheathing give rise to too rapid oxidization, any other equally strong metal can be substituted.

Small boats.—One life-boat, two yawl-boats for ten oars each, and one skiff for four oars; all to be of the best and most durable material, and of the best models for speed and efficiency. Two sets of davits, with complete rigging for lowering and hoisting these boats at convenient points, one on each side of the

dredge-boat.

The dredge-boat is to be fitted up with all devices, fixtures, appliances and accessories (except such as may hereafter be specially designated) that are usually provided for first-class ocean steamers; everything pertaining to the boat to be of the very best material and constructed and applied in the most skilful and workmanlike manner.

Respectfully submitted:

M. D. McALESTER,

Brevet Brigadier General, Major of Engineers.

Major General A. A. HUMPHREYS,

Chief of Engineers, Washington.

Deductions from a special survey made by parties of the Coast Survey during April and May, 1867, under direction of Brevet Brigadier General M. D. Mc Alester, engineers.

	Southwest Pass.	Pass à Loutre.
Distance across the bar between 20 feet curve	14,583 feet	8,750 feet.
channel line being the line of deepest soundings	150 feet	150 feet.
Mean depth of water in present channel	17. 19 feet	15. 38 feet.
20 feet channel	2.81 feet	
tion for a 20 feet channel Least depth of water on mid-	221,661	
channel line	11 feet	11 feet. 3 feet 9 inches.
points	Station A, water 6 feet deep, soft mud for a depth of 8 feet; no sand.	Station A, water 12 feet deep a stratum of hard sand an clay 2 feet thick; soft mu below.
	Station B, water 7 feet deep, soft mud for a depth of 8 feet; no sand.	Station B, water 11 feet deep very hard sand and mu- unable to sink pole to greater depth than 4 feet
	Station C, water 15 feet deep, stratum of hard sand and clay 6 inches thick; soft mud and sand below.	Station C, water 14 feet deep soft mud; pole thru down 5 feet.
	Station D, water 15 feet deep, clay and sand for a depth of 5 feet; clay very sticky.	Station D, water 17 feet deep very soft mud.
•	Station E, water 16 feet deep, soft mud for a depth of 2 feet.	•
	Station F, water 18 feet deep, soft mud and sand. Station G, water 19 feet deep, soft mud and sand.	

REMARKS.

A comparison of these data with the Coast Survey chart shows that the bottom of the channel of the Southwest Pass, between the upper 20-foot curve and the wreck, a distance of about two and one-third miles, is composed of soft mud mixed with a small quantity of sand; between the wreck and the outer 20-foot curve, about one-half of a mile, incrustations of a fine sticky clay with occasionally some sand, varying in thickness from six inches to eight feet, are found.

These incrustations exist in patches throughout the greater portion of the channel between the limits just named; below, and in the intervals between

them, the bottom is of soft mud.

At Pass à Loutre, between the upper 20-foot curve and the crest of the bar, a distance of about one and one-fourth mile, the bottom is composed of soft mad with a little sand; from the crest of the bar to the outer 12-foot curve, about eight hundred feet, a hard incrustation of sand and mud, varying in thickness from two to four feet, is found; from the outer 12-foot to the outer 20-foot curve the bottom is of soft mud.

There is a mud lump covered with a hard crust two feet thick in the south

side of the channel, about 175 feet from the outer 20-foot curve.

This lump is about 300 feet long and 160 feet wide, and would project about 60 feet into a channel 150 feet wide, passing to the north of it.

Respectfully submitted:

M D. McALESTER,

Brevet Brigadier General.

Major General A. A. Humphrbys, Chief of Engineers, Washington, D. C.

APPENDIX H.

Report of Colonel J. N. Macomb.

OFFICE OF WESTERN RIVER IMPROVEMENTS, Cincinnati, Ohio, September 20, 1867.

GENERAL: I respectfully submit herewith my annual report upon the duty of improving the western rivers, &c., to which I was assigned by Special Orders No. 418, dated War Department, Adjutant General's Office, August 22, 1866, and your letter to me of September 3, 1866.

By Special Orders No. 447, War Department, Adjutant General's Office, September 7, 1866, Brevet Major C. W. Howell, captain of engineers, was assigned

to duty as my assistant, and reported to me on my arrival here.

The appropriation for building snag-boats and for making the requisite examinations and surveys, and for commencing the improvement of these rivers, were made by the act of Congress approved on the 23d of June, 1866.

I have thus far merely caused such examinations of the Arkansas, Missouri, and Mississippi rivers to be made as would suffice to show at what points it would be most important and useful to commence work when all the means and appliances therefor shall be in readiness for use, it seeming on all sides to be conceded that the amount of work required for these improvements is so great as clearly to come under the designation of indefinite, both as to extent and as to the time required for accomplishing it.

My attention was called in the first place to the duty of procuring the proper boats and machinery for this work. On inquiry I found that there was nothing suitable to be had by purchasing, and I recommended that we should build the

boats and machinery.

About the end of November I received from the engineer department drawings of one of the old snag boats used on these waters some twenty years since, and after a careful study of the plan on the part of my assistants and myself, I called to my aid Mr. E. M. Shield, one of the most experienced and judicious mechanical engineers of the west, by whom were prepared the models and specifications for a snag boat and machinery, which were submitted to the engineer department on the 13th of February, 1867, and approved on the 26th of the same month.

Important changes were made in the plan of the boat and machinery. In the first place the machinery for hauling out snags is quite independent of the propelling wheels of the boat, thus giving a much better command of the whole power, and affording the means of controlling the position of the boat while grappling with a snag or other obstruction to be removed from the river. Auxiliary engines are also introduced for various duties which were done by hand in the old plan; this will probably increase the efficiency of the boat, while the crew may

be much smaller than it was found necessary to have in the old boats.

In accordance with instructions, I advertised for proposals for building three boats, with appliances as above, and at the expiration of the time for receiving bids I reported to the department touching the embarrassment which I felt, arising from the fact that while the lowest bidder for hulls of boats worked in one city, the lowest bidders for cabins and other important parts worked in quite different places, and I asked that I might so give out the contracts as to have each boat completed and equipped at some one locality; but this was deemed to be not in accordance with the law, and I was directed to give out the contracts strictly in conformity with the lowest bids for separate parts of the boats, so that one of the boats which we are building at Cincinnati will be towed to Madison to receive its cabin and machinery, and the boat which is building at New Albany will need to be towed up to Louisville to receive its machinery; and there are other anomalies of similar import incidental to the requirement of taking, in all cases, the "lowest responsible bidder."

The plan of receiving bids only from persons manufacturing or dealing in the articles wanted is doubtless the best one both for the government and for the dealers; but there should be a condition introduced to prevent the difficulties and delays experienced in regard to building these boats or any similar work, and that condition should be such as to require that each boat should be completed and equipped where built. This, I suppose, would be accomplished by fixing upon the towns or places where such work could be advantageously done, and then taking bids for the several classes of work to be done at those places. I trust that authority may be given to the departments having such work in charge to conduct the business in that way or in some better one.

Abstracts of bids and contracts thus far given out under the head of

snag-boats are herewith submitted, marked A, B, C, D, E, F, and G.

The hulls of the boats are all well advanced, and can be launched as soon as the water shall have sufficiently risen to make it safe to launch them. The machinery is in a good state of forwardness, and will be ready for setting up as soon as the boats can be placed within the reach of the parties contracting for it.

On the 12th of December, 1866, I sent an assistant to proceed from Cairo, Illinois, to Fort Smith, Arkansas, and note the positions of all the principal dangers to navigation on those parts of the Mississippi and Arkansas rivers embraced in that route. The result of that mission was explained to the department in my letter of the 21st of February, 1867.

A list of the more important obstructions to the navigation of the Arkansas

is berewith submitted, marked H.

An inspection of this list will show that there is a great amount of work to be done in that river.

On the 28th of February, 1867, I submitted to the department a report from

my assistant, Major C. W. Howell, relative to the removal of wrecks in these rivers, and recommended that a steamboat should be purchased and fitted out with submarine armor, diving-bells, and electro-magnetic batteries for exploding torpedoes, and with derricks and all other appliances for blowing up wrecks and hoisting out the detached parts. This recommendation was approved, and I was directed to advertise for proposals for the sale to the government of a suitable steamboat, which was described in the advertisement. Only one bid was received under this head, and a contract was entered into by which the steamer Commodore was purchased for this service. The alterations and fitting up of this boat being of a nature not easily covered by a contract, I was authorized to have them done by days' work, under the supervision of Mr. Pierce, the assistant whom I had placed in charge of the boat.

This boat, on being fitted up for the wrecking business, was called by me

the General J. G. Totten, as stated in my letter of 28th May, 1867.

The boat has been employed in blowing up wrecks in the Mississippi river with considerable success, although laboring under the disadvantage of working in a very high stage of water, which will doubtless render it necessary that the wrecks which were first undertaken should be revisited during the low water of this fall.

My first instructions to the assistant in charge were to proceed in the steamer with the wrecking party to commence removing two wrecks near St. Louis, which were considered as prominent dangers. On reaching St. Louis a supply of powder for use in the torpedo cans was procured at the United States arsenal. the ordnance department having authorized its sale to me for this purpose. The first work undertaken was the removal of the White Cloud and Belie Memphis, lying together just off the lower part of the city of St. Louis. The work upon these wrecks was continued until the 25th of June, when, as well as could be judged at the high stage of water then prevailing, it was deemed to have been accomplished, and was left with the intention of returning to make further examination during the low water of the fall. While the boat was at the above work there occurred the lamentable accident of the sinking of the steamer Governor Sharkey in that vicinity, on which occasion our wrecking party was instrumental in saving life, as reported by me in a letter dated July Ī, 1867.

On the 27th of June the wrecking steamer reached Memphis with a view to the removal of the wreck of the Jeff. Thompson, which had been the means of causing the disaster to the steamer Platte Valley, with great loss of life. The party continued to work in the vicinity of Memphis during the greater part of July, and worked at the wreck of the old wharf boat, which had encumbered the Memphis landing. This wreck and that of the Jeff. Thompson were worked upon until they seemed to be destroyed, as well as could be judged in the high stage of water then obtaining. I doubt, however, if it is, in all cases, possible to clear away a wreck without the aid of the powerful machinery of the snagboats in addition to the shattering first done by the torpedoes.

While working in the vicinity of Memphis the crew suffered greatly from sickness, incidental to the climate. My assistant gratefully acknowledges, under date of 15th of July, the important and timely aid rendered by Assistant Sur-

geon Tremaine, of the medical department United States army.

The party continuing to suffer from sickness, the steamer was brought up to the mouth of the Ohio, where I visited it as soon as I could, and on the 5th of August reported to the Chief of Engineers the condition of the party, and requested authority to send the boat up the Mississippi to work at the removal of the wreck of the Northern Light, in Coon Slough, being in the district allotted to General Warren, who, not being yet prepared to undertake that piece of work, had expressed to me a wish that the wrecking boat could be sent thither for

that purpose. My request was approved and granted, and the work has been

done, as reported by me in a letter of the 20th of September, 1867.

In the course of the month of April, it being evident that a long time must elapse before any of the boats authorized to be built and equipped for this service could be in readiness for use, I was directed to advertise for proposals for boats fully equipped with crews and machinery to commence the work of clearing away the snags and other impediments of that sort in the upper waters of the Arkansas and Missouri rivers. I accordingly published the required notice in the latter part of April, and on the 28th of May, 1867, reported the result of that advertising to the department. The bids were deemed to be excessive, and I was ordered to advertise again. On the 31st of July I reported the result of this second advertising, and on the 7th of August I was authorized to conclude the contracts under these last bids.

These contracts were approved on the 19th of August, and the contractors are now at work upon the Arkansas river with the steamer North West, and upon the Missouri river with the steamer Underwriter, on board of each of which steamers there is a supervisor or inspector, in the employment of the government, whose duty it is to make reports to this office in writing once a

week, giving a full account of the work done.

Abstracts of the bids and contracts for this sort of work are herewith sub-

mitted, marked No. 1, No. 2, and No. 3.

In order to ascertain where work upon the Missouri river was most needed, I sent Brevet Major C. W. Howell, with instructions dated 24th of June last, to make a careful examination, and report upon that river from Fort Benton to its mouth. In this duty he is assisted by Major F. V. Werner, late of the New York 1st volunteer artillery, who is employed by me for this purpose.

Major Howell has not yet returned, and it will be some time after his return

before his report can be prepared. It will, however, be finished before the new

boats can be got ready to avail of the information which it will afford.

The improvement of these great western rivers is clearly a work which does not admit of permanent completion, for the channels which are in use during a season or more may be suddenly abandoned by the rivers in the course of the changing stages of the water. The rivers cut out for themselves new channels, which, in their turn, require to be freed from snags to render them safe for navigating; their improvement must, therefore, be carried on with successive annual appropriations for working the snag-boats, and for constructing such additional oues as may be shown to be necessary from the experience to be gained by the use of those now in the course of construction.

From the lists of dangers to the navigation herewith submitted, it is apparent that there is enough to be done to employ snag-boats and their working parties for many years to come. (See lists marked H and K, also L, herewith submit-

ted.)

It was suggested by the convention of steamboat masters and owners held at St Louis, Missouri, on the 14th and 15th of May last, that the establishment of lights by the general government at points where dangerous obstructions exist, or long crossings are required to be made, would contribute greatly to the safety of lives and property. This is doubtless true, and where the obstructions consist of permanent rocky ledges or reefs is well worthy of attention.

The amount of commerce and navigation to be benefited by the carrying on of these improvements is the whole great inland commerce of the west, and the navigation of all the great western rivers, which greatly affects the commerce of the whole United States, the facilitating of which will promote that of the world.

The amount that can be profitably expended upon these improvements in the next fiscal year is shown in the estimates which I have forwarded to the engineer department.

The experience to be gained by the practical working of the means and appliances about to be used in the removal of obstructions can alone determine to what extent it may be advisable to modify these estimates in the future.

All of which is most respectfully submitted by your most obedient servant,

J. N. MACOMB,

Colonel Engineers, Brevet Colonel United States Army.

Brevet Major General A. A. HUMPHRBYS,

Brigadier General Commanding Corps of Engineers, United States Army, Washington, D. C.

A.—Abstract of proposals for hulls and attachments for three double-hulled steam snag-boats.

No.	Name of bidder.	'Residence.	For one beat.	For two boats.	For three boats.
1 2 3 4 5 6 7 8	Morton & Startzman	do do New Albany, Ind Jeffersonville, Ind dodo	28,500 26,300	\$51,000 49,800 48,800 51,100	73,000

B.—Abstract of proposals for boilers, engines, and machinery, for three double-hulled steam snag-boats.

No.	Name of bidder.	Residence.	For one boat.	For two boats.	For three boats.
1	Niles Works		\$32,000	\$64,000 78,200	\$96,000 115,800
2 3 4	Malley & Stemler	do	39,600 49,000 28,500	57,000	85,500
5	Miami Machine Works Ainslee, Cochrane & Co	do	50,000 30,000	57,500	85,500
7 8	Denis Long	New Albany, Ind .	26, 500 37, 000		
9	Neal Manufacturing Co	Madison, Ind	28,500		

C.—Abstract of proposals for joiner work and cabins for three double-hulled steam snag-boats.

No.	Name of bidder.	Residence.	For one boat.	For two boats.	For three boats.
1 2 3 4 5	Hill, Hewitt & Co	New Albany, Ind . Cincinnati, Ohio . Jeffersonville, Ind . Cincinnati, Ohio . Madison, Ind	\$7,500 7,500 8,000 9,644 7,000	\$14,500	

D.—Abstract of proposals for tackle, blocks, and cordage, for three double-hulled steam snag-boats.

No.	Name of bidder.	Residence.	Blocks, spars, and derricks for one boat.	Нешр горе.	Wire tiller rope.	
1 2 3	Deacon & Depew	New Albany, Ind Louisville, Ky Cincinnati, Ohio	\$1,662 00 1,471 00 1,606 50	Per lb. \$0 25 23 25	Per lb. \$0 60	

E.—Abstract of proposals for painting and glazing for three double-hulled steam snag-boats.

No.	Name of bidder.		For two	o boats.	boat nd.	For three boats.
		Residence.	One at Cincinnati, Ohio.	One at Madison, Ind.	For one bat New bany, Inc	
1 2 3	W. O. Williams Hill, Hewitt & Co Riggs & Murray D. J. Williams & Co.	Louisville, Ky New Albany, Ind . Cincinnati, Ohiodo	\$1,500	\$2,500	\$2,850 2,043	\$7,200
5	Vance & Armstrong. D. J. Williams & Co.	Madison, Ind	1,800	3,500 1,800		41,200

F.—(Miscellaneous.)—Abstract of proposals for iron for chains, making chains, and salt for salting hulls for three double-hulled steam snag-boats.

No.	Name of bidder.	Residence.	Iron.	For making chains.		w ba
				2-inch.	1-inch.	Kanawha salt.
1 2 3 4 5	Swift's Iron and Steel Works J. W. Malinee Henry Werry A. H. Moore Hedrick & Laidly	Louisville, Ky Cinciunati, Ohio		Per lb. \$0.05 05 05	Per lb. \$0 07 06 08	Per bush.

G.—Abstract of contracts made on account of the construction of snag-boats and apparatus for clearing western rivers.

t. Remarks.	00 00 00 00 00 00 00 00 00 For second tiller rope and shives, \$735; hemp	88488
Amount.	24, 800 88, 500 88, 500 7, 500 7, 500 7, 100 1, 130 1, 130	3,600 2,043 1,500 11,500
Designation.	May 20, 1867 Hulls for two snag-boats. May 20, 1867 Hull for one anag-boats. May 20, 1867 Machinery for one snag-boat. May 22, 1867 Machinery for one snag-boat. May 20, 1867 Machinery for one snag-boat. May 21, 1867 Cabin and joiner work for one snag-boat. May 22, 1867 Cabin and joiner work for one snag-boat. May 22, 1867 Cabin and joiner work for one snag-boat. May 20, 1867 Cabin and joiner work for one snag-boat. May 20, 1867 Blocks, tackle and cordage for three snag-boats.	Cincinnati, Ohio. July 31, 1967. Painting two snag-boats. New Albany, Ind. Aug. 12, 1967. Painting one snag-boat. Cincinnati, Ohio. July 29, 1867. Iron for chains for three snag-boats. Cincinnati, Ohio. July 29, 1867. Making chains for three snag-boats. Cincinnati, Ohio. 300 barrels Kanawha salt for hulls.
Date of contract.	May 20, 1867 May 20, 1867 May 20, 1867 May 22, 1867 May 21, 1867 May 21, 1867 May 21, 1867 May 22, 1867 May 20, 1867	July 31, 1867 Aug. 12, 1867 July 29, 1867 July 29, 1867
Residence.	Cincinnati, Obio New Albany, In Cincinnati, Obio Madison, Ind Louisville, Ky Cincinnati, Obio New Albany, In Madison, Ind Louisville, Ky	Cincinnati, Ohio New Albany, Ind Cincinnati, Ohio Cincinnati, Ohio Cincinnati, Ohio
Name of contractor.	Marine Railway and Dock Co. William Jones. C. T. Dumont. Neal Manufacturing Co. Denis Long. Morton & Startzman. Hill, Rewitt & Co. Vance & Armstrong.	D. J. Williams & Co. Hill, Hewitt & Co. Swift's Iron and Steel Works. Henry Wery.
No.	-80470500	511224

("Approximate,) a inch chain, 6 cents per pound; 2 inch chain, 5 cents per pound. (Approximate,) 50 cents per bushel.

H.—List of some of the obstructions to the navigation of the Arkansas river, from its mouth to Little Rock, including White River cut-off.

Entering White river about ten (10) miles above Napoleon, on the Mississippi, to go into Arkansas river through the cut-off, a distance of eighteen (18) miles; up White river, for twelve (12) miles before reaching cut-off some few snags and logs; also the wreck of the steamer Lady Jackson; through the cut-off, a distance of six (6) miles, very loggy and full of snags. In low water very difficult to make.

On entering the Arkansas river at Napoleon, there are fifteen (15) bends up to head of cut-off, a distance of thirty (30) miles, a great many snags, and three rack heaps, in the way. There are snags and logs in all the bends, some more difficult than others to navigate, especially after night; also two or three wrecks

of steamers. No towns, but a great many landings, in that distance.

From the head of the cut-off to Red Forks, a small place on the left-hand side of the river, a distance of twenty (20) miles, there are ten (10) bends. River crooked and snags all the way, and all the crossings in low water get very shoal. Channel narrow, and at extreme low water only navigable for small steamers. Three (3) wrecks of steamers sunk a long time; names of them I do not know. Breaks can be seen in low water from Red Fork to Arkansas Post. The next important place is ten (10) miles; some few landings between. The river considered very good, although in low water a great many bad breaks to be seen. On up the river for five (5) miles you come to Smith's cut-off, a very bad place for logs and snags. On up the river some few snags in the way.

Up to Floyd Smith's landing, a distance of four (4) miles, going on up, snags all the way for seven (7) miles. There you come to the new cut-off, called "Moore's or Smith's Bend cut-off," very crooked and snaggy. Some five (5) miles above you come to Forroley's bend; all snags; a narrow track for a steamboat. From Forroley's cut-off to Pine Bluff, a distance of eighty-nine (89) miles, there are thirty-six (36) bends; all of the bends have snags in, and in low water the river is shoal, excepting in the bends among the logs, which makes the channel very crooked. In this distance there are a great many high snags, of all shapes and sizes; a great many of them we ran over going down stream after night.

Along this one hundred and seventy (170) miles, from the mouth of the river to Pine Bluff, there are upwards of two hundred (200) landings, including woodyards; there are thirteen (13) wrecks of steamers and barges sunk, that I know of. Only few of them are in the way; some of them show breaks in low

water, and some of them are covered up in the bars.

From Pine Bluff to Little Rock, a distance of one hundred and ten (110) miles, there are thirty-three (33) bends and very crooked low-water channel, narrow, and in the crossings always changing; the same nature as the lower part of the river; snags and logs all the way, and bad bars. There are four (4) bluffs come in on the left-hand side (going up) of the river, called "Dug's Bluff," "Yellow Bluff," "White Bluff," and "Red Bluff."

In this distance of one hundred and ten (110) miles, there are upwards of ninety (90) landings, wood yards included, and fourteen (14) wrecks of steamers and wrecks that I know of. Several of these are very much in the way, and some are covered up in the sand, and some of the breaks can be seen in low water.

The distance from mouth of river to Little Rock I call two hundred and eighty (280) miles. All pilots have their own distance is why I mention this.

EUGENE G. HANDLAN.

Cincinnati, New Orleans, and Arkansas River Pilot.
[Steamer "Berths."]

J. M. MACOMB,

A true copy:

Colonel Engineers, Brevet Colonel U. S. A.

No. 1.

Abstract No. 1, on account of "improving Mississippi, Missouri, and Arkansas rivers." Proposals for sale to the United States of one side-wheel steamer for removing wrecks, &c., in the above rivers.

No.	Name of bidder.	Town—	State.	Date of advertisement.	Amount.	Remarks.
1	Thomas Murdock.	Cincinnati.	Ohio.	March 11, 1867	\$21,000	Only bid received.

Abstract No. 2, on account of improving Missouri and Arkansas rivers, proposals for furnishing boats and equipment for the removal of snags and other obstructions from the upper channels of said rivers by contract.

No.	Name of bidder.	Under advertise- ment dated—	Residence.	Missouri river.— Price per day of I steamer, crew, and equipment of ten hours.	Arkansas river.— Price per day of I steamer, crew, and equipment of ten hours.
		1867.			
1	A. Covey	April 27.	Post Perry, Pa.	\$800 per day.	
2	Thomas Murdock	April 27	Cincinnati, Ohio.	240 per day.	
3	Cincinnati Wrecking Boat Company.	April 27	Cincinnati, Ohio.	300 per day.	
4	A. B. Hopkins	April 27	Cincinnati, Ohio.		\$175 per day.
5	Arkansas and White River Wrecking Boat Company.	April 27	Cincinnati, Ohio.		170 per day.
6	Thomas Murdock	April 27	Cincinnati, Ohio		245 per day.
1	Cincinnati Wrecking Boat Company.			\$275 per day.	
2	Arkansas and White River Wrecking Boat Company.		Cincinnati, Ohio.		\$160 per day.

Abstract No. 3, of contract made on account of improving Mississippi, Missouri and Arkansas rivers.

No.	Name of contractor.	Residence.	Date of contract.	Designation.	Amount.
1	Thomas Murdock	Cincinnati, Ohio.	March 29, 1867.	One side-wheel steamer for removal of wrecks,	\$21,000.
2	Cincinnati Wrecking Boat Company.	Cincinnati, Ohio.	Aug. 14, 1867.	Three months' snag- ging, &c., in Missouri river.	\$275 a day.
3	Arkansas and White River Wrocking Company.	Cincinnati, Ohio.	Aug. 13, 1867.	Four months' snag- ging, &c., in Arkan- sas river.	\$160 a day.

K.

620 OLIVE STREET, ST. LOUIS, MISSOURI, September 12, 1867.

Sir: In accordance with your request, I beg leave to submit the following report of the present condition of the Mississippi river from St. Louis to Cairo. In making up this report, I have only taken notice of snags, breaks, and wrecks that are obstructions and dangers to navigation at the present time. The channel of the river is constantly shifting from place to place, and with every change other snags, &c., not mentioned in this report, will become obstructions.

This river is now ten feet above low-water mark, and falling at the rate of three inches in twenty-four hours. There is seven feet water in the channel hence to Cairo; as the river falls the channel will shift and cut out, and the dangers (snags, breaks, &c.) will become more numerous.

Distance from St. Location. Obstructions. Louis in miles. White Cloud Foot of Ann street, St. Louis, one wheel four feet dry, and break to the right of wheel, which I suppose to be the bow of the Belle Memphis. 4 breaks Head of Quarantine island. In crossing to Cahokia bend. 1 snag. Opposite La Pere towhead. 7 miles . 1 break Two above and four just below Jim Smith's; caving-in bank, with heavy timber on it. 20 miles 6 snags..... 33 miles Cornish island. 3 snags... 3 snags... Rush tower. 40 miles 4 snags.. 41 miles Rush crossing. 4 breaks Do. 44 miles Rush Island bend, with caving bank and 6 snags..... heavy timber. Head of Fort Charter's island. 48 miles 1 wreck..... Head of Fort Charter's island, caving bank 8 snags..... and heavy timber. 60 miles ... Head St. Genevieve towhead. 1 snag... 3 breaks Do. 4 snags...... Foot of St. Genevieve bend. Foot of St. Genevieve bend, caving bank and heavy timber. 65 miles Kaskaskia bend, caving bank and heavy 6 snags. timber.

25 w----Vol. ii

Obstructions, &c., in Mississippi river-Continued.

66 miles	2 snags	Kaskaskia island.
		Do.
/1 miles	3 snags	Salive island, caving bank. Pratt's bend.
	8 snags 1 wreck	Pratt's bend, caving bank, heavy timber.
82 miles	3 breaks	Mary's river.
84 miles	3 snags	Manscoe's island.
84 miles	2 breaks	Do.
88 miles	3 snags	Head of Liberty island.
88 miles	2 breaks	Head of Liberty island, caving bank.
92 miles	1 snag	Sheep island.
92 miles	3 breaks	Do.
98 miles	3 snags	Below Hening's, caving bank.
104 miles	4 snags	Wilkinson's island, caving bank.
107 miles	3 snags	Kelly's landing, caving bank.
110 miles	1 break	Hat island; (this break cost the steambo interest over \$100,000 last fall.)
120 miles	10 snags	Around Tower island.
	12 breaks	Around Tower island, caving in bank a
100!	2 hasalas	heavy timber.
130 miles	3 breaks	Neely's. Devil's Tea-table.
132 miles	2 breaks	Do.
139 miles	4 wrecks	Bainbridge's.
140 miles	4 snags	Key West Point.
142 miles	3 snags	Kinney Point, caving bank.
143 miles	6 snags	Devil's island, caving bank and heavy timb
150 miles	4 snags	Cape Giradeau bend.
166 miles	4 snags	Head of Powers's island.
167 miles	3 snags	Foot of Burnam's island.
169 miles	1 snag	Goose Island landing.
180 miles	2 snags	Corner Buffalo island.
Ĭ	1 break	Do.
185 miles	1 wreck	Head Dog-tooth bend.
156 miles	2 snags	Dog-tooth bend above Thompson's.
	4 breaks	Dog-tooth bend at Thompson's.
	2 snags	Dog-tooth bend below Thompson's; one the snags and two of the breaks bel- Thompson's are very difficult (on accou
	1	of the narrow channel) to pass in safet
187 miles	1 snag	snags four feet in diameter. Sister island.
190 miles	18 snags	Greenleaf's bend, bank caving in rapid very heavy timber.
192 miles	4 snags	Missouri Point below Sister island.
194 miles	2 breaks	Elk island.
194 miles	1 snag	Do.
197 miles	3 breaks	Head Bird's towhead.
198 miles	6 breaks	Fort Bird's towhead.
	4 spags	Do.
200 miles	1 snag	Mouth of the Ohio river.
200 miles	1 rock	Do.
	1 rock	At the foot of the Grand chain, called t
		Paul Jones Rock. The Totten can ta these two rocks and the big snag at the fo of Dog-touth.

Pilots from below report having sounded and passed over the place where the Jeff. Thompson lay, and find no remains of that vessel. When the Totten was at the wrecks of the White Cloud, Belle of Memphis, and Jeff. Thompson, she worked under many disadvantages—high water, rapid current, &c.

The Jeff Thompson's casements were over three feet solid oak, locked and interlocked; her head solid and well fastened for ramming. After what she (the Totten) has accomplished under such unfavorable circumstances, what can she not do with a fair chance?

In the above report you will find eight wrecks of steamers, 63 breaks or snags under water, and 137 dry snags that are now obstructions or dangers to navigation between this place and the junction of the Ohio and Mississippi rivers. Truly a formidable array; but when this river has fallen six or seven feet more, the number of snags and breaks will be more than doubled.

Those caving in banks, with heavy timber, is the principal cause of snags; the heavy roots of trees are a sufficient anchor to hold until the tree becomes firmly bedded. But I fear to trespass, and will close. I arrived here at six p. m., and leave at daylight in the morning for Helena, Arkansas, consequently I have had but very little time to write up the above report.

I shall take notes on my trip to Helena and return, and if they would be of

any service to you, would be pleased to send them.

Your learning and experience forbid my making suggestions until called for. I have had some experience in anagging, (as we called it on the old boats,) and do not hesitate to offer my services to you in any capacity.

I will be absent some eight or ten days on this trip, and on my return, with

your permission, will write again.

I have the honor to be, very respectfully, your obedient servant,

JOHN S. TENNYSON,

St. Louis and New Orleans Pilot.

Colonel JOHN N. MACOMB, Corps of Engineers U. S. A., Cincinnati, Ohio.

A true copy:

J. N. MACOMB, Colonel Engineers, Brevet Colonel U. S. A.

L.

List of steamers sunk in the Mississippi river between St. Louis, Missouri, and Cairo, Illinois.

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*Belle Memphis, by ice, foot of Ann street, St. Louis, Mo.
*White Cloud, struck wreck, foot of Ann street, St. Louis, Mo.
*Governor Sharkey, overloaded, foot of Ann street, St. Louis, Mo. Shepherdess, struck break, Cahokia bend.
                   -, opposite La Pere river.
                 -, opposite Jefferson Barracks.
Virginia, struck break, head Carroll's Island.
Fisher, struck break, foot Carroll's Island.
                    , opposite Cave hollow.
Elvira, struck break, foot Widow Beard's island.
                  ·, at Foster's.
Tonaleuka, struck break, foot Widow Beard's island.
Missouri, burned, above Merrimac.
                  -, above Merrimac.
                   , at Widow Waters'.
Windsor, struck break, opposite Sulphur springs.
Amazon, struck rocks, opposite Rattlesnake springs.
                  , Harlow's.
Robert Fulton, rocks, Platin rocks.
Emma Boyd, break, opposite Tilly's.

*Wm. L. Ewing, break, opposite Tilly's.

*_____, break. Wreck of Ewing, opposite Tilly's.

Dunkirk, break, opposite Rush island.
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-, break, opposite Rush island.
West Wind, break, opposite Rush Tow head.
_____, head Fort Charter's island.
Cambria, break, head Turkey island.
Corsair, break, foot Turkey island.
Rienzi, wreck of Corsair, foot of Turkey island.
Yucatan, wreck of Rienzi, foot Turkey island.
Atlants, break, foot Turkey island.
Rubicon, break, foot Turkey island.
Sultan, burned, St. Genevieve.
Omega, break, St. Genevieve.
                    head St. Genevieve island.
Wyacondia, break, St. Genevieve bend.
Leander, break, head Kaskaskia island.
J. M. Convers, ice, Ferry island.
Columbus, old age, Saleve island.
Naugatuck, break at Saline, St. Mary's landing.
*Thos. H. Larkin, burned, Pratt's bend.
                  -, head Horse island.
Vermillion, rock, above Mary's river.
Anglo-Saxon, rock, below Mary's river.
Little Franklin, break, Manscoe's island.
Defiance, break, at liberty.
                    , foot Liberty island.
Tennessee Valley, break, big Eddy.
Glasgow, collision, Devil's Back-bone.
                   , rocks, at Wilkinson's.
*Honduras, break, Kelly's landing.
——, break, opposite Kelly's landing.

Duke of Orleans, burnt, foot Lacour's island.

Maid of Orleans, break, head Hat Island.
General Jessup, wreck United States, head Hat island.
United States, break, head Hat island.
New York, break, head Hat island.
Hamlet, break, head Hat island.
Manona, break, head Hat island.
New York, break, head Hat island.
Jas. E. Woodruff, break, head Hat island.

    head Fountain bluff.

                 —, opposite Kount's landing.
—, head Tower island.
Meridian, break, opposite Upper Mouth Muddy.
Georgetown, break, below Upper Mouth Muddy.
Walk-in-the-Water, old age, Sheffield.
Gondolier, collision, Preston.
                     -, Crawford.
Loyune, -
*Olive, snag, Devil's Tea-table.
*_____, snag, Devil's Tea-table.
Wiota, break, Sablett's.
Missouri Belle, break, Bainbridge.
*Chester, break, Bainbridge.

*General Pike, Wreck Chester, Bainbridge
Key West, break, Key West Point.
*Union, break, opposite Sheppard's.
Swiftsure No. 3, break, opposite Sheppard's.
Kate Kinney, break, Kinney point.
_____, above head Devil's island.
Fort Pitt, break, around Devil's island.
Jas. Montgomery, break, Devil's island.
John Swasey, wreck Montgomery, Devil's island.
*Bell Golding, break, Devil's island.
Lasalle, break, foot Devil's island.
Jas. Robb, supposed rocks, foot Devil's island.
India, powder, Cape College.
Talisman, collision, foot Cape Girardeau bend.

———, foot Cape Girardeau bend.
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J. M. White, rocks, Grand chain. Homer, rocks, Grand chain. Wm. Parish, rocks, Grand chain. John Hancock, rocks, Grand chain. *Paul Jones, rocks, Grand chain. Alhambra, rocks, Beaver dam. Reanoke, rocks, Beaver dam.

A. M. Phillips, break, Doolan's slough

———, break, Doolan's slough. New Orleans, break, foot Burnam's island. Aunt Letty, break, foot Burnam's island. Paragon, break, Illinois side Goose island.

———, break, Illinois side Goose island. , break, Illinois side Goose island. , break, Illinois side Goose island. J. P. Tweed, break, Illinois side Goose island. Mentor, break, Mentor point. *Felix Grundy, foot Goose island. *Rowena, break, Buffalo island. , break, Buffalo island. -, break, Hucker's bend. -, break, Hucker's bend. -, ice, Widow Brooks's. -, ice, Widow Brooks's. Saladin, break, head Dog-tooth bend. -, head Dog-tooth island. Orient, break, in Dog-tooth bend. Narraganeett, break, foot Dog-tooth bend. Manhattan, break, foot Dog-tooth bend. Swatara, collision, head Missouri Sister. Sam Lay, break, Illinois Sister. Boston, ice, 100t Auto Eliza, sneg, Eliza Point.

Mound City, want of caulking, opposite Eliza Point.

Daniel Pollard, ——, Eliza Point. Boston, ice, foot Able's towhead. Ben. Lewis, explosion, junction Ohio and Mississippi rivers. Grand Tower, Cement rocks, junction Ohio and Mississippi rivers. Glendy Burke, Cement rocks, junction Ohio and Mississippi rivers. Chancellor, Cement rocks, junction Ohio and Mississippi rivers.

The above list is made from memory; the names of some of the boats are unknown or forgotten, left blank. These marked * are obstructions to navigation. Break is a hidden obstruction, stumps, logs, &c., under water.

I have a full list, but it is not with me at present. Very respectfully, your obedient servant,

JOHN S. TENNYSON. St. Louis and New Orleans Pilot.

A true copy:

J. N. MACOMB. Colonel Engineers, Brevet Colonel U. S. A.

APPENDIX H.

OFFICE OF WESTERN RIVER IMPROVEMENTS, Cincinnati, Ohio, February 28, 1867.

GENERAL: I beg leave to submit herewith the report of my chief assistant, Brevet Major C. W. Howell, in relation to the removal of wrecks in western rivers, which was alluded to in my letter of 21st instant as in the course of preparation.

It will be seen by this report that a suitable boat can probably be purchased

for about \$20,000, and fitted for the service for about \$5,000, in addition to which would be the cost of a diving-bell, or submarine armor. The gunpowder for this service could, doubtless, be furnished from the public supplies now on hand.

Instead of using Beardsley's exploding apparatus, I should prefer to avail myself of the benefit of the recent researches which, I understand, have been

made by Brevet Major King, of the corps of engineers.

Believing it to be of the utmost importance that the chief wrecks named in the report should be removed as soon as possible, I would respectfully request to be authorized to apply the requisite funds from the appropriation for the "improvement of the Mississippi, Missouri, Arkansas, and Ohio rivers," for carrying out the project now submitted.

I remain, very respectfully, your most obedient servant,

J. N. MACOMB,

Lieutenant Colonel Engineers, Brevet Colonel U. S. A.

Brevet Major General A. A. HUMPHREYS,

Brigadier General and Chief of Engineers,

Washington, D. C.

Office Western River Improvements, February 28, 1867.

Report of Brevet Major C. W. Howell, captain engineers, upon the removal of wrecks in the Mississippi river, &c., approved, with the slight modifications named in the accompanying letter of this date, with which this report is now respectfully submitted to the Chief of Engineers.

J. N. MACOMB.

Lieutenant Colonel Engineers, Brevet Colonel U. S. A.

OFFICE WESTERN RIVER IMPROVEMENTS, Cincinnati, Ohio, February 28, 1867.

COLONEL: In obedience to your orders of the 21st instant, I have the honor to make the following report, viz., first, as to the number and location of wrecks in the Mississippi river; second, the manner in which it is proposed to remove them; third, the boats and tackle necessary; fourth, the comparative cost and advantages of chartering and of buying a steamboat:

First. Accompanying this report I send a list of wrecks, for which I am indebted to Mr. A. G. Hiner, a well-known river pilot. Although the list is incomplete, there is enough shown to make the importance and extent of the work

appear.

Second. To remove the wrecks, it is proposed either to charter or buy a steamboat and furnish it with torpedoes and electrical machine for blowing up the obstructions, so that, either by lifting with powerful cranes or by pulling, they

may be removed piece by piece.

Third. The torpedoes will be simply tin cans of gunpowder; the exploding apparatus, Beardsley's; the cranes worked by steam; also a double-geared capstan forward and a hand capstan aft; diving-bell and armor. The boat should be of light draught, strongly built, with powerful engines and large wheel. For particulars of machinery I will refer you to the report of Mr. Pierce, submitted herewith.

Fourth. I have been unable to find a boat suitable for this service that can

be chartered, and I do not think it would be advisable to make further efforts in that direction, for the following reasons:

1st. The insurance on a boat licensed to carry gunpowder will be very heavy, and, as the owner will insist upon insurance, the expense must be added to the charter.

2d. I know of no boat on the river properly arranged for this service. There must be a clear deck in front of the chimneys, and the hull forward must be strengthened by strong timber bulkheads. The first will necessitate the cutting away the forward cabin of the boats here in use, and the second will fill up the forward hold so as to render it almost useless for freight.

3d. A wrecking boat will be needed for the next five years, and will in that time pay for itself several times over; that is, if we are to judge from the aver-

age charters paid here during the war.

Accompanying this report you will find a list of steamboats now offered for sale; also a general description of each boat. I have informed myself as well as possible about each, and would recommend the side-wheel boat Commodore as the one best suited for the service. The report of Mr. Pierce will give you her description in detail.

I have examined the ferry-boats in use here, and find them in very bad condition. They are not calculated to run against a stiff current, and are deficient

in power. I would recommend them as worthless for our use.

I have the honor to be, colonel, very respectfully, your obedient servant, C. W. HOWELL,

Captain Engineers, Brevet Major U. S. A.

Brevet Colonel J. N. MACOMB, Corps of Engineers, Colonel U. S. A.

List of boats offered for sale.

	Price.
Commodore, side-wheel, (see report of C. E. Pierce,) about	\$ 20,000
Bertha, stern-wheel; 3 boilers. 24 feet by 36 inches; 15-inch cylinders,	
5-foot stroke; length, 165 feet; beam, 32 feet; about	15,000
Argosy, stern-wheel; 3 boilers; 16-inch cylinders, 5-foot stroke; length,	
165 feet	20,000
Nymph, stern-wheel; 10-inch cylinders; length, 149 feet; not ser-	
viceable	6,500
Delaware, stern-wheel, at Pittsburgh, (not first class;) dimensions	
about same as Bertha	10,000
Charleston, stern-wheel, at Memphis, (not first-class;) dimensions	
about same as Delaware	14,000

List of wrecks between St. Louis and Vicksburg.

Belle Memphis lies between St. Louis and Carondelet.

Hat island, opposite Wittenberg, a number of old wrecks not now in the channel, but may be, on account of channel shifting.

Montgomery lies in the channel near Devil's island; room enough for boats

passing.

Paul Jones lies in Grand chain, between Thebes and Commerce, Missouri.

Goose island, one wreck; not now in the channel; was formerly.

B. M. Runyon, James White, and Fred Tron, island No. 10. These wrecks lie between New Madison and Point Pleasant. The Fred Tron and James White are considered dangerous.

Sunnyside lies at island No. 16; not a channel obstruction.

Ben Stickney lies between islands No. 20 and No. 21, near the shore. H. D. Newcomb, near Hale's point, Tennessee; not an obstruction.

Old wreck at Plum point, in the channel.

Niagara and Empress lie at island No. 34; also one old wreck. These, however, do not now obstruct the channel, but may on account of shifting.

Rebel gunboat Jeff. Thompson lies between Memphis and island No. 46,

in the channel.

City of Memphis in the channel near island No. 52.

Di Vernon lies near island No. 69.

Wrecks near Columbia, Arkansas, not now in the way.

Wreck near island No. 103, not dangerous.

OFFICE WESTERN RIVER IMPROVEMENTS, Cincinnati, Ohio, February 28, 1867.

COLONBL: In accordance with your orders dated February 25, 1867, I have the honor to state that I proceeded to Carrollton, Kentucky, and made a thorough examination of the side-wheel steamer Commodore, the result of which is as follows:

Extreme length on deck, 175 feet; depth of hold, four feet six inches; breadth of beam, twenty-eight feet; one capstan, worked by hand; tounage, 348 tons;

full-length cabin, with twenty-eight state-rooms and office.

Machinery.—Three double-flued boilers, twenty-two feet long, thirty-eight inches in diameter. Cylinders nineteen inches in diameter, stroke six feet. One doctor engine and pumping apparatus complete. Diameter of wheels twenty-seven feet.

Draught of water.—Forward, sixteen inches; midships, twenty-two inches;

aft, nineteen inches.

Condition of hull.—The timbers are 3½-inch by 5-inch oak, sound and in good order, the planking is good and well fastened, the deck beams all sound and well put in, deck plank sound and in good condition, the cabin and upper works are all in good order and well painted, steering apparatus in perfect order. She is between two and three years old, and appears to be in perfect running order;

machinery in good repair and works well.

In the event of the purchase of the above steamer, I would suggest the following alterations and additions: To get necessary room to work advantageously forward, the boiler and hurricane decks should be cut off forward of the chimneys. Two bulkheads should be added fore and aft, and hull should also be sheathed nearly one-half its length inside, for the purpose of supporting the strain that may occur in the use of cranes. Two cranes and one diving-bell, and all necessary machinery for use of same. One steam capstan should also be added forward, and the hand capstan now on the boat be placed aft.

Five thousand dollars will be the approximate cost of the above repairs and

additions.

Respectfully submitted:

CHARLES E. PEIRCE, Assistant.

Colonel J. N. MACOMB, United States Engineers.

Schedule of approximate cost of working a wrecking boat per month.

WAGES.

One pilot	\$250
One mate and 2d mate	230
One steam-engineer and 2d engineer	

One carpenter \$100	
Two diving-bell men	
Two firemen 100	
Twelve laborers, \$40 each	
One cook and assistants	
One cabin boy	
	\$1,835
One blacksmith	100
Total wages	1, 935
Fuel	750
Oils, &cc	50
	2,735
Stores	265
Total working expenses	3,000
	====

Respectfully submitted:

CHARLES E. PEIRCE, Assistant.

H 2.

ENGINEER DEPARTMENT, Washington, April 17, 1867.

SIR: I respectfully transmit herewith a copy of a letter from Colonel J. N. Macomb, corps of engineers, with schedules of bids received for building double-hulled snag-boats for the western rivers.

Separate proposals were submitted as follows:

For hulls and attachments.

For joiner work and cabins.

For boilers, engines, and machinery.

For painting and glazing.		
For tackle, blocks, and cordage.		
The lowest bidders for hulls and attachments are:		
William Jones, of New Albany, Indiana, for one hull, &c., at Marine Railway and Dock Company of Cincinnati, Ohio, for two	\$24,000	00
hulls, &c., each at	24, 400	00
The lowest bidders for joiner work and cabins are:		
Vance & Armstrong, Madison, Indiana, for one boat, at	7, 500,	00
Morton & Stratzman, Cincinnati, Ohio, for one boat, at	7,500	00
Hill, Hewett & Co., of New Albany, Indiana, for one boat, at	7, 500	00
The lowest bidders for boilers, engines, and machinery are:		
Dennis Long, of Louisville, Kentucky, for one boat, at	26, 500	00
C. T. Dumont, of Cincinnati, Ohio, for one boat, at	28, 500	00
Neal Manufacturing Company, of Madison, Indiana, for one boat, at	28, 500	00
The lowest bidders for tackle-blocks and cordage are:		
J. J. Hair, of Louisville, Kentucky, for blocks, spars, &c., each	1,471	00
For hemp rope, &c., per pound		23
Deacon & Depere, of New Albany, Indiana, for tiller rope, per		
pound		60
I recommend that the contracts he awarded to the shove on	the requisi	ita

I recommend that the contracts be awarded to the above, on the requisite security being furnished.

The lowest bidders for painting and glazing are:

Riggs & Murray, Cincinnati, Ohio, who propose for three boats \$4,500 00 I recommend that their bid be accepted, provided they will do the work for one boat at New Albany or Louisville, and furnish the required security, &c.

If they will not do the work at New Albany or Louisville, then the next lowest bidders for one boat are Hill, Hewett & Co., of New Albany, Indiana,

who propose at \$2,043, to whom the contract may be awarded.

It will be observed that Colonel Macomb, in view of difficulties which he suggests, recommends that all the bids be rejected; but as this, in the opinion of this department, would not be in accordance with the provisions of the law, his recommendations are not concurred in.

Very respectfully, your obedient servant,

A. A. HUMPHREYS,

Brig. Gen. and Chief of Engineers, Maj. Gen. Vols.

Hon. E. M. STANTON, Secretary of War.

MAY 6, 1867.

Report of Chief of Engineers approved.

E. M. STANTON, Secretary of War.

OFFICE OF WESTERN RIVER IMPROVEMENTS, Cincinnati, Ohio, April 6, 1867.

GENERAL: I herewith transmit abstracts of the bids received, under the following heads, for the construction of snag-boats:

Hulls and attachments; joiner work and cabins; boilers, engines, and ma-

chinery; painting and glazing; tackle, blocks and cordage.

On examining the bids it will be found that they come from the following named places, viz: Cincinnati, Ohio; Madison, Indiana; Jeffersonville, Indiana; Louisville, Kentucky; and New Albany, Indiana; and that whilst the lowest bid for hulls and attachments is in New Albany, Indiana, the lowest bid for machinery is in Louisville, Kentucky; the lowest bid for cabins and joiner work is in Madison, Indiana; and the lowest bid for painting is in Cincinnati, Ohio. I find, on inquiry of several of the parties making bids, that their expectation is to do the work at their own places of residence or of business, so that if I have the hulls built at New Albany I must bring them up the river to Louisville to be fitted with machinery, and thence up to Madison, Indiana, for cabins, and thence to Cincinnati to be painted. This would certainly lead to great delay and trouble, and to expense which no bidder has included or considered in his bid; and yet the law requires the work to be given to the lowest responsible Now if "responsibility" means money responsibility, to be determined by the bonds which the bidders offer as security, all would seem to be on a par, for in all cases the bondsmen are vouched for by the proper government officials; but if I understand this business it is not bonds or money responsibility that is wanted in this case so much as good mechanical work, for which the government will pay to the parties doing the work in cash, (reserving the usual percentage,) at reasonably short intervals, upon proper inspections, estimates, and reports, as may be agreed upon when the contracts shall be drawn up.

I observe that the instructions of the department require me to "reserve the right of rejecting any and all bids," and I will therefore recommend that all the bids be rejected, and that we establish by the aid of these bids, and by what we know of the value of the work required to be done, a price for a snag-boat completely finished, as our specifications call for, and then offer the boats to be

built and finished at such established price, as follows: One to the mechanics at Louisville, Kentucky, and Jeffersonville, Indiana, and two to those of Cincinnati, Ohio, and that the offers be confined to such bidders as have the reputation of doing the best work in their several branches of business.

I believe that, by this plan, the community will be satisfied, and that the government will get the best result and in the shortest possible time, as the rivalship existing among the several different localities above named will naturally conduce to this favorable result.

I remain, very respectfully, your obedient servant,

J. N. MACOMB,

Lieutenant Colonel Engineers, Bvt. Col. U. S. A.

Brigadier General A. A. HUMPHREYS,

Maj. Gen. Vols., Chief of Engineers, U. S. Army, Washington, D. C.

APPENDIX I.

Annual report of W. Milnor Roberts, superintending engineer Ohio river improvement, made to Major General A. A. Humphreys, Chief of Engineers United States army, for the year ending June 30, 1867.

GENERAL: In obedience to instructions contained in the circular of the engineer department, No. 11, dated January 10, 1867, I respectfully present the following annual report upon the surveys and works under my charge as super-

intending engineer of the Ohio river improvement.

My appointment to this duty is dated August 3, 1866. After making the necessary preliminary arrangements, in accordance with the written instructions from the department, dated September 5, 1866, I chartered the steamer Greenback, Captain John Rodgers, and taking with me Captain George W. Rowley, consulting pilot, and Thomas P. Roberts, assistant engineer, made a preliminary examination of the entire length of the river, as described in my report dated December 25, 1866.

I also made a preliminary special report dated September 25, 1866, recommending the construction of riprap dams at the following points on the river, upon all of which work had formerly been done under the direction of Captain Sanders, between the years 1837 and 1844. Upon all of these work has been in progress during the present year, and it is expected that they will be completed on or before December 1, 1867.

The distances of these works below Pittsburg and names of places are shown

in the annexed table:

WORKS UNDER CONSTRUCTION.

White's ripple, 11 miles below Pittsburg. Logtown bar, 18 miles below Pittsburg. Twin island, 85 miles below Pittsburg. Captina island, 107 miles below Pittsburg. Fish Creek island, 112 miles below Pittsburg. Petticoat bar, 1461 miles below Pittsburg. Muskingum island, 174 miles below Pittsburg. Blennerhassett's island, 185 miles below Pittsburg. Buffington island, 214 miles below Pittsburg.

SURVEY OF THE OHIO RIVER.

The fall of 1866 proved to be very unfavorable for surveys on the river, owing to a succession of freshets keeping the river from falling to a low stage. No engineering party was organized that season.

In accordance with instructions of the engineer department, I organized two surveying parties, which were placed on their respective survey boats on the 25th of June last; one party under the direction of Sigismund Low, esq., assistant United States civil engineer, to survey the river from near Gallipolis to Cincinnati, about 200 miles; and the other under Alonzo Livermore, esq., assistant United States civil engineer, to survey between Cincinnati and Louisville, about 145 miles. Upon completing which sections, if the season admits of it, they are to proceed with the surveys between Louisville and Cairo, about 400 miles, or as far down as they can before the severe weather sets in.

Each party consists of one assistant in charge, one transitman, one leveller, two rodmen, two flagmen, two chainmen, two axemen, and two boatmen, with such occasional help as might be found needful; each party having a floating

boat to live in.

Mr. Livermore accepted the appointment on this river with the understanding that in case the work of improving the Des Moines rapids of the Mississippi river should be put under contract he would take a position there as a civil engineer on the engineering staff of Major General J. H. Wilson. Accordingly he left the Ohio river survey at the close of July, to go to his present post at Keokuk. The survey of that part of the river has since been under the charge of James E. Day, esq., assistant United States civil engineer, a competent gentleman of much experience, who had formerly been associated with me in engineering for a number of years.

The object of these surveys is to gather the necessary detailed information respecting the pools, ripples, shoals, islands, &c., below the termination of the old surveys made by Captain Sanders and Mr. Fuller, which ended, (so far as our charts show,) in 1844, 271 miles below Pittsburg; and to obtain reliable maps and charts of the same. These will be useful hereafter when examining and discussing the different plans for the improvement of the navigation of the Ohio, and should always be preserved as valuable records of the present aspect

of the stream.

On the 12th of September, instant, the survey of Mr. Low's party had advanced 126 miles, leaving about 73 miles only to reach Cincinnati.

At the same date the survey of Mr. Day's party had advanced 120 miles below

Cincinnati, leaving only about 20 miles to reach Louisville.

It may be in place to refer here to the fact that the surveys now being made by General Weitzel, at Louisville, &c., with a view to a proposed canal around the falls, will probably render it unnecessary for my parties to spend much time at that particular point this season.

General Weitzel's surveys, added to those formerly made by the government, will be likely to present all the engineering information needful in connection with the falls, with the old canal, with the new one partly built on the Louis-

ville side, and with the projected canal on the Indiana side.

The present surveys along the Ohio river being but partially completed, and still in active progress, the data obtained is, of course, not yet in shape for a

complete report, and cannot be until after the close of the season.

At some of the ripples, at the time the surveying parties passed, the water was rather high for careful investigation and soundings for showing the bars, low-water channels, rate of currents, &c. These can be obtained at a more favorable period hereafter, either this fall or next season.

The maps of the river between the end of the old survey, 271 miles below Pittsburg and Louisville, will, however, be ready for the department not long after the respective parties conclude their field duties. They are upon the same

scale as those on the upper part of the river, 1,000 feet to an inch.

Until the survey is completed, at least as far as Louisville, showing the upper Ohio, it would be premature and unsatisfactory to enter upon a minute description of part of the details eliminated.

My report of December, 1866, published in the appendix to the report of the Chief of Engineers United States army, contains a general description of the river, with approximate distances, &c. These assumed distances will be, of course, somewhat modified by the surveys, and will be correctly given hereafter from the actual measurements. But the main features of the river will be found as described in that report, to which the department is respectfully referred.

At the end of the fiscal year ending June 30, 1867, the surveying parties had only been organized six days, namely, at the date of June 25, so that the expenditures under the particular head of "surveys" of the river will come in

the statement of the fiscal year ending June 30, 1868.

The expenditures for "preliminary examinations and surveys" made during the autumn of 1866, in September and October, and up to the 30th of June, 1867, amounted to \$6,677 82.

The total amount expended on preliminary examinations and surveys, up to

the 31st of August, 1867, is \$11,955 38.

I estimate that the cost of the necessary examinations and surveys of the Ohio river, from the beginning of September, 1866, (upon the plan adopted,) to the end of the fiscal year, ending June 30, 1868, will be \$22,000. Of which there were expended, charged to that fund, during the fiscal year ending June 30, 1867, \$6,677 82. Leaving to be expended during the fiscal year ending June 30, 1868, \$15,322 18.

During the season of 1868, after June 30th, I think it would be advisable to

have a small appropriation applicable to this fund, say \$10,000.

WORKS ON THE RIVER.

At the time that contracts were made, owing to the stage of the river, the quantities of stone necessary to complete them could not be ascertained with accuracy. Quantities were therefore assumed less than the probable quantities that might be finally required. In the construction of the dams it was afterwards found that the additional quantities needed to perfect them were so considerable that the department directed me to advertise for new proposals for the additional stone, which was accordingly done.

The annexed tabular statement exhibits the distances, names of places, contractors' names, prices, number of cubic yards of stone contracted for, and

amount of the original contracts made in 1866:

Quantities, price, &c., at nine riprap dams.

Miles below Pittsburg.	Places.	Contractors.	Price of quarrying and delivering, per cubic yard.	Price of boating and putting into dam, per cubic yard.	Number of cubic yards.	Amount of contract.
11 18	White's ripple	Swan & Fenion	\$1 75 1 25	\$1 35 1 00	5, 000 5, 000	\$15, 500 11, 250
85 107	Twin island	Manfull & Kerr	1 13 1 25	1 47 1 00	4, 000 3, 000	11, 250 10, 400 6, 750
112	Fish creek	do	1 25	100	3,000	6, 750
146	Petticoat bar	King, Reinhart & Co	1 75	85	3,000	7,800
174	Muskingum island	C. M. Cole	1 10	55 55	4,000	6,600
185 214	Blennerhassett's island	T T Power	1 10 1 75	1 20	5, 000 4, 000	8, 250 11, 800
-17	Damingson mand	J. J. F OW 01	1	. ~~	3,000	
	Total					85, 100

It is not probable that a full report upon the surveys can be presented this fall, as it is not expected that they will reach (lairo this season, but a report upon them as far as they shall have advanced at the close of the present surveying season will soon after be in readiness.

The following tabular statement exhibits the distances, names of places, contractors' names, prices, number of cubic yards, and amount of contracts for additional stone at eight riprap dams.

The quantity contracted for originally at Fish Creek island being sufficient, no additional stone has been let:

Additional quantities, price, &c., at eight riprap dams

Miles below Pittsburg.	Places.	Contractors.	Price of quarrying and delivering, per cubic yard.	Price of boating and putting into dam, per cubic yard.	Number of cubic yards.	Amount of contract.
11	White's ripple	Swan & Fenion	\$1 50	\$1 00 50	3, 500 1, 800	\$8, 750 00 900 00
18	Logtown bar	Routh & Lane	1 25	1 00	1, 275	2,868 75
85	Twin island	James Kerr	1 13	2 25 1 37	1,000	2,25000
107	Captina island	Routh & Lane	1 25	1 00	1,000	2 250 00
1464	Petticoat bar	King, Reinhart & Tripp	1 75	85	500	1,300 00
174	Petticoat bar	C. M. Cole	i 10	65	4, 875	8,331 25
185	Blennerhassett's island	do	1 10	75	4, 500	8,325 00
214	Buffington island	Charles Cable	1 25	75	5, 200	10,400 (0
	Total	• • • • • • • • • • • • • • • • • • • •				48, 575 00

Statement showing the estimated amount of work done on the dams up to the end of July, 1867.

Miles below Pittsburg.	Places.	Contractors.	Price of quarrying and delivering, per cubic yard.	Price of boating and putting into dam, per cubic yard.	Number of cubic yards contracted for.	Amount of contract.
11 18	White's ripple Logtown bar	Routh & Lane	\$1 75 1 25	\$1 35 1 00	5, 000 5, 000	\$15,500 11,250
84 107		Manfull & KerrRouth & Lane		1 47 1 00	4, 000 3, 000	10 400 6 750
112	Canting island	do	1 25	1 00	3,000	6,730
1464	Petticont bar	King, Reinhart & Co C. M. Coledo.	1 75	85	3, 000	7, 800
174	Muskingnmis land	C. M. Cole	1 10	55	4,000	6,600
185	Blonnerhassett's island	do	1 10	55	5,000	8, 250
214	Buffington island	J. J. Power	1 75	1 20	4,000	11, 800
	Total					85, 100

Statement showing the estimated amount of work done on the dams up to the end of July, 1867—Continued.

Miles below Pittsburg.	Places.	Contractors.	Amount paid to Augrat 15.	Yet to be paid.	Ten per centum re- perved.	
11 18	White's ripple Logtown bar	Swan & Fenion		\$1,793 00 5,141 25	\$1,370 70 610 87	
85	Twin island			1,499 90	890 00	
107	Fish creek			3,960 00 4,500 00	979 00 225 00	
1464	Petticoat bar	King Reinhart & Co.	6, 867 00	933 00	686 70	
174	Muskiogum island	C. M. Cole	4,063 95	2,536 05	406 39	
185	Blennerbassett's island	do	6,831 00	1,419 00	683 10	
214	Buffington island	J. J. Power	10,080 00	1,720 00	1,008 00	
	Total	•••••••	61, 597 80	23, 502 20	6, 159 76	

The contracts for the additional stone are only now being signed, but the parties, as it happens, are either the original contractors or others who operate in harmony with them, so that no material delay has taken place in the management of the work, and I think that all of them will be completed by the contract period, (the 1st of December, 1867.) unless there should be a season of much high water this fall to interfere, which, however, is not anticipated.

The system of laying out and superintending the works with a small engineering party, having the use of a light-draught government steamer, is believed to be more advantageous and more economical than the old plan of having persons stationed at each point. The greater the number of dams that may be going on simultaneously, the better and more economically this system will work.

The plan of measuring the stone by the cubic yard, instead of by the ton, is also believed to be an improvement upon the old mode of measuring by the ton

or the draught of the stone boats.

Some of the dams are advanced far enough to show a marked improvement on the depth of water in the channels, and I have no doubt that at each of the works under contract the navigation will be materially benefited by their completion. We may not at the first effort secure exactly the best results; some modifications may become necessary which only experience can regulate.

WHITE'S RIPPLE, (11 miles below Pittsburg.)

It is expected that the work at this dam will be finished about the latter part of October, 1867. Meanwhile the construction of the main longitudinal dam has added about one foot to the low-water depth in the channel and increased its velocity also. The precise effect altogether upon the navigation in this vicinity cannot be accurately determined until after we have had opportunities of examining it in different stages of the water. We have to guard against having too strong a current running obliquely to the left across the longitudinal dam in a coal-boat stage. Possibly it may hereafter become necessary to raise the old cross-dam between the channel and the left shore to equalize the flow, but it is not proposed to do anything of the kind at present, as I hope that a sufficient flow for that purpose may be allowed to pass down to the left of the towhead along the longitudinal dam.

LOGTOWN BAR, (18 miles below Pittsburg.)

The dam at this place, running from the left shore to the bar and ending obliquely against the old partly built dam, is about finished.

That part of the old dam extending from the bar some distance obliquely up stream, pointing towards the right shore, has been removed and put into the new dam. This arrangement leaves a clear, straight, and wide channel near the middle of the river, which, it is believed, will be a permanent improvement of the navigation in low water and in coal-boat stages.

It can be determined hereafter whether any additional work may be neces-

sary to render this part of the navigation complete.

TWIN ISLAND, (85 miles below Pittsburg.)

Nearly all the stone required for the dam at this place has been put in: the main dam is good and will fulfil its purpose.

At the foot of the island there has been a good deal of washing away, leav-

ing much water to escape to the left of the centre bar.

There ought to be a dam put in here, which would take about one thousand cubic yards, and would improve the low-water depth materially. The necessity for this has only recently become apparent.

With this arrangement the navigation at this place will be very much improved. This is one of the instances where everything that might be needed

did not appear in the beginning.

CAPTINA ISLAND, (107 miles below Pittsburg.)

The contractors expect to complete this dam about the last of October. The same contractors have the dam at Fish creek and at Logtown bar, and their work is carried on from the same quarry, with their own steamer. There is not yet sufficient stone in this dam to enable me to refer to the practical effect of this improvement, but no doubt is entertained respecting its success.

FISH CREEK ISLAND, (112 miles below Pittsburg.)

At the beginning of this month (September) there remained only about eight hundred and fifty cubic yards of stone to be put in to complete the dam at this place. It will be finished this fall in good season. This is a small dam, of about three thousand cubic yards in all.

Thus far it has been well built, and it will, when finished, improve the

navigation at this point materially.

PETTICOAT BAR, (1462 miles below Pittsburg.)

The dam at this place is finished. It presents a fine appearance, and is in all respects complete. Its effect is good, and the low-water navigation is somewhat better than it was.

It only remains to see how this dam, as well as the other dams in process of construction, will stand the ice freshets. The precise action of ice freshets cannot be determined in advance, although no serious injury from that cause is anticipated at this place.

Less work was required here than at most other points, and the advantage to navigation is much less striking than in some of the other cases. It has

made a safer navigation at a tow-boat stage.

Muskingum Island, (174 miles below Pittsburg.)

The dam at this place is well advanced and progressing satisfactorily.

will be finished by the first of December.

It is probable that all that is desired at this point can be obtained with fewer cubic yards of stone than were estimated (in all) as necessary, and as a rather larger quantity is needed than was estimated to be necessary at Blennerhassett's Island dam, it is proposed to decrease the quantity to be put in at Muskingum and increase the quantity at Blennerhassett's.

The same contractor has both jobs at the same price per cubic yard, and he is willing to transfer the stone as proposed, so that it will be the same to the government.

BLENNERHASSETT's ISLAND, (185 miles below Pittsburg.)

There are two dams at this place—one at the head of the island and one at the foot, known as the head dam.

There has been some trouble at the upper or main dam by the sudden breaking away of sixty feet of the bar at the head of the island. This gap was promptly closed by Mr. Cole, the energetic contractor in charge.

A six feet freshet in August washed out considerably more of this sand and gravel bar. It will continue to need attention and some additional stone through

another season.

The construction of the Blennerhassett dam has had the effect of raising the water at the dam more than three feet above its former low water height, and

the pressure of water is therefore considerable.

When the dams at the head and foot of the island shall have been completed and in use for a time, a better judgment can of course be formed as to their full effect. I have no doubt that they will greatly improve the navigation along that part of the river. It will probably require somewhat more stone (in all) than was estimated to be necessary, partly in consequence of the washing out of the bar, the proper remedy for which is to fill the space thus washed out with stone.

The work will, it is expected, be finished this season before December.

BUFFINGTON ISLAND, (214 miles below Pittsburg.)

The works at this place are in a forward state, and will be completed before December next.

At least half of the flow in low water passes down the narrow chute on the Ohio side of the island, and navigation here becomes difficult and uncertain, and finally ceases sooner than at any place lower down the river. After closing this Ohio chute, it is necessary to build up the long wing dam on the Virginia chute, (shown on the chart,) to prevent the spread of the water out of the channel.

The Ohio chute dam, which is well advanced, is now turning a large volume of water, but a considerable washing out of light sand and gravel in the gap, 175 feet long, is taking place, rendering an additional quantity of stone necessary. Although these works will be completed before December, another freshet season must pass before we can determine finally whether any other work may be necessary at this important point.

It is believed that the completion of the present plans will make a very

material improvement of the navigation.

While the low water navigation will be improved at each of these places, the channels generally will also be rendered better than they were for the coal boating, which, as explained in former reports, has of late years become a most important interest on the river, and should always be kept prominently in view, in considering any proposed plans for the improvement of the Ohio river.

The barge system is on the increase and gradually gaining favor, and it is highly probable that for freighting coal, iron, and heavy mineral and agricultural products, it will come into more general use, not only on the Ohio, but on the

Mississippi.

This is a very important fact in the business of the Ohio river—the general substitution of fleets of barges for the former single steamers, or the plan of floating boxes. It is reasonable to believe that after a while a large proportion of the steamers engaged in freighting will be tow-boats, running in connection with barges. Some single steamers will of course still be useful in carrying on

the local passenger and freight business between the numerous commercial points along the river, and there will always be steamers engaged during good boating stages in the long voyages from the waters of the Ohio to the upper Mississippi and Missouri, and also to the lower Mississippi; but the bulk of the freighting will probably be ultimately carried on by means of barges towed by steamers.

Although this branch of the subject belongs appropriately to another report upon which I am now engaged, describing the several plans which have been proposed for the radical permanent improvement of the navigation of the Ohio,

it is also deemed proper in this place thus briefly to advert to it.

One object is to show clearly that there is nothing now in progress of construction on the river calculated to interfere injuriously with the barge system of boating, and wherever the navigation is good for fleets of barges, it is good for single steamers.

REMOVING OBSTRUCTIONS FROM THE RIVER.

In accordance with the instructions of the engineer department, I advertised for proposals for furnishing a light-draught steamer, with crane boat, flats, and all necessary tools and fixtures, to be employed by the day, in removing obstructions from the river. Captain John Rodgers, of the steamer Greenback, was awarded the contract, at the rate of eighty-eight and a half dollare (\$88 50) per day's work.

Captain John Shouse, of Steubenville, an experienced, intelligent and reliable Ohio river pilot, was appointed inspector for this special service, and entered

upon the duty June 11, 1867.

This plan for removing river obstructions, by contract, at a fixed price per day, the contractor furnishing vessels, tools, men, &c., at his own cost and risk, was to some extent experimental. I had formed the opinion that such work could probably be done under this arrangement more economically and satisfactorily than by purchasing vessels, &c., and employing the men by the day or month.

A regular steam snag-boat, it is known, is very costly to construct and manage, and it was believed that a light-draught steamer, with a strong crane boat and a couple of decked flats, would accomplish all or nearly all the work that

could be done by the more expensive snag-boat.

Under this contract plan, as under any other plan of government operations on such work, much depends on the experience, ability, and faithfulness of the persons employed. In this respect we have, I think, been fortunate in securing the services of an able, energetic, and honest man for contractor, (Captain John Rodgers,) who is well qualified for this particular duty, and in the appointment of a government inspector, (Captain John Shouse,) in charge of the operations, who is the right man in the right place.

The experiment of removing obstructions on this plan has therefore been con-

ducted under very favorable auspices, and I regard it as a success.

The contract price, as already stated, is eighty-eight and a half dollars (\$88 50) per day, and, including the pay of the inspector, the whole cost is ninety-three and a half dollars (\$93 50) per day, or for a full month of 26 working days, twenty-four hundred and thirty-one dollars, (\$2,431.)

Allowing that in a favorable season this sort of work might be advantageously carried on between April and November, or during seven months of actual work, it would make the cost for the season seventeen thousand and seventeen dollars

(\$17,017) for a full season for one boat.

So far as we can now tell, judging from what has been accomplished during about three months in the removal of obstructions, it might require say two more steamers another favorable season to clear the entire river between Pittsburg and Cairo.

At the end of the operations there would be no steamers, boats, &c., on hand

to be disposed of.

If two more such steamers were employed this year, and three should be employed next year, the estimated total cost, assuming the same rate per steamer as the present contract price, would be nearly as follows:

Estimated expense of one boat, &c., during the season of 1867 Estimated expense of two others, two months, say	\$14,000 9,724	
Estimated expense of three boats during the season of 1868, at \$17,017 each.	51, 051	
Total from the beginning in 1866	74, 775	00

This is believed to be a liberal estimate. The probability is that it may cost something less. It is possible that competent parties might undertake the operations during the ensuing year, at a somewhat lower rate, as there seems to be a downward tendency in prices.

When it is recollected that these obstructions are numerous, many of them sunken barges and wrecks, exceedingly difficult of removal, the total estimated cost appears small when put in comparison with the great benefit their removal is conferring upon the river commerce.

According to my report, dated October 31, 1866, we had noted the following

obstructions in the Ohio river:

List of known obstructions, October 31, 1866.

Kind of obstructions.	Upper Ohio, Pittsburg to Louisville, 615 miles.	Lower Ohio, Louisville to Cairo, 400 miles.	Total, 1,015 miles.	
Snags, (places)	49 28	15 17 18 11	90 66 46 83	

It was remarked in connection with this list, that in some places several snags or trees might be found where only a single snag is mentioned; and at points marked "loggy places" there may be a number of logs to be taken out; and that there are also more single wrecks and sunken boats than the table shows; and after that report was written some additional obstructions became known to me.

Having become satisfied that there is not probably any more economical method of removing these obstructions from the river, I had the honor to make a communication to the department, dated September 2, recommending that proposals be received for furnishing two additional steamers, crane-boats, &c., for employment this fall. The department, at the date of September 4, 1867, instructed me to advertise for proposals accordingly, which has been done. The proposals are to be received until noon of the 21st instant, at the office in Pittsburg, Pennsylvania. From the nature of the case, perfectly accurate estimates of the total cost of removing all the obstructions cannot be arrived at in advance; but I think the data now in our possession are sufficient to warrant the assumption that the entire cost will not be likely to exceed the above sum of \$74,775. From this should be deducted the sum of \$6,140 75 already paid up to the end of August, leaving to be expended, according to the foregoing esti-

mate for work to be done after August 31, 1867, the sum of \$68,634 25. Possibly a considerable saving may be effected by modifying the system somewhat, so as to work two sets of crane-boats, flats, &c., under the management of one steamer passing from one set to the other, keeping the two sets of crane-boats always within a few miles of each other.

Under instructions from the department dated December 28, 1866, I have a special report in preparation and nearly finished upon all the different plans which have been proposed for the permanent improvement of the Ohio river. It embraces the reservoir plan, the plan of dams, mounds, and wide, river canal; the plan of locks, dams, and the plan of dams with patent chutes without locks. Either of these plans will necessarily involve the expenditure of a large sum of

money, as the report will show.

This special report might have been finished some time ago, but for other imperative calls upon my time and attention, which could not be deferred without injury to the public service. I refer to it here partly for the purpose of mentioning that no works will at this time be recommended to be put under contract which would interfere with either of the more costly plans referred to above. I may add that the longer I study the characteristics of the Ohio river. its peculiar regimen, and its commerce, in connection with the topographical and hydrographical features of the country through which it flows, and of the region of its head streams on the Monongahela and Alleghany rivers, the more complicated and difficult the problem of its radical improvement becomes. will not in this place enter upon the discussion which belongs to the other report, and which would be merely repetition.

The riprap dams now in progress, and the others which are recommended. the excavation of parts of bars, the taking away of some rocks, and the complete removal of the numerous other river obstructions, are works deemed to be immediately and urgently needed; and yet the whole of this character of improvements on the entire river will not require a very large sum of money. The direct advantage of navigation to be gained will be far more than commensurate with the outlay; and this expenditure ought not to be delayed on account of other and grander projected or proposed improvements which may follow.

REFERRING TO PROPOSED WORKS ON THE RIVER NOT YET UNDER CONTRACT.

It has been already mentioned that there are a number of places on the river upon which it is proposed to offer special reports, the data for which could not be prepared in detail in time for this annual report. At the same time I consider it important for the interest of western river commerce that provision should be made by reasonable but adequate appropriations for the continuation of the present general system, which is, in brief, a concentration of the water into single channels, by means of proper wing dams; the excavation of portions of certain bars, the removal of some rocks, and the removal of numerous obstructions, consisting of snags, roots, logs, old wrecks, and sunken barges, coal boats, steam-

An estimate of the probable cost of removing the obstructions, and which can be effected during the season of 1868, has been given in this report. Also, an estimate of the amount required to complete the river contracts now in progress between Pittsburg and Buffington. There were apportioned to the Ohio river the following sums:

By the act passed in 1866, to be applied to improvements, &c..... \$100,000 By an act dated March 2, 1867, to be applied to improvements, &c. 100,000 By an act dated June 23, 1866, to be applied to removal of obstructions. 50,000 By an act dated June 23, 1866, to be applied to examinations and 22,000

£72,000

If no more contracts were to be made, these apportionments of the appropriations would not be exceeded; but it is presumed that the government, having resumed the care of the Ohio river and other western rivers, intends to go on with such proper works as will eventually improve their navigation, and that further appropriations will be made for the improvement of the Ohio river.

Without enumerating at this time every point on the river where work may, perhaps, be advantageously put under contract at an early period, I will refer, in a succinct manner, to the general features of the more prominent cases, from a consideration of which the department can form a judgment as to the propriety of the appropriations asked for the Ohio river.

Description of places where work is required on the Ohio river between Pittsburg and Cairo.

GLASS HOUSE RIPPLE, (2 miles below Pittsburg.)

This is one of the worst places on the river in low water, and urgently needs improvement, by a dam of some kind. Within a few days my special report upon this place will be forwarded to the department.

MERRIMAN'S RIPPLE, (101 miles below Pittsburg.)

Dredging is advisable for the purpose of straightening the channel and improving the approach to White's ripple. Perhaps half an acre in area.

DEADMAN'S RIPPLE, (14 miles below Pittsburg.)

An area of about 250 by 60 feet, about one-third of an acre, which has washed in, should be dredged away. Also, the head of the channel may be greatly improved by a considerable amount of dredging.

BEAVER SHOALS, (27 miles below Pittsburg.)

Five or six hundred feet of old boulder dam, about ten feet wide, two feet deep, should be taken away at upper end and put across from left shore to head of dam, and the left chute should be closed. There should be some dredging of heavy gravel and boulders at the foot to straighten the channel. Some rock excavation will probably be necessary.

RACCOON SHOALS, (29 miles below Pittsburg.)

A moderate amount of dredging here will straighten and improve the channel, to be done simultaneously with the improvement of Beaver shoals.

MONTGOMERY ISLAND, (32 miles below Pittsburg.)

Several rocks here should be taken out.

PHILLIS'S ISLAND, (35 miles below Pittsburg.)

Several rocks here should be taken out.

GEORGETOWN ISLAND, (38 miles below Pittsburg.)

Rocks here are dangerous at a low stage of water, ten or twelve in number, each requiring one or two blasts.

LINE ISLAND, (411 miles below Pittsburg.)

About one hundred by seventy feet, about one-sixth of an acre, ought to be dredged two feet deep.

BAKER'S ISLAND, (49½ miles below Pittsburg.)

The dam here leaks badly; about twenty-five hundred cubic yards of stone will perfect this work.

CLUSTER'S ISLAND, (52 miles below Pittsburg.)

A dam was planned here by Captain Sanders in 1844. To build it properly will require five or six thousand cubic yards of stone. There is no doubt that the concentration of the water by a dam would be beneficial; but I am not yet prepared to recommend a definite plan.

BLACK'S ISLAND, (54 miles below Pittsburg.)

There is an old unfinished dam here. Fifteen hundred cubic yards of stone will probably complete it. A few rocks should be removed.

Brown's Island, (613 miles below Pittsburg.)

Gaps in the old curved dam waste considerable water; also some stone needed on the Ohio side. About eight hundred cubic yards in all will probably suffice.

FERRY BAR, (66 miles below Pittsburg.)

A number of considerable rocks should be removed, as the channel is narrow. Many have been taken away through private enterprise.

MINGO ISLAND, (70 miles below Pittsburg.)

There is an old unfinished dam here, which should be finished to improve the low-water navigation. Not heavy work.

Wells's Bar, (- miles below Pittsburg.)

Our Captain Rogers removed a large rock from here. The place is nearly as shoal as the Sisters. All the water runs in the channel, and as the bottom is hard gravel, it is probable that dredging will improve it. A boat lately grounded here, after passing the Sisters or Twin islands.

Cox's BAR, (72 miles below Pittsburg.)

This place can be improved by levelling off lumps in the channel. It is only troublesome in extremely low water.

BEACH BOTTOM BAR, (772 miles below Pittsburg.)

The old unfinished dam at this place should be completed. It will require about 2,500 cubic yards of stone.

BURLINGTON BAR, (864 miles below Pittsburg.)

It may be found advisable to build the dam planned here by Captain Sanders for the improvement of the low-water navigation. The water is not deep. It will require about 6,000 cubic yards of stone.

WHEELING ISLAND, (90 miles below Pittsburg.)

A low-water dam at the head is needed to close the Ohio chute, say 1,500 feet long; about 3,500 cubic yards of stone.

Bogg's Island, (922 miles below Pittsburg.)

Some clearing away of boulders necessary.

McMahen's Creek Bar, (94 miles below Pittsburg.)

There are two channels here. It is likely that the channel could be improved by dredging. If dredging is done along the river it might be tried here. Ultimately one of the channels may be shut if deemed advisable.

FISHING CREEK, (1271 miles below Pittsburg.)

The old unfinished dam at this place should be completed. It will require about 3,000 cubic yards of stone.

WILLIAMSON'S ISLAND, (133 miles below Pittsburg.)

The old unfinished dam at this place should be completed. It will require about 2,500 cubic yards of stone.

WHITTEN'S TOWHEAD, (1341 miles below Pittsburg.)

The old dam at this place is about 800 feet longer than necessary. During the last winter (1866-'67) the ice carried away the lower portion, and pilots now complain that it gives trouble. It certainly ought to be repaired. About 3,500 cubic yards of new stone, in addition to what could be obtained from the upper end of the old dam, will probably suffice.

Wells's Island, (1381 miles below Pittsburg.)

The old unfinished dam at this place should be completed, requiring about 3,500 cubic yards of stone. It will improve the low-water navigation.

MILL CREEK ISLAND, (1402 miles below Pittsburg.)

At the old dam, at the island end, the water is cutting some. This should be stopped and the dam completed. It will need about 1,200 cubic yards of stone.

GRAND VIEW ISLAND, (1412 miles below Pittsburg.)

About one thousand cubic yards of stone can be advantageously applied here in closing gap and strengthening the old dam.

THREE BROTHERS, (1581 miles below Pittsburg.)

This place can be improved at a moderate expense by dredging Rowland's Race, so as to straighten the channel.

CARPENTER'S BAR, (166 miles below Pittsburg.)

A dam at the head of either side of Marietta island, one and three-fourths miles below, will doubtless benefit this bad place, but it is not certain that it would wholly remedy it. If a dam is built at Carpenter's bar, it will require abou; 10,000 cubic yards of stone. It is one of the worst places. Sand settles here in some years. In 1859 it was worse than any place between Wheeling and Cincinnati.

MARIETTA ISLAND, (1673 miles below Pittsburg.)

A special report is to be made. See remarks in my report of December last. It is difficult to decide respecting the mode of removing the trouble at this island in consequence of the peculiar location of the city of Marietta.

COLE'S ISLAND.

The foundation of the old dam here is very good. If raised to a four-feet stage it will benefit medium navigation, which is important at this place. It will require about 3,000 cubic yards of stone.

NEWBURG BAR, (193 miles below Pittsburg.)

There is some cutting by the water at the island end of the old dam at this place. To fill the gap and raise and finish the whole dam will take about 3,000 cubic yards of stone.

BELLVILLE ISLAND, (202 miles below Pittsburg.)

This is a long series of bars and dams, extending for a mile and a half. It may take about 4,000 cubic yards of stone in all to improve this place properly.

SAND CREEK BAR, (219 miles below Pittsburg.)

This point should be improved. A dam seven or eight hundred feet long, of a curved shape, through shoal water, should be built so as to close the left chute. About 900 cubic yards of stone is required.

GOOSE ISLAND, (227 miles below Pittsburg.)

This place can be improved by shutting one channel. If either is closed it must be the right side, as the left is the regular tow-boat channel. About 3,800 cubic yards of stone will answer for this purpose.

LETART'S ISLAND, (232 miles below Pittsburg.)

A valuable low-water improvement will be to finish the dam, to close up the Virginia chute, requiring about 3,000 cubic yards of stone.

EIGHT MILE ISLAND, (255 miles below Pittsburg.)

Requires only a low-water improvement—a dam to contract the spreading water, 600 feet long over shoal water; about 1,200 cubic yards of stone.

RACCOON ISLAND, (273 miles below Pittsburg.)

The channel is very crooked and needs a landmark, say two posts twenty-five feet high, with white boards, for the present, until some trees grow up. Perhaps a little work in channel.

GREEN BOTTOM RIPPLE, (2891 miles below Pittsburg.)

The channel here is crooked, with a hard bottom. Dredging, if practicable, may benefit this place. The channel would be materially improved if straightened.

GUYANDOTTE BAR, (3021 miles below Pittsburg.)

The river is very wide here, and shoal all over. One or two long, low wing dams will materially improve this place, say about 1,200 cubic yards of stone.

BURLINGTON OR TWELVE POLE BAR, (312 miles below Pittsburg.)

There are two chutes here at low water. The left chute should be closed with a curved dam built low and flat. About 4,000 cubic yards of stone.

Poag's Shallows, (3223 miles below Pittsburg.)

From a point 21 miles above Poag's Shallows to Burk's Point 25 miles below, making forty-six miles distance, a general clearing out of loose rock is urgently needed. Some of the rocks contain ten cubic yards each, in five feet water, requiring blasting. A floating crane boat and flats may be used here to advantage. Some of this sort of work was done in 1844 by Captain Burch, at this place and Doyhan's bar, Guyaudotte and Sandy Shallows.

BRUSH CREEK ISLAND, (381 miles below Pittsburg.)

There are three channels here at low water, consequently the water spreads. There is an old partly built wing dam, thrown out from the Virginia side some distance above, which has nearly disappeared in the lapse of time. It has been proposed to make this up and then to throw in another wing dam to the head of the island, or else to extend a wing dam down from the old one. It may take about 5,500 cubic yards of stone to make it complete.

MANCHESTER ISLAND, (391 miles below Pittsburg.)

It is probable that a number of sunken boats at this place hold the sand and obstruct the navigation. Their removal will materially improve it. Until they are removed and the effect observed, I would not recommend the commencement of any dam here. Before these boats were sunk the Manchester channel was much better than it is now.

CHARLESTON BAR.

There is shoal water at the foot, close to Kentucky side, which is the channel only in extreme low water. Half the water wastes here in ordinary low stages. It can be closed by a dam with about 4,500 cubic yards of stone.

FOOT OF AUGUSTA BAR, (426 miles below Pittsburg.)

There is a low, changeable, gravel shoal here. The navigation is bad for about ten miles below. It is probable that if the river is thoroughly cleared of rubbish, a better channel might maintain itself without aid from dams. Only future experience can determine this finally. These remarks apply as far down as Big Snag bar, which is about ten miles below Augusta bar.

FOUR MILE CREEK BAR, (4591 miles below Pittsburg.)

The navigation has been injured here by the accumulation of wrecks. There are two channels. One of them might be closed, making an improvement similar to that at Petticoat bar. It would take about 4,000 cubic yards of stone.

Bosley's or Medoc's Bar, (4881 miles below Pittsburg.)

There are two channels at this point, which are alternatively preferred. A dam may yet be deemed advisable; but a thorough clearing out of the wrecks and rubbish should first be attempted, and the effect noted.

RISING SUN BAR, (508 miles below Pittsburg.)

This is a difficult place. It is reported this year (1867) as the shoalest spot between Cincinnati and Louisville, and I also found this by sounding. There is a sort of backbone across the channel at the head. Experienced captains and pilots, who have been many years in the Cincinnati and Louisville trade, complained to me that this place has been greatly injured by extensive ploughing on the large flat bar for the purpose of obtaining paving stones for Cincinnati and other cities. Persons take off the large stones, and the next succeeding flood sweeps off a portion of the loosened bar. (Further reference will hereafter be made to this evil, which is not confined to this one place.) It occurred to me when viewing this place that a spur dam from the Indiana shore, and a long, oblique wing dam from the Kentucky shore on the bar, by concentrating the water, would materially aid navigation, say at a five feet stage. Dredging also, I think, can be advantageously done at this place. It might require 10,000 cubic yards of stone, in all, for this place.

GUNPOWDER CREEK BAR, (516 miles below Pittsburg.)

This is likewise a bad place in extreme low water. There are two channels here. The Kentucky channel should be closed, (if Rising Sun bar be improved, as it should be.) A long; low dam, requiring about 8,500 cubic yards of stone, would probably make this good.

WARSAW BAR, (534 miles below Pittsburg.)

A broad shoal. The water could be concentrated here by means of two wing dams, requiring about 6,000 cubic yards.

GRASSY FLAT BAR, (5951 miles below Pittsburg.)

Many years ago Captain Shreve built a pile dam here from the Kentucky shore. A new dam from the Indiana shore would leave the middle channel open, and effect an improvement of the navigation. The water is shoal, but the dam would be long, requiring about 6,500 cubic yards of stone.

This brings us to Louisville, at the falls of the Ohio.

End of Upper Ohio.—Beginning of Lower Ohio.—Falls at Louisville.

FALLS OF THE OHIO, LOUISVILLE, (615 miles below Pittsburg.)

The total fall here, at extreme low water, is twenty-six feet in three miles. No work has been done in the river at this point by the government for many years. The old canal and locks, and the new partly constructed canal and locks, remain about as described in my report of December, 1866. But Congress at its second session last spring ordered a special survey of the falls, with a view to a canal improvement on the Indiana side, and the department assigned the duty of making the necessary surveys and report to Brevet Major General G. Weitzel, United States army, who, as I have learned from himself, has been for some time engaged in that duty. One of my engineering parties, which commenced at Cincinnati, has about reached Louisville with our survey of that part of the Ohio river; but assuming that General Weitzel's surveys, added to those previously made by the government, will furnish all needed information at that important point, I have instructed Mr. Day, the assistant in charge, to proceed without unnecessary delay with the survey of the river below Louisville.

PORTLAND BAR, (620 miles below Louisville.)

The channel is changeable here. It crosses from Portland, on the Kentucky side, to New Albany, on the Indiana side. At this crossing there are two channels, which are never both good at the same time. It is a difficult shoal place. On account of the shifting sand it is doubtful whether ordinary riprap wing dams would avail to keep a regular channel. (I am not yet prepared to suggest any precise plan, or to make a definitive recommendation respecting work at this point, or at any place between Louisville and Cairo.)

FLINT ISLAND, (707 miles below Pittsburg.)

The main channel is generally down the Kentucky shore. If dams should be put in, they would be quite long. Nothing determined.

OII. CREEK BAR, (711 miles below Pittsburg.)

There is a shifting sand channel, which often gets very shoal.

HOLT'S BAR, (721 miles below Pittsburg.)

At the head it becomes quite shoal, in extreme low water. Owing to the shifting sands it is difficult to decide upon the plan of improvement.

PUPPY CREEK, (776 miles below Pittsburg.)

Troubled with shoals in low water; there is a hard bottom at this place. Possibly riprap dams may be available here.

YELLOW BANK ISLAND, (7794 miles below Pittsburg.)

A shoal, crooked place at low water. Dredging here would temporarily straighten the channel.

French Island, (7921 miles below Pittsburg.)

This is a very bad place. It ought to be helped, if it is possible at any reasonable cost. It should be improved permanently. A large amount has been expended in dams in years past, but they get covered or swamped in the

river sands, the channel sometimes making directly over them. No definitive plan yet prepared.

Scuffletown Bar, (801 miles below Pittsburg.)

A bad place. It is likely that if the old dam were raised, so as to turn more of the water, it would make an improvement of the navigation. As it is a long dam, it would require about 5,000 cubic yards for the purpose.

THREE-MILE ISLAND, (804 miles below Pittsburg.)

This place in late years has not been as bad as formerly. The shoal usually exists a mile below the dam that was put in many years ago. No definite plan has yet been arranged here.

HENDERSON ISLAND, (830 miles below Pittsburg.)

This is a difficult shoal place. Doubtless the cause of some of these sand shoals is the sunken logs which catch and hold the sand. Our consulting pilot, Captain Rowley, is of opinion that elsewhere in the lower Ohio, as well as here, these sunken logs have greatly tended to render such places troublesome to navigation. Their removal will probably materially benefit navigation.

FOOT OF SLIM ISLAND, (853 miles below Pittsburg.)

In my report of last December it was remarked that this place is shoal, difficult at times, and might be improved.

SHAWNEETOWN BAR, (879 miles below Pittsburg.)

This is an especially bad place, partly owing to the great width of the river in this vicinity. A concentration of the water would certainly give a greater depth for navigation, but it will require a considerable quantity of stone, or extensive dams of some kind, to be effectual. Limestone rock exists in the neighborhood. It has not yet been sufficiently examined to decide the particular plan, but it might be assumed, approximately, that 20,000 cubic yards of stone, or an equivalent cost, would make a much better navigation at this point.

CINCINNATI BAR, (890 miles below Pittsburg.)

A changeable place. Latterly it seems to have been rather better than it was some years ago. No plan for its improvement has yet been fixed upon.

BATTERY ROCK, (897 miles below Pittsburg.)

This place frequently becomes troublesome in low water. It has not been reported bad during the present season, up to this time, (September 13.)

TREADWATER BAR, (900 miles below Pittsburg.)

It becomes very shoal at low water, but this part of the river has not been at a very low stage this season.

WALKER'S BAR, (910 miles below Pittsburg.)

This bar sometimes gets very shoal, and it has been reported difficult this year, although the river has not been at its lowest stage at this point.

BIG HURRICANE ISLAND, (913 miles below Pittsburg.)

This is a shoal, crooked, and difficult place. It could be improved by closing the right side with a dam. It should only be improved in connection with an approvement of Walker's bar, by a series of dams across sluices, &c., for three

and a half miles. The extent of this work, even should it be deemed advisable, I am not yet prepared to state. There are several wrecks at the head of Big Hurricane island, which must be taken out. Their removal will improve the navigation.

TRYER, OR UPPER SISTER ISLAND, (938 miles below Pittsburg.)

A few loose rocks at this place should be taken out. A steamer was wrecked here during the war. Pilots, in trying to avoid these rocks, sometimes run aground.

SECOND SISTER ISLAND, (9382 miles below Pittsburg.)

It is often very shoal, and may be improved. No plan yet devised.

CUMBERLAND ISLAND, (951 miles below Pittsburg.)

This is a difficult place. The dam, to be effectual here, should be built from the head of Dog island to the head of Cumberland island. It would be nearly a mile longer, but would have generally a harder bottom, and would hold better than the old dam, constructed many years ago. At the foot of the island, dams like the dams at Petticoat bar might be required, though longer. But as there is less current in this place, and nearly all other places below Louisville, than there is on the Upper Ohio, lighter dams will answer.

COTTONWOOD BAR, (956 miles below Pittsburg.)

This is sometimes a difficult place. When the other shoals of the Lower Ohio are improved, an improvement at this place may be devised.

TOWHEAD, FOOT OF TENNESSEE ISLAND, (965½ miles below Pittsburg.)
The same remarks as above apply here.

GRAND CHAIN, (990 miles below Pittsburg.)

This well-known and sometimes very troublesome place extends for about eight miles. The difficulties here are created by a complication of causes, consisting of crooked channels, rocks, sunken logs, wrecks, and the absence of proper land-marks and lights. I have, however, deferred my special report upon this important locality until after an opportunity for a critical examination in a low stage of water.

CACHE ISLAND, (1,007 miles below Pittsburg.)

Navigation at this point only becomes troublesome when the Mississippi, at Cairo, is very low.

This completes the enumeration of all the most prominent points on the Ohio requiring the immediate attention of the government. There are a number of intermediate minor matters, which come under notice as we progress in our operations, which cannot yet be satisfactorily introduced into an annual report.

From the foregoing description of the difficulties in the way of a perfect navigation of this important national highway, it will be seen that, while accuracy as to the probable actual necessary expenditure at particular points is yet unattainable, a very fair general approximate estimate may be made of the probable cost of perfecting such works as are likely to be found necessary and reasonably practicable, and which can be effected at moderate cost, considering the great advantages to be secured to commerce. Without, therefore, having adequate data at a number of places needing improvement, I feel warranted in offering at this time an approximate estimate of the probable cost of completing the necessary improvements of the navigation upon the general system adopted. Above

Louisville, and especially above Cincinnati, a nearer approach to the probable requirements can be made than at points below, owing to the difference in the natural regimen of the river. Below Louisville, on the Lower Ohio, the question is complicated by the river being so much wider, the fall and current so much less, and by the formation of extensive moving sand shoals, the perfect management and control of which has not yet been attained.

RIPRAP DAMS, AND OTHER WORK.

Although I must await the result of the surveys now in progress before reporting all the details of the various points that seem to require special work for the improvement of the low-water navigation, yet an approximate estimate may be made nearly enough to enable the department to decide upon the amount proper to be appropriated for the coming year in this connection. I am now able to make a closer approximate estimate than could be made in September of last year upon the works then recommended to be put under contract, at which time I was also unaware of the rigid character of the act of Congress respecting appropriations in connection with limitations of contracts. I was chiefly desirous at that time that the government should not advertise for more stone than might be ultimately found necessary at those points which were afterwards put under contract between Pittsburg and Buffington island.

Approximate estimate of the cost of completing the system of improvements on the Ohio river, on the general plan of wing dams, with occasional dredging.

1.—BETWEEN PITTSBURG AND LOUISVILLE, 615 MILES.

From Pittsburg to-

*Glass House ripple, two miles	\$30,000
Horse-tail ripple, five and a half miles	500
Head of Davis island, five and a half miles	3,000
Duff's ripple, eight and a half miles	2,000
Merriman's ripple, ten miles	1,500
Deadman's ripple, fourteen miles	1,500
Beaver shoals, twenty-seven miles	20,000
Raccoon shoals, twenty-nine miles	1,000
Montgomery island, thirty-two miles	200
Phillis island, thirty-five miles	200
Georgetown island, thirty-eight miles	300
Line island, forty-one and a half miles	800
Baker's island, forty-nine and a half miles	6,000
Chester's island, fifty-two miles	15, 000
Black's island, fifty-four miles	4,000
Brown's island, sixty-one and three-fourths miles	2, 400
Ferry bar, sixty-six miles	500
Well's bar, sixty-eight miles	1,000
Mingo island, seventy miles	6,000
Cox bar, seventy-two miles	5 00
Beach Bottom bar, seventy-seven and a half miles	7,000
Burlington bar, eighty-six and a fourth miles	15,000
Wheeling island, ninety miles	10,000
Bogg's island, ninety-two and a half miles	400
McMahan creek bar, ninety-four miles	9,000
Fishing creek bar, one hundred and twenty-seven and half miles	8, 500

^{*}In case of the adoption of another plan, involving a dam below Burnott's island, the cost would be \$240,000.

From Pittsburg to-

From Pittsburg to—	
Williamson island, one hundred and thirty-three miles	\$ 7,000
Whitton's towheads, one hundred and thirty-four and half miles	10,000
Wells's island, one hundred and thirty-eight and a half miles,	10,000
Will Creek island, one hundred and forty and a half miles	3, 500
Grand View island, one hundred and forty-one and a half miles,	3,000
Three Brothers, one hundred and fifty-eight and three-fourth	
miles	1,500
*Carpenter's bar, one hundred and sixty-six miles	25, 000
†Marietta island, one hundred and sixty-seven and three-fourth	
miles	20,000
Cole's island, one hundred and eighty miles	7,000
Newburg bar, one hundred and ninety-three miles	7,000
Bellville island, two hundred and two miles	10,000
Grand Creek bar, two hundred and nineteen miles	2,500
Goose island, two hundred and twenty-seven miles	9, 500
Letart's island, two hundred and thirty-two miles	7,500
Eight Mile island, two hundred and fifty-five miles	3,000
Raccoon island, two hundred and seventy-three miles	700
Green Bottom ripple, two hundred and eighty-nine and a half	
miles	1, 0 00
Guyandotte bar, three hundred and two and a half miles	3, 500
Burlington, or Twelve-pole creek, three hundred and twelve	•, • • •
miles	10,000
†Poag's shallows, three hundred and twenty-two and a half miles,	17,000
Brush Creek island, three hundred and eighty-one miles	14,000
Manchester island, three hundred and ninety-one miles	2,000
Charleston bar, four hundred and eleven miles	11,000
Foot of Augusta bar, four hundred and twenty-six miles	3,000
Four Mile creek, four hundred and fifty-nine and a half miles,	10,000
Bosley's, or Medoc's bar, four hundred and eighty-eight miles,	8, 000
Rising Sun bar, five hundred and eight miles	25, 000
Gunpowder Creek bar, five bundred and sixteen miles	21,000
Warsaw bar, five hundred and thirty-four miles	15,000
Grassy Flat bar, five hundred and ninety-five and a half-miles,	16,000
Louisville, six hundred and fifteen miles.	10,000
Down Hilly will be before the before the before	
	430,000
Add ten per cent. for superintendence and contingencies	43,000
Total	473,000
· · · · ·	=
2.—BETWEEN LOUISVILLE AND CAIRO, 400 MILES.	
From Pittsburg to-	
§ Portland bar, six hundred and twenty miles	910 000
Flint island, seven hundred and seven miles	15 000
Oil creek, seven hundred and eleven miles	12, 000
Holt's bar, seven hundred and twenty-one miles	14,000
Puppy creek, seven hundred and seventy-eight miles	15,000
"It is not certain that it will be necessary to build this dam. It will depend the dame at Mariette island.	upon what
may be done at Marietta island. † It is believed that any less expenditure at this point would be ineffectual, or	any plan
that would be deemed proper.	,
t This applies to a distance of forty-six miles along the river.	
§ For work between the falls and New Albany. Probably the construction of a dam here might reduce the work at Flint island	3
" and and an a same note will be teduce the work at Little istant	••

\$ 12, 000
20, 000
15, 000
12,000
10,000
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20,000
30,000
330, 000
33, 000
363,000
\$473,000
363,000
836, 000

The sum of eight hundred and thirty-six thousand dollars, in addition to the former appropriations applicable to the improvement of the Ohio river, which former appropriation may be considered as about absorbed by present contracts and necessary general expenses, will, it is presumed, complete the work on the Ohio river, between Pittsburg and Cairo, a distance assumed to be 1,015 miles. All of the work can probably be done during the residue of the fiscal year 1867, and the season of 1863. Contracts cannot be made, as I understand, in advance of appropriations; I would therefore respectfully recommend that the above sum be appropriated in such a manner that the department may be able to make contracts without being compelled to lose any of the favorable parts of the season. If the contracts could be made not later than April, 1868, and a favorable season should follow, the whole series of works might be entirely completed before the setting in of winter the same year, and in many of them time for the fall navigation.

^{*}This would apply to work that may have to be done near the island, and for a mile or more below.

[†]In addition to removing numerous sunken logs, &c., it is possible that a dam may be made available.

[†]This is intended to apply to dams that will probably be found necessary between Walker's bar and foot of Big Hurricane island.

A special report to be shortly presented to the department, relating to several different plans for the improvement of Glass House Ripple, two miles below Pittsburg, will show upon one of the plans proposed a considerably larger estimated cost on the alternative plan than is included in the foregoing. It is referred to in a note in the tabular statement of estimated amount required.

The department will understand that I do not profess to have yet arrived at a perfect knowledge of the Ohio river, or all its requirements, in connection even with the present system of improvement; nevertheless, I feel confident that the benefits to be immediately secured to the navigation and commercial interests connected with the river, by the completion of the proposed works will be very great, considering the comparatively small sum necessary for this purpose. It may well be asked, what is a million of dollars compared with the advantages of a much safer river, one thousand miles in length, improved so far as to have always a better low-water navigation, lasting through a longer season each year, and at the same time a superior navigation to the old river in medium and coalboat stages.

Before concluding, it may not be improper for me to refer to a matter which may be worthy of consideration. Practical difficulties, as the department is aware, sometimes arise in awarding and making contracts for the government, owing to the peculiar wording of the acts of Congress regulating letting of public The intention obviously was to establish guards against favoritism in awarding contracts, and so secure as far as possible experts for doing the different branches of work. I think, however, that the public interests could be subserved and proper security still retained by some modification of the present laws, whereby the highest officer of a department should be clothed with some discretionary authority sufficient to present an unnecessary division into separate contracts of parts of a work which ought properly to be in one contract. has been comparatively little trouble from this source, as yet, on this improvement, owing to the simple character of the operations hitherto commenced. But in case extensive and various kinds of public improvements by the government should continue, it may be important to adopt the best and most effective mode of letting and awarding contracts, as a matter of real economy in the end, and also as a means of securing the best class of contractors. Probably the present laws were arranged more with reference to army contracts than to works of internal improvement.

I would respectfully state that all the (civil) officers engaged in making the river surveys, and in superintending the operations on the improvement under my charge, have conducted themselves in the most satisfactory manner. It is due also to the contractors engaged in constructing the riprap dams, that I should bear testimony to their uniform and most faithful management of the

works undertaken by them.

I would take occasion to mention that in my report of December 20, 1866, printed in the "Appendix to report of Chief of Engineers, respecting certain public works," Ex. Doc. 56, part 2, 39th Congress, 2d session, at pages 321 and 322, there is an error in the heading of a table of the stages of water, by the transposing of "Cincinnati" and "Louisville," at the top of the table.

Respectfully submitted:

W. MILNOR ROBERTS,

Superintending Engineer Ohio River Improvement.

PITTSBURG, PA., September 14, 1867.

APPENDIX K.

ENGINEER OFFICE, HARBOR DEFENCES. Baltimore, Md., July 1, 1867.

GENERAL: In obedience to circular from engineer department, dated June 10, 1867, calling for information on certain points connected with the work for "river and harbor improvements" under my charge, viz., the improvement of the Patapsco below Fort McHenry, and of the Susquehanna below Havre de Grace, I respectfully report as follows:

IMPROVEMENT OF PATAPSCO.

Paragraph 1.—Thorough and elaborate surveys have been completed of all the space below Fort Carroll covered by this improvement, and the result has shown conclusively the advisability of a change in the line of the channel from that of the lower part of the old dredged route, or "Brewerton channel." The plan adopted is to dredge a channel 200 feet wide and twenty-two feet deep at mean low water, following the line of the Brewerton channel to a point just below Seven-foot Knoll light-house, where it leaves the old route and takes a nearly due south direction, striking the deep water of the bay near foot of Gibson's island. The expenditures under this plan have been confined to repairs of dredges, scows, and tug.

Paragraph 2.—The estimated amount required to complete this improvement

is **\$**250,000.

Paragraph 3.—The amount to be profitably expended during next ensuing fiscal year is \$125,000.

Paragraph 4.—The work is located in the collection district of Baltimore.

Paragraph 5.—Near port of Baltimore, extends from Lazaretto light, past Fort Carroll and Seven-foot Knoll light, and below Bodkin Point light.

Paragraph 6.—The amount of revenue collected during last fiscal year was **\$**5,613,983.

Paragraph 7. The whole foreign and coastwise trade of Baltimore, including regular lines of packets to Liverpool, Bremen, Hamburg, the West India islands, and ports on the coast north and south.

Paragraph 8.—Abstract of proposals for repairs of hulls and wood-work of United States steam dredges, tug, and scows.

Bids received.	Cooper & Slicer.	Hooper &	Jones & Ashcroft.	J. T. Tardy & Bro.	J. H. Smith & Co.
MATERIALS.					
	No. 1.	No. 2.	No. 3.	No. 4.	No. 5.
Select white oak plank, per foot,	1	1			
board measure	80 07	\$ 0 06	\$ 0 06	\$0 07	\$ 0 06
Select white oak timber, per cubic foot.	pads 72 dippers 60	48	54	60	48
Select North Carolina timber, per cubic foot.	pads 54 dippers 42	48	54	60	· 54
Select North Carolina plank, per	arppoint in			1	
foot, board measure	05	06	06	07	06
Hand-made wrought iron spikes .	. 13	12	11	15	10
Machine-made wr't iron spikes	12	10	11	15	091
Bolt iron, per pound	07	0년	07	10	07
Navy oakum, per pound	13	12	12	16	12
North Carolina pitch, per gallon .	35	40	30	50	12 28
Gas tar, per gallon	20	25	20]	25	25
00 37-1 ::	•			•	

Paragraph 8.—Abstract of proposals for repairs, &c.—Continued.

Bids received.	Cooper & Slicer.	Hooper & Co.	Jones & Ashcroft.	J. T. Tardy & Bro.	J. H. Smith
LABOR. Sawing, per running foot	No. 1.	No. 2.	No. 3.	No. 4.	No. 5.
Use of shears, per day	25 00	\$2 5 00	\$3 0 00	\$ 15 00	\$5 00 35 00
Hauling upon railway, per job, tug.	10 00	15 00	10 00 20 00		10 00 12 00
Use of ways, per day, dredges Dodoscows Dodotug	12 00 5 00	price.	15 00 5 00 10 00	5 00	17 (3) 5 (0) 6 (0)
Ship carpenters per hour. Ship smiths do Ship caulkers do	37± 37± 36	35 35 35	35 38 33	35 40 35	35 35 35
Ship scrapersdo		35 25	33 28	35 20	30 25

J. H. Smith & Co. accepted.

Abstract of proposals for repairs of machinery of dredges, scows, and tug.

Bids received.	Walls & Son.	II.C. Laws- bee, sgent.	Hazelhurst & Co.	Murray, Clark & Co.	E. J. Cord
MATERIALS. Boiler iron per pound. Boiler rivets do. Wrought iron do. Hammered iron do. St el do. Iron castings do. Brass castings do. Composition castings, per pound. Sheet copper do. Three-inch copper boiler tubes, per foot.	No. 1. \$0 09 09 07 10 30 (†) 30 38 3 50	No. 2. (*) \$0 12 10 12 45 45 45 (\$)	No. 3. \$0 06} 081 07 08 25 044 30 30 38	09 07 12 24	No.5. \$0 10 10 10 22 (‡) 40 35 (¶)
Boiler makers per hour. Forging and forge do Drilling and drills do Boring and tools do Planing and tools do Turning and tools do Carpenters' work do Pattern makers do Fitters do Machinists do	60 · 35	35 75 60 75 75 75 35	30 75 40 75 50 50 35 40 32	35 76 55 70 80 75 35	36 65 69 80 65 77 31

^{*} Flange 10 cents, refined 8 cents.
† Loam 8 cents, machinery 4 cents.
‡ Loam 8 cents, machinery 6.

Bid of Hazlehurst & Co. accepted.

^{§ 15} per cent. above manufacturers' prices. || 25 per cent. above manufacturers' prices. || Lowest price ruling when required.

Abstract of proposals for coal.

Bids received.	Price per ton of 2,240 lbs.
1. S. M. Johnston. 2. Taylor & Chamberlain	\$5 30 5 80 5 25

Bid of J. Hankey accepted.

Abstract of proposals for canvas roof-covering.

	-	-	•		-		_					
Bids received.					Pr	ice p	r squ	are	ya	rd c	ompl	ete.
No. 1-J. J. Gray & Co.				 	 			 .			\$0	85
No. 2—J. C. Brown				 	 						. 0	99
No. 3—J. C. Brown				 	 						. 0	97
No. 4—Tall & Edgar				 	 	•					. 1	03
Bid of J. J. Gray & Co	acce	pted	l .					٠				

Paragraph 9.—Abstract of contracts for repairs of machinery of tug and dredges—Hazlehurst & Co., contractors; prices as per schedule of proposals.

Contract for repairs of hulls and wood-work of tug, dredges, and scows—J.H. Smith & Co., contractors; prices as per proposals.

Contract for coal—J. Hankey, contractor; \$5 25 per ton.

Contract for canvas roofing—J. J. Gray & Co., contractor; eighty-five cents per square yard.

IMPROVEMENT OF SUSQUEHANNA BRLOW HAVRE DE GRACE, MARYLAND.

The resurvey of the area covered by this improvement showed that, under the existing regimen of the river, the decided tendency was to obliterate the effects of dredging almost entirely, and in view of this fact it has been determined to try the effect of narrowing the width of the water-way in connection with dredging.

The plan adopted (but not yet commenced) is to construct, near the point where the river expands greatly, a movable dam or breakwater to deflect the

current of water toward the line of the main channel.

It is proposed to form the deflector of large rafts, chained together, each raft carrying a row of sheet-piling which will penetrate the bottom only enough to steady the rafts. The rafts in sections will be supported in position by three cribs of timber filled with stone.

The deflector is arranged with a view to its removal in the winter, and it should be replaced by a permanent work, should it prove effectual in its operations.

It is estimated that \$50,000 would be required to make this improvement permanent, and this sum can be advantageously expended next year in dredging and building a permanent deflector.

The work is located in the collection district of Baltimore. Baltimore is the nearest port of entry. The work commences nearly opposite Point Concord

light, and extends past Donohue's Fishing Battery light.

The coal and lumber trade between Baltimore and Philadelphia and the lumber and coal regions of the upper Susquehanna all passes through the channel to be improved.

No proposals issued as yet. No contracts executed as yet.

Respectfully submitted:

WM. P. CRAIGHILL.

Brevet Lieutenant Colonel, Major of Engineers.

Major General A. A. HUMPHREYS, Chief of Engineers, U. S. Army, Washington, D. C. BALTIMORE, MARYLAND, March 15, 1867.

GENERAL: Congress, by appropriating at its last session for the improvement of the Patapaco river below Fort McHenry \$75,000, (the amount asked for one year's operations in my report on the subject to you, dated January 15, 1867,) may perhaps be considered as having thereby approved the project of improvement upon which the estimate in that report was based. This project, it will be recollected, contemplated a change in the direction, for a considerable portion of its length, of the improved main ship channel from the bay to the city of Baltimore.

The direction which it is proposed to change was adopted some fifteen years ago, at the suggestion of Brevet Brigadier General Henry Brewerton, then captain in the corps of engineers, who had given much study to the subject and had had an extensive experience in similar improvements. His plan was approved, it is supposed, by the board of engineers for such works and by the

engineer department.

My opinion with reference to the propriety of the change of direction is very decided. The reasons for it have been given in previous communications.

Considering all the circumstances of the adoption of the direction of the existing "Brewerton" channel, and the extent to which the interests of the city of Baltimore—the third city on the Atlantic coast in commerce and population—are involved in the selection of the best direction for the main ship channel to its wharves, it is respectfully requested that the subject be committed to a board of engineers for examination and report before a final decision is made.

I request, also, that the consideration of the improvement of the Susquehanna

river at Havre de Grace be committed to the same board.

Very respectfully, your obedient servant,

WM. P. CRAIGHILL,

Major of Engineers, Brevet Lieutenant Colonel. Major General A. A. Humphreys,

Chief of Engineers, U. S. Army.

WASHINGTON, D. C., May 23, 1867.

GENERAL: I have the honor to transmit herewith, in two sheets, the map of the recent survey of the Brewerton channel in the Patapsco river, and of a portion of Chesapeake bay south and east of the Seven-foot Knoll light. The ground covered by these sheets is included in Coast Survey coast chart No. 31, a copy of which was transmitted to you with my letter of this date relative to the Suquehanna river. A reference to that chart is invited, in connection with the sheets now enclosed.

The original design for the improvement of this entrance was to open a channel, one hundred and fifty feet in width and twenty-two feet in depth at mean low water, from the mouth of the basin, between Fort McHenry and the Lazaretto light, to the entrance buoy at the mouth of the Patapsco river. The channel, when completed, was intended to be in two straight lines, intersecting at a point a little below Fort Carroll.

The best water along the proposed route being in the portion above Fort Carroll, it was obviously proper to commence the work of improvement by dredging below Fort Carroll, and the dredges have thus far been worked only

in that portion of the proposed channel.

It may usually be anticipated that a long straight cut like this will not be a permanent improvement. There were peculiar reasons for the expectation that this case might prove an exceptional one. These were, that the Patapeco river above Baltimore is a small stream, and when expanded into a much broader

bed, as it is above Fort McHenry, the current becomes sluggish, and deposits in that part of its course the matter it may have brought down. The water is then comparatively free from such matter, and this probably accounts for the greater depth of water observable from Fort McHenry to Fort Carroll than below. Moreover the mean rise and fall of the tides is only about one foot. In consideration of the small volume of water coming down past Fort McHenry, with a velocity diminished even from what it had above, owing to a continued expansion below of the water-way, and in consideration of the small change in the water-level, owing to the small mean rise and fall of the tides, it was supposed that the water was in a condition approaching stagnation, and that hence a straight dredged channel would be permanent.

This expectation was, to a certain extent, well founded, and the dredged channel is quite fixed in condition until it crosses the line tangent to the right bank of the Susquehanna, when it becomes subject to the influences of the

current of that river sweeping almost perpendicularly across it.

There is a great shoal or bar seen below Fort Carroll, the cause of which is not positively known, inasmuch as Curtis creek and Bear creek are not streams in the proper sense, but single short projections into the land. What is meant is that they have little or no current except what is produced by the tide. A certain amount of matter is undoubtedly brought down them on the ebb tide, and this assists in forming the great shoal below Fort Carroll; but the existence of this shoal is supposed to be chiefly due to the conflict of the currents thereabouts, caused by the tides in the bay and the two rivers, as well as by the outflow of the two latter, independent of the tides.

It is my belief that the straight cut below Fort Carroll will in time fill up, to a certain extent, if left entirely to the action of natural causes. The portion above North Point will be more permanent, but the same causes which produced that shoal will, if their operation continues unaltered, reproduce the same effect, which in this case amounts to a filling of the channel; but the process will be a slow one. The remaining portion of the straight cut below North Point will be more rapidly filled. This belief led me to see whether a better direction, and one likely to be more permanent, could not be given to this portion of the channel.

An examination of the map of the space to the south of the Brewerton channel and to the east of Seven-foot Knoll light showed that, were it not for the lumps scattered over that space, a decidedly better direction for the channel would be obtained by leaving the Brewerton cut at a point a little to the east of the Seven-foot Knoll light, running almost due south for a distance of about three miles and then turning off in a southeasterly course, passing to the north of the Belvidere shoal. The direction thus indicated is about that of the resultant current of the Patapsco and Susquehanna rivers. The principal obstructions are the lumps referred to. These are hard, and their remaining where they are, taken with the fact that the water is considerably deeper in their immediate vicinity, showed that everything movable by the current has been removed. If these lumps were taken out by dredging, it is believed a permanent and good channel would remain in this part.

I am told that it is apprehended that some of the lumps remaining near, but not in the channel, might prove dangerous to vessels which should happen from any cause to get out of the channel and strike upon them. This is a danger undoubtedly to be considered as of importance so long as a channel of but small width is provided, and that marked out only by buoys; but this danger would disappear by substituting fixed beacons, for use both day and night, and giving the channel greater width thereabouts. It is proposed to ascertain the opinion on this point of the principal ship-owners, shipmasters, and intelligent pilots of

the city of Baltimore.

The advantages of this proposed direction are cheapness in first cost, permanence when once obtained, a smaller accumulation of ice in winter, and a small gain in distance.

To obviate, to a considerable extent, the danger mentioned above, it is proposed not to confine ourselves simply to the removal of lumps in the channel-

way, but to remove them for some distance on either side.

To keep open the straight cut from the Seven-foot Knoll light to Sparrow's Point, which cannot be improved as to direction, will require occasional dredging. There are in the possession of the engineer department, already, four dredges and appurtenances, purchased from funds appropriated some years ago by Congress for the improvement of the Patapsco river. When the funds provided from the same source fail, the machinery remains available, and it would be well worth the money that would be expended if the authorities of the State and city would raise a fund sufficient to continue this machinery in action, whenever requisite to keep open a good channel to the wharves of the chief city of their State, and one of the most important on the Atlantic coast.

It is considered that 150 feet is not a sufficient width for the channel, and

that it should be made 200 feet at least.

Seventy-five thousand dollars are now available for this improvement, which sum was appropriated at the last session of Congress. It is intended to apply this at once in repairing the machinery, and then working it in the channels at such points as to give, as far as practicable, a uniform width and depth throughout, which will not, of course, be the width and depth ultimately expected, as the sum available is not sufficient therefor. Estimates on this point have already been submitted to you, and through you to the Secretary of War, and to Congress.

The question which, it seems to me, should now be settled is, whether to consider the straight direction to the entrance buoy as finally adopted, or shall the change above proposed be approved. Upon this point I respectfully request

an early decision.

In connection with this subject one or two other remarks may be pertinent. There are in the possession of the authorities of the city or State several dredges, which are usually occupied in dredging in the basin, immediately about the wharves of the city, certainly above Fort McHenry. The matter dredged is that which comes from the sewers of the city, and from the small stream called Jones's Falls. This matter is deposited on the flats outside of Fort McHenry, but without much consideration as to locality, and the probable injurious effect of this deposition on the channel below.

If the general government supplies any or all of the money for improving the entrance to the harbor of Baltimore, it is directly interested in seeing that nothing is done, through ignorance or want of due consideration, to thwart its efforts in that direction. Some means should be adopted of regulating the place of the deposit referred to above. The excavated matter might be utilized as

manure, or in some other way.

The great length of the straight Brewerton cut, and the consequent length of time during which vessels entering by that channel continue upon a single course, led me to think this an unusually favorable occasion for the use of fixed

range beacons.

The attention of the Light-house Board being called to the matter, the suggestion was adopted, and Congress, upon application from that board, made the necessary appropriation. I was gratified, yesterday, to learn that the construction of one of the beacons is in progress, and the other will be shortly begun. This will be a most important aid to properly marking out the exact position of the Brewerton cut, and calling attention to it. The pilots of Baltimore generally decry the Brewerton cut, the real reason therefor being found in the fact that its straightness, and the consequent simplicity of its navigation when it is

once known and accurately marked, would almost, if not entirely, supersede the necessity for their aid.

This is one consideration with them, and another is found in the tendency among persons of this class to cling to old things, and to look with suspicion on

what is a novelty, even if an admitted improvement.

There has been some ground for complaint as to the difficulty of using the Brewerton cut—that it is necessary to have the channel, being so narrow, very carefully buoyed.

Buoys are very likely to get out of place, either by accident or by their deliberate removal by those whose interest it is to increase, in the eyes of all others than themselves, the difficulty of the navigation. Such removal of the buoys

in this channel is known to me to have occurred.

This difficulty will be removed by the erection of the beacons, and the advantages of the improvement more thoroughly appreciated by the commercial community than has hitherto been the case.

Additional current observations are needed near the Seven-foot Knoll light,

and these have been ordered.

It is proposed during the present season to complete the survey of the channel above Fort Carroll. This is necessary, as the erection of Fort Carroll itself, as well as the deposition of the matter excavated from the basin, must have had a considerable effect upon that portion of the channel, and it is very desirable to know certainly what this effect has been.

Very respectfully, general, your obedient servant,

WM. P. CRAIGHILL,

Major of Engineers, Brevet Lieutenant Colonel.

Major General A. A. HUMPHREYS,

Chref of Engineers U. S. A., Washington, D. C.

Engineer Department, Washington, May 29, 1867.

SIR: The act of Congress making an appropriation of \$75,000 for the improvement of the Patapsco river provides that "it shall be the duty of the Secretary of War to apply the sum herein appropriated for other purposes than for examinations and surveys by contract provided, however, that when, from the nature of the work to be done, the same cannot, in the judgment of the Secretary, be made the subject of contract, the necessary expenditure may be otherwise ordered."

At the time of the passage of the act there were already on hand belonging to the United States, on account of the improvement of the Patapsco river, one

steam-tug, four steam-dredges, and a number of scows.

The question now arises whether this valuable machinery should be placed in the hands of contractors to be worked by them, or shall the government proceed to work it by contracting for the fuel and hiring, at the usual market rates, employés of the various grades, namely, engineers, pilots, firemen, deck hands, dipper tenders, and laborers.

The latter course would be most advantageous to the interests involved; the

work is not adapted to or suitable to be done by contract.

If it is thought that the language of the act quoted must be taken literally and strictly and without qualification, growing out of the nature of the work to be done, then the boats and machinery must be put in the hands of contractors. If, however, the language be construed to mean that when, from the nature of the work to be done, the same cannot properly be made the subject of con-

tract, it may be otherwise ordered, then the engineers and others may be hired by the officer in charge of the work.

Very respectfully, your obedient servant,

A. A. HUMPHREYS,

Brig. Gen. and Chief of Engineers, Maj. Gen. Vols.

Hon. E. M. STANTON, Secretary of War.

As from its nature this work cannot be made the subject of contract with due regard to the public interest, the Secretary of War authorizes the Chief of Engineers to have it done otherwise.

ED. SCHRIVER, Major General.

WAR DEPARTMENT, June 6, 1867.

WASHINGTON, June 28, 1867.

GENERAL: All the data requisite seem now provided upon which to base a determination of the question whether the ship channel in the Patapaco river, below Fort Carroll, shall be in a straight line, or there shall be a change of driection towards the south of a point near Seven-foot knoll, as proposed some time since.

My opinion on the subject is known to you, in which I am the more confirmed by the results of the late survey. The map was transmitted to the engineer bureau with my letter of May 23, and additions to it of later current observations requested by letter of June 26.

From information received from the president of the Board of Trade of Baltimore, and from others, I believe it to be the general sentiment of those who are most deeply interested in the commercial prosperity of the city that the ship channel should turn to the southward near Seven-foot knoll.

I enclose an outline tracing showing the particular location proposed for the channel. If this tracing be placed over the proper sheet of soundings, transmitted with my letter of May 23, already referred to, the reasons for the particular location selected will become more apparent. The directions given to the lines of the channel are taken as they are, in order that in its navigation use may be made of the beacon already constructed on the Seven-foot knoll. It is expected that a new beacon will be erected, at the point shown on tracing, where the channel is to change its direction from a north and south line to one bearing northwest or southeast.

In entering the channel from the bay a vessel would keep upon the range of the proposed angle beacon and Seven-foot Knoll beacon until she passed the angle beacon, when she would steer due north until reaching the line of the Brewerton channel, indicated by the range beacons for that channel, about to be put in place by the light-house department. In passing out of the river a vessel would keep in the Brewerton channel until the new angle beacon was in a due southerly direction, when she would steer upon that course until she reached the angle beacon, and then she would steer upon the range of the angle beacon and the Seven-foot Knoll beacon. Vessels should always pass to the west of the new angle beacon.

I request authority to consider as adopted the change of direction of the channel proposed, and to apply the funds available in the manner to be indicated below.

It is thought that the channel should at no place have a less width than 200

feet; and estimates for such a width and a depth of 22 feet at mean low water have been submitted. The sum appropriated and available is insufficient for the completion of the improvement, which includes work between Forts Carroll and McHenry. It is proposed to apply the whole sum in hand below Fort Carroll.

The matter to be excavated from the Brewerton channel has a very different character from that of the material to be removed from the new location, being in the former a quite soft mixture of mud and sand continuously distributed; but in the latter it is a rather hard concrete of oyster shells, sand, and mud in isolated lumps. The process of dredging with the Osgood machine will answer in both cases.

Operations have been delayed by causes beyond my control. In consideration of this fact, and influenced by the desire, since the proper plan of improvement now seems clear, to produce as great a result in the remainder of the present season as the available means will allow, I propose to employ in the new part of the channel, in the manner authorized by the Secretary of War, that is, by days-work, the dredges belonging to the United States on account of this improvement, and to engage other dredges, if possible, to work, under contract, in the Brewerton channel by the cubic yard. This, it is considered, will be the most advantageous distribution of the owned and hired dredges.

The hard lumps are a much more dangerous obstruction than soft mud, and it is therefore proposed to give the channel through them a greater width than 200 feet, but to confine the work of this season in the Brewerton channel to clearing out to a width of 150 feet and a depth of 22 feet, and afterwards, if

the means should be provided, to increase the width to 200 feet.

An early decision on the propositions of this letter is respectfully requested. Very respectfully, your obedient servant,

WM. P. CRAIGHILL,

Major of Engineers, Brevet Lieutenant Colonel.

Major General A. A. HUMPHRBYS,

Chief of Engineers U. S. A., Washington, D. C.

K 1.

United States Engineers' Office, Baltimore, Maryland, June 26, 1867.

GENERAL: In connection with this letter I request that reference be made to sheet C, transmitted with my letter of 23d ultimo, relative to the Susquehanna river improvement. That sheet is the map of the last survey of the river near Havre de Grace. The following estimate is derived from the data of that map: To reopen the channel formerly dredged below Havre de Grace to a width of

The price per cubic yard is that at which dredging by contract is now in progress at that place.

The money appropriated by Congress and now available for the improvement

of this river is \$26,400.

The estimate (see my letter of 4th instant) for a movable deflector amounted to \$20,200, of which \$3,600 were to be applied to three fixed cribs as points of support.

If the available funds are applied to dredging by contract a channel in the old direction, and only eight feet deep, there would remain but \$9,000 for the

deflector, which would be insufficient to make it properly.

Sheet C shows a considerable filling at the lower end of upper cut. Representations have lately been made to me that the commercial interests of the community are seriously impaired by this obstruction. There is a strong desire that immediate steps be taken to remove it. I propose to open at once, by contract, a channel, shown by the dotted red lines on enclosed tracing, 100 feet wide and eight feet deep. This will, I am informed, give relief to the immediate and pressing wants of the community. The cost of the work will be \$3,000. I ask authority to undertake this at once.

The Coast Survey party at my disposal have finished what is now required in connection with the Patapsco river improvement, and will immediately proceed with the additional current observations needed near Havre de Grace in the Susquehanna, and with the additional soundings requisite to complete sheet C

to the new railroad bridge.

I request authority to proceed as soon as practicable with the execution of the movable jetty, for plans and estimates of which see letters of 23d ultimo and June 4. I propose to locate first one or more of the cribs, the precise position to be determined after the completion of the current observations. The construction of the movable part of the deflector would be commenced later, to be extended this season as far as the means available will permit. It will be understood that a part of the plan is to swing the deflector out of the way of danger from ice when that danger threatens, restoring it to its place in the early spring when the ice has cleared out of the river.

In addition to the dredging recommended in this communication as of immediate importance, it will probably be desirable to dredge more at a later period near the same point, and also near the lower half of the lower cut. This should

also be done by contract and possibly during this season.

Last year, before the passage of the law making appropriations for the improvement of the Patapsco and Susquehanna rivers, the Chief of Engineers authorized me to draw for \$20,000 from the general appropriation for harbors on the Atlantic coast, to be divided according to my discretion equitably between

the improvements.

A large portion of this sum was consumed in repairs of the machinery belonging to the Patapsco river, with the intention of dividing the remainder equally between the two places. When the special appropriations were made, the smaller sum of \$5,200 was provided for the Patapsco, the more important object, and the larger sum of \$26,400 for the Susquehanna, the less important object. I then proposed to the engineer department to expend on the Patapsco last season the entire balance from the allotment from the general appropriation, and in addition the small special appropriation, and this season to spend its special appropriation on the Susquehanna, using, however, the machinery belonging to the Patapsco, but repaired from funds derived from the general appropriation.

This proposal was approved and would have been carried out, but the conditions of the case were again changed by the appropriation at the last session of Congress of \$75,000 for the Patapsco, and nothing for the Susquehanna, which required the machinery belonging to the Patapsco to be retained in its own im-

provement, and left the Susquehanna to be otherwise provided for.

In consideration of the facts stated above, and that the funds available for the Susquehanna may not suffice for what it is very desirable to do for it, I propose that the special appropriation for the Patapsco now return to the general appropriation for harbors on the Atlantic, \$10,000, and that that sum be held in reserve to be applied to the Susquehanna upon the exhaustion of the present

special appropriation, and in the event of no additional appropriation being made therefor at the next session of Congress.

This arrangement I consider entirely equitable, and I request your approval

of it.

Very respectfully, your obedient servant,

WM. P. CRAIGHILL,

Major of Engineers, Brevet Lieutenant Colonel.

Major General A. A. Humphrbys, Chief of Engineers U. S. A., Washington, D. C.

WASHINGTON, D. C., June 4, 1867.

GENERAL: I transmit herewith the estimates promised in my letter of 23d ultimo relative to the Susquehanna river, with a sketch of the proposed floating dam.

The sketch exhibits merely the idea. If that is approved the details will be elaborated.

It seems to me as settled that dredging alone, unless continuous, is not the expedient to be resorted to for the improvement of the navigation there. The

continual use of dredges cannot now be expected.

The probability seems great of a favorable result from the use of one or more jettees. If the system be adopted, the first to be erected (and that would probably be all that would be needed) would be about in the position indicated in my letter of 23d ultimo, referred to above. If it be decided to construct a jettee in that position, there are several objections to making it of stone alone, or of wooden cribs filled. The first and a very serious one is the expense. There is certainly not money enough available at this time for such a structure. Another objection is, that if built it might be found to act disadvantageously, or not as well as possible, owing to a faulty location. In this event its removal would be very expensive.

If the plan of a stone structure is given up we are brought to consider the propriety of adopting a piling which must be strongly braced to resist the action of ice. There is money enough available for such a structure, but with this exception the same objections, though less in degree, may be made to it as to the

stone jettee.

The floating dam has several advantages in its favor. The first cost is not great, as shown by the estimate, which is made upon a most liberal allowance as to prices. If the first position selected for it is found not to be the best, it is readily removed to another. In winter it may be entirely or partially removed, if necessary, to avoid danger to it from the ice. If found to fail utterly of the expected effect, the materials may be sold for nearly their first cost. If found to succeed, and it is desired to replace it by a fixed structure of wood alone, or of wood and stone combined, its materials themselves, or the proceeds of their sale, may be applied to the new work.

It is not considered necessary, and it may be disadvantageous, that the contact between the movable uprights shall be perfect, and that they shall all, or nearly all, touch the bottom. Some of them, however, should reach the bottom and penetrate to a small depth, in order to assist in steadying the rafts against the tendency to be partly submerged or moved out of place under the action of the currents. It may be necessary or advantageous, when considerable certainty is reached as to the best position for the jettee, to drive two or three piles along

each raft to give fixed points of support.

With reference to this location it may be properly stated that whatever may be the effect upon the channel of any structure there, and whatever its character, no damage will result from it, considered simply as an obstruction to navi-

gation, as it will be entirely out of the track of vessels.

It may be objected that a jettee in the proposed location, if it acts as a deflector for the downward current, will equally prevent the ascent of the flood. This is a well-founded objection, but it is not so forcible upon consideration as at first glance. In one view of the case we may disregard altogether the tidal currents, looking only to controlling the water belonging to the river, supposing it unaffected by the tides.

It is highly probable that the flood current is deflected to a very great degree at the southern edge of the large shoal at the mouth of the river, and that the flood current at the position selected for the jettee is almost inappreciable, and may be practically disregarded. If the flood is checked by the jetty there will be a proportional increase of flood in the channel, which will be at least not dis-

advantageous.

Additional current observations are about to be made here, the result of which will be speedily reported.

Very respectfully, your obedient servant,

WM. P. CRAIGHILL, Major of Engineers, But. Lieut. Col.

Major General A. A. Humphreys, Chief of Engineers U. S. A., Washington, D. C.

Estimate of cost of deflecting current of Susquehanna river at head of Devil's Island shoal, below Havre de Grace, Maryland, to accompany report of Brevet Lieutenant Colonel Craighill of June 4, 1867.

By rafts and sheet piling, forming floating dams, anchored to stone cribs, three (3) cribs, at \$1,200 each	\$ 3,600 1 5,600	00
Total cost	20, 200	00
By continuous piling, with back row and braces, ninety thousand (90,000) feet of piling and braces, at 25 cents per running foot. Prices used for above—	322, 500	00
Lumber, per 1,000 feet, board measure	25	00
Iron and labor, per cent. of value of lumber		30
Piles, running foot		15
per pile)	3	GO
Stone, per cubic yard, delivered in cribs	1	5 0

Engineer Office, Harbor Defences,

Baltimore, June 4, 1867.

WASHINGTON, D. C., May 23, 1867.

GENERAL: In compliance with your oral directions I have the honor to submit the following report with reference to the improvement of the Susquehannariver at Havre de Grace.

Accompanying this report are three sheets, as follows, viz: Coast Survey coast chart No. 31, marked A; tracing of survey of 1846, marked B; tracing of survey of 1867, marked O.

Upon B and C are marked in red the straight cuts dredged out seven years ago to a width of 100 feet and a depth of 10 feet, which was in conformity to the project of the board of engineers. B shows the channel and shoals as they were in 1846, fourteen years before the completion of the dredging. C shows them as they were in 1867, seven years after the completion of the dredging.

The general position and character of the channel and the shoals were not greatly different in 1846 and 1867; that is to say, the variations are probably not materially greater in the interval of twenty years than may occur in the same year, as the shoals are subject to changes from the action of freshets

and ice.

The conclusion to be derived from the facts before us is, that if a channel in this location be dredged out to a given depth and width, and then left untouched for several years, it will be filled up to such an extent as to be virtually in the

same condition as before the improvement was attempted.

It is then clear that a permanent improvement requires some plan different from that which has been executed thus far. If the cut formerly made is to be reopened, something is needed to keep it open. This may be done by constant attention, and dredging whenever a filling-up is seen to have begun. For this purpose machinery must be procured, and the means should be always available to set it in motion whenever required. It is not probable that these conditions will be satisfied, if at all, for many years to come, when the population of this section of the country may have become so dense and the importance of the navigation of the Susquehanna so great as to justify the continuous expenditure of the money requisite to keep this channel open.

In my last annual report the opinion was expressed that it might be found advantageous, after more study of this subject founded upon a survey which was necessary, to improve the shore channel by dredging in the spots where it was

needed and cutting through the shoal on line A A, sheet B.

A comparison of the maps of 1846 and 1867 leads me to abandon this idea. An examination of sheet A shows that the bed of the Susquehanna river for a number of miles above the Havre de Grace light is about one mile in width. At that point there is a sudden and very great expansion of the water-ways, so that the width of the opening is over five miles, if measured on a straight line drawn from near Carpenter's Point through the fishing battery light.

The effect of this sudden expansion is the formation of extensive shoals and of a channel with a diminished depth. A striking feature is the great shoal immediately in prolongation of the narrower portion of the river. This shoal may be supposed to have been formed in course of time by matter brought down the river and there deposited, owing to the diminution of the velocity of the cur-

rent consequent upon the sudden and great expansion of the water-way.

The volume of water coming down now impinges against this great shoal. The effect of this impact is seen in the deep sack formed in the edge of the shoal. A considerable portion of the water of the river spreads over the shoal and passes out eventually between Sandy and Turkey Points. The navigable channel from the head of Spesutie island to near the town of Havre de Grace, receives only the water which is deflected from the great shoal, and, indeed, only a part of that, as another portion is spread over the shoal to the westward of the channel.

If the water which does not now pass through the channel below Havre de Grace could be made to pass through it, there would be strong ground for expecting that the channel would be correspondingly and permanently improved.

To effect this object a deflector seems an obvious expedient, so placed and constructed as to catch the water now lost on the shoal, (which is itself a partial deflector,) and with such a direction that the desired deflection shall be gradual and complete.

I think the proper direction for the deflector is a line about parallel to the west-

ern edge of the great shoal near its northern portion.

The proper position for the deflector is probably a little to the eastward of the western edge of the shoal, and it should extend across the deep sack in the shoal, at or near its mouth. The line of it would be about b c b, sheet B.

As it is always impossible to foresee the effect of any interference with the natural regimen of a water-course, I should prefer that the deflector be temporary and removable if found to act disadvantageously, or when endangered by ice, and my wish was, and is, to devise a floating structure which would act as a deflector when anchored in position.

It is difficult, however, to arrange such a structure on account of the considerable depth to which it should sink in order to be effectual. I propose, therefore, a more fixed arrangement, which is, to sink three cribs filled with stone at the points b, c, and b, and connect them by a single row of piles driven very nearly, though not quite, in contact with each other, and connected at the top by a cap-

The effect of this structure should be very carefully observed. It may be found necessary to extend it towards the north or south, or to remove a portion of it. It is not improbable that it may be found expedient to erect a jettee also on the Havre de Grace side to give a better direction to the water, which now

spreads over the shoal westward of the channel.

An estimate is in course of preparation of the cost of such a jettee as is proposed above. If found to do what is expected of it, there should be formed a solid stone jettee on the line of the proposed piling, for which an estimate will be submitted.

There will also be prepared an estimate of a floating substitute for the piling. It would be desirable, simultaneously with the construction of the jettee, or antecedent thereto, to dredge through the shoal which has formed at the lower end of the upper cut, and to widen the channel about the lower half of the lower cut. An estimate will be prepared of the cost of reopening the old cuts to the depth of ten and the width of one hundred feet.

Some additional current observations are requsite in order to decide definitely

the precise position for the proposed jettee. These have been directed.

There is no pressure from any quarter for rapid progress in the work of improvement at this point, and it would seem advantageous, therefore, to proceed deliberately with it.

Very respectfully, your obedient servant,

W. P. CRAIGHILL,

Major of Engineers, Brevet Lieutenant Colonel.

Major General A. A. HUMPHRBYS, Chief of Engineers, U. S. A., Washington, D. C.

APPENDIX L.

U. S. ENGINBER'S OFFICE, PHILADRLPHIA, PA., September 11, 1867.

Six: In accordance with the requirements of circulars Nos. 11 and 24, engineer department, dated respectively the 10th of June and 29th of August, 1867, I have the honor to transmit herewith annual reports of the progress made in the following river and harbor improvements and surveys in my charge, together with the necessary abstracts of proposals, and of contracts, &c., relating thereto, viz: Delaware breakwater, Delaware bay; improving harbor, &c., at Marcus Hook, Delaware river; improving harbor, &c., at Chester, Delaware river; sur-

vey at Reedy island, Delaware river; survey at Liston's Tree point, Delaware.

In all fifteen papers.

I would take this opportunity of calling attention to the unhappy effect of some of the provisions of the act of Congress making appropriations for the above works. By law, all power is taken from an officer to reject, after due advertisement, a bid merely because it is extortionate. If the lowest bidder be a responsible man, the law requires a contract to be made with him, though he offers to furnish the articles or labor at a thousand fold its market price. If an attempt is made to regulate bids, by stating in the advertisement that unreasonable bids will be rejected, the bidder quietly remarks, "I have read the law," and avails himself in his bids of the particular information gained by poring over congressional publications, to the detriment of the public interest. provision of the law offers a premium to contractors to enter into combination to force prices to such a point as they may choose to fix upon, after obtaining the control of the market in regard to certain articles. Such a combination in one instance has already caused much difficulty and delay in the prosecution of certain work at Marcus Hook, and will give much more, perhaps, ultimately rendering a suspension of operations for weeks or months necessary.

The proviso requiring separate contracts to be made for each class of materials or labor for each work, instead of a single contract embracing all the means and appliances requisite for a proper prosecution of the work to its entire completion, is producing all the trouble, confusion, and dissatisfaction that were anticipated by the department when endeavoring to have the proviso modified last year. It has resulted in parcelling out to four or five distinct individuals what should have been under the control of one, and rendering it almost impracticable for the different parties to work harmoniously and to the greatest advantage for the interests of the United States. They are loud in their denunciation of the absurdity of the provision, judging from their experience as

practical men, as experts in matters of construction.

In the works under my charge it has prevented some of the most reliable men from making bids. When informed of the provisions of the law, seeing at once the endless complication which might arise, the constant jarring and quarreling between the different parties, and loss to which they themselves might be subjected through the delay or failure of another party, upon whose work their own might depend, they refuse to look at drawings, specifications, or anything else; stated under such conditions they did not wish to know anything more; "they would not touch the work with a ten-foot pole, or have anything to do with it," and they did not. By thus preventing the reception of advantageous bids, further injury to the public service results.

Under this distribution of separate contracts for labor and materials for the same work among different parties, owing to the fault of one party, heavy claims for damages have been made by others; the claims are just, but the aggressor refuses to pay them, and how they are to be paid under the law remains to be

settled.

If it was desired to cause work to be done at great expense, in the most unsatisfactory manner, and to assist in further demoralization of contractors—if, indeed, with many of them, that be practicable—a more happy set of provisions to attain these objects could hardly be devised.

I have the honor to be, sir, very respectfully, your obedient servant, C. SEAFORTH STEWART,

Lieutenant Colonel of Engineers.

Major General A. A. Humphreys, Chief of Engineers, U. S. A., Washington, D. C.

L 1.

Report of progress made in construction of the Delaware breakwater during the year ending June 30, 1867.

It was ascertained, after an examination of this work on the 9th of August, 1866, that preparations could not be made and materials procured in time to

accomplish any work during that year.

On the 7th of January, 1867, proposals by advertisement were invited for furnishing as much stone for continuing this work as was supposed could be advantageously received during the working season; and on the 31st of the same month proposals, in like manner, were invited for putting the stone to be furnished in position.

Three sets of bids for material were sent in and six for labor. Contracts

were accordingly entered into with the lowest responsible bidders.

On the 23d of May the placing of stones in position on the breakwater was

begun.

Through miscalculation as to means to be employed, the contractor for labor, although thoroughly forewarned on all such points, did not put up machinery of suitable character and strength for the work to be done. Vessels, therefore, could not be unloaded as rapidly as they should have been, nor as fast as through his representations the masters had been led to anticipate. Some vessels before discharging were kept waiting for weeks. Hence have arisen claims for demurage as yet unpaid by the contractor for labor, from whom it is justly due.

Owing to the machinery and the inexperience of the hands employed at the labor, very little progress in the construction had been made up to the 30th of

June, 1867—the close of the fiscal year.

In all, at that date, about 1,200 tons of stone had been received and put in position, completing to the full height of superstructure 115 feet in length. The whole length completed is about 2,000 feet, and some 560 feet in length remain to be raised to the proper height.

No work has been done at the ice-breaker; as soon as it can be completed a crane will be placed there, but the principal effect will be directed to complete, so far as practicable, in the first place, the breakwater proper, its present base.

Expenditures to June 30, 1867.

Advertising, stationery, &c	\$527 74
Contractors for stone furnished	3, 235 32
Services of United States inspectors, &c	
Total to June 30, 1867	4, 253 35
In hands of agent July 1, 1867	\$ 23, 546 65
1868	189,603 70
Total available for year ending June 30, 1868	213, 150 35

This sum will probably complete the works on their present bases.

Amount that can probably be profitably expended during the year ending June 30, 1868, \$100,000.

The breakwater is located in the collection district of Delaware. The nearest port of entry is Wilmington. There is a light-house upon the breakwater and another upon Cape Henlopen. Fort Delaware is the nearest fort. The amount of revenue collected at Wilmington during the fiscal year ending June 30, 1867, is \$41,733 96.

It is estimated that during the past year upwards of 30,000 vessels have passed in and out of Delaware bay and by these works. Many of these vessels have taken shelter in the artificial harbor.

The following table, taken from such imperfect records as have been kept from time to time since the breakwater was raised above the surface of the water, gives the number of vessels of each class sheltered behind that work during the d fferent periods therein specified:

Years.	Ships.	Barks.	Brigs.	Schooners	Sloops.	Pilot boats	Steamers.	Total.
1833 4	22		178	372	167	. 127		866
1834 6	48	l	315	667	303	411		1, 744
1635	133		569	1,719	461	644		3, 526
ls36	301		1, 027	2,719	620	767		5, 434
1837	227		478	2,777	629	732		4, 843
1838	165		732	3, 191	765	685		5, 5, 8
1699	165		504	3, 561	734	697		5, 661
1840 c	172		279	1,969	308	371		3, 039
1841 d	îii		902	3, 916	590	483		6, 002
842	107		1, 060	5, 335	802	744		8, 098
1843	103		841	4, 981	1, 167	792		7, 884
844	231		969	5, 767	854	744		8, 595
1845	265		1, 042	5, 446	597	776		8, 126
1846	258		1, 625	6,711	614	781		9, 989
847	342		1, 937	7,742	358	874		11, 253
848	340		1, 457	6,037	374	918		9, 126
849.	329		804	3, 261	168	553	•••••	5, 115
1854 e	35	247	1, 085	5,098	90	1	114	6, 669
855	65	240	855	6, 808	128		243	8, 339
856	116	425	1, 062	6, 125	256		286	8, 270
857.	79	331	981	5, 348	218		220	7, 177
858 f	6	34	78	1.030	174		38	1. 360
862 8	55	239	879	8,087	253	1	246	9, 759
863	231	345	1, 040	7.092	251		347	9, 306
864.	231	308	1, 155	7, 781	189		299	9, 963
865	274	378	1, 179	6,056	209		372	8, 468
866.	28	227	694	6, 747	269		316	8, 311
867 Å	97	207	490	3, 929			179	5, 049
Total	4, 536	2,981	24, 217	130, 242	11, 725	11, 149	2, 660	187, 510

a From Sept. 1, inclusive.
b July 1 to Oct. 17 not recorded.

c To June 3, inclusive.

g From Feb, 1, inclusive. A To June 30, inclusive.

It is thus seen that 187,510 days shelter have been afforded to vessels of every class. Upon an average for each recorded day of these 28 years, 21 vessels have been at anchor in this harbor.

Abstracts of proposals of contracts for the work and for each class of materials and labor therefor are transmitted herewith.

It is thought that probably, at intervals of ten years, or thereabouts, the damages to the work caused by ice and storms may require an appropriation of about \$15,000 for repairs.

Abstract of contracts for Delaware breakwater: Contractors, Henry Barker and J. R. Bodwell, stone; William H. Miller, labor.

Abstract of contracts for each class of materials and labor for Delaware breakwater: Contractors, Henry Barker and J. R. Bodwell, stone, at \$4 73 per ton; William H. Miller, labor, at \$1 22 per ton.

Respectfully submitted:

C. SEAFORTH STEWART, Lieutenant Colonel of Engineers.

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d From May 1.
e From April 1, inclusive.
f To March 31, inclusive.

Abstract of proposals for Delaware breakwater.

	Per ton f	or stone.	ton.	cter
Name of bidder.	One-fourth to twc tons.	Two tons and upwards.	Labor, rate per t	Remarks - chars of atone.
John C. Leiper and Richardson Shoemaker. William H. Groves Henry Barker and J. R. Bodwell Cornelius and Charles F. Kennedy E. D. Hitchens, E. J. Morris, G. Chambers, D. A. Marshall, and C. Burton William H. Miller	2 70 4 73		\$1 30 1 97 1 25 1 69 1 22 1 37	Gneiss. Do. Granite and gasiss.

NOTE.—The proposals of William H. Groves were rejected, he not being considered, under the circumstances of the case, a responsible bidder.

C. SEAFORTH STEWART, Lientenant Colonel of Engineers.

L 2.

Report of progress made in repairs of government piers, wharves, and landings. and in improving harbor at Marcus Hook, on Delaware river, Pennsylvania. during the year ending June 30, 1867.

Having on the 21st of July, 1866, been charged by the department with the prosecution of these repairs, after an examination of the piers and harbor early in August following, a report of the results of this examination was made, and on the 20th of September, in accordance with the act of Congress relating to the appropriation, proposals were invited by advertising for making the required repairs.

But one person made bids, and, as stated in my report of last year, owing to delay in perfecting the bids and to the insufficiency of the appropriation to make the repairs needed, the proposals were not accepted, and further operations were postponed until the present season.

During the winter session of Congress, in consequence of reports and estimates submitted, an additional appropriation was made for repairs and also for improving the harbor by the addition of four new piers, in accordance with plan also submitted.

On the 15th of May, 1867, proposals for repairs of old piers, wharves, &c., were invited by advertisement, and on the 22d of same month for furnishing materials and labor for the construction of the four new piers to be placed on the prolongation of the old.

There were three bidders for the repairs, and contracts for each class of labor and materials were entered into with the only bidder whose proposals conformed to the requirements of the act of Congress, and work under these contracts was to commence by the 5th of July, 1867. No work therefore had actually been done on these repairs at the termination of the fiscal year ending June 30, 1867.

For materials and construction of the four new piers there were in all twelve

sets of bidders. Four sets of contracts were in process of preparation with the lowest responsible bidders at the close of the fiscal year, and under these work was to be commenced early in July, the first month of the fiscal year for which the appropriation for the new piers was made.

The expenditures during the year have been as follows, viz:

Advertising for proposals	\$ 828 9	18 80
Total to June 30, 1867	837	98
In hands of agent, July 1, 1867	\$4, 212 93, 950	
Available for year ending June 30, 1868	98, 162	02
Probable amount required for the entire and permanent completion of the work	\$ 98, 162	02

This amount can be profitably expended during the fiscal year ending June 30, 1868, and nothing further is asked for to complete the work.

Marcus Hook is located in the collection district of Philadelphia; the nearest port of entry, light-house, and fort are, respectively, Wilmington, Delaware, Christiana light, and Fort Mifflin.

The amount of revenue received at Wilmington during the past fiscal year is

\$41,733 96.

Before completion of the work it will be of benefit to commerce and navigation so far as to afford occasionally a convenient ice harbor to vessels passing up and down the Delaware when ice is running therein heavily. The works may also aid in building up local commerce.

Abstracts of proposals, of contracts for the work, and of contracts for each

class of materials or labor for the work, are transmitted herewith.

For a few years after completion of the work it is believed no large expenditure will be required unless for repairs caused by unforeseen accidents.

The probable annual expenditure may be estimated as follows, viz:

Material and labor	120
Total per year for ten years	240

At the expiration of about ten years the wood-work of piers, wharves, and landings now to be repaired may require renewing to the extent of probably \$5,000.

Abstract of contracts for improving harbor at Marcus Hook, Pennsylvania.

Repair of piers:

William H. Rotan, for all timber, iron, and labor.

Four new piers:

Abraham P. Eyre, hemlock timbor and labor on same.

John C. Leiper, for all stone.

Robert Crane and John Keaveny, for labor on stone.

James R. Grant, for all iron.

C. SEAFORTH STEWART,

Licutenant Colonel of Engineers.

Abstract of contracts for each class of materials and labor for improving harbor at Marcus Hook, Pennsylvania.

Repairs of piers:

William H. Rotan—Hemlock wharf timber, 14 cents per foot, face; hemlock piles, ties, and drag logs, 7 cents per foot, lineal; white pine wharf timber, cap logs and caps, 30 cents per foot, face; white pine bridge timber and plank, 4 cents per foot, board measure; mooring posts, 4 cents per foot, board measure; iron bolts, 7 cents per pound; corner plates and spikes, 10 cents per pound; screw bolts, 14 cents per pound; ring bolts, 16 cents per pound; labor on wharf timber, 14 cents per foot; labor on piles, 14 cents per foot, lineal; labor on plank, mooring posts, fenders, &c., 1½ cent per foot, board measure.

Four new piers:

Abraham P. Eyre—Hemlock wharf timber, 131 cents per foot, face; labor on

wharf timber, 5 cents per foot, face,

John C. Leiper—Hammered facing stone, 79 cents per foot, cubic; paving stone, 73 cents per foot, cubic; rubble backing stone, \$3 per perch; rubble filling stone, \$1 40 per perch,

Robert Crane and John Keaveny—Labor on hammered facing stone, \$3 per yard; labor on paving stone, \$2 50 per yard; labor on rubble backing stone,

\$2 per perch; labor on rubble filling stone, \$1 per perch.

James R. Grant-Iron bolts, 62 cents per pound; screw bolts, 10 cents per pound.

C. SEAFORTH STEWART,

Lieutenant Colonel of Engineers.

Abstract of proposals for repairs at Marcus Hook, Pennsylvania.

No. 1. A. P. & Edwin S. Eyre—White pine cap logs, 32 cents per foot, face, square measure; white pine bridge timber, \$30 per 1,000 feet; white pine mooring posts. \$14 each; white oak fenders, \$48 per 1,000 feet; hemlock square timber, 14 cents per foot, face; hemlock round and piling, 8 cents per foot, lineal; white oak timber for piling, 14 cents per foot, lineal; stringers or side of bridging, 30 cents per foot, lineal; digging out, removing, piling, &c., 14 cents per foot; furnishing and laying white pine bridge plank, \$50 per 1,000 feet; driving piles, \$5 each; putting on fenders, preparing for capping, \$150 each; iron work, 8 cents per pound.

No. 2. Milo W. Locke—White pine timber, 30 cents per foot, lineal; hemlock timber, including piles, 16 cents per foot, lineal; white oak timber, \$60 per 1,000 feet, board measure; iron work, 8 cents per pound; on white pine, 10 cents per foot, lineal; on hemlock cribs, 6 cents per foot, lineal; driving piles, 15 cents per foot, lineal; stone, \$2 50 per perch; removing old work.

&c, \$1,000.

No. 3. W. H. Rotan—Hemlock piles, ties, and drag logs, 7 cents per foot, lineal; hemlock wharf timber, 14 cents per foot, face; pine wharf timber, 30 cents per foot, face; pine caps and cap logs, 30 cents per foot, face; pine bridge timber, 4 cents per foot, board measure; pine plank, 4 cents per foot, board measure; oak fenders, 5½ cents per foot, board measure; mooring posts, 4 cents per foot, board measure; iron belts, 7 cents per pound; iron corner plates, with nails, 10 cents per pound; iron screw bolts, 14 cents per pound; iron ring bolts, 16 cents per pound; laying wharf timber, 14 cents per foot, lineal; driving piles, removing old work, filling up, &c., 14 cents per foot, lineal; bridge work,

planking, putting on fenders, mooring posts, and all sawed timber, 14 cent per foot, board measure.

The proposals of William H. Rotan, being more nearly in conformity with the requirements of law and the lowest in the aggregate, were accepted.

C. SEAFORTH STEWART,

Licutenant Colonel of Engineers.

Abstract of proposals for new piers at Marcus Hook, Pennsylvania.

Reaney, S in & Co.—Iron bolts, 7 cents per pound.

James R. Grant-Iron bolts, 61 cents per pound; screw bolts, 10 cents per pound.

John R McNeil-Rubble filling, \$1 47 per perch.

Edward J. Lanman—Rubble filling, \$1 95 per perch; rubble backing, \$5 per perch; hammered facing stone and paving stone, \$9,417 for each pier.

George W. Smith—Hemlock timber, 163 cents per foot, face.

Robert Crane and John Keaveny—Hammered facing stone, \$24 per cubic yard; paving stone, \$19 per cubic yard; rubble backing, \$5 50 per perch; rubble filling, \$3 per perch; hemlock timber, 17 cents per foot, face; iron bolts, 10 cents per pound; labor on hammered facing stone, \$3 per yard; on paving stone, \$2 50 per yard; on rubble backing, \$2 per perch; on rubble filling, \$1 per perch; on hemlock timber, 15 cents per foot, face.

John Stewart, jr., and John S. Stevens-Iron bolts, 7 cents per pound.

William H. Rotan-Hemlock timber, 16 cents per foot, face; iron bolts, 7 cents per pound; labor on hemlock timber, 5 cents per foot, face.

A. P. Eyre—Hemlock timber, 13½ cents per foot, face; iron bolts, 7½ cents

per pound; labor on hemlock timber, 5 cents per foot, face.

Henry Barker and J. R. Bodwell-Hammered facing stone, \$1 20 per cubic foot; paving stone, \$1 20 per cubic foot; rubble backing. \$4 per perch, rubble filling, \$2 per perch; labor on hammered facing stone. \$6 per cubic yard; on rubble backing, \$1 50 per perch; on rubble filling, 25 cents per perch.

John C. Leiper-Hammered facing stone, 79 cents per cubic foot; paving stone, 73 cents per cubic foot; rubble backing, \$3 per perch; rubble filling,

\$1 40 per perch.

John Honan-Labor on hammered facing stone, \$4 75 per yard; on paving stone, \$4 75 per yard; on rubble backing, \$2 per yard.

C. SEAFÖRTH STEWART,

Lieutenant Colonel of Engineers.

L 3.

Report of progress made in improving harbor at Chester, Pennsylvania, during the year ending June 30, 1867.

Congress having, during the last winter session, made an appropriation for improving Chester harbor by repairing the government piers, &c., and the department having charged me with the prosecution of these repairs, proposals therefore were invited by advertisement on the 29th of May, 1867.

Six sets of proposals were received, and those of the lowest responsible bidders

accepted.

It being about the close of the fiscal year, preparations were made to enter into the necessary contracts, that work might be commenced as soon as practicable after the beginning of the new fiscal year, for which the new appropriation had been made.

Proposals for repairs to the causeway connected with the lower pier had not been invited in May, as it was believed this work might be done to greater advantage at a later day. No work upon repair had therefore been begun at the close of the year ending June 30, 1867. Expenditures for that year have been as follows, viz:

Advertising for proposals	
Total to June 30, 1867	

This sum will probably be sufficient for the completion of the contemplated repairs, and can be profitably expended during the fiscal year ending June 30, 1868. No further appropriation is asked for.

Chester is located in the collection district of Philadelphia. That city is the nearest port of entry. Fort Mifflin is the nearest fort, and the nearest lighthouse is Fort Mifflin light. The amount of revenue collected at Philadelphia during the fiscal year ending June 30, 1867, is \$8,845,772 43.

It is reported that at one time during the past winter about seven sea-going steamers, two ships, five or six barks, and some thirty-five small crafts were ice-bound at Chester.

Since October, 1866, from incomplete records, it would seem that at least eleven steamers, eight barks, nine brigs, and many smaller vessels have arrived there, besides the steamboats touching daily on their up and down trips.

there, besides the steamboats touching daily on their up and down trips.

Inasmuch as private wharves have been built in what was once the harbor between the government piers, and may apparently be extended at will, having already greatly diminished the capacity of the harbor, as other piers and wharves have been built above and below, and it is intended, by private individuals, to make like and extensive arrangements for the accommodation of vessels in landing and receiving freight, and to protect them from ice during the winter months, it would seem it is no longer necessary for the United States to be at the expense of keeping these piers in repair for the sake of what was once the only shelter for vessels, the so-called harbor of Chester, and that it would be more advantageous to retrocede the piers to the State of Pennsylvania, that the latter might dispose of them to private individuals, or in such other way as to it might seem best.

Abstracts of proposals, of contracts, and of contracts for each class of materials or labor for the work, are forwarded herewith.

The amount required for yearly expenditures, for casual repairs, is estimated as follows, viz:

as follows, viz:	
Materials and labor	\$200
Advertising for proposals for same	120
Contingencies	40
·	
Total per year for ten years	360

After some ten years a sum of \$7,000 may be necessary for renewing upper parts of piers, wharves, and bridges.

Respectfully submitted:

C. SEAFORTH STEWART.

Lieutenant Colonel of Engineers.

Abstract of contracts for improving harbor at Chester, Pennsylvania.

Name of contractor.	For what purpose.
Bartram Booth	Timber.
James Nelson	Labor.

Abstract of proposals for repairing piers at Chester, Pennsylvania.

Reaney, Son & Co.—Iron bolts, 7 cents per pound. James Nelson—For all labor, 18 cents per foot, face.

Bartram Booth—White pine cap logs, \$34 per 1,000 feet; white pine bridge timber, 30 cents per cubic foot; white pine mooring posts, 30 cents per cubic foot; white pine upper courses, 30 cents per foot, face; white pine bridge plank, \$31 per 1,000 feet; oak posts and fenders, \$47 50 per 1,000 feet; hemlock tie timber, 7 cents per foot.

James R. Grant-Iron bolts, 6 cents per pound; screw and ring bolts, 10

cents per pound.

W. H. Rotan—White pine wharf timber and caps, $27\frac{1}{2}$ cents per superficial foot; white pine ties, 20 cents per lineal foot; hemlock square wharf timber, 15 cents per superficial foot; hemlock tie timber, 8 cents per lineal foot; oak fenders, 5 cents per foot; white pine bridge timber, \$37 50 per 1,000 feet; white pine mooring posts, plank, &c., \$37 50 per 1,000 feet; iron bolts, $6\frac{3}{8}$ per pound; screw bolts, $10\frac{1}{2}$ cents per pound; wrought spikes, 7 cents per pound; labor on all square timber, $1\frac{1}{2}$ cent per foot; laying timber and balance of work, 12 cents per foot.

A. P. Eyre—White pine side and caps, 31 cents per foot, face; white pine bridge timber, \$32 per 1,000 feet; white pine stringers, 32 cents per lineal foot; white pine plank, \$37 per 1,000 feet; white pine mooring posts, \$30 per 1,000 feet; hemlock tie timber, 7 cents per lineal foot; oak fenders, \$45 per 1,000 feet; labor on timber, 13½ cents per foot, face; placing sleepers for bridges, \$4 per 1,000 feet; bridge covering, \$6 per 1,000 feet; placing and securing mooring posts, \$10 per 1,000 feet; for linear work, 10 cents per foot;

putting on of fenders, \$15 per 1,000 feet.

C. SEAFORTH STEWART,

Lieutenant Colonel of Engineers.

Abstract of contracts for each class of materials and of labor for improving harbor at Christer, Pennsylvania.

Bartram Booth—White pine cap logs, \$34 per 1,000 feet, board measure; white pine bridge timber and mooring posts, 30 cents per cubic foot; white pine wharf timber, 30 cents per foot, face; white pine bridge plank, \$31 per 1,000 feet, board measure; hemlock tie timber, 7 cents per foot, lineal; white oak posts and fenders, \$47 50 per 1,000 feet, board measure.

James R. Grant—Iron bolts, 6 cents per pound; corner plates, spikes, screws,

and ring bolts, 10 cents per pound.

James Nelson—For all labor, 18 cents per foot face of wharf and pier work.
C. SEAFORTH STEWART,

Lieutenant Colonel of Engineers.

L 4.

Report of progress made in survey at Reedy island during the year ending June 30, 1867.

This survey was commenced by R. M. Bache, Assistant United States Coast Survey, on the 1st of May, 1867. The stormy, rainy weather prevalent during the months of May and June interfered with its progress greatly. At the close of the fiscal year, on June 30, 1867, the field-work had not been completed.

It will be some time before the results of the survey can be ascertained and be made available to determine what plan shall be adopted and the expendi-

tures necessary under such plan.

The expenditure on the survey on June 30, 1867, had been as follows:	ows, viz:
Services of hired men	\$ 391 95
Subsistence	
Signals and contingencies	36 56
Total to June 30, 1867	506 51

Probably as much more will be required to complete the survey.

Reedy island is in the collection district of Delaware. The nearest port of entry is Wilmington, Delaware.

There is a light upon the island—Reedy island light.

Fort Delaware is the nearest fort.

The amount of revenue collected at Wilmington during the fiscal year ending June 30, 1867, is \$41,733 96.

Respectfully submitted:

C. SEAFORTH STEWART,

Lieutenant Colonel of Engineers.

L 5,

Progress made in survey at Liston's Tree Point, Delaware, during the year ending June 30, 1867.

An officer of the Coast Survey has been detailed for this work upon the completion of the survey at Reedy island, upon which he was engaged at the close of the fiscal year ending June 30, 1867.

To make this survey may require an expenditure of \$1,000. This will be

expended during the fiscal year ending June 30, 1868.

Liston's Tree Point is located in the collection district of Delaware. The nearest port of entry is Wilmington. Bombay Hook light-house is the nearest light; Fort Delaware the nearest fort.

The amount of revenue collected at Wilmington, Delaware, during the fiscal year ending June 30, 1867, is \$41,733 96.

Respectfully submitted:

C. SEAFORTH STEWART,

Lieutenant Colonel of Engineers.

APPENDIX M.

NEW YORK, August 6, 1867.

GENERAL: I have the honor to submit the following report of operations upon the improvement of the Hudson river during the year ending June 30, 1867:

By letter of August 6, 1866, the department was furnished with a report of the condition of the Hudson river, and of the works erected for its improvement by the United States engineers and by the State commissioners, from Troy to New Baltimore.

The works erected by the United States from 1834 to 1843, consisting of the Overslaugh dike below, and the Port Schuyler dike above, Albany, were found to need extensive repairs, and to this purpose it was proposed to devote the whole sum of \$83,000 then available. Brevet Lieutenant Colonel John M. Wilson, captain of engineers, having reported to me September 6, as assistant upon the improvement of the Hudson river, repaired to Albany, furnished with the necessary instructions to undertake the repairs of the United States dikes above and below Albany. Under his efficient supervision repairs have been vigorously pushed during the year.

Notwithstanding the lateness of the season at which work was commenced, and the unusual number of freshets which postponed active operations of this

season nearly to June, considerable progress has been made.

The repairs of the dikes consisted in repaving slopes with large stone laid by haud, in restoring the original cross-section of the dike by a filling of stone, and by repaving the surface; and when the destruction of the dike had gone too far, in rebuilding it, using the construction known as the half-dike.

November 9, 1866, a full report and estimates for the improvement of the Hudson river, made as complete as possible from information compiled from former

surveys and reports. was rendered to the department.

Under this report a further appropriation of \$305,188 was granted by Congress.

A board of engineers was convened at Albany on May 14, 1867, and the

system described in the report of November 9 substantially indorsed.

A survey of the river near New Baltimore, and also at Cuyler's island, above Albany, was made in May and June, for the express object of locating correctly the works of improvement required at those localities. These surveys will hereafter form portions of the general survey of the river.

Tide-gauges have been set at Troy, Castleton, Albany, and New Baltimore, and temporarily, for specific objects, at Coeyman's, Bogart's island, and Cuyler's island. The extension of the freshet into the month of June has prevented, for want of sufficient length of observation from the cause above assigned, a report

of the tidal state of the river for the last year.

Current observations, giving velocities of surface and the set, have been made at Coeyman's, and the results transmitted, in a sketch of Barren island and the resignborhood, to the department. The surveys and examinations described have not sufficiently advanced as yet to have particular bearing upon the system of provement adopted.

These surveys will continue until all necessary information has been obtained.

Money statement.—Appropriation of 1864.—Repairs of harbors on the Atlantic

Amount available July 1, 1866	\$ 33,000 00
Expended during the year	24, 123 76
Balance available July 1, 1867	8, 876 24

Appropriation of 1866.—Improving the Hudson river.

Amount available July 1, 1866	\$50,000 00 17,325 52
Balance available July 1, 1867	32,674 48
Examinations and surveys on the Atlantic coast.	

Information supplied to conform to the acts of Congress making appropriations for this work:

1. Resurvey has not sufficiently progressed to furnish results.

2. There is required to be appropriated to finish the scheme of improvement, according to the estimate of November 9, 1866, the sum of \$474,109 75.

3. The amount that can be profitably expended during the next fiscal year is \$335,000.

4. This work is situated in the fourteenth collection district.

5. The nearest port of entry is Albany, New York.

Applied to the survey of the Hudson river:

6. The amount of revenue collected there for the last fiscal year is \$19,974.

7. The amount of commerce and navigation to be benefited by this work is about \$500,000,000.

8. Abstract of proposals, with names of bidders sent herewith.

9. Abstract of contracts, with names of contractors sent herewith.

10. Abstract of contracts for each class of materials or labor. (Included in No. 9.)

The works to be finished this year are the repairs of the United States dikes, new dike at Cuyler's island, and cutting away the face of Mull's island, with a probable total expenditure of \$162,960.

During the next season the long dikes at Castleton, long dike between Lower and Upper Patroon's island, long dike connecting Bogart's island with Westerloo island, the dike from Base island to eastern bank of river, and dredging, will be

the probable operations.

For these essential operations there would be required \$335,000, and deducting therefrom the balance of the present appropriations, there would be left in round numbers the sum of \$152,000, which is asked to be appropriated for the fiscal year ending July 1, 1869.

Respectfully submitted:

JOHN NEWTON,

Lieut. Col. of Engineers, and Bot. Maj. Gen. U. S. A.

Major General A. A. Humphrbys, Chief of Engineers.

JOHN NEWTON, Lieutenant Colonal of Engineers and Brood Major General U. S. A.

Abstract of proposals for materials and labor on improvement of navigation of Hudson river, New York.

1121 0	ai O	•	•			•		3 C	, Li		•			_	•	,,		*	, ,		-						
Romarks,				•											No despuit stracted.			Misunderstood advertisement.		Misunderstood advertisement.						Refused to enter into contract.	
4,000 exb. yards rabble- stone, more or less, October, 1866.	P. c. yd.										:			:	:					:	:	:	:	:			
Sawing off pile heads.	Eack.					:					:		:	:	:					į	:	:		:	20 25		
Delivering 8,000 cable yards of rabble-stone, May, 1867.	P. c. yd. \$1.55	3 -									:	:	:	:							-	-		8	86	1 49	
Labor on 5,000 running. feet of timber work on dike.	Per foot.			8		:		2	2		:	*	:	:	:	:		7	:	8	:	:	:	2	8	•	
Delivering 10,000 cubic yards of stone chips.	P. c. yd.	3					9	88		:	:		:	:		:			:	:	:	:	:		98		
Driving 30,000 feet of piles.	Per foot. \$0 09\$	2	•	90				9		:	:	:		ক 		:	_	•	0		<u>:</u>	:	:	٤	88		
Delivering 3,000 lbs. of bols, with nuts and washers.	Per 16.					30	15			2					5	25	_			:	의 -	:	:	:	2 23	1	
Delivering 15,000 ibe. wrought spikes.	Per 16.					示	₹	3		•	7	6				ā					74	<u>:</u>	:	:	3 35		
Delivering 4,000 15-foot piles.	Per foot. \$0 13	:	181		:	:	S	2			:		:		\$		2			:	:	<u>:</u>	9:	28	38		
Delivering 100,000 feet 8 by 12 inch hemlock timber.	Per M. \$30 00	8	8					8			:		8	2		:				<u>:</u>	•	8 7	:	•	88	•	
Delivering 6,000 cable yards 4th class build- ing stone.	P. c. yd.	2					3.74	8											:	-	:	:	:	5	3 2		
Laying 15,000 sq. yda. 4th class building stone in a slope wall,	Per yd. \$0 50				'n	1	G æ	\$:	:		2	:						:	:	:	8	38		
Name,	Emory R. Seward	William Fuller	Morean Lewis	Templeton & Payne.	Thomas Brennan and John Higgins	Stmeon Cunliff.	McClosky & Grimes	R. Nelson Gere	Stephen Miles	John D. Hutchingon	M. McGlanis	Francis Besulac	Thomas Knowlson	James Brady	Hebry M. Smith	Whiteh H Taylor	E. K. Scovill		Patrick Riley	Franch Jacques and John Smith	Skinner & Arnold	S. L. Griffith	Edmund Raymond	Peter G. Canneld	Author Clark	Eph Owen	

I certify that the above is a true abstract.

ABSTRACT OF CONTRACTS FOR WORK AND MATERIALS FOR IMPROVING HUDSON RIVER, NEW YORK.

Emory R. Seward.—October 17, 1866, for 4,000 cubic yards of rubble stone, more or less, at \$1 50 per cubic yard, to be delivered on the dike.

R. Nelson Gere.—April 25, 1867, for laying 15,000 square yards, more or less, of fourth class building stone in a slope wall on the dikes and islands in the Hudson river, at forty cents per square yard.

Emory R. Seward.—April 26, 1867, for 3,000 cubic vards, more or less, of fourth class building stone, at \$1 75 per cubic yard, to be delivered on the

dikes and islands.

William Fuller.—April 25, 1867, for 3,000 cubic yards, more or less, of fourth class building stone, at \$1 75 per cubic yard, to be delivered on the dikes and islands.

Morgan Lewis.—April 29, 1867, for 100,000 feet, more or less, of hemlock timber, 8 by 12, at \$25 per thousand feet, to be delivered at the docks at Albany, New York.

Emory R. Seward.—April 29, 1867, for delivering 4.000 piles, more or less,

each 15 feet long, at 12 cents per foot, to be delivered at the dikes.

Simeon Cunliff—April 23, 1867, for delivering 15,000 pounds of wrought spikes, more or less, at five and a half cents per pound.

Simeon Cunliff.—April 23, 1867, for delivering 3,000 pounds of bolts, more or less, with nuts and washers, at eight cents per pound.

Lorenzo D. Loomis.—April 27, 1867, for driving 30,000 feet of piles, more

or less, at the dikes and islands, at five and a half cents per foot.

Emory R. Seward.—May 27, 1867, for preparing and putting in the timber work on 5,000 running feet of dike, more or less, at forty cents per running foot of dike.

William Fuller.—May 4, 1867, for 8,000 cubic yards, more or less, of rubble stone, at \$1 34 per cubic yard, delivered on the dikes and islands.

I certify that the foregoing abstract is correct.

JOHN NEWTON,

Lieut. Col. of Engineers, and Brevet Major General, U. S. A.

M 1.

Proceedings of a board of engineers convened in accordance with engineer order, dated Engineer Department, Washington, May 4, 1867.

The board met Tuesday, May 21, 1867, at 11 o'clock a. m., in accordance with the following order:

[Engineer Order, |

Engineer Department, Washington, May 14, 1867.

A board of engineers, consisting of Brevet Brigadier General Hartman Bache, colonel of engineers; Brevet Brigadier General Henry Brewerton, colonel of engineers; Brevet Major General John Newton, lieutenant colonel of engineers; Brevet Brigadier General H. L. Abbot, major of engineers, will assemble at Albany, New York, on Tuesday, May 21, 1867, or as soon thereafter as practicable, for the consideration of the project of improvement of Hudson river, proposed by Brevet Major General Newton, lieutenant colonel of engineers.

The board will report its views, embracing such modifications of the project

of improvement, if any, that it may deem essential, and including the proper order of time for the execution of the various portions of the plans.

Brevet Lieutenant Colonel John M. Wilson, captain of engineers, will act as

recorder of the board of engineers.

A. A. HUMPHREYS,

Brig. Gen. and Chief of Engineers, Major General of Volunteers.

All the members of the board and the recorder were present.

The order assembling the board having been read, at the request of the board, Brevet Major General John Newton, lieutenant colonel of engineers, read his report on the improvement of the Hudson river, and gave his views upon the subject.

At half past two p. m. the board adjourned, to meet to-morrow morning at

nine a. m., on board the steamboat Seneca, to inspect the river.

WEDNESDAY, May 22, 1867.

The board met pursuant to adjournment on steamboat Seneca; present, all the members and the recorder.

The board proceeded to inspect the Hudson river between Albany and New Baltimore, thoroughly examining the dikes built by the United States and the State of New York.

At 2.30 p. m. the board adjourned, to meet on steamer Seneca to-morrow at nine a. m., to inspect the Hudson river between Albany and Troy.

At three p. m. the board met informally, and discussed the general subject of river improvement until quarter past five p. m.

THURSDAY, May 23, 1867.

The board met pursuant to adjournment at nine a. m., on steamboat Seneca; present, all the members and the recorder. The board proceeded to examine the condition of the Hudson river between Albany and Troy.

The board returned to Albany at two p. m., and at 2.15 p. m. assembled at

the engineer office.

The board then entered into a general discussion of the report of Major General Newton; after which, on motion of General H. L. Abbot, the following resolution was unanimously adopted:

Resolved, That the board, after carefully considering the project of Brevet Major General John Newton for the improvement of the Hudson river, and after a personal examination of the river from Troy to New Baltimore, adopt

that project, in which are laid down the following essential principles.

1st. A system of longitudinal dikes, designed to confine the current sufficiently to allow the ebb and flow of the tidal current to keep the channel clear. These dikes to be gradually brought nearer together from New Baltimore towards Troy, so as to assist the entrance of the flood current and increase its height; their height to be kept approximately at the level of the tidal high water, so as not to confine the freshets; the exact level, however, being left to be determined by experience as the work progresses.

2d. That the dredge be used so far as necessary to open the channels above described, which the current should not be allowed to do except very gradually,

lest accumulations dangerous to navigation be formed below.

3d. Keeping as far as practicable the side reservoirs open to the passage of tidal currents by gaps at their lower extremities, in order to increase the tidal flow.

4th. Dumping all dredged materials in secure places, where it cannot be moved back into the channel by the current.

5th. Constructing the dikes of timber and stone in a manner to secure their

permanency at the minimum cost, the details varying with the locality, to be left to the discretion of the local engineer, but to be so designed as to admit of having an increased height given to the dikes if necessary.

6th. To protect when necessary the banks and islands against the abrading

action of the currents by revetments.

7th. That limits beyond which no encroachments upon the channel should be made be prescribed, and that any such encroachments be reported by the engineer in charge.

The following resolution was then unanimously adopted:

Resolved, That the plan of improvement proposed by General Newton in his report, and modified by him in a paper herewith laid before the board, (marked A,) be approved, with the remark that the information collected is not yet sufficient to prescribe a plan complete and thoroughly digested in all its details.

General Abbot then presented the following resolution, which was unanimously

adopted:

Resolved, That if the amount appropriated for the improvement of the Hudson river was sufficient to complete the entire system above laid down, the work should be begun at New Baltimore and extended regularly upward in all essential features towards Trov.

The existing appropriation being, however, inadequate for such a purpose, the opinion of the board is, that the money should be distributed throughout the entire distance, in such a manner as to meet the most pressing needs of navigation, beginning at the New Baltimore section.

The following resolution was then presented by General Abbot and unani-

mously adopted:

Whereas the report of General Delafield refers to various accurate surveys of the river, made in 1819, 1831, 1843, and 1852, only two of which surveys are now in the hands of the engineer in charge and available for the use of the board; and whereas the earlier records, both maps and reports, are of the first importance in studying the effects of the works of improvement—

Resolved, that every effort should be made to secure the missing maps, and that they should all be reduced to the same scale for easy reference, and if possible be represented on the same sheet of paper. Also, that a full collection of all reports relating to the river should be made and carefully preserved, and that

such as only exist in manuscript should be printed.

There being no further business before the board, it adjourned sine die at

6.15 p. m.

HARTMAN BACHE,
Colonel Engineers, Brevet Brigadier General.
HENRY BREWERTON,
Colonel and Brevet Brigadier General, U. S. A.
JOHN NEWTON,
Lieutenant Colonel Engineers, Brevet Major General.
HENRY L. ABBBOT,
Major of Engineers, Brevet Brigadier General.

Δ

Modification proposed by Brevet Major General John Newton to the Board of Engineers, at their session of the twenty-third instant.

The works of improvement in the first section, at and near New Baltimore, as laid down in my report of November 9, 1866, and now modified to conform to later information, are recommended as follows:

The existence of a ledge of slate rock across the western channel, at Barren

island, renders it impossible without blasting to lead up a much greater quantity of flood tide through this pass, and the filling up of the re-entering below Coeyman's no longer possesses the importance once assigned to it—that of directing

the flood current through the western channel.

The channel edge of the filling above Coeyman's should have such a direction as to throw the main body of the ebb through the east channel; and this channel, to compensate for the want of space, as above, in the west channel, to be much enlarged by cutting away the face of Mull's island, for the purpose of admitting a larger volume of flood, and at the same time of the ebb tide.

The widening of the channel between Houghtailing island and New Balti-

more to be made the last in the series of improvements in this section.

In the second section, to remove the end of the old State dam, projecting into

the channel beyond the contemplated dike.

In the third section, the proposed dike across the mouth of Cooper's Kill, which was designed to counteract the apparent cutting away of the shore of Papscannee island, is probably unnecessary, as further information shows this place to be reinforced against such further abrasion by the uncovering of rock.

The principal works designed for this river, whose general position can now be designed, even in the want of much detailed information appropriate to the

case, may be enumerated as follows:

First. The widening of the east channel at Barren island; the amelioration

of the re-entering above Coeyman's by a filling of earth, or by a dike.

Second. The long dike between Thorn's dock and Schermerhorn's island, and probably a short one between Campbell's and Low island.

Third. The extension of the United States dike from the head of Bogart's

Third. The extension of the United States dike from the head of Bogart's island to the head of Westerloo island, or even to the wharves of Albany.

Fourth. The long dike from upper to lower Patroon's island, and the short dike from Base island to the east shore.

These works should be located in their exact position, and in other particulars,

after sufficient information has been obtained from observation.

The want of appropriate data for a finished plan of river improvement has been severely felt, and it is proposed to supply this deficiency by proper observations upon the silt and velocities of the currents, the slopes in the tidal and freshet states of the river, the rises of tides and freshets, by soundings, by borings in the bed of the river, &c

Such observations, continued for a sufficient period of time to constitute a complete record, would not only be of importance in the execution of the works of improvement proposed, but would serve to suggest works of minor importance which cannot now be certainly specified, and finally enable a comparison

to be made in the future of this river and others under discussion.

Although the improvement of a tidal river would suggest, as a general rule, a commencement below, and a gradual working up above, yet it is suggested that an uncertainty or irregularity of future appropriations may render a violation of this rule expedient in the present case, and a devotion of the available funds to the most essential works in the first, second, and fourth sections.

JOHN NEWTON,

Licutenant Colonel Engineers, Brevet Major General, Member of the Special Board of Engineers.

A true copy:

JOHN M. WILSON, Captain Engineers, Brevet Lieut. Col. U. S. Army

APPENDIX N.

United States Engineer Office, Newport, R. I., September 11, 1867.

GENERAL: I have the honor to submit the following annual report of progress on the works of river and harbor improvements and surveys in my charge for the year ending September 1, 1867:

IMPROVEMENT OF THAMES RIVER, CONNECTICUT.

The work on Thames river, this season, has consisted in dredging out the channel immediately below the city of Norwich, in the manner recommended by me in my last annual report. Work was commenced on the 5th of June by Mr. E. A. Bill, to whom the contract for dredging was awarded. Up to the first of the present month 14,820 cubic yards of material have been excavated and removed from the channel. The material excavated (which has up to this time been principally sand) has been deposited on flats near the shore, so that there is no danger of its being moved in any way to interfere with the channel. I have now three (3) dredging machines at work on this river, and the work is progressing satisfactorily. I do not propose to attempt to finish the work the present season, for the reason that I desire to observe the effect of the ice and spring freshets on the work done this year. It is possible that the channel may be varied by the above-mentioned causes, and I think it desirable to reserve a portion of the appropriation until the next season, in order to remedy any injury that may occur.

In answer to the requirements of engineer department circular dated June

10, 1867, I have to report as follows:

of the present season	\$ 83, 074	50
tember 1, 1867	8, 653	41
Total amount available September 1, 1867	74, 421	09

First. Total amount available for this work at the commencement

Second. I consider that the amount appropriated (\$84,000) is sufficient to complete the work of excavating the channel to a depth of fourteen feet at high water, and to repair the piers which were constructed in this river by the government for the purpose of contracting the channel.

Third. \$74,421 09; the amount available for the work on the 1st of Sep-

tember, 1867.

Fourth. Third (3d) collection district of Connecticut.

Fifth. New London, Connecticut.

Sixth. -

Seventh. Fully set forth in my report of November 19, 1866.

Eighth. Enclosure marked A.

Ninth. Enclosure marked B.

IMPROVEMENT OF PROVIDENCE RIVER, RHODE ISLAND, OFF PAWTUXET BAR, AND AT THE CROOK.

The work in this river has consisted in raising and removing from the channel off Pawtuxet bar the sunken wreck of the schooner Mary Stewart, and dredging the channel at the Crook. This wreck was raised in the month of May last by Mr. G. W. Townsend, to whom the contract was awarded. The wreck was moved entirely out of the channel and placed in about seven feet at low water, so as to interfere in no respect with navigation. The cost of raising and removing this wreck was \$2,000.

The dredging at the Crook has consisted in deepening and widening the channel at that point Up to the 1st instant the amount of material excavated and removed is 42,219 cubic yards. It is principally mud.

This work will be of great benefit to the harbor of Providence, as it will enable vessels to pass each other with ease, which formerly was impracticable

at low water in many cases.

I submit no estimate of funds for this improvement, as the work done this season will, I think, be all that is required in the present condition of the harbor.

No vessels drawing over seven feet can go up to the wharves at low water. Now, all vessels that reach the wharves have an easy access to the harbor.

The city of Providence has appropriated money from time to time for deepening the channel above the Fox Point wharf. This part of the river, on which are located most of the wharves where the principal commercial business of the place is transacted, will require constant dredging to accommodate the increasing trade of the city, and all appropriations made by the general government ought, in my opinion, to be assisted by appropriations from the city or State, and the whole work be placed under one superintendence, with a definite plan fixed upon for the improvement of the harbor.

In answer to circular of June 10, 1867, I report as follows:	
First. Total amount of appropriation	\$25,000 00
Amount expended under present plan of operations up to 1st September, 1867:	420,000 00
Removing sunken wreck, Mary Stewart \$2,000 00)
Dredging at the Crook	
Total expenditures	23, 012 94
Amount available for this work, 1st September, 1867	1, 987 06
Second. No appropriation recommended at present. Third. \$1,987 06—the amount available September 1, 1867. Fourth. Providence district. Fifth. Providence, Rhode Island.	
Sixth. Gold	\$164 007 72
Currency	11, 272 90
Total	175, 370 63

Seventh. The city of Providence and the country about, for which it is the shipping port, contains many large manufactories of almost every description. The cotton mills, iron works, &c, are among the largest in the country. The continued increase of the manufacturing interest, and the present rapid growth of the city, indicate that all additional facilities afforded to the navigation of the harbor will be required to meet the demands of commerce. The number of vessels arriving at Providence during the past fiscal year was 5,414.

Eighth. Enclosures marked C. Ninth. Enclosures marked D.

IMPROVEMENT OF PAWTUCKET RIVER, RHODE ISLAND.

The work on this improvement has consisted in dredging the channel, so as to obtain a depth of six feet at low water. This is a tidal river, and it is possible that the present work may only have temporary results. The material excavated, so far, is fine sand, and it has been deposited on flats on either side of the channel. I am of opinion that after the channel is deepened so as to give a

depth of six feet at low water, up to the wharves of Pawtucket, it will be kept open by the constant passage of vessels which are towed up and down the river.

Total amount excavated up to September 1, 1867, 12,430 cubic yards.

In answer to circular of date June 10, 1867, I report as follows:

Total amount available for this work September 1, 1867 12,591

- 2. I think that the present appropriation is sufficient to afford the depth of water desired, and no further appropriation is asked for at this time.
 - 3. \$12,591—the amount available September 1, 1867.
 - 4. Providence district.
 - 5. Providence, R. I.
- 7. The city of Pawtucket, like Providence, is a large manufacturing depot. The supplies for the factories, in the way of building material, raw material, fuel, &c., and their productions, give employment to a great deal of shipping, and with the present depth of water in the river a large portion of these supplies have to be landed at Providence and carted to Pawtucket. The number of vessels arriving at Pawtucket during the last fiscal year was seven hundred. This number will be much increased by the present plan of improvement of the river, as nearly all vessels engaged in this trade will be able to ascend the river to the city.
 - 8. Enclosures marked G.
 - 9. Enclosures marked H.

IMPROVEMENT OF CONNECTICUT RIVER BETWEEN HARTFORD AND ITS MOUTE.

The survey of this river, with a view to its improvement, was commenced on the 10th of August by Theodore G. Ellis, civil engineer.

It is not possible at this time for me to offer any accurate views as to the work required to be done on this river to improve its navigation. From what I have seen, however, I am of the opinion that it is only a question of dredging the channel, and that the result of the survey will be simply the determining of the amount of material to be removed in order to obtain the desired channel.

The character of the river for twenty miles below the city of Hartford, where the navigation is most difficult, is very similar to that of the Mississippi. The country is alluvial, and the bed of the river is constantly shifting. It was formerly attempted to improve the channel of this river by constructing jetties or piers, but some of these are now under ground, formed by the changes in the river bed, while others are directly in the way of vessels navigating the river. One probable part of the plan of improving the navigation of the Connecticut river will be the removal of a portion of these piers.

At the mouth of the river is a bar, on which there is, at low water, a depth of from four to seven feet, as shown by the Coast Survey charts. An examination of this bar, with a view to its improvement, will be made.

The information required by department circular of June 10, 1867, will be furnished with the detailed report of the survey.

REMOVAL OF MIDDLE ROCK, NEW HAVEN HARBOR, CONNECTICUT.

The amount of \$5,937 25 of the appropriation for the removal of middle rock, New Haven harbor, having been transferred to me by Captain Mansfield, corps of engineers, brevet lieutenant colonel United States army, it occurs to me that it would be advisable to expend it, if only as a matter of experiment.

The original appropriation for this work, made in 1852, (?) was \$6,000, and of this amount \$67 75 was expended in attempts to remove the rock by exploding charges of powder on the surface. As far as I can ascertain, these efforts were futile, owing, perhaps, to the want of sufficient depth of water.

Being of opinion that the true way of removing this rock was by drilling holes and blasting in the ordinary way, I requested and obtained authority from you to expend the balance of the appropriation. Mr. G. W. Townsend made me an offer to remove four feet from the top of the rock for the sum of \$5,000, or he would work by the day and do what he could with the money available. The department having preferred that the work should be done by days-work, I made an agreement with Mr. Townsend, by which he was to furnish one sloop and crew, diving apparatus, divers, scow, drilling machines, galvanic battery, powder, and all material for blasting rock under water, for the sum of eighty dollars per day.

He commenced work on the 13th of July and is yet at work. On the 23d of August I visited the rock and found that there was on the rock fourteen feet at low water, an increase in the depth of five feet. Mr. Townsend is of the opinion that he can take off another foot with the fund available. The work has progressed much better than I anticipated, even under the most favorable

circumstances, and this season has been unusually unfavorable.

I estimate the amount required to cut down this rock so as to give the depth of water originally proposed, viz: seventeen feet at low water, at \$30,000.

In answer to circular of June 10, 1867, I report as follows:

Amount available for the work September 1, 1867...... 2, 437 25

2. \$30,000.

- 3. \$2,437 25—the amount available September 1, 1867.
- 4. New Haven district.
- 5. New Haven.

BLOCK ISLAND.

The survey of this island, with a view to the construction of a breakwater to form an artificial harbor, was commenced on the 19th of July last, and finished on the 5th instant. The survey was made by Colonel G. W. Dresser, formerly of the fourth artillery, and he is now engaged in making the necessary drawings to illustrate the results of his examination.

I enclose herewith his preliminary report, marked I, fully indorsing the views expressed by him. There can be no question but that a harbor of refuge at Block island would be of great benefit to the major part of the commercial interests of our Atlantic coast, and in time of war it would be a most convenient rendezvous for naval vessels belonging to a squadron stationed on this coast.

Detailed plans and estimates of a breakwater at Block island will be forwarded as soon as practicable.

The information required in circular of June 10, 1867, will be forwarded with detailed report.

WESTPORT HARBOR, CONNECTICUT.

No work has been done on this improvement up to the present time, except a survey to ascertain as near as possible the cost of making the necessary improvements.

The amount appropriated for this work (\$2,500) was evidently inadequate to perform the work required, which consisted in, first, the repairs of the breakwater on Cedar Point. This will require 120 cubic yards of stone. Second. Repairs of the walls of the canal leading from the harbor to the sound, and excavating the canal. This will require 619 cubic yards of stone, and 7,290 yards of excavation to obtain a depth, at low-water, of two feet. To obtain a depth of four feet, at low water, will require 15,054 cubic yards of excavation. Third. Removal of obstructions in the channel of the river.

I make the following estimate of the cost of the work:

•	
739 cubic yards of stone laid in walls	3, 763
	12, 153

The only work that can now be executed to advantage is the repair of the breakwater on Cedar Point and the walls of the canal. This work will be executed this fall as far as the appropriation will permit. The amount required to complete the work of improving this harbor, in addition to the amount already appropriated, is \$10,000

In answer to circular of June 10, 1867, I report as follows:

- 1. _____.
- 2. \$10,000.
- 3. \$10,000.
- 4. Fairfield, Connecticut, district.
- 5. Nearest port of entry, Bridgeport, Connecticut.
 I am, general, very respectfully, your obedient servant,

D. C. HOUSTON,
Major of Engineers, Brevet Colonel U. S. A.

Major General A. A. Humphreys, Chief of Engineers, U. S. A., Washington, D. C.

A.—Abstract of proposals for improvement of Thames river, Connecticut, during the year ending August 31, 1867.

To dredge out the channel of Thames river immediately below the city of Norwich, Connecticut, to obtain a channel of fourteen feet at high water:

James Y. Smith, eighty-five (85) cents per cubic yard.

E. A. Bill, forty-five (45) cents per cubic yard.

A. J. Hackley, sixty thousand (\$60,000) dollars to complete the work.

D. C. HOUSTON,

Major of Engineers, Brevet Colonel U.S.A.

B .- Abstract of contracts for improvement of Thames river, State of Connecticut.

To dredge out the channel of the Thames river, Connecticut, immediately below the city of Norwich:

E. A. Bill, forty-five (45) cents per cubic yard.

D. C. HOUSTON,

Major of Engineers, Brevet Colonel U.S. A.

C .- Abstract of proposals for improvement of Providence river, off Pawtuxet bar and at the Crook, during the year ending August 31, 1867.

For removing the sunken wreck Mary Stewart from the channel of Providence river, off Pawtuxet bar:

Eben W. Eaton, twenty-four hundred dollars, (\$2,400.) Jacob Palmer, twenty-two hundred dollars, (\$2,200.)

George W. Townsend, two thousand dollars, (\$2,000.)

For dredging out the channel of the Providence river at the Crook:

James Y. Smith, one and seven-eighths $(1\frac{7}{8})$ cent per cubic foot.

Sampson Wardell, one and one-quarter (11) of a cent per cubic foot.

Thomas J. Hill, one and one-half (112) cent per cubic foot.

D. C. HOUSTON.

Major of Engineers, Brevet Colonel U. S. A.

D.—Abstract of contracts for improvement of Providence river, off Pawtuxet bar and at the Crook, during the year ending August 31, 1867.

Removal of sunken wreck Mary Stewart from the channel of Providence river, off Pawtuxet bar:

George W. Townsend, two thousand dollars (\$2,000.)

Dredging the channel of the Providence river at the Crook:

Thomas J. Hill, one and one-quarter (11) cent per cubic foot.

D. C. HOUSTON.

Major of Engineers, Brevet Colonel U. S. A.

G .- Abstract of proposals for improvement of Pawtucket river, Rhode Island, during the year ending August 31, 1867.

For dredging out the channel of the Pawtucket river, Rhode Island, between the city of Pawtucket and the Red bridge:

S. S. Wardell, five (5) cents per cubic foot.

Thomas J. Hill, four and one-quarter (41) cents per cubic foot.

C. A. Nichols, three (3) cents per cubic foot.

(All the above bids were rejected.)

Templeton and Payne, thirty-five (35) cents per cubic yard.

A. Sickles, fifty-four (54) cents per cubic yard.

Thomas J. Strong, forty-five (45) cents per cubic yard.

S. S. Wardell, eighty-one (81) cents per cubic yard.

E R. Seward, seventy-five (75) cents per cubic yard.

Charles A. Nichols, seventy-four (74) cents per cubic yard.

Thomas J. Hill, eighty-one (81) cents per cubic yard.

D. C. HOUSTON.

Major of Engineers, Brevet Colonel U. S. A.

H .- Abstract of contracts for improvement of Pawtucket river during the year ending August 31, 1867.

To dredge out channel of the Pawtucket river, State of Rhode Island, between the city of Pawtucket and the Red bridge, to obtain a channel of six (6) feet at low water and seventy-five (75) feet wide:

Templeton and Payne, thirty-five (35) cents per cubic yard.

D. C. HOUSTON,

Major of Engineers, Brevet Colonel U. S. A.

I.—Report of Colonel G. W. Dresser.

NEWPORT, RHODE ISLAND, September 10, 1867.

COLONEL: I have the honor to submit the following as a preliminary report of the survey made by me "with a view to the location of a breakwater which shall form an artificial harbor," at Block island.

Pursuant to your instructions contained in letter to me, dated Engineer Office, Newport, Rhode Island, July 11, 1867, I proceeded to Block island, arriving

there July 19, 1867.

I was engaged until July 24 in making an examination of the entire shore of the island, as well as most of the interior of it. There is not a ledge of rock to be found on the island anywhere, and no appearance of rock on the shores excepting boulders, which have been unearthed by the action of the sea against the land. These vary in size, but are mostly small, and are quite uniformly distributed around the shore, from the eastern, around the southern to the western point of the southern or larger part of the island. There are some deposited also on the shore at Clay Head, on the northeastern point of the island. None of these are available in the construction of a work, for two reasons, viz. First, they could not be removed from the positions they now occupy to the point required so economically as stone could be procured from quarries abounding on the shores of the neighboring mainland even if the quantity available on the island shores was sufficient. Secondly, they should be left where they are to protect, as far as they will, the shores of the island from wearing away by the action of the sea, which, though very slight, should not be disregarded.

The surface of the island is rolling land, with a fertile soil, and the whole is underlaid with clay, which, in most cases, is dry and tough. On the south end of the island the land is high, and these clay cliffs stand almost vertically at a height of from 75 to 125 feet above the ocean, at the foot of which sand

and gravel beaches slope off gradually and uniformly into the sea.

The two points presenting themselves as fit ones for a work to form an artificial harbor were, first, a point on the west beach near the Salt pond, where it has been proposed to make an opening through the beach to form a ship channel into the pond, which, in that case, would form a harbor. Second, a point near Sands's landing, at the southeast point of the bay, on the east side of the island, where, by constructing a breakwater of stone, running in a northerly direction, a perfect harbor, with smooth bottom and excellent holding ground

for anchorage, could be obtained.

At the first point careful surveys were made, and in my final report I will submit profiles of the cutting, estimates of the amount and cost of excavation required, together with a map of the pond, with the soundings thereon. The area of this pond is about eight hundred acres, but at least one-third of it is shoal water, not exceeding five or six feet deep. The deepest water found was sixty feet, with several feet of mud at the bottom, but the average depth of that portion of the pond which could be available for vessels would not exceed twenty-five or thirty feet. The bottom is very uneven. The pond is separated from the sea on the east, and the sound on the west, only by sandy beaches, which have probably been thrown up by the action of the seathus uniting in one what probably was once two distinct islands. I am told that in heavy storms and high tides in the winter, the water rushes over the east beach into the pond, frequently cutting off communication along that shore between the north and south ends of the island.

This pond freezes up in the winter, ice sometimes forming from eighteen

inches to two feet thick.

In making a cut through the west beach into the pond, it would be necessary to excavate a channel, nearly at right angles to the line of the beach, in the

sand for a considerable distance into the sound. This would fill up at once to low-water mark, unless a work was constructed to protect it and keep the sand out; and to whatever extent a pier was built for this purpose, the sand would eventually work around the head of it, and together with the sand blown from the shore, would form a bar which would constantly be shifting its position, as the rise and fall of the tide would form a strong current to and from the pond through the opening.

The present level of the pond is about three feet above low-water mark. The average rise and fall of the tide is about three feet and a quarter. The ordinary spring tides are about four and a half feet. An opening at this point would be subject to a heavy sea surf directly from the ocean, along the westerly shore of the island, in a southwest storm, and during a southeast or easterly storm, a vessel would have a head wind to beat through a channel not over six

hundred and fifty feet wide into the harbor.

From the careful consideration of this subject, after examining the point myself, and getting all the information possible from the inhabitants, I conclude that any expenditure at this point would fail to secure a harbor available at all times, and that a large and constant annual outlay would be required to keep open any channel cut there.

With reference to the second point—a work on the eastern part of the island which would form a protection to vessels anchored in the bay—I have made a survey of the shore of the bay and procured the necessary information and data to make a map, showing the depth of water, character of the bottom, &c., of

the entire bay.

The point at which I would propose to locate the starting point of a breakwater is on the shore about five hundred feet east of the present landing. The bottom is smooth and hard, free from rocks, and the water deepens gradually to six or seven fathoms at a distance of half a mile from the shore, and attains a depth of ten fathoms at about one mile from shore, in the direction of Clay Head.

The only reasonable limit to the number of vessels that could be safely sheltered here is the extent to which the work is carried. There are no obstacles to be overcome in the construction of a work here, other than those which ordinarily arise in works of this kind upon a good solid foundation, where the stone can be placed by vessels anchoring at the line of the work and throwing stone overboard to make the foundation for the sea-wall on top of them. The protection to vessels would be complete from all winds.

The demands of our commerce, and the necessities of our navy, in defending Long Island sound in case of a foreign war, would be amply met by a work of

reasonable extent and expense at this point.

In my final report, which I am now preparing, I will submit plans, &c., of

the work, with the maps, estimates, &c., in full detail.

Every courtesy and facility was most cordially extended to me by the people of the island during my stay there. I kept a record of the tide taken once an hour during the day, and also of the thermometer and the wind, taken three times a day during the time I was on the island, which I shall submit in tabular form with my final report.

I am, colonel, very respectfully, your obedient servant,

GEORGE W. DRESSER, C. E.

Colonel D. C. HOUSTON,

United States Engineers, Newport, R. I.

APPENDIX O.

NEW BEDFORD, MASS., June 7, 1866.

GENERAL: I have the honor to report that, in obedience to the instructions of the department dated the 19th ultimo, I visited Duxbury and the neighboring beach, and conferred with Mr. G. B. Weston. I beg to submit the following remarks on the object of the instructions of the department:

Duxbury bay and harbor are separated from the Atlantic ocean by a long, narrow beach extending for five or six miles from the mainland, in a southeasterly direction, to a headland called the Gurnet, thence westerly a mile or more to another high point known as Saquish head. The beach, throughout most of its length, is about seventy-five yards in width, and rises to a general level of about ten feet above ordinary high tides. In many places depressions of several feet occur, through which it is said the sea is liable to flow on the occa-

sion of a spring tide accompanied by the prevalent easterly wind.

On the Gurnet there is a field-work, and on Saquish head another, both built during the war, which constitute the defences of Plymouth as well as Duxbury harbor. Mr. Weston represents that these points are at times, for a couple of months in the winter, inaccessible by water, the beach being the only communication. In this statement, to my mind, lies the main argument that can be urged with any force in favor of the proposed repair. As the harbor appeared to me, and from the best information I could gain, it is of little value, on account of the flats which occupy almost all its extent. I have not seen a chart of the harbor, and have been compelled to form my opinion from a cursory examination, but I would respectfully refer to the Coast Survey report for the depth of water, &c. There is no commerce, and even the fishing smacks have sought other ports.

About thirty years ago an appropriation of \$5,000 was expended upon this beach, under the direction of Colonel Totten, in planting two rows of stakes, three or four feet apart, and filling the euclosed space with brush and seaweed, which served to collect and retain the sand, upon which beach grass was set out. The grass gave cohesion to the mass, and held it in place long after the woodwork had disappeared This simple measure seems to have served a good purpose, and if it be decided to repair the beach now, I would recommend this as the most economical means.

The aggregate in length of the breaches or depressions is supposed to be about two miles, and will require about \$4,000. The estimate as to the length is that of Mr. Weston rather than my own, as the stormy weather prevented me from ascertaining the fact for myself. I was the less inclined to spend much time in measurements, as the whole project struck me as one I could not recommend.

I am, very respectfully, your obedient servant,

G. H. MENDELL.

Major Engineers, Brevet Colonel U. S. A.

Brevet Maj. Gen. RICHARD DELAPIELD, Chief Engineer U. S. A., Washington, D. C.

NEW BEDFORD, MASS., October 14, 1867.

GENERAL: In accordance with instructions of the 7th instant, I have visited Duxbury beach, Massachusetts, and would respectfully submit the following report:

The harbor of Duxbury at time of high water is very large, but so shallow that when the tide is out it consists only of a few narrow winding channels and a vast extent of marshy flats. It seems to be used only as a resort for a few fishing vessels or occasional coasters with articles for local consumption.

This harbor is separated from the outer sea by a narrow beach rising ten to twelve feet above the level of high water, and about six miles long. A further description of this beach is given in the report of Colonel Mendell on this subject, to which I would respectfully refer. Several depressions occur in the crest of this beach, of an aggregate length of about 1,500 feet. These, save 500 feet, require but slight repairs to prevent the water from breaking over. The remaining parts would require a substantial work.

From observing the cost of repairs on the adjacent and similar beach at

Plymouth, I make the following estimate, viz:

500 feet, at \$4 per foot	\$2,000 2,000
Total for completing the repairs	4,000

In case it be decided to make these repairs, I would recommend a similar means to that used many years ago, viz: two rows of stakes, with the space between filed with seaweed. Parts of this old work still remain, and in some places still serve a good purpose in protecting the sand from translation by the wind. The more exposed positions would require a substantial work, for which a triangular frame similar to that used at Plymouth would be well adapted.

I fully concur with Colonel Mendell in feeling that neither the value of the harbor nor the present danger of injury to the beach are sufficient to justify a

recommendation in its favor.

Very respectfully, your obedient servant,

JARED A. SMITH,

Brevet Major U. S. A., Captain of Engineers.

Brevet Maj. Gen. A. A. HUMPHREYS,

Chief of Engineers, U. S. A., Washington, D. C.

APPENDIX P.

Boston, Mass., September 5, 1867.

GENERAL: In accordance with your circular of June 10, No. 11, I have the honor to make the following report in relation to the works carried on during the past year for the "preservation of Provincetown harbor, Massachusetts."

As more fully stated in my report of last year, this work at the commencement of the previous year was under the charge of the late Colonel J. D. Graham, corps of engineers, who, in November, 1866, had made a report and

estimate for the works at that harbor.

In March, 1867, a further examination, report, and estimate was ordered by the engineer department to be made by Major Blunt and myself, which was completed in April and at once approved by the department, and the estimate

(\$8,000) was called for and received in July, 1866.

This report of April, 1866, proposed that some repairs should be made to the old bulkhead at Long Point, next the light-house, and an additional work of about the same size and character with that should be placed in front of the site of the farther or upper battery, about from twelve hundred to sixteen hundred feet from the light-house. Also, a similar work on Beach Point, east of Province-town, from one-fourth to one-half mile southeast of the inlet or bridge. Also, (in which it agreed with Colonel Graham's report,) the planting of a brush and beach grass protection at three specified points on the seaward side of the sand ridge, at the east of East Harbor Meadow, to secure those positions from any possible inroads of the ocean upon the main harbor through east harbor and its lowlands.

The work was just commenced n June, 1866, at Beach Point, and was vigorously pushed on all the positions above referred to in the next month, July, the first of the fiscal year, now reported upon, and by the month of September about ten acres of beach grass, with interlacing fences of brush, &c., had been planted at the points on the sand ridge referred to east of East Harbor Meadows, giving a satisfactory protection, and all that thus far appears to be needed to

this position. At Long Point, in front of the upper battery, a bulkhead about seven hundred feet long was constructed, with three jettees, each projecting about sixty to seventy feet, and four brace jettees, of about twenty-eight to thirty-two feet, placed intermediately with the longer jettees. A large space in the rear of central position of this bulkhead, where the sea had made in towards the battery, was filled in with sand, and for some thirty to forty feet in width, in rear as well as at the extremities in front, it was planted with beach grass, while belts of beach grass were planted in different directions in the peninsula, opposite and near this work, so as to secure the drifting sand and prevent currents of water, &c. This bulkhead, of the character of those previously used with success in that locality, consists simply of a two-inch plank fence from five to say seven feet high, with batten of one-fourth to one-fifth, with posts four feet apart and six to eight inches across, set three to five feet in the sand and secured to the rear by one or two braces ten to twenty feet long dove-tailed to the posts and held at the lower or inner end by pickets or short cross-ties, the earth being filled in rear to near the top of the fence and planted with beach grass roots eighteen to twenty-four inches apart. The front of the bulkheld is protected by jettees, which also secure and raise the sand on the outside. The best form as thus far found, and as used in the bulkhead at this battery, are placed at intervals of eighty to one hundred feet, projecting outward from fifty to sixty feet, two to three feet high at the outer end, and four to five feet high, or two or three feet lower than the top at the bulkhead, built of posts at the bulkhead, as far in the ground as out, and planked on both sides and at top; and these, with intermediate or brace jetters, (every twenty-five to forty feet,) to be of an oblique bracing timber twenty-five to thirty feet long, running from near the top of the bulkhead to the surface outside and supported by posts planked on both sides like the other jettees, constitute the best protection of a simple and inexpensive character as thus far used for these localities; and the bulkhead erected in front of this upper battery has thus far stood perfectly well, answering all the purposes expected of it in securing the beach in rear and in causing a considerable accumulation of sand in front.

I would report in relation to the bulkhead next to the Long Point light-house. erected by Colonel Blunt in 1853, to which some repairs were given by myself in 1857, and which is of the same general character with that above described, for the last twelve or fourteen years had answered most effectively the objects intended, to preserve and widen the narrow strip of sand beach, with its planted beach grass, which connects the site of the light-house with the broader parts of peninsula of Long Point. Some slight repairs and modifications, all that were deemed necessary, were given to this position last season, and it was left in its usual good condition at the approach of winter. I regret to state, however, that during the month of January last a violent storm raged there which very singularly did not injure the new work, first described as erected last year and almost similarly situated, and within a half mile of that next to the light-house at Long Point, yet it almost completely destroyed this latter work, carrying away large portions of the bulkhead and the jettees for the most part, and sweeping out large portions of the sand that had accumulated between the jetties and been placed in rear of the bulkhead, and it reduced the width of the isthmus there in some parts to some thirty feet only, which has fortunately still remained unbroken, being protected mainly by the beach grass still in position there. No repairs, however, have as yet been made there during this present year, as a

board of engineer officers had been asked for to consider the best protection advisable for the future for this and other positions where the bulkhead had

been injured in this harbor.

This board of officers, consisting of Generals Bache, Foster, and myself, convened there as ordered about the close of June, and, substantially indorsing the former action of the engineer board and officers in relation to the works, it recommended the re-erection of a bulkhead essentially similar to that just destroyed, and the continuance or prolongation in front of it of a breakwater or riprap of large rough stone, which at just below the level of half tide has remained very permanently in position and appeared to give very effective protection to that pier for the past twenty years or more.

In accordance with the recommendation of this board and the law of Congress of 1866, making the special appropriation for this harbor, proposals were called for and a contract has been made within the past month for the construction of this work and the extension of this breakwater, for which the preparations are now being made so as to complete them during the present season and before the

close of November.

The position, however, not where in my opinion the main injury is likely to occur, which is at Long Point, but to which the principal complaints of some of the citizens have been directed, and where the main difficulties have as yet occurred, are at Beach Point and the inlet of East Harbor. The principal portion of the sum of \$43,000, estimated for by Colonel Graham, or over \$38,000, was proposed to close this inlet by a loaded crib-work, this opening being nearly 900 feet in width at the end of Beach Point.

This peninsula of Beach Point is of great value to the people of the vicinity as a means of communication with the main portion of Cape Cod, and the closure of this inlet would undoubtedly be very desirable for this purpose, either for the present common road or for any projected railway. But the necessity for this closing of this inlet as a means of protecting or preserving the harbor was not apparent to the first board of engineers, as shown in our report of April 28, 1866, nor does it appear at present expedient, as stated in the report of the second board ordered June 6, 1867, especially in view of the fact that this inlet, by the testimony given to us, had narrowed by some 3,000 feet within the memory of old residents who accompanied us there, and latterly at an accelerated rate, and the water way at the opening in April, 1866, did not show a depth generally of more than twenty-five to thirty inches at low water for hundreds of feet on either side of the bridge; yet as there was at that time a deep hole within or above the bridge, and the peninsula about one-third of a mile southeast was worn by the sea to a narrow ridge of some twenty to thirty feet, the possibility of this part being broken through and the present point being left as an island by which a current might be established through that deep hole to the bridge, that possibly could thus force the sand out to the harbor, induced Major Blunt and myself to recommend a bulkhead protection at this point of the Beach Point peninsula, one-third of a mile from the bridge.

This recommendation was approved and carried out, and during the last half of 1866 a bulkhead was constructed at this point much stronger than either of those at Long Point, though generally of the same character. It was about 1,250 feet long, curved or indented inward, or concave to the sea, some 300 feet from a right line—about half the curvature of the shore—that this roadway ridge at the indentation might be widened and strengthened. This bulkhead had four jettees, each projecting from 45 to 60 feet, and fourteen brace jettees of 25 to 35 feet, and the earth in rear was filled up to nearly its full height with sand and planted with beach grass, its object being to secure this narrow ridge and to increase its width by retaining the sand drifting towards it from the south. I regret to say, however, that when this work was about half completed, and again when it was just on the point of being finished very satisfactorily, a suc-

cession of gales of unusual violence from the southwest occurred there, and at each time they injured the then unfinished work to a very great extent, and the labor was continued at intervals actually till the close of December in the attempt to secure as much as possible of the work—to preserve the ridge during the winter months—at which time, as by the report of my assistant, Lieutenant Lockwood, about two-thirds of the work executed remained in tolerable condition to afford protection to this ridge, and without much greater injury it so remained until the visit of the last board of engineers in June last.

My opinion then expressed to this board is still my conviction, that I could not rely upon it that any similar structure would be strong enough to insure a protection to the peninsula in that position, and that how valuable soever this might be as a roadway to the inhabitants, I did not think that the expensive works that alone could be relied upon to resist the waves there were either called for or justified by any possible danger to the harbor from injuries to the shore line, and though the report of this board, which was approved, favored further construction of this less expensive character here, I was quite gratified to be able to report, as on August 5, that at my visit about the first of that month I found very great and advantageous changes, strongly sustaining the position I had held. I found within the month a very great change towards the closing of the inlet, the "deep hole" of last year filled up, and a bar across it at low water, and for acres above and to within about 100 yards of the bridge, where there had been two to three feet water at low water in April, 1866, there was a bank of sand two feet out of water, leaving a width of water-way less than 100 feet, where it had been 800 to 1,000 in April, 1866, and at the bulkhead a still more extraordinary change for the better was found; for though the sand had previously made for some 100 to 200 feet along its southeast part, it had within the month or two previous extended along nearly the whole of the about 800 feet remaining, filling up in rear to high-tide lines and filling up to the level of all the jettees, even burying some out of sight, and in fact accomplishing most completely the very purpose and every purpose which was expected from its condition originally.

Now, though of course I cannot feel any positive surety of the continuance of this recent increment upon this treacherous sandy shore, yet as so much is recently accomplished of what had been most to be desired from the filling of that deep hole, and from the new bank at the inlet, I now can have no fear of any island or island eddies being formed from the inlet to injure the harbor, and I can anticipate no damage more than perhaps a temporary injury to the roadway, and I must feel assured that no expensive structures of timber or stone can be required here, while with the belief that the dangers of last year are being rapidly repaired by natural causes, I have the hope even that this slight bulkhead will not need to be replaced, though I have provided for such work in the contract just completed for the Long Point bulkhead, should I find it necessary and give notice within one month from the date of that contract, which I

trust will meet the approval of the department.

The funds on hand and in the treasury, it is expected, will be ample for all expected expenditures according to the report of the board of engineers for the

present year, and for the ensuing fiscal year ending June 30, 1869.

In recapitulation I would state that the works for the protection of Province-town harbor during the fiscal year are as follows: a new plank fence bulkhead about 700 feet long, with the necessary jettees, has been constructed for the protection of the shore and battery about one-third of a mile southwest from Long Point light, the extreme point of the Cape, to which this work has given full security thus far, and during the autumn of 1866 the old bulkhead adjacent to the light was repaired, though it was, to a great extent, destroyed by the storms of the past winter. For the restoration of the bulkhead, however, to the extent

of 600 to 700 feet, and for a rough stone breakwater, a contrict has been made, by which it is expected to be completed during the present autumn.

At the outer sea beach, east of East harbor, the necessary brush "catch-sands" and beach grass have been planted to give all the present security desirable

there, the spaces thus planted comprising about ten acres.

At Beach Point peninsula, at one-third mile from the bridge, a plank-fence bulkhead was constructed, about 1,250 feet long, with the necessary jettees, in the autumn of 1866, and this, though seriously injured by the autumnal gales and by the storms of winter, has yet for the most part remained in position; and though at the first it gave some protection and an increase to the sand-bank in front of its eastern end for perhaps 100 to 150 feet only, I now am able to report that, just after the close of the fiscal year, the two-third portion of this bulkhead remaining was giving most efficient protection—all that had ever been hoped from it—to the shore of this peninsula, having carried large deposits of sand, in front as well as in its rear, of the work, while the contract made for the Long Point bulkhead provides for the extension or repairs of this, if it shall prove necessary, of which necessity much doubt still exists. And the inlet of East harbor, at the extremity of Beach Point, the closure of which is desirable, I am able to report has during the past year, by natural causes alone, filled up to a very great extent, so as to give the expectation that but little, if any, expenditure will be required for closing the same by artificial means.

Although no expenditures have been made under either of the laws referred to in Circular No. 12, I append the information required as to the items Nos. 4, 5, 6, 7 of this circular, as reported upon in the accompanying paper A, from information derived principally from the collector's office in this city. And as to proposals and contracts, I would remark that an advertisement was issued in October, 1866, inviting offers for contract, to which no response or bid was offered within the time required, and no contract was made during the fiscal

year ending June 30, 1867.

The funds available, expended, and on hand, are as follows: Furnished for use of this work from the appropriation for harbors		
on the Atlantic coast from July to December, 1866	\$ 15,000	00
From appropriation for "preservation of Provincetown harbor," made in 1866	3, 000	00
Expended during the fiscal year	18, 000 11, 292	
	6, 707	95
On hand July 1, 1867, of funds from appropriation for harbors on the Atlantic coast	\$ 3, 707	95
On hand July 1, 1867, of funds from appropriation for preserva- tion of Provincetown harbor	3, 000	00
T. A	6, 707	95
In treasury of the United States undrawn, from the appropriation for preservation of Provincetown harbor	40, 068	44
Total available	-	

And no further appropriation is deemed requisite at present. I am, sir, very respectfully, your obedient servant,

H. W. BENHAM, Colonel of Engineers, Brevet Major General.

Major General A. A. Humphrbys, Chief of Engineers. BOSTON, MASSACHUSETTS, April 12, 1867.

GENERAL: I have the honor to forward herewith, as useful for the files of the department, a sketch of Cape Cod harbor, with enlarged plots of Long Point and Beach Point, with the plans of the bulkheads at these positions. The bulkhead nearest the light-house at Long Point was constructed by Colonel Blunt some fifteen years ago, and stood very well up to this past winter, having received some repairs, under my direction, last summer. During this past season, however, serious damage is reported to it; and when the good working season shall have fairly come I propose to make these repairs, applying for the purpose what, I think, will be useful in that position—the continuous longitudinal box, filled with sand, in addition to, and in rear of, the bulkhead.

The bulkhead farthest from the light-house at Long Point, in front of the five-gun battery, was constructed last summer, and is reported to be in essen-

tially good condition and uninjured.

As to the bulkhead at Beach Point, in front of East harbor, the plan of which as left at the close of work in December is also shown, I would state that, as previously reported, many injuries occurred to it repeatedly in the course of construction, so that, after several repairs, it was left, as shown, somewhat irregular in plan upon the 31st of December, 1866. By the report of the watchmin in charge, the bulkhead as left at that time has not changed essentially for the worse up to this date, although, from his report, I have some reason to fear it has suffered as much from the depredations of the people near as from the sea, "three boats, loaded" with the, perhaps, broken materials, being reported as "seen at one time leaving this work."

Upon the 13th of February I suggested to the department that, in the proposed visit of the board of engineers to select a site to be surveyed for the principal military work at Provincetown, we should be authorized to examine this position, (if not that at Long Point also, with the same object,) that the views of these additional officers might be had as to the expediency of any works of much greater expense for this position. I refer to this again, as a further reflection makes me doubt—in which doubt Colonel Blunt states that he coincides with me—whether any great danger to the main harbor can possibly be anticipated from even the breaking through of this neck or tongue of land into East harbor that would justify any great or extraordinary expenditure for its further protection.

Undoubtedly the greatest value of this tongue of land now is from its giving the best—I might say the only tolerable—means of communication between Provincetown and the main part of the peninsula of Cape Cod, and for this purpose it has been sedulously cared for and its injuries repaired from time to

time by the inhabitants of the adjacent towns.

The injuries that were to be feared, and on account of which Colonel Blunt and myself felt justified in recommending some expenditure there in our report of April 28, 1866, were mainly that if this tongue of land were broken through and a new cut made into East harbor, an island being left to the east of the bridge, it was possible that eddying currents might carry sand to a considerable extent out into the main harbor, to its injury. This, it is true, we considered perhaps rather as a possible than a probable danger, and, as such, we felt the small expenditure of \$2,000 to \$3,000 might be justified, although, from the peculiarly bad season, the cost of the work there already done has been about double this amount, and the work is very defective and the protection has been by no means perfect. And in consideration of further work there, if nothing will stand permanently except an expensive structure of large piles or heavy timber, or stone and timber work, at an expense at least of from \$15,000 to \$30,000, as I think it must be, I would say that my present opinion is that this contingent or probable danger to the harbor is not such as to justify such an expenditure in this position.

To enable me to use most efficiently and economically the appropriation that was last made for this work, I would respectfully recommend that authority should be given (as I understand the law permits) that these funds should be expended and used, as the other funds have been heretofore, for the preservation of this harbor. I may add, in explanation of this request, that after having, to the best of my ability, last autumn, prepared the advertisement for work there, to meet the requirements of the law and also to protect the public interest, I did not get a single bid according to the terms of my advertisement.

Very respectfully, your most obedient servant,

H. W. BENHAM,

Brevet Major General.

Major General A. A. Humphreys, Chief of Engineers.

Boston, Massachusetts, July 4, 1867.

GENERAL: I have the honor to enclose herewith (which had been left in my office for copying) the report of the board of engineers for Provincetown harbor. My further reflection since the meeting of the board makes me feel it proper to say that I had earnestly wished that the other officers had been willing to designate more distinctly the kind of structures they deem most expedient at Beach Point and at Long Point, though it is true they have generally approved those already placed there. Yet I have not been satisfied with these structures myself, and yet could not think of others that would probably answer and withstand the force of the sea, except such as would involve a very largely increased outlay, and for such greater outlays, especially at Beach Point, I can see no justification for any danger we could anticipate to the harbor. For this Beach Point peninsula, in fact, as I believe I previously reported to you, the great value is as a roadway, for which it is of the highest importance to the people of the vicinity. But I think all probable injury to it by the sea would scarcely be likely to injure the harbor, and I believe this is the opinion of the other members, though they did not think it necessary to express it in a formal report; and entertaining this opinion myself, if this report is approved, as I cannot now see a justification of expensive structures at Beach Point, I have great doubts, I must say, about following essentially the plans adopted last year, in the fear that, with any of the slighter modifications recommended. a succession of similar southwest storms might again destroy them.

In other words, this slighter work will, of course, be as well constructed as possible, if ordered; but I cannot have the confidence in its strength and en-

durance that I would wish.

I am, sir, very respectfully, your obedient servant,

H. W. BENHAM,

Brevet Major General.

Major General A. A. HUMPHRBYS,

Chief of Engineers.

The board of engineers—composed of Brevet Major General Hartman Bache, colonel of engineers; Brevet Major General H. W. Benham, colonel of engineers; Brevet Major General J. G. Foster, lieutenant colonel of engineers—constituted by Engineer Order 43, dated June 6, 1867, "for the consideration of the plans for the preservation of the harbor" at Provincetown, Massachusetts, embracing the nature and extent of the works to be constructed at Beach Point and Long Point," assembled, at the call of the president, on Wednesday,

June 26, 1867, at Boston, and proceeded thence the same day to Provincetown to make examinations of several localities in regard to which the attention of the board had been called. These examinations, at once entered upon, were completed on the 27th, and the board returned on the 28th to Boston, and held a meeting in furtherance of the object of its appointment.

Besides information in the knowledge of Brevet Major General Benham, the engineer of the harbor, communicated verbally by him, the following described

papers were laid before the board and read:

1. Report of Colonel J. D. Graham, corps of engineers, dated Boston, Mas-

sachusetts, November 28, 1865, with tracings numbered 1 and 2.

2. Report of board of commissioners of the Commonwealth of Massachusetts, dated Boston, Massachusetts, November 16, 1865, H. Doc. No. 395,

May 8, 1866, marked A.

3 Report of board of engineers, consisting of Brevet Brigadier General H. W. Benham and Major C. E. Blunt, corps of engineers, to the Chief of Engineers, upon the best plan "for protecting Provincetown harbor in the most permanent and effective manner," dated Boston, Massachusetts, April 28, 1866. (See Senate Doc. No 198, Commonwealth of Massachusetts, marked D, May 1, 1867.)

4. Letters of Alpheus Hardy in behalf of board of commissioners of Commonwealth of Massachusetts, dated Boston, May 2, 1866, addressed to the chairman of the State senate committee on protection of Provincetown harbor,

(H. Doc. No. 395, marked B, 1866.)

5. Report of joint special committee on Provincetown harbor, dated May 8,

1866, (H. Doc. 395, Commonwealth of Massachusetts, 1866.)

6. Report of board of commissioners for Provincetown harbor, dated Boston, December 31, 1866, enclosing letter of the board to Brevet Brigadier General H. W. Benham, dated November 20, 1866, and the reply of General Benham, of same date, (House Doc. No. 62, Commonwealth of Massachusetts, 1867.)

7. Letters of Brevet Major General H. W. Benham to his excellency A. H. Bullock, governor of the State of Massachusetts, dated Boston, February 18, 1867, with two enclosures, (Senate Doc. No. 198, Commonwealth of Massachu-

setts, May 1, 1867, marked A, B, and C.)

8. Letter of Brevet Major General H. W. Benham, to his excellency A. H. Bullock, governor of the State of Massachusetts, dated Boston, Massachusetts, February 27, 1867, (Senate Doc. No. 198, May 1, 1867, marked E.)

9. Report of joint committee on Provincetown harbor, dated May 9, 1867,

(House Doc. No. 408, May 9, 1867.)

Also, the following maps:

1. Map of Cape Cod harbor, of 1833-'34-'35, by Major J. D. Graham, corps of topographical engineers; scale 1-10,560.

2. Coast Survey map of same harbor of 1857; scale 1-50,000.

3. Coast Survey tracing of part of the same map on the scale of 1-10,000.

 Enlargement made at Coast Survey office of Major Graham's map to scale 1-10,000.

After the reading of the foregoing papers, and a consideration of the question involved, the board adjourned, to meet at 9 a. m. on Saturday.

The board met, pursuant to adjournment, Saturday, June 29, when, after a

full discussion of the question presented for consideration, it was

Resolved That, in the opinion of the board, the course recommended in the joint report of Brigadier General H. W. Benham and Major C. E. Blunt, corps of engineers, should be carried out, modified as follows:

 That the closing of the outlet of East harbor be deferred until such time as it is shown that active operations, from natural causes, tending to effect that

object, shall have ceased; and

That besides the combined modes of bulkheads and jettees heretofore used to preserve Long Point, that the deposit of stone be continued, at least for a limited extent, to further test the efficiency of that mode to effect the desired

protection.

The board would further express the opinion, that in all cases where jettees are used, these structures, which, by checking the currents, cause deposits to be made, should possess, in some degree, an open and flexible character. The board approves of the plan of the bulkheads heretofore used, but hesitates to suggest, as in the case, also, of the jettees, both at Beach Point and Long Point, the number, position, or length of these works—matters which, in the opinion of the board, should be left to the judgment of the local engineer, with full authority to modify them as may seem best in the progress of the operations.

The business of the board being completed, it was duly adjourned sine die.

HARTMAN BACHE,

Colonel of Engineers, and Brevet Brigadier General. H. W. BENHAM,

Colonel of Engineers, and Brevet Major General. J. G. FOSTER,

Brevet Major General U. S. A., Lieutenant Colonel Engineers.

NOTE.—The printed legislative documents referred to accompany this report.

BOSTON, MASSACHUSETTS, September 17, 1867.

GENERAL: Having obtained the necessary information from the collector of the Barnstable district in relation to Provincetown—which was delayed from my supposing Provincetown to be in the Boston collection district, as it is in the Boston light-house district—I have the honor to report on certain additional matters which the circular No. 11 of the department, of June 10, 1867, states is called for by the recent appropriation laws for the works at Provincetown harbor and the sea-walls at Great Brewster, Deer, and Lovell's islands; although, as intimated in a previous letter, it is not understood that this report is expected where no disbursements have as yet taken place from the appropriations made by those laws.

Provincetoron.—This port is in the Barnstable collection district, and the harbor is from four to five miles from the Highland light, at Cape Cod, one of the most important lights on this coast. The revenue collected for the last fiscal year is \$3,471 22. And as to article 7 of engineer circular, I would respectfully refer to the enclosed copy of letter of Collector Swift, of Barnstable.

The abstract of proposals for contracts for work at Provincetown I forward to-day, with the new set of contracts prepared and completed by the Messrs. Blaisdells, which firm is the only one with whom contracts have as yet been

made for this work.

As to the sea-walls of Deer island, Lovell's island, and Great Brewster island, I would state that they are all within from one to three miles of Fort Warren, Boston harbor, and within about the same distance of Boston light; that they are in the collection district of Boston and Charlestown; that the revenue of this district for the last fiscal year was, as reported by the collector, as per enclosed copy of his letter, \$17,546,914 51; the item 7, of engineer circular No. 11, being also replied to in said letter of the collector.

The abstracts of proposals for Great Brewster island and Deer and Lovell's islands are forwarded to-day, with a letter in relation to the contracts recently

made for these works.

I am, sir, very respectfully, your most obedient servant,

H. W. BENHAM,

Brevet Major General.

Major General A. A. Humphreys,

Chief of Engineers.

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Custom House, Boston, Collector's Office, September 9, 1867.

SIR: In answer to your question in letter of September 4 as to the amount of commerce and navigation at this port, I have already made a partial statement, viz:

Amount of duties received at the port of Boston and Charlestown—in which the Great Brewster, Lovell's, and Deer islands are situated—in gold,

\$17,344,830 53; in currency, \$202,084 98.

2. The works in Provincetown are in the collection district of Barnstable, and for the amount of duties received I would respectfully refer you to Hon. Charles F. Swift, the collector. His office is in the town of Barnstable. His residence is in Yarmouth.

3. Question numbered 7 is more difficult to answer correctly. The number of foreign vessels entering this port last year was, in round numbers, 3,000, the number boarded by our officers being precisely 2,980; the number taking pilots

being 1,575.

The number of arrivals other than foreign I estimate, after much inquiry, at 9,000, including, of course, many small packets and fishing vessels, which arrive many times in a year. The number of vessels that took pilots in a year, from June, 1866, to June, 1867, was 2,518. I mention this as these vessels are generally of considerable draught of water.

The tonnage of vessels registered and enrolled and licensed in this port for the year ending June 30, 1867, was, in round numbers, two hundred and ninety thousand (290,000) tons, new measurement. Accuracy is impossible, as a por-

tion of the returns were of old measurement.

I am, very respectfully, your obedient servant, THOMAS RUSSELL. Collector.

H. W. BENHAM,

Major General United States Army.

CUSTOM HOUSE, BARNSTABLE, Collector's Office, September 13, 1867.

DEAR SIR: I am in receipt of your letter of September 12, 1867, asking for information upon several points in relation to Provincetown harbor, &c. In reply I have the honor to say—

1st. There was collected at the port of Provincetown, for the fiscal year ending

June 30, 1867, from all sources, the sum of \$3,471 22.

2d. As to the "amount of commerce and navigation that would be benefited by the completion of the works for the preservation of that harbor," I can only give with accuracy the local statistics of that place. There was, on the date specified, an enrolled and registered tonnage at that port of 218 vessels, comprising an aggregate of 16,229 tons. The local navigation of the port, as you are doubtless well aware, forms a very inconsiderable amount of the commerce that will be benefited by the preservation of the harbor. I have myself seen 400 vessels at anchor there at one time; and as a port of refuge it has probably no equal in the country.

Very respectfully, your obedient servant,

C. F. SWIFT, Collector.

Brevet Maj. Gen. H. W. BENHAM, U. S. Engineers' Office, Boston, Mass.

P 1.

BOSTON, MASSACHUSETTS, September 10, 1867.

GENERAL: I have the honor to forward herewith the annual report for the last fiscal year for the Great Brewster island sea-walls and for Deer and Lovell's island sea-walls, in this harbor.

The statement A, for these and the Provincetown works, referred to near the close of the report for the latter work, already forwarded, will be sent as soon as it can be prepared from information just received from the collector of this port, and for these three works, although it is not understood to be strictly required at this time, as no disbursements have as yet been made of moneys appropriated under the law requiring that information.

Very respectfully, your obedient servant,

H. W. BENHAM,

Colonel of Engineers, Brevet Major General.

Major General A. A. HUMPHREYS, Chief of Engineers.

BOSTON MASSACHUSETTS, September 6, 1867.

GENERAL: I have the honor to make the following report on the operations for the protection of Great Brewster island for the year ending June 30, 1867.

As in my last annual report a resume of the previous operations from the time it was first commenced by myself in 1849 was given, I propose here to limit myself more closely to the operations only of the past year, and these have been mostly as was anticipated in that report.

There was on hand, as stated at the date of last report, a quantity of cutstone facing sufficient nearly for 150 linear feet of wall, and, as was then proposed, an engagement was made for the delivery of about a like amount additional during that autumn, and a contract was made, after advertisement, for the further amount of facing for about 550 linear feet, being about all that was required for the original proposed amount of main wall; and, by another contract, the rough granite paving in rear of coping was provided for, and the stone under these different engagements have been received essentially as provided for and within the fiscal year, the delivery of the same being much facilitated by a wharf crane which I have had erected this season, and provided a portable engine for working it.

With the stone on hand the wall was recommenced in August, 1866, on the west branch of the wall of the south head, at 140 feet from the angle where Colonel Graham's work had commenced, and it was carried on about 250 feet to the end of the main line of this west branch, though not finished for the last fifty feet of the upper courses, when it was necessary to stop that part of the work and close it with a dry wing wall for the worst weather of the winter; and, during the bad weather, the men were occupied in laying the large rough paving in rear of the coping, and in filling with earth the very large space left in rear of this wall. In April the laying of the main wall at this head was resumed and carried to completion, paving, filling, and all, to a total length, in this fiscal year, of 350 running feet on west branch of south head, including the return wing wall at the west end of 104 running feet into the bank, there having been laid there during the fiscal year 5,900 feet, face measure, of wall, or about 1,680 cubic yards of faced masonry, and, with 350 yards of concrete foundation, 2,030 cubic yards were laid here, and about 550 superficial yards (in fact, cubic yards with the boulders beneath) of dry masonry paving in rear of the coping, and of earth work there was about 750 yards of excavation for foundation, and the mass of earth that was required to be filled in rear of this position of the wall, from the faulty location I found given to it, amounted to over 6,000 cubic yards, at a cost of over \$5,000. As usual, the filling in rear is an inappreciable portion of the This last expenditure, I regret to say, was caused mainly by the inproper site selected so far from the bluff for this west branch of wall. And another unnecessary large expenditure has been recently necessary at the north head to throw over the wall large quantities of earth that fell upon it from the high bluff, because the walls had been run too near it in the concave portion. In the latter case, the wall built in 1853 and 1854 was run some seventy feet further northward or into the bluff on the south line of the wall of the north head than was proposed by the plans sent to the engineer department in 1850. the case at the south head, although there had been the abrasions by the action of the sea for fifteen years upon that bluff, which is thus worn away at least three feet a year as I had formerly estimated, the wall of this south branch in 1865 was placed some fifteen feet beyond or without the suggested line of 1850. Both errors, as I understand, were mainly due to the same overseer, in the absence of the senior engineer in charge, and are necessarily alluded to here to explain an expenditure of some \$6,000 to \$7,000 on this account, an amount in fact sufficient for about 100 running feet of main wall.

When the wall of the south head was approaching completion in June the work was resumed upon the north head, and this north wall was constructed, except the paving in rear, to the extent of 130 feet at the coping, and 150 feet at the foundation, or an average of 140 running feet by the close of June, or of the fiscal year. This added about 2,520 superficial feet of faced masonry of the main wall, or 720 cubic yards, and, including 150 yards of concrete foundation, 870 yards of wall, in addition to that at south head, or 2,900 cubic yards of masonry altogether, and about 1,000 cubic yards of earth-filling was placed in addition to that at south head, or 7,000 yards of earth-filling in rear of both walls, and 610 yards excavated for foundation, or, with excavations at south head,

1,360 yards altogether.

As to the proposed operations of the present fiscal year, I would state that the work was progressing most favorably to the completion I had expected in the most economical manner. The whole front of the main bluff was protected, and the short line of wall on the north was carried to within, at the foundation, some twenty feet of the last return angle on the northwest, when I found that the appropriation of \$25,000, available for this year, could be used for either labor or materials only, under contracts duly advertised for, and this made it necessary, early in August, to issue such advertisements, and as I fortunately obtained one offer, (though only one,) and that from the overseer of that work, I was able to contract, though at additional cost, for the continuance of the work from the middle of August, and without any great delay. And my expectation is that the main wall, as formerly projected, extending around the north end of the north head, and for from 350 to 380 feet on the northwest side, where, as far as the bluff is worn by the action of the elements, will be completed this autumn, and that most, if not all, the required paving in rear of the coping will be laid, and that a jetty will be built, where it appears now to be required, at the west end of the west branch of the south head.

It is hoped that the funds appropriated will suffice for this purpose, though I am as yet somewhat in doubt upon this subject, my estimates having been made for the conduct of the work as heretofore, with contracts for materials only, while in contract for labor I cannot calculate on less than an addition of 33 to 50 per cent. for the profits and risk of the construction to furnish the same quantity of work.

In considering the future expenditures for this work, I would state that the sums now available, I trust, will be all that is required for the work as originally projected, but my observation of the action of the sea between the two main

walls on the east, that is, between the two bluffs, which I do not doubt were separate islands formerly, leads me to the conviction that the space between these walls, about 250 feet, should be closed by a wall of nearly or quite the same strength as those which protect the bluffs, or there will be great danger hereafter that the sea will force its way between them, cutting this island again into two parts, eddying in rear of the walls already built, and doing other damage

that cannot now be especially designated.

The original plan of Colonel Thayer, (drawn by Lieutenant Welcker,) as I saw it when I commenced this work in 1849, as I now have it in my office, and as I presume is also in the engineer department, contemplated only a riprap wall of rough stone for this position, and during my first operations on this work I doubted if even this riprap would be necessary, as I saw that the high bank of shingle and boulders between these walls continued full and unbroken during all the storms we then had. I found it not to be greatly changed even on my taking charge some sixteen to eighteen months ago. But I now find that it is greatly affected by the ocean storms, being much lowered and driven inwards to the rear of the lines of the walls. I do not doubt that the facts are that until the supply of shingle, boulders, &c., from the north head, was cut off by the completion of the wall in front of its eastern face, the drift or debris from this bluff was sufficient to keep up fully this bank, and supply any wearing action of the sea. But now that this wall is finished and there is no increment from this, or in fact any other source, and the abrading action of the sea is still continuing upon this drift bank of gravel and shingle, it will undoubtedly soon level it, and force it inward, with the probability of the results stated, in any violent storm. To remedy this, I think that the sea-wall should be built up at this space between the walls of the two heads, especially as a riprap wall, if effective, would be nearly or quite as expensive from the much larger amount of stone required, which even of rough stone is now so expensive, as nearly or quite to counterbalance the cutting and fitting of stone in a And for the 250 feet I estimate, as necessary, at the rate of \$75 per linear foot, the lowest sum that my calculations of cost of details will permit at present prices, \$22,500. This estimate is deemed sufficient, if the work is to be carried on as usual, with former appropriations, which permit the officer to use his judgment as to whether contracts should be made for labor or not. But if the amount is to be used under conditions as required by the last law making appropriations, I shall not feel safe, except with estimates at least 50 per cent. additional as necessary for the profit and risk of the contractors, or for the sum of \$33,750, or, say, \$34,000.

In recapitulation I would state as follows, as to the work for the protection of Great Brewster island for the past year, that as soon as the appropriation was available the work was resumed in laying the wall of the west branch of the south head, which was completed, and to the extent of 350 linear feet, and the space in the rear filled, and the paving set before the close of the fiscal year, and early in the present season the wall of the north head was resumed and completed except as to a small portion of the coping of the main or most exposed face on the east face, and, excepting the paving in rear, to the extent of about 140 linear feet, or 490 linear feet altogether on both walls by the close of the fiscal year. Contracts and engagements were also made, and mainly executed during the year, for the facing of about 700 linear feet of wall, and for most of the required paving in rear. A wharf crane was erected and a portable engine

provided for the work.

And it is expected that by the close of the present working season all the main sea wall, as originally planned, or about 2,500 linear feet, altogether, will be completed, which will give ample protection to all the bluffs of the island.

No future work is required or estimated for, except for about 250 feet of wall between the two main walls as originally planned, which, from the continued

wearing action of the sea, appears now to be rendered necessary, and for this estimate of \$22,500, or, if the law requires the labor to be done by contract, \$34,000 is asked, as ample for all that I now anticipate can be required in future.

Report of funds for Great Brewster island sea wall.

On hand with assistant treasurer at Boston, July 1, 1866	\$900 86
On hand in United States treasury from appropriations in 1866. Received for sale of public property	75, 0 00 00 2, 250 00
Total available for fiscal year ending June 30, 1867	78, 150 %
Expended during the year	\$62, 240 82
In the United States treasury July 1, 1867, less amount of taxes paid direct from treasury to revenue department Of which there was due to the appropriations of other works for	\$25, 218 44
debts paid, as per note 6 of weekly money statement	9,308 40
Leaving balance from former appropriation available July 1, 1867 Appropriation of March, 1867	15, 910 04 25, 000 00
Total available for the service of the year ending June 30,1867	40, 910 04

I am, sir, very respectfully, your obedient servant,

H. W. BENHAM,

Colonel of Engineers Brevet Major General.

Major General A. A. HUMPHREYS, Chief of Engineers U. S. Army.

Boston, Massachusetts, September 7, 1867.

GENERAL: I have the honor to make the following report in reference to the operations on the sea-walls of Deer and Lovell's islands for the year ending June 30, 1867:

As stated in my last annual report, the work at Deer island has been closed from the autumn of 1865, in consequence of the exhaustion of the appropriation; the work of the season and of the year preceding, under Colouel J. D. Graham, having consisted of the erection of the boarding-house, storehouse, and stable, and the rebuilding of the sea-wall of the north head in two places, where it had been most injured, and to the extent of about 300 linear feet.

As, both in my last report and in Colonel Graham's for the previous year, the character of the sea-walls protecting the three heads or bluffs of this island are given, and in my last report the proposed manner of rebuilding them, this is not repeated here, more than to state that the old "dry-laid" sea-walls, with earth backing, are being relaid in mortar, with a good concrete backing to give them an average thickness of some eight feet, or about one-half their average height.

The work of the fiscal year was commenced on the middle head in August, 1866, as soon as practicable after the notice of the funds received as mentioned in my report in the last week of July; and though I then had the full expectation, notwithstanding unusual delays and difficulties in obtaining workmen, of being able to complete the necessary repairs upon that portion during that

working season, I regret to say that, although the work was pushed forward to late in December, the difficulty and expense of removing the old wall, with the injuries received by storms, were so great that I was not able to have accomplished more than the relaying of about 180 linear feet of wall with the paving in rear, where the gap was closed up for the winter. As the season opened this year in April, and early in May, the work of relaying the wall of the middle head was resumed with a more effective working force, and it has progressed much more satisfactorily thus far, fully double the amount of work or more having been accomplished daily, so that by the close of the year, or 30th of June, about 160 linear feet additional had been relaid, with the most of the paving in rear, making, as a total of the work for the fiscal year, the rebuilding of 340 linear feet of wall, 17½ feet high by about eight feet thick, with its concrete backing of about 1½ yard to the linear foot.

Since the last date, however, it may be stated, though coming more properly in the report of the present fiscal year, this work has gone on with increased rapidity, so that at the *date* of this report the wall of the middle head is completed to its south extremity, and to the extent of about 200 linear feet since June, being all there now appears necessary at this bluff; the curved end or wing wall at the north end, about 290 linear feet, being in such good condition and so little exposed, that I have thus far doubted the necessity of relaying

this portion.

Upon the south head also the work of rebuilding has been commenced at the north end, where it had been most injured by the storms, and at this date about 120 linear feet is relaid, (without the paving in rear,) and I have now little doubt, with my working force continuing as at present, that we shall be able to complete the rebuilding of the wall of this head during the present working season. The more advantageous prosecution of the work this season gives me a more sure basis for estimating for funds needed for the work yet to be accomplished, than I had at first at the date of my last report, when I thought it would be done much less, or at the close of the last season, when I had feared it would be so much more expensive than what I now think it will prove to be.

The expenditure for the working season of last year, to December 31, during which I was only able to get, as stated, about 180 linear feet of wall rebuilt, was \$7,844 08, or \$43 58 per linear foot; while the expenditures this season, (exclusive of \$2,500 charged on account of one-quarter part of steamer,) to September 1, are about \$11,000 for about 470 linear feet as laid this season at that date, or nearly \$24 per linear foot of wall, which I trust will enable me to estimate closely for the funds necessary to be appropriated in addition to what may be on hand for the completion of the rebuilding of the wall of the north head, which, I doubt not, may be completed with the necessary funds

available in two more seasons.

At Lovell's island, the funds for which are included in the same appropriation with Deer island, but little has been done this season, thus far, beyond the completion of the boarding-house, and the securing of the hay crop for future use. The increase of cost of the wall at Deer island last season, over what I expected, made me fear to make the expenditure I had proposed on this island, where less necessary, until further sums should be available. I say less necessary, because a very critical examination of the old mortar wall of this island showed that this is not at all essentially injured by the action of the sea up to this time, and there is only required, as formerly reported for this part, the placing of one or two new jettees at parts where the outer surface near the front of the wall is most abraded, and the relaying of an old jettee at its west extremity, and no special injury to be expected from delaying this work till the next season, when the erection of the new wall at the southeast bluff, for which the facing is now being prepared, can also go on as now planned.

The information of the appropriation of the \$25,000 estimated for this new

wall was received in the department letter of the 13th of June, and proposals were called for within the week, and a contract was completed for the furnishing of the cut-stone facing for this wall (eight feet high, at \$15 the linear foot) on the 19th of July; and at the date of this report several cargoes have been already received of this stone, as well as some loads of jettee stone contracted for at the same time; and it is expected that the whole will be delivered as per contract this autumn, and that early in the ensuing year this new wall will be commenced and completed during the working season, and all the required repairs to the old wall executed.

I would state that the estimate which I had made for this wall at the southeast head of Lovell's island came out most closely, in proportion to its size, with the actual cost this year of the Great Brewster wall, as laid without contract, the facing being just about one-half the total cost of the wall; and this facing, as contracted for, will cost about \$12,000, leaving thus \$13,000 of my estimate for purchases of cement, obtaining concrete material, and laying the walls; that is, if done without contract, as formerly. If the work, however, is not executed in this way, but according to the law making the last appropriation, I must add fifty per cent. for risks and profits of contractors, and other incidental expenses,

or \$6,500 in addition to my former estimate. For each jettee about 3,000 cubic feet of stone are required, which have been contracted for at thirty-five cents per foot, or say 331, or \$1,000 for the split stone for each jettee; and as the cost of laying the stone with concrete materials, &c., is about twenty per cent more than the cost of the stone. I thus find as the estimate necessary for Lovell's island, besides the above..... **\$**6.500 For one jettee fifty feet long, for stone and laying..... 2, 200 For one jettee twenty-five feet long, for stone and laying..... 1, 100 For relaying jettee at west end of old wall..... 1,200 Add for contingencies, hauling stone, &c., from the beach, this 1,500 12, 500

Of which there has been expended to September 1st, about And there is estimated to complete 292 feet at south head, this	\$11,000 00
season, at \$24 per foot	7,000 00
Add for contingencies, closing work, &c	529 30

18, 529 30

This leaves an unexpended balance for Deer island, at the close of the working season, of \$41,000.



And neglecting the difference as to labor on the jettees, if the labor of the wall is to be executed by contract, add for profit and loss to contractors fifty per cent. on main wall	20, 000
	64, 500
Or less the \$41,000 above reported as expected to be on hand December 1st	\$23, 500
	12, 500 36, 000
Total required for both works, with labor by contract	30, 000

Or if the work is executed as heretofore by days' labor, less the \$26,000 estimated by contract for both works, there will only be required for the next fiscal

year \$9,500, or say, to complete the work, \$10,000.

I would therefore state, in recapitulation, as to the work executed on Deer and Lovell's island sea-walls for the fiscal year, that the rebuilding of wall of the middle bluff was commenced in August, 1866, and by the close of June, 1867, 340 linear feet of the dry wall were securely rebuilt with mortar joints and concrete backing, with the paving of heavy flat stone twelve to fifteen feet in rear of coping; the wall being about 17½ feet high, with an average thickness of eight feet.

At the date of this report, the balance of wall requiring relaying upon that middle head, about 200 linear feet, has been completed, and about 110 linear feet of the wall of south head, of the same size, has been relaid, and it is expected that the remainder of the wall of this head will be rebuilt during the present

working season.

At Lovell's island but little has been done during the fiscal year, except the completion of the boarding-house for workmen, and the call for contracts for facing stone for a new wall to the southeast head, and for jettee stone for the old wall. These contracts were completed in July, 1867, and it is expected that all the necessary stone will be delivered this autumn, and the work of building the jettees to the old wall and of constructing the new wall will be commenced as soon as the season permits in 1868.

Statement of funds received and expended on sea-walls for Deer and Lovell's islands for the fiscal year ending June 30, 1867.

On hand July 1, 1866	\$60 <i>5</i> 533 18, 0 00	14
Expended for Deer island		
Expended for Lovell's island		
_	16, 061	13
On hand in assistant treasurer's office, Boston, July 1, 1867 On hand in treasury undrawn, Boston, July 1, 1867, including the \$25,000 estimated and appropriated for Lovell's island, and the	2, 544	35
\$25,000 for Deer island, March, 1867	81,984	95
Total available for both works	84, 529	
island	59, 529	30

I am, sir, very respecfully your obedient servant.

H. W. BENHAM,

Colonel of Engineers, Brevet Major General U. S. Army.

Major General A. A. Humphreys, Chief of Engineers.

APPENDIX Q.

Boston, September 5, 1867.

GENERAL: I have the honor to make the following report of the operations on the works under my charge during the fiscal year ending June 30, 1867:

PRESERVATION AND IMPROVEMENT OF BOSTON HARBOR, MASSACHUSETTS.

Brevet Major General J. G. Foster was assigned to the charge of this work in May, 1867, and arrived and assumed charge on May 28, 1867. Preliminary surveys were at once made, and proposals were advertised for, for dredging a channel across the upper middle bar and the west extremity of Lovell's island, and for blasting and removing the rocks in the Narrows, (Tower Rock and Corwin Rock,) lying between Fort Warren and the Narrows light, (commonly called Bug light,) upon the extremity of Great Brewster spit. Pending the securing of the titles to the land at Long island, Gallup's island, and Point Allerton, upon which the sea-walls are to be constructed, no work has been commenced at these points, nor have proposals for their construction been invited.

It is proposed during the current year to remove by dredging about 40,000 cubic yards from the upper middle bar, so as to make a channel across that bar 300 feet in width and twenty-three feet deep at mean low water; to dredge off the southwest point of Lovell's island, and off the extremity of Great Brewster spit, removing about 145,000 cubic yards, so as to widen the channel at that point to 500 feet, at twenty-three feet depth at mean low water; to remove entirely "Tower Rock" by blasting it off to the depth of twenty-three feet at mean low water, and removing the fragments from the channel, and then to blast off and remove as much of "Corwin Rock" as the unexpended portion of the appropriation assigned to that object will accomplish. An additional amount will be required for its entire removal. The construction of the sea-walls upon Long Island head, Gallup's island, and Point Allerton, which latter is called in the laws of Massachusetts "Point Alderton," will also be commenced. For this purpose contracts will be made for the materials and labor, and for the construction of a temporary wharf at Long Island head and Point Allerton, with a view to the commencement of the work early in the spring, and for its rapid prosecution during the summer.

Expended during the fiscal year ending June 30, 1867	\$239	06
Probable amount to be expended during the fiscal year ending	299, 760 .	0.4
June 30, 1867	299, 700.	74

Estimated amount required for the fiscal year ending June 30, 1869.

Estimatea amount requirea for the fiscal year enaing sune 30	, 1509.	
For the preservation of north head of Long island	\$ 75,000 (00
For the preservation of Gallup's island	54,000	00
For dredging the southwest point of Lovell's island, to widen the		
channel to 685 feet at 23 feet depth of mean low water, as re-		
quired by the project of the Boston harbor commissioners	130,000	00
For dredging the upper middle bar to 23 feet at mean low water,		•
and a width of 1,000 feet, as called for by the project of the		
Boston harbor commissioners	110,000 0	0(
For the preservation of Point Allerton	21,000 0	
For blasting and removing Corwin Rock	24,000 0	0

414, 000 00

202,084 98

17, 546, 915 51

I have also the honor to report the points called for in circular No. 11 of the engineer department, series 1867, as follows, viz:

1st. The plan adopted for the works of preservation and improvement, under

my charge in this harbor, is substantially that of the Boston harbor commis-
sioners, which has met the general approval of the Chief of Engineers. The
items of this plan are as follows, viz:
For the improvement of the channel across the upper middle bar,
by dredging a channel 23 feet deep at mean low water, and
1,000 feet wide, at a total estimated cost of \$157,085 00
For the improvement of the channel at the Narrows, by dredging
off the southwest point of Lovell's island and the extremity of
Great Brewster spit, so as to widen the channel to 685 feet at
23 feet depth of water, at a total estimated cost of 188, 805 00
For the improvement of the channel at the Narrows, by the re-
moval by blasting of Tower Rock and Corwin Rock, at an esti-
mated cost of
For the preservation of the north head of Long island, by the con-
struction of a sea-wall, at an estimated cost of 150, 000 00
For the preservation of the north end of Gallup's island, by the
For the preservation of the north end of Gallup's Island, by the
construction of a sea-wall, at an estimated cost of 103, 585 63
For the preservation of Point Allerton, by the construction of a
sea-wall, at an estimated cost of
2d. The amounts that are required for the entire and permanent completion
of each work of preservation and improvement under my charge is given in the
previous paragraph, No. 1, with the exception of the removal of Tower Rock and
Corwin Rock, the cost of which will be \$44,000.
3d. The amount that can be profitably expended upon each work during the
next fiscal year is as follows, viz:
For the preservation of the north head of Long island \$75,000 00
For the preservation of Gallup's island
For the preservation of Point Allerton
For the preservation of Point Allerton
For dredging the southwest point of Lovell's island and the ex-
For dredging the southwest point of Lovell's island and the extremity of the Great Brewster spit
For dredging the southwest point of Lovell's island and the extremity of the Great Brewster spit
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For dredging the southwest point of Lovell's island and the extremity of the Great Brewster spit

7th. By information received from the collector of this port, I am enabled to report, in regard to this item, that the number of vessels that will probably be

Revenue in currency

Total revenue.....

Thomas Russell, as follows, viz:

benefited in some degree by the widening of the channel will be about 24,000, which is the aggregate number of vessels estimated to enter and leave this harbor annually by the channel through the Narrows. The amount of tonnage during the fiscal year ending the 30th of June, 1867, was as follows:

Tonnage entered from foreign ports	731, 930
Tonnage entered from domestic ports	956, 133
Tonnage cleared from foreign ports	689, 822
Tonuage cleared from domestic ports	1, 243, 366

This amount of tonnage will no doubt be benefited, more or less, by the widening of the channel. The deepening of the channel to twenty-three feet will, the collector thinks, benefit about 3,000 vessels of an aggregate tonnage of about 1,000,000 tons.

The amount of commerce to be benefited by the proposed improvement it is difficult to estimate, as no approximation can be made of the amount of the coasting trade. The imports and exports, however, are accessible, and amounted during the last fiscal year to the following:

Total imports	\$47, 288, 747
Total exports	19, 317, 874

Aggregate amount of foreign commerce 66, 606, 621

8th. The work being on June 30 at its commencement, no proposal or contracts had at that time been made.

Respectfully submitted:

J. G. FOSTER,

Bvt. Maj. Gen. U. S. A., Lieut. Col. Engineers.

Major General A. A. HUMPHREYS, Chief of Engineers, Washington, D. C.

Q 1.

ENGINEER DEPARTMENT. Washington, February 11, 1867.

SIR: I respectfully return herewith the letter of Hon. T. D. Eliot, acting chairman of the Committee on Commerce of the House of Representatives, of the 29th ultimo, with enclosed papers relative to the improvement of Boston harbor, referred to this department for report, and beg leave to submit the following remarks in relation thereto.

The preservation of certain of the islands and headlands in Boston bay is essential to the maintenance of the existing channels. So important is this considered in connection with existing and proposed permanent fortifications for the defence of the city of Boston, that the construction of sea-walls for the preservation of Deer and Lovell's and Great Brewster islands has been carried on for some time past under regular appropriations.

Estimates for the completion of these walls, amounting to \$25,000 for each, are submitted in the last annual report from this office, excepting for that of Deer island, the report upon which did not reach this office in time to be included,

but is now presented.

The debris from these islands form shoals that are advancing regularly into the channels, have already obstructed them, and threaten to make still more

serious changes. The growth of these shoals or spits should be arrested and

portions of them removed by dredging.

In addition to the islands named, works for the preservation of the north head of Long island and of Gallup's island should be at once commenced as part of the system of defence, as well as for the interests of commerce, as the debris of these islands form shoals that threaten to obstruct seriously the main channels.

With reference to the other headlands named in the petition, that of Point Allerton is of the first importance in the interests of navigation, and should be preserved in the manner indicated in the papers accompanying the petition.

Of the same general character but of less importance is the preservation of

Nantasket bluff.

Respecting the south head of Long island, Moore island, and Winthrop's head, I am not prepared to report. Their preservation would prevent the deterioration of subordinate channels, convenient to local and perhaps other trade, but the closure or material shoaling of these channels would aid the defence, and if that shoaling or the process causing it should not in any way impair the main channels, I am not satisfied that the deterioration should be arrested. The question is one that will require careful investigation.

The removal of Tower Rock and Corwin Rock, which stand on the sailing line in mid-channel, and the dredging of Great Brewster and Lovell's Island spits, are works of the first importance in rendering the navigation of the main

channels leading to Boston harbor secure and commodious.

The removal of the upper middle bar, though not so imperatively required for the safety of navigation, would yet be of great advantage to it. There are certain considerations, however, (which may be presented fully at some other time,) as well as those of economy, which may restrict the removal of Lovell's Island spit and the upper middle bar within the limits of the project of the Harbor Commission, (which proposes a channel one thousand feet wide at the upper middle bar, and 685 feet wide at Lovell's island, dimensions well adapted to the great commerce of Boston.) A portion of the amount of the estimate of the commission judiciously expended will be of great advantage.

I have had the benefit of a personal conference with the Harbor Commission of Boston, and an examination of their reports, plans and estimates. Under their direction extensive and careful surveys and examinations in Boston bay and harbor have been carried on for several years, and estimates of the extent and cost of the works for the preservation of the headlands and the removal of

the obstructions in the channels have been prepared.

The amounts required for the fiscal year ending June 30, 1868, for the preservation of headlands and removal of obstructions in the main channels which are deemed essential for the restoration and preservation of those channels, are as follows:

For completion of sea-wall on Great Brewster island	\$ 25, 000
For completion of sea-wall on Deer island	25, 000
For completion of sea-wall on Lovell's island	
For the preservation of the north head of Long island	
For the preservation of Gallup's island	
For the removal of Tower Rock and Corwin Rock	
For dredging Great Brewster spit, Lovell's Island spit, and upper	
middle bar	
For the preservation of Point Allerton	50, 000

For the completion of these works there will be required in addition for the fiscal year following that ending June 30, 1868—

For the	preservation	of north	head Long	ieland	,	\$7 5,	000
							000

For dredging Lovell's spit, if it should be found necessary to carry it to the full width of the project of the Boston Harbor Commission, 685 feet
For dredging the upper middle bar, if it should be found necessary
to carry it to the full width of the project of the Boston Harbor
Commission, 1,000 feet
For the preservation of Point Allerton
For the preservation of Nantasket bluff
Very respectfully, your obedient servant,
A. A. HUMPHREYS,

A. A. HUMPHREYS, Chief of Engineers.

Hon. E. M. STANTON, Secretary of War.

APPENDIX R.

Report on the Saco river improvements for the fiscal year ending June 30, 1867, by Brevet Brigadier General George Thom, lieutenant colonel of engineers.

United States Engineer Oppics, Portland, Maine, September 10, 1867.

By acts of Congres approved June 23, 1866, and March 2, 1867, appropriations were made for improving the navigation of this river. That made in 1866 (to wit, \$40,000) was, as I understand, based on the report and estimates of George Davidson, esq., assistant United States Coast Survey, dated May, 1866, and the balance appropriated in March, 1867, was based on the estimates submitted by me in my report to the engineer department, dated January 5, 1867. Accompanying that report were plans and estimates for rebuilding the piers and removing the sunken rocks, amounting to \$211,701 05, which included General Alexander's estimate of \$192,500 for the breakwater uniting the piers (Nos. 11 and 12) near the mouth of the river.

In obedience to instructions from the department, dated March 28, 1867, (a copy of which is hereto appended, marked 3,) I issued a notice dated April 8, 1867, inviting "proposals for furnishing rough stone for a breakwater at the mouth of Saco river, Maine." A copy of the notice is hereto appended, marked 4.

Four bids were received for this work, the lowest being that of Messrs. James M. Deering & John W. Deering, of Saco, Maine, for \$1 81 per ton of 2,240 pounds, as will be seen in referring to the abstract of proposals hereto appended, and marked 1. Before entering into a contract with these parties for the work, I awaited the determining of the position of the breakwater by a board of engineers to be appointed for that purpose, as instructed by department letter of the 28th of March, 1867. On the 13th of June the report of the board of engineers on this subject was transmitted to the department, a copy of which is hereto appended, marked 5. On the 14th of June I entered into a contract with Messrs. Deering, who were the lowest bidders, for furnishing the stone for the breakwater. The report of the board of engineers and contract made with Messrs. Deering having both been approved by the department, the contractors commenced the delivery of the stone on the last of June.*

In pursuance of instructions from the department, dated March 28, 1867, I advertised for proposals for removing a sunken rock in Saco river, a copy of the

notice being hereto appended, marked 6.

^{*} Up to the 8th of September, 1867, about 14,000 tons of stone were delivered.

Two bids only were received for this work, the lowest being that of Mr. James Andrews, of Biddeford, for six hundred dollars, as will be seen on referring to the abstract of proposals hereto appended, marked 1.

A contract was made with Mr. Andrews, and it has been fulfilled by him.*

The amount that will be required for the entire permanent completion of the breakwater and other improvements in Saco river,

as exhibited in my report dated January 5, 1867, (since printed\$211,701 05

by Congress,) is To which add for 105,000 cubic feet additional coping

of breakwater on its exterior face down to low-

water mark, at 50 cents per cubic foot...... \$52,500 00

10 per cent. for contingencies..... 5, 250 00

57,750 00

269, 451 05

Say, \$270, 000.

Total amount appropriated by acts of Congress approved March 2, 1867, and June 23, 1866, \$80,000; amount required for completion of work, \$150,000, of which \$75,000 can be profitably expended upon it during the next fiscal year.

The works for the improvement of the navigation of Saco river are located in the collection district of Saco and near the ports of Saco and Biddeford, Maine. The nearest light-house is on Wood island, opposite the mouth of Saco river, and the nearest forts are those in Portland harbor.

The amount of revenue collected at Saco, (the port of entry,) as furnished by

the collector, was for the fiscal year ending June 30, 1867, \$755 76.

As to the amount of commerce and navigation that would be benefited by the completion of these works of improvement I will here repeat the information kindly furnished by Mr. Edward Eastman, then United States deputy collector,

contained in my special report of the 5th of January last, to wit:

"At the inner harbor, which is inside the bar, we have averaged about forty coal vessels and about one hundred and fifty to two hundred vessels in the coasting business, bringing general merchandise for this place and Biddeford and the back country, and carrying out timber, &c., exclusive of what comes by railroad. This port is the entry for the supplies, including the back districts, of a population of about thirty thousand, and is the natural shipping port of the timber interest of nearly the whole of York county, except some towns in the extreme western part which lie near Portsmouth, New Hampshire.

"In addition to our former business since this appropriation was made an ice company has been formed by persons from Philadelphia in connection with residents of Saco, who are making arrangements for the exportation of ice. They are erecting large store-houses on or near one of our wharves, and say they will employ about one hundred coasting vessels in the ice-carrying trade the

coming season.

"The Saco Ship-building Company also has just fairly started, and will give a new impetus to our commerce, as they obtain a considerable portion of their timber from the southern markets."

The improvements now in progress may be regarded as permanent, and such as after completion will not require further expenditure.

The papers described as follows are hereto appended:

1. Abstract of proposals for each work, with name of bidders, &c.

^{*}On the 24th of July, 1867, another contract for the entire removal of the sunken rocks at Little islands was made with Mr. James M. Andrews, of Biddeford, Maine, for the sum of \$2,000. On the 8th of September, 1867, the contract was fulfilled.

- 2. Abstract of contracts for each work, with name of contractors.
- 3. Copy of instructions from the engineer department.
- 4. Copy of notice inviting proposals for breakwater.
- 5. Copy of report of board of engineers.6. Copy of notice inviting proposals for removing sunken rock.

GEORGE THOM, Lieut. Colonel of Engineers, Brevet Brig. Gen. U. S. A.

No. 1.—Abstract of bids received for the improvement of Saco river, Maine.

Nature of work.	No. for reference.	Names of bidders.	Amount bid for.	Price.
Furnishing rough stone for break-	1	Wm. Courtenay, Baltimore, Maryland.	25,000 tons	\$2 23 per 2,240 lbs.
water.	2	Isaac Hamilton, Cumber- land. Maine.	25,000 tons.	2 15 per 2,240 lbs.
	3	Jas. Andrews, W. G. Gooch & Co., Goodwin & Buck, Ira Andrews, W. & J. Moore, all of Biddeford, Maine,	50,000 tons, (more or less.)	1 83 per 2,240 lbs.
	4	Jas. M. Deering and John W. Deering, of Saco, Maine.	50,000 tons, (more or less.)	1 81 per 2,240 lbs.
For removing sunk- en rock.	1	Ira Andrews, of Biddeford, and Jas. M. Deering, of Saco.	Whole work.	\$ 2,360 00
	2	Jas. Andrews, of Biddeford, Maine.	Whole work.	600 00

GEORGE THOM, Lieut. Col. of Engineers, Brevet Brig. Gen. U. S. Army.

No. 2.—Abstract of contracts made for the improvement of Saco river, Maine.

Nature of work.	No. for reference.	Names of contractors.	Quantity.	Price.
Furnishing rough stone for break- water.	1	Jas. M. Deering and John W. Deering, of Saco, Maine.	50,000 tons, (more or less.)	\$1 81 per 2,240 lbs.
For removing sunk- en rock.	2	Jas. Andrews, of Biddeford, Maine.	Whole work.	\$600 00

GEORGE THOM. Lieut. Col. of Engineers, Brevet Brigadier General.

No. 3.

ENGINERE DEPARTMENT, Washington, March 28, 1867.

Colonel: Your letter of the 26th instant has been received. Your recommendation that the funds appropriated for the improvement of the Saco river be applied to the construction of a breakwater uniting piers Nos. 11 and 12, except so much as may be required to rebuild pier No. 6, and to remove the sunken rock in the channel at Little's island, and (should it be hereafter desirable) to rebuild pier No. 8 at "Ferry Narrows," is approved, with the condition that the position of the breakwater shall first be determined by a board of engineers. You can proceed, however, to advertise for proposals for furnishing the necessary materials for the work, in order that no unnecessary delay may occur in its vigorous prosecution at as early a day as practicable.

Very respectfully, your obedient servant,

A. A. HUMPHREYS.

Brig. Gen. and Chief of Engineers, Maj. Gen. of Vol.

Brevet Colonel GEORGE THOM, U. S. A.,

Lieut. Colonel of Engineers, Portland, Maine.

No. 4.

Proposals for furnishing rough stone for a breakwater at the mouth of Saco river. Maine, will be received at this office until 10 o'clock a. m, on Tuesday, the 30th instant.

The quantity required, under recent appropriations, will be fifty thousand tons, more or less. The stone must be of suitable size and strength, and be deposited in such places and in such manner as may be directed by the superintending engineer, in conformity with the plans adopted.

The delivery of the stone must commence as soon as practicable after the approval of the contract, and be completed on or before the first day of December next. Persons in making proposals will state the price per ton (of 2,240 pounds) of stone delivered and deposited in position; the weight to be ascertained at the expense of the contractor and to the satisfaction of the superintending engineer.

Proposals will be considered for one-half or for the whole of the above-named

Should any person find it impracticable to undertake the whole job in the time above specified, they are requested to state in their proposals the earliest period (to be not later than the 1st of July, 1868) in which they will contract to complete it, with the understanding that not less than 35,000 tons will be delivered on or before the first day of December next.

Each proposal must be accompanied by a written guarantee signed by two responsible persons, in the required form, that the bidder will, when called on, if his proposal be accepted, enter into a contract and bond, with good and sufficient security, (the sureties and their places of residence to be named in the proposal,) for the true and faithful performance of his contract. The contract will be awarded to the lowest responsible bidder, and be subject to the approval of the Secretary of War.

The undersigned, however, reserves the right to exclude the bids of any persons who there is reason to believe will not faithfully and promptly perform the contract; any informal bids, as well as those that are above a responsible price for the work; and no member of Congress, officer or agent of the government, nor any person employed in the public service, shall be admitted to any

share in the contract, or any benefit which may arise therefrom.

Payments will be made monthly—20 per cent. to be reserved therefrom until the whole work is furnished, and to be forfeited in the event of the non-fulfilment of the contract in the time and manner as therein required.

Persons desiring to make proposals will please call on the undersigned at his

office, in Morton block, on Congress street, for forms of same, and for more definite information, if desired, and, on transmitting them, will indorse thereon, "Proposals for improvement of Saco river."

GEORGE THOM.

Brevet Brigadier General, U. S. Army.

UNITED STATES ENGINEER OFFICE, Portland, Maine, April 8, 1867.

No. 5.

Proposals for removing a sunken rock in Saco river, Maine.

Proposals will be received at the office until 2 o'clock p. m., on Friday, the 17t instant, for removing a sunken rock in mid-channel of Saco river, at Little island. The rock has an irregular shape, being about seventeen feet long by five wide, and six feet in height. The depth of water in the channel at this place is from eight to nine feet at mean low water. The rock must be entirely removed from the channel, and its fragments be deposited on the site of the brenkwater at the mouth of the river. The work must be commenced immediarely after the approval of the contract, and be completed not later than four weeks from that period. In making proposals bidders will state the price for which they will perform the job. Each proposal must be accompanied by a written guarantee, signed by two responsible persons in the required form, that the bidder will, when called on, if his proposal be accepted, enter into a contract and bond with good and sufficient security (the sureties and their places of residence to be named in the proposal) for the true and faithful performance of his The contract will be awarded to the lowest responsible bidder and be subject to the approval of the Secretary of War. The undersigned, however, reserves the right to exclude the bids of any persons who there is reason to believe will not faithfully and promptly perform the contract; also any informal bids, as well as those that are above a reasonable price for the work; and no member of Congress, officer or agent of the government, nor any person employed in the public service, shall be admitted to any share in the contract or any benefit which may arise therefrom. Payment will be made on the satisfactory completion of the work.

Persons desirous to make proposals will please call on the undersigned, at his office in Morton block, on Congress street, for forms of same, and for more definite information, if desired; and on transmitting them, will indorse thereon "Proposals for removing rock in Saco river, Maine."

GEORGE THOM, Brevet Brigadier General U.S. A.

UNITED STATES ENGINEER OFFICE, Portland, Maine, May 7, 1867.

R 1.

UNITED STATES ENGINEER'S OFFICE, Purtland, Maine, June 13, 1867.

GENERAL: I have the honor to transmit herewith a report of board of engineers, convened in a mpliance with Engineer Order No 43, dated June 6, 1867, "for the consideration of the plan proposed for, but especially to determine the

proper position and extent of, the breakwater at the mouth of the Saco river." Also, to forward herewith a copy of the plan recommended by the board for adoption.

I am, very respectfully, your obedient servant,

GEORGE THOM,

Lieut. Col. Corps of Engineers, Brevet Brig. Gen. U. S. Army. Major General A. A. Humphreys,

Chief of Engineers, U. S. Army, Washington, D. C.

In pursuance of Engineer Order No. 43, dated June 6, 1867, the board of engineers—consisting of Brevet Brigadier General Hartman Bache, colonel of engineers; Brevet Brigadier General George Thom, lieutenant colonel of engineers; Brevet Colonel Thomas L. Casey, major of engineers—assembled at Saco, Maine, on Tuesday, June 11, 1867, "for the consideration of the plans proposed for, but especially to determine the proper position and extent of, the the breakwater at the mouth of Saco river."

The board made a personal examination of the river, the channels, piers, and bars at its mouth, after which it adjourned, to meet at Portland, Maine, at 9 a.

m. Wednesday, June 12, 1867.

The board, pursuant to adjournment, met at Portland, Maine, at 9 a.m.

Wednesday, June 12, 1867; present, all its members.

All the information relating to this work in the possession of Brevet Brigadier General George Thom, lieutenant colonel of engineers, was laid by him before the board, including the following papers, viz: Notes on the piers in the Saco river, Maine, submitted by Captain J. G. Barnard, corps of engineers, dated October 18, 1853, with accompanying drawings; report of Mr. George Davidson, assistant, United States Coast Survey, dated May 19, 25, and 27 1866, with accompanying maps; and report of Brevet Brigadier General B. S. Alexander, lieutenant colonel of engineers, dated October 16, 1866, with accompanying plans.

After a free discussion of the various plans presented, it was-

Resolved, That this board accepts the plan proposed by Brevet Brigadier General B. S. Alexander, lieutenant colonel corps of engineers, for the breakwater at the mouth of Saco river, Maine, as set forth in his report of October 16, 1866, and recommends that it be constructed to the extent and in the position therein laid down. At the same time, the board would leave it to the direction of the constructing engineer to make such changes in the inner slope of the cross section of the breakwater as may be found expedient.

The board having concluded the duties for which it was convened, adjourned

sine die.

THOMAS LINCOLN CASEY,

Major of Engineers and Brevet Col. U.S. Army. GEORGE THOM,

Lieut. Col. of Engineers, and Brevet Brig. Gen. U. S. Army. HARTMAN BACHE,

Colonel of Engineers, Brevet Brigadier General.

R 2.

Report on the survey of Richmond's island, ('ape Elizabeth, Maine, for the year ending June 30, 1867, by Brevet Brigadier General George Thom, lieutenant colonel corps of engineers.

United Statks Engineer Office, Portland, Maine, September 10, 1867.

The act of Congress approved March 2, 1867, providing for the survey of this locality, does not state its purpose; but as a small appropriation has heretofore been made for building a breakwater connecting Richmond's island with the mainland, and as an attempt was made about the years 1853 and 1854 to construct such a work, for the purpose of making a harbor of refuge at that place. I have caused a survey to be again made with a view to submitting a plan and estimate of the cost of such a work as, in my opinion, would be best adapted to that purpose.

The bar connecting the island and the mainland is, doubtless, caused by the meeting of the currents passing around both the eastern and the western points of the island; that portion of the bar nearest to the island being, as it appears fixed and unchanged in its position, while that nearest the mainland is moved a short distance from the westward to the eastward, as the east or west winds may respectively predominate. Its general position, however, is on a line con-

necting the two nearest points of the island and mainland.

A breakwater constructed on this bar, uniting these two points, would form a good harbor of refuge, affording a safe anchorage and good holding ground, with the wind from any point between north and southwest; and as the winds from the northward and eastward bring the most violent and destructive gales which occur on this coast, there can be no question as to the very great importance and advantage of such a work at this place, in thus affording refuge to vessels which are prevented by these storms from entering Portland harbor or other places on this part of the coast.

The breakwater, to be permanent, should, in my opinion, be built of stone The proposed location with its longitudinal section and a general cross section

are exhibited on the accompanying map.

As the capping of this work would greatly increase its cost, it is not estimated for; but should it hereafter be found necessary to resist the action of the sea, it can be added. It is probable, however, that it will not be necessary, as the proposed structure will prevent the shifting of the bar and increase its foundation, and will thereby acquire additional strength.

There is an abundance of rubble stone to be found on Cape Elizabeth, which

will reduce the probable cost of the work.

Of which \$50,000 can be profitably expended upon it during the next fiscal year.

The following information is furnished in compliance with the requirements of the act providing for this survey, viz:

The proposed work is located in the collection district of Portland and Falmouth; it is distant from Portland, the port of entry, by water about 12 miles, and eight by land; and from Saco, another port of entry, about 13 miles by water and about the same distance by land.

Cape Elizabeth light-houses lie about two miles to the northeast, and Wood

Island light-house (opposite the mouth of Saco river) about eight miles to the south of this locality. The nearest forts are those in Portland harbor, viz: Forts Gorges, Preble, and Scammel.

At Portland, the nearest port of entry, the amount of revenue collected for

the fiscal year ending June 30, 1867, was-

The amount of duties assessed at this port on merchandise entered during the same period, which includes, in addition to the foregoing, duties on goods in transitu for the British provinces, and on those entered for other ports in bond, was \$7,682 650.

The amount of commerce and navigation that would be benefited by the completion of this work may be inferred from the following information furnished by Hon. J. Washburn, jr., United States collector at Portland, Maine: "The number of foreign entrances and clearances at Portland, Maine, for the year ending June 30, 1867, was 1,171; the number of domestic entries and clearances during the same period was 1,430." But this statement exhibits but a part of the commerce that would be benefited by the work referred to in Portland harbor and at Richmond's island.

The number of vessels that put into Portland harbor and that at Richmond's island during the year, which do not make entry at Portland, is very large.

GEORGE THOM,

Lieut. Col. Corps of Engineers, Bvt. Brig. General.

R 3.

Report on the extension of the breakwater in Portland harbor, Maine, by Brevet Brigadier General George Thom, lieutenant colonel corps of engineers, for the fiscal year ending June 30, 1867.

United States Enginber Office, Portland, Maine, September 10, 1867.

By an act of Congress, approved June 23, 1866, an appropriation was made for this work amounting to \$105,111 05. This is the amount that was esti-

mated for the entire completion of the work.

In November, 1866, I relieved Brevet Brigadier General B. S. Alexander, lieutenant colonel of engineers, from the charge of this work. He had already caused soundings to be made for a distance of 180 feet from the outer end of the breakwater, and had submitted a plan for its extension. As this involved a departure from the original plan, I was instructed by the department, (see Appendix A.) before determining upon the plan of this work, to make a full series of current and tidal observations at such points in the harbor as "might be found useful aids in arriving at proper conclusions."

After repeated experiments it was found impracticable to make the current observations called for during the winter, owing to the winds, storms, and floating ice. The tidal observations, however, were continued day and night for two

or three months, and satisfactory results obtained.

A board of engineers appointed by the department order of June 6, 1867, "to investigate the question of the proper direction and length of the extension of the breakwater," approved the plan recommended by General Alexander.

Before further action thereon by the department, the current and tidal observations called for are to be furnished.* Having been instructed under date of June 27, 1867, to have the breakwater repaired, and the unfinished portion southwest of the light-house and the shore end completed, I advertised for pro-

posals for the same.

The amount already appropriated for this work is deemed sufficient for its completion, and it is probable that it will be completed during the next fiscal year. This work is situated in the collection district of l'ortland and Falmout. It is in Portland harbor, has a light-house built on its outer extremity, and is in the immediate vicinity of Fort Gorges, Fort Preble, and Fort Scannel. The amount of revenue collected for the fiscal year ending June 30, 1867, was, as stated by the United States collector, as follows, viz:

Duties on importations	\$986 , 318 17 47 , 574 37
Total	1, 033, 892 54

The amount of duties assessed at this port on merchandise entered during the same period, which includes, in addition to the foregoing, duties on goods in transit to the British provinces and on those entered for other ports in bond, was \$7.862.650.

The amount of commerce and navigation that would be benefited by the completion of this work, as also stated by the collector, is as follows, viz: "The number of foreign entrances and clearances at this port for the year ending June 30, 1867, was 1,171; the number of domestic entries and clearances during the same period was 1,430. But this statement exhibits but a part of the commerce that would be benefited by the work referred to. The number of vessels that put into this harbor and that at Richmond's island during the year, which do not make entry at Portland, is very large."

The proposed completion of this work may be regarded as permanent, and

after completion will not require further expenditure.

GEORGE THOM,

Lieut. Colonel of Engineers, Brevet Brig. Gen. U. S. A.

A.

Engineer Department, Washington, November 5, 1866.

COLONEL: Brevet Brigadier General Alexander, in his report upon the break-water at Portland, expresses an opinion adverse to its "extension further than is necessary to give it a proper termination," and this would involve a departure from the original plan. It is suggested that before determining upon it, a full series of current and tidal observations might be found useful aids in arriving at proper conclusions. Current observations might be made at the following positions, viz:

1st. Middle of main channel, between the end of the breakwater and Fort Gorges.

2d. Mid-channel, off Pomroy's Rock.

3d. Off Fort Gorges, in the middle of the channel from Casco bay that runs west of Fort Gorges.

† Contracts have been made for this work, and operations were commenced on the 1st of

September, 1867.

^{*} In July and August, 1867, a very minute and satisfactory series of current and tidal observations were made by Assistant Engineer Mr. A. Grant Childs, it having been the earliest opportunity for so doing.

4th. On the Middle Ground.

5th. In mid-channel between the Middle Ground and the end of the break-water.

6th. In mid-channel of inner harbor.

Tidal observations might be taken at the same time with the current observations, to extend through at least one lunation, as follows, viz:

1st. At the end of the breakwater.

2d. At Fort Gorges.

3d. At the Atlantic and St. Lawrence railroad bridge.

4th. At Portland bridge.

Very respectfully, your obedient servant,

A. A. HUMPHREYS.

Chief of Engineers, Brig. and Brevet Maj. Gen.

Brevet Colonel George Thom, U. S. A., Lieut. Col. of Engineers, Portland, Me.

R 4.

United States Engineer Office, Portland, Me., June 13, 1867.

GENERAL: I have the honor to transmit herewith the report of the board of engineers convened under Engineer Order No. 43, dated June 6, 1867, to investigate the question of a proper direction and length of the extension of the breakwater.

A copy of the drawings and plans recommended for adoption will also be for-

warded as soon as completed.

These drawings will be duplicates of those accompanying the report of Brevet Brigadier General B. S. Alexander, lieutenant colonel corps of engineers, dated September 25, 1866, to which, for the present, the department is respectfully referred.

I am, very respectfully, your obedient servant,

GEO. THOM,

Lieut. Col. Corps of Engineers, Brevet Brig. Gen. U. S. A.

Major General A. A. HUMPHREYS,

Chief of Engineers U.S.A., Washington, D.C.

In pursuance of Engineer Order No. 43, dated June 6, 1867, the board of engineers, consisting of Brevet Brigadier General Hartman Bache, colonel of engineers; Brevet Brigadier General George Thom, lieutenant colonel of engineers; Brevet Colonel Thomas L. Casey, major of engineers, assembled at Portland, Maine, on Wednesday, June 12, 1867, to investigate the question of the proper direction and length of the extension of the breakwater.

The board made a personal examination of the breakwater; after which it

adjourned to meet at 10 a.m., June 13, 1867.

The board, pursuant to adjournment, met at 10 a.m., June 13, 1867; present, all its members.

All the information relating to this work in the possession of Brevet Brigadier General George Thom, lieutenant colonel of engineers, was laid by him before the board, including the following papers, viz:

Three letters from Major Z. B. Tower, corps of engineers, to the engineer

Three letters from Major Z. B. Tower, corps of engineers, to the engineer department, dated November 30, 1852, January 17, and May 16, 1853; report of Brevet Brigadier General B. S. Alexander, lieutenant colonel of engineers, to

the engineer department, dated September 25, 1866, with accompanying drawings; United States Coast Survey map of Portland harbor, 1862; tidal and current observations in Portland harbor, made in the winter of 1866, under the direction of Brevet Brigadier General George Thom.

After a full discussion of the various plans presented, it was

Resolved, That this board approves the plan proposed by Brevet Brigadier General B. S. Alexander, lieutenant colonel of engineers, "of the proper direction and length of extension of the breakwater" in Portland harbor, as set forth in his report of September 25, 1866, and accompanying drawings, and does recommend that the work be completed in accordance with the plan.

The board deems it proper to call the attention of the department to the fact that among the papers laid before it there was nothing showing the original plan of the work, or when and under whose direction it was built, nor the reports and estimates of the late Colonel J. D. Graham, corps of engineers, upon which

the present appropriation was based.

The board having completed the duties for which it was convened, adjourned

sine dic.

HARTMAN BACHE,
Colonel of Engineers and Brevet Brig. Gen.
GEO. THOM,

Lieut. Col. of Engineers and Brevet Brig. Gen.
THOMAS L. CASEY,

Major of Engineers and Brevet Col. U.S. A.

R 5.

Report on the examination or survey of Kennebec river above Gardiner, Maine for the year ending June 30, 1867, by Brevet Brigadier General Thomlieutenant colonel of engineers.

UNITED STATES ENGINEER'S OFFICE, PORTLAND, MAINE, September 10, 1867.

On my being placed in charge of this work in November, 1866, I commenced a careful survey of Kennebec river, extending from Shepard's Point, at Hallowell, up to Augusta, Maine, with a view to determine the obstructions to navigation between those points, for the improvement of which Congress, by an act approved June 23, 1866, made an appropriation of twenty thousand dollars.

The result of this survey, together with the maps and estimates for the improvement of the river between those points, were submitted to the department in my special reports of the same, dated January 21 and 29, 1867. By an act approved March 2, 1867, Congress made an additional appropriation, based on my estimate sufficient for the entire completion of the improvements recommended by me to be made between Shepard's Point and Augusta. In an examination of the river since made between Shepard's Point and Gardiner, I ascertained that there are two shoals, known as "Hinkley's shoal and Brown's Island shoal."

Hinkley's shoal is about one-half a mile below Shepard's Point. It extends across the river and has about five and a half feet of water on it at low water, and is about 300 feet wide.

Brown's Island shoal is about one and a quarter mile below Shepard's Point and extends from Brown's island across to Brown's Island wharf, on the left bank of the river. It is about 150 feet wide, and has about six feet of water on it at

low water. In addition to these shoals, there is a rock in the river abreast Grant's ship yard, which should be removed.

Grant's snip yard, which should be removed.			
To excavate a channel through these shoals of the same width and			
depth as that now being excavated through the shoals above			
would require about 5,000 cubic yards of dredging, which at 45			
cents per cubic yard would cost		250	00
To remove rock in the river, (say)		150	
	2, 7	700	00
Adding 10 per cent. for contingencies	2	270	00
	2, 9	970	00

Say \$3,000.

A more careful survey of this river soon to be made between Gardiner and Shepard's Point may modify this estimate a little, but probably will not increase it. The amount above named (to wit, \$3,000) will probably be all that is required for the entire completion of the improvement of the river between those points, all of which can be profitably expended upon the work during the next fiscal year.

The following information is furnished in compliance with the act of Congress

providing for this survey, viz:

The proposed improvements are located in the collection district of Bath, Maine.

Bath is the only port of entry in this district; it is situated on Kennebee river, about twenty-five miles below Gardiner, Maine.

The United States arsenal is located at Augusta, on the left bank of the Kennebec, about six miles above Gardiner, and Fort Popham is at the mouth of the Kennebec, about twelve miles below Bath.

Seguin and Pond Island light-houses are near the mouth of this river.

The amount of revenue collected at this port of entry (Bath) during the fiscal

year ending June 30, 1867, was \$40,820 56.

The amount of commerce and navigation that would be benefited by the completion of the proposed river improvements, as stated by the United States collector of customs at Bath, is copied from my special report on this subject, dated January 21, 1867, as follows: "The whole tonnage of this district is something over 80,000 tons, all of which would be more or less benefited by the improvement of the navigation of the Kennebec, the works near the mouth of the river, such as Fort Popham. Seguin and Pond Island lights are designed for the benefit of the whole district."

The improvement proposed between Augusta and Shepard's Point, in Hallowell, would more particularly benefit vessels bound to and leaving those ports, and the completion of the work would doubtless cause great increase of business

at those points.

One steamboat has plied between Hallowell and Boston, and one between Gardiner and Boston, during the past season, carrying many passengers and much freight. These steamers would extend their trips to Augusta if the contemplated improvement is made. The arrivals at Hallowell and Augusta in 1865 were 42 coastwise and 5 foreign vessels. At Gardiner, four miles below Shepard's Point, the arrivals were 223 coastwise and 10 foreign vessels.

GEORGE THOM,

Lieut. Col. of Engineers, Brevet Brig. Gen. U. S. A.

R 6.

Report on the improvement of Kennebec river, in the State of Maine, between Shepard's Point and Augusta, by Brevet Brigadier General George Thom, lieutenant colonel corps of engineers, for the fiveal year ending June 30, 1867.

UNITED STATES ENGINERR OFFICE, Portland, Maine, September 10, 1867.

This work consists in straightening and deepening the channel of Kenneber river, through the several shoals obstructing its navigation, between Shepard's Point and Augusta, Maine.

49,500

Say \$50,000.

tal 50,000

which is estimated to be sufficient for completing the proposed improvement. The nature of the bottom is such that it is not probable that any material change will occur in the channel, when once properly dredged.

Under authority of the department, dated February 18 and March 20, 1867, I advertised three separate times for proposals for dredging the proposed channel through the shoals at Hallowell, before a reasonable and satisfactory bid was received—the bids ranging from forty cents to one dollar thirty-five cents per cubic yard, as will be seen on referring to the abstract of same hereto appended marked No. 1. A copy of the last notice, inviting proposals, is hereto appended marked No. 3.

Mr. Augustus R. Wright, of Geneva, New York, having been the lowest bidder, a contract was made, May 23, 1867, with him, to perform the work at forty cents per cubic yard, and to commence operations on or before the 1st of August, 1867. The contract requires Mr. Wright to complete his work on or before the 1st of August, 1868.

The following information, called for by the act of Congress making appropriation for this work, is furnished, viz:

The improvements are located in the collection district of Bath, Maine.

Bath. Maine, is the only port of entry in this collection district, and is situated on the Kennebec river, about thirty miles below Hallowell.

The United States areenal is located at Augusta, on the left bank of the Kennebec; and Fort Popham is about twelve miles below Bath, at the mouth of the river. Seguin and Pond Island light houses are near the mouth of this river.

The amount of revenue collected at this port of entry (Bath) during the fiscal year ending June 30, 1867, was \$40,820 56.

The amount of commerce and navigation that would be benefited by the completion of the proposed river improvements, as stated by the United States collector of customs at Bath, is copied from my special report on this subject, dated January 21, 1867, as follows: "The whole tonnage of this district is something over 80,000 tons, all of which would be more or less benefited by the improvement of the navigation of the Kennebec. The works near the mouth of the river, such as Fort Popham, Seguin, and Pond Island lights, are designed for the benefit of the whole district. The improvement proposed between Augusta and Shepard's Point, in Hallowell, would more particularly benefit vessels bound to and leaving those ports, and the completion of the work would doubtless cause great increase of business at those points."

One steamboat has plied between Hallowell and Boston, and one between Gardiner and Boston, during the past season, carrying many passengers and much freight. These steamers would extend their trips to Augusta, if the contemplated improvement is made. The arrivals at Hallowell and Augusta, in 1865, were forty-two coastwise and five foreign vessels. At Gardiner, four miles below Shepard's Point, the arrivals were 223 coastwise and ten foreign vessels.

The following described papers are appended hereto, to wit:

- 1. Abstract of proposals.
- 2. Abstract of contracts.
- 3. Copy of notice inviting proposals for dredging.

GEORGE THOM, Lieut. Col. of Engineers, Brevet Brig. Gen. U. S. A.

No. 1.—Abstract of proposals received for the improvement of Kennebec river, Maine.

Nature of work.	Names of bidders.	Amount bid for.	Price.
FIRST SERIES OF BIDS.		Whole work.	
Dredging a new channel	Bernard Daly, Port-	45,000 cubic yards,	\$1 35 per cubic yard.
through Shepard's Point shoal and Hallowell shoal.	land, Maine. George P. Wescott, Portland, Maine.	(more or less.) 45,000 cubic yards, (more or less.)	1 19 per cubic yard.
SECOND SERIES OF BIDS.			
	Chas. Woolley, Boston, Mass.	45,000 cubic yards, (more or less.)	1 00 per cubic yard.
THIRD SERIES OF BIDS.			
	A. B. Cooley, & Co., Philadelphia, Pa.	45,000 cubic yards, (more or less.)	1 10 per cubic yard
	Chas. Woolley, Bos- ton, Mass.	45,000 cubic yards, (more or less)	90 per cubic yard
	Jonas H. Perley,	45,000 cubic yards,	88 per cubic yard
	Portland, Maine. Thomas J. Strong. Sandy Hill, N. Y.	(more or less.) 45,000 cubic yards, (more or less.)	65 per cubic yard
	A. R. Wright, Geneva, N. Y.	45,000 cubic yards, (more or less.)	40 per cubic yard.

GEORGE THOM, Lieutenant Colonel of Engineers, Brevet Brigadier General.

No. 2.—Abstract of contracts made for the improvement of Kennebec river,

Marne.

Nature of work.	Name of contractor.	Quantity.	Price.
Dredging a new channel through Shepard's Point shoal and Hallowell shoal.	Augustus Wright, Geneva, N. Y.	Whole work, 45,000 cubic yards, (more or less.)	\$0 40 per cubic yard.

GEORGE THOM, Lieut. Col. Corps of Engineers, Brevet Brig. Gen. U. S. Army.

No. 3.

Proposals for dredging a new channel through Shepard's Point shoal and Hallowell shoal in the Kennebec river, at Hallowell, Maine.

U. S. ENGINBER OFFICE,

Portland, Maine, May 6, 1867.

Proposals will again be received for this work (those heretofore received being unreasonably high) until 2 o'clock p. m. on Saturday, the 18th instant.

The object of the proposed improvement is to obtain a clear channel, not less than seven feet deep at low water and seventy-five feet wide on the bottom, with sides having a slope of two feet to one foot rise.

The channel will first be excavated through Shepard's Point shoal for the distance of about 450 yards, requiring 20,000 cubic yards of excavation, more or less, and afterwards through Hallowell shoal, for a distance of 575 yards, requiring 25,000 cubic yards of excavation, more or less. This amount of excavation may be increased or diminished, as the engineer in charge may direct after further examination of the river.

The material taken from the shoal is to be deposited in the river in such manner as may be required by the engineer in charge and in such localities as may be designated by him, not exceeding 650 yards in distance from Shepard's Point above and below it.

In making proposals, bidders must state the price per cubic yard actually excavated, to be measured in the scows, with the understanding that the price stated is to include the depositing of the material taken out, in such localities as may be designated within the limits above named.

The work must be commenced as soon as practicable after the approval of the contract, and be completed not later than the 1st of August, 1868, with the understanding that not less than 25,000 cubic yards must be dredged on or before the 20th of November next.

Each proposal must be accompanied by a written guarantee, signed by two responsible persons, in the required form, that the bidder will, when called on, if his proposal be accepted, enter into a contract and bond, with good and sufficient security, for the true and faithful performance of his contract:

The contract will be awarded to the lowest responsible bidder, and be subject

to the approval of the Secretary of War.

The undersigned, however, reserves the right to exclude the bids of any persons who there is reason to believe will not faithfully and promptly perform the contract; also, any informal bids, as well as those that are above a reasonable price for the work; and no member of Congress, officer or agent of the govern-

ment, nor any person employed in the public service, shall be admitted to any share in the contract or any benefit which may arise therefrom.

Payment will be made monthly; twenty per cent. to be reserved therefrom until the whole work is finished, and to be forfeited in the event of the non-ful-

filment of the contract in the time and manner as therein required.

Persons desiring to make proposals will please call on the undersigned at his effice, in Morton block, on Congress street, for forms of same and for more definite information, if desired; and on transmitting them, will indorse thereon, "Proposals for improvement of Kennebec river."

GEORGE THOM, Brevet Brigadier General U. S. Army.

R 7.

U. S. ENGINEER OFFICE, Portland, Maine, January 21, 1867.

GENERAL: I have the honor to transmit herewith a map, showing the position and extent of the shoals in Kennebec river, near Shepard's Point, at Hallowell, Maine, with a plan and sections of the excavation for the proposed new channel through the shoals; showing, also, the direction and extent of the proposed dam for closing the present channel near Shepard's Point after the new channel shall have been opened, surveyed and drawn under my direction, in obedience to your instructions of the 3d of November last.

The soundings on this map are referred to the *lowest* water indicated by the lower tide-gauge during the time of the survey, which was on the 14th and 15th of November last. At the upper tide-gauge the water stood one and a half $(1\frac{1}{2})$ feet higher at low water than it did at the lower tide-gauge; whilst, at high

water, it stood at the same height as it did at the lower gauge.

The proposed excavation, which is estimated from the lowest water indicated by the lower tide-gauge, will, therefore, in all cases be as great, and generally will exceed the amount actually required, when the water is not at a lower stage than it was during the survey. In the lowest stages of the river, however, it is probable that the water reaches a level from one to two feet lewer than is indicated on the map, a fact only to be ascertained by actual observation during the "dry season." In which case, the estimate will have to be correspondingly increased.

The bed of the river is composed of sand, gravel, and pebbles. Borings were made in several places to a depth of a few feet below low water, near the line of the proposed channel, without finding any ledge or boulders of any considerable size.

The present channel of the river lies near its right (or west) bank, and makes a very abrupt bend around Shepard's Point, the maximum ebb current, at the

time of the survey, being about three miles an hour.

Brevet Brigadier General Alexander, (my predecessor in charge of this work,) in his report, dated the 20th of September last, proposed a plan for improving this channel, which has received the approval of the department, to wit, "by dredging a new channel through Shepard's Point shoal; the dredging to be made with a view of obtaining a clear channel, seven feet deep at low water, with a width of sixty feet on the bottom, which would require a width at the top, in order to obtain the proper slopes, averaging about ninety feet."

The channel which the accompanying map shows to be most practicable to be excavated is, in my opinion, that indicated by the two parallel lines extending above and below Shepard's Point, from A to S, as it follows the general course of the river, and requires less excavation than any other location equally

direct. It is proposed that the channel lying between the points marked M and S, through the shoal next below Shepard's Point, should first be opened, and the excavated material be deposited inside, or west of the line marked U V, leaving the present channel between Shepard's Point and U open, until the completion of the new channel between M and S; also that the material, afterwards excavated between the points A and L, be deposited, a portion of it immediately below Shepard's Point, inside or west of the line T U, so as to fill up the present channel and divert it into the new one, and at the same time give a uniform width to the river below Shepard's Point, and the remaining portion to be deposited above Shepard's Point, between the foot of the small island and the point W on the left bank below it.

The amount of excavation estimated for a channel, located as above described, and having a depth of seven feet below the *lowest* water observed during the survey, with a width of sixty feet on the bottom, and ninety feet on top, is 20,635 cubic yards, which, at 45 cents per cubic yard, would cost \$9,285 75; and, adding ten per cent. for contingencies, \$928 57, making the total

\$10.214 32.

This estimate includes the removal of the material, and depositing it in the required localities. But a channel of the same dimensions, at a stage of the water two teet lower than that indicated on the map, (and actual observations might show, though it is not probable that in the dryest season it reaches a still lower level,) would require an amount of excavation estimated at 35,360 cubic yards, which, at 45 cents per cubic yard, would cost \$15.912; and, adding ten per cent.-for contingencies, \$1,591 20, would make total \$17,503 20.

Colonel S. H. Long, late of the corps of topographical engineers, in his report dated. September 20, 1837, recommended the excavation of a new channel through the shoals as above, but to be eight feet in depth at low water, with a width of one hundred feet on top. To make a channel of this depth, with a width of seventy-five feet on the bottom and 107 feet on top, which I too would recommend, would cost about fifty four (54) per cent. more than for the above

described dimensions.

About twenty years ago, ten years subsequent to the date of Colonel Long's report, the citizens of Augusta and thereabouts undertook to dredge out a channel above Shepard's Point, and they improved it in some localities. The accompanying map shows where some of the dredging was made between the points marked A and L, in the proposed channel through Hallowell shoals, immediately above Shepard's Point. The unfinished condition of the work, and the fact that the excavated material was not removed far enough from the channel, has caused it to be more or less filled up since that period.

In addition to the survey of the river at Hallowell, I have also had it surveyed above Hallowell as far as the bridge at Augusta, so as to include "Wyman's," "Britt's," and "Gage's" shoals, the maps of which, with estimates for the necessary excavation and cost of same, will soon be completed and submitted, and "the amount required for the entire completion" of the improve-

ment between Shepard's Point and Augusta will then be reported.

The total amount already appropriated, therefore, (to wit, \$20,000,) can be

profitably expended upon the work during the next fiscal year.

In contracting for this work, my opinion, which differs from that of my predecessor, is, that it should be let out and paid for by the cubic yard, as the amount of excavation will depend upon the level of low water to be assumed as a plane of reference, which can only be ascertained by observations on the tides made in the lowest stage of the river, during the "dry season." These observations can be made while the work is in progress.

I have, therefore, before advertising for proposals, to respectfully ask to be instructed by the department which course I am to adopt in this matter—to let out the work by the job, as recommended by my predecessor, with an uncer-

tainty as to the amount of excavation to be done, or by the *cubic yard*, to be measured from time to time, as the work progresses, the final depth and dimensions of the channel to depend on further examination of the river.

The following additional information, called for by the bureau circular dated the 22d of September last, in compliance with the act of Congress making the

appropriation for this work, is furnished, viz:

1. The work is located in the collection district of Bath, Maine.

2. Bath, Maine, which is on the Kennebec river, about thirty miles below

Hallowell, is the only port of entry in this collection district.

The ports of delivery are Phipsburg (in which Fort Popham, at the mouth of the Kennebec river, is located) and Georgetown, situated between Bath and the mouth of the Kennebec; also, Bowdoinham, Richmoud, Gardiner, Pitiston, Hallowell, and Augusta, above Bath, on the Kennebec, Topsham, on the Androscoggin, and Brunswick, on the Androscoggin and New Meadow rivers.

The United States arsenal is located at Augusta, Maine, on the left bank of the Kennebec, and Fort Popham is about twelve miles below Bath, at the mouth

of the river.

Seguin and Pond island light-houses are near the mouth of this river.

3. The amount of revenue collected at this port of entry during the last fiscal year, as furnished by the collector, was \$37,208 95.

- 4 The amount of commerce and navigation that would be benefited by the completion of the proposed river improvements is stated by the collector as follows:
- "The whole tonnage of this district is something over 80,000 tons, all of which would be more or less benefited by the improvement of the navigation of the Kennebec.
- "The works near the mouth of the river, such as Fort Popham, Seguin, and Pond island lights, are designed for the benefit of the whole district. The proposed improvement between Augusta and Shephard's Point, in Hallowell, would more particularly benefit vessels bound to and leaving those ports, and the completion of the work would doubtless cause great increase of business at those points. One steamboat has plied between Hallowell and Boston, and one between Gardiner and Boston, during the past season, carrying many passengers and much freight. These steamers would extend their trips to Augusta, if the contemplated improvement is made.

"The arrivals at Hallowell and Augusta, in 1865, were forty-two coastwise and five foreign vessels; at Gardiner, four miles below Shephard's Point, the arrivals were two hundred and twenty-three coastwise and ten foreign vessels."

Appended hereto is a copy of a letter bearing further upon this subject, from Hon. S. Caldwell, mayor of Augusta, Maine.

GEO. THOM,

Lt. Col. Corps of Engr's, Bot. Col. U. S. A.

Brevet Major General A. A. HUMPHRBYS, Chief of Engineers, U. S. A., Washington, D. C.

> MAYOR'S OFFICE, Augusta, Me., December 28, 1866.

DEAR SIR: I have your favor, enclosing one from Colonel Thom, of the United States engineers, making inquiries in regard to the necessity and anticipated benefit of the improvement of the Kennebec river, between Shepard's Point and this city.

In reply 1 beg leave to state that for many years past all the interests of this vicinity depending upon cheap transportation have seriously felt the lack of

steamer and sail-vessel communication with the outer world.

Steamers of large burden, and sailing vessels of very considerable tonnage, come within two miles of our wharves, and could they ascend higher, their

numbers would be largely enhanced.

The business of this place is already, as you well know, very considerable, and, with the now certainly anticipated location of some very large manufactories upon our water-power, we can count upon a great increase of our business, thus rendering our lack of easy water communication more and more inconvenient.

The location of the United States arsenal at this place gives the government a more direct and immediate interest in the improvement than it would otherwise have. During the war, in the movement of ordnance stores to and from this place, the cost would have been greatly lessened, and the convenience to the government greatly promoted, could a sea-going steamer have come directly to the arsenal wharf.

If a depth of seven or eight feet at low water can be secured, I feel very sure that the commercial interests of this whole section of country would be greatly henefited.

Very respectfully, yours, &c.,

S. CALDWELL, Mayor, &c.

E. S. J. NEALLY, Esq., U. S. Collector, Bath.

R 8. .

United States Engineers' Office, Portland, Maine, January 29, 1867.

GENERAL: I have the honor to transmit herewith two maps, (marked 1 and 2) showing the position and extent of the shoals in the Kennebec river, above Hallowell, Maine, and the location and sections of the excavations, for a proposed new channel, the survey and drawings of which have just been completed, under my direction, to enable me to furnish an estimate of the amount that is required for the entire completion of the improvement of the river between Shepard's Point and the city of Augusta, Maine, as contemplated by the law approved June 23, 1866, making appropriations for this work.

Sheet No. 1 shows that portion of the river extending from the bridge at Augusta down, to include Gage's shoal, and sheet No. 2 shows the river as far down as the bridge at Hallowell, including Britt's and Wyman's shoals.

Sheet No. 3, which was transmitted to you with my report dated the 21st instant, shows the river and shoals from the bridge at Hallowell down to include

the shoal next below Shepard's Point.

The soundings shown on sheets Nos. 1 and 2, at Gage's and Britt's shoals, are referred to the lowest water observed at their respective tide-gauges in the month of December, whilst those soundings were being made, at which time the low water at Gage's and Britt's shoals was about two feet higher than the lowest water which was observed the month previous in the survey at Shepard's Point. indicated on sheet No. 3. It is probable, however, that in "dry seasons," when the river is at its lowest stage, the water falls about two feet still lower than was observed at Shepard's Point, or about four feet lower than is indicated by the soundings on sheets 1 and 2.

At Gage's shoal (the first below Augusta) the main channel of the river runs west of the shoal and near the right or west bank of the river. The shoalest water found in this channel, during its survey, was eight and one-half feet deep at low water, which would probably be reduced to four and one-half

feet at the lowest stages of the river in the "dry season."

At Britt's shoal the shoalest water found in the main or west channel during the survey was seven and three-tenths feet deep, which, in the lowest stages of the river, would probably be reduced to three and three-tenths feet; whilst in the eastern channel, near the left bank, the shoalest water found during its survey was six and three-tenths feet deep, which, in the lowest stages of the river, would probably he reduced to two and three-tenths feet. These reduced depths, both at Gage's and Britt's shoals, agree very closely with those indicated on the map accompanying Colonel Long's report of 1837. Tidal observations, made at the several shoals during the "dry season," could alone give the exact corrections for reducing the soundings on the maps to the lowest stage of the river.

It having been decided by the department to give to the channel through the Hallowell and Shepard's Point shoals a depth of seven feet of water at low water, I would recommend that the channel to be excavated through the shoals above Hallowell (to wit, Britt's and Gage's shoals) should be seventy-five feet wide at the bottom, with sides having slopes of two feet to one foot rise, and have a depth of six feet in the lowest stages of the river, so that when the river is in that stage vessels drawing from nine to ten feet of water may, at high tide,

ascend to Augusta.

The channel which I recommend to be opened through Gage's shoal is shown by the two parallel lines between the points A and G. This location coincides nearly with the present channel of the river. It is more direct, and it requires

less excavation than any other location would require

At Britt's shoal there are two channels that are to be considered, to wit: the western channel, which lies between the shoal and the right bank of the river, and the eastern channel, which runs near the eastern bank. The eastern channel is more direct, and the estimated cost of its excavation is a little less than that for the western channel; but the nature of the river bed may, judging from its rocky shore, present greater obstacles to its excavation than the western channel, (a fact only to be ascertained by further examinations,) so that, to be on the safe side, I have adopted the estimate for improving the western channel.

At Wyman's shoal, immediately above Hallowell bridge, no excavation is

necessary.

The bed of the river at Gage's and Britt's shoals is of the same character as at Shepard's Point shoals, being composed of sand, gravel, and pebbles, with occasional small boulders, and there does not appear to be any ledge or other obstacles to making the proposed excavations. This fact can easily be deter-

mined by borings, which I propose making.

The sections on sheets Nos. 1 and 2, showing the excavation required for the proposed channel through Gage's and Britt's shoals, give a depth of ten feet of water (at low tide) when the river is at the same stage as it was in December, at the time of the survey of those shoals; but a depth of eight feet only (at low tide) if reduced to the stage of the river as it was about the middle of November last, when the Hallowell and Shepard's Point shoals were being surveyed, and a depth of six feet only in the lowest stages of the river.

ESTIMATES.

I.—Gage's shoal.

For a channel having a depth of six feet at the lowest stages of the river, and seventy-five feet wide at the bottom:

8,870 cubic yards excavation, at 45 cents	\$ 3, 991	50
Adding ten per cent. for contingencies	399	15
· · · · · · · · · · · · · · · · · · ·		

II .- Britt's shoal, (western channel.)

For channel of same depth and dimensions as above:	
23,270 cubic yards of excavation, at 45 cents	\$10,471 50 1,047 15
Total	11, 518 65
III.—Britt's shoal, (eastern channel.)	
For channel of same depth and dimensions as above:	
17,180 cubic yards of excavation, at 45 cents	
Total	8, 504 10
It is probable that a further examination of the river in its would somewhat modify but not increase the above estimates. For the entire completion of the improvement of the river betwee Point and the city of Augusta, Maine, as contemplated by the la June 23, 1866, making an appropriation for this work, I have the mit the following estimates, viz: I. Hallowell and Shepard's Point shoals.—For a channel seven feet deep and sixty feet wide, as approved by the department, (see my report of 21st January, 1867) 2. Britt's shoal, (western channel.)—For a channel six feet deep and seventy-five feet wide 3. Gage's shoal.—For a channel six feet deep and seventy-five feet wide	sen Shepard's aw approved honor to sub-
Total	33, 412 50 20, 000 00
Additional amount required for the entire completion of the work,	13, 412 50
say	
I am, very respectfully, your obedient servant,	\ \ \ \ \ \ \ \ \ \ \ \
GEO. THO Lieut. Col. Corps of Engineers, Brevet Colon.	
Brevet Maj. Gen. A. A. Humphrbys,	CD U. D. 481
Diever maj. Gen. A. A. Homentaro,	

R 9.

Chief of Engineers U.S.A., Washington, D. C.

Report of the survey of the "gut" opposite the city of Bath, Maine, by Bretel Brigadier General George Thom, lieutenant colonel curps of engineers.

> United States Engineers' Office, Portland, Maine, September 10, 1867.

The act of Congress approved March 2, 1867, providing for the examination or survey of this locality, does not specify the object for which it is required. This gut was surveyed by the United States Coast Survey in 1865, rendering

any further survey of it at this time unnecessary. Accompanying this is a copy of the chart of the same, furnished by the Superintendent of the United States Coast Survey. In a personal examination of the gut or Upper Hell Gate, as it is generally called, I became satisfied that the object of the "examination or survey "called for was the improvement of its navigation.

Owing to the contraction of the channel of Back river at the Upper Hell Gate, which is about forty yards wide at low water, (caused by a ledge projecting out from the south shore.) the tidal currents run through this gate with such violence as to render its navigation exceedingly difficult and dangerous at any other time than at high and low water, except for steamers. The difficulties are still further increased by a large rock known as Boiler rock, which lies in midchannel, about seventy-five yards below the gate, compelling vessels to make an abrupt change of course so as to page south of the small island below.

This rock I carefully examined by the aid of a submatine party. from three to four fathoms of water, at low water, its upper or highest point being only about three feet below the surface at mean low water. It is an immense boulder, about thirty feet long and fifteen feet wide at the bottom, and twenty feet long by ten feet wide on its top. The tide here rises and falls from six to seven feet; so that at high water the rock has about ten feet of water

For the improvement of the navigation of this place I would respectfully recommend-

1. That the Boiler rock be removed to a depth of twelve feet at mean low water, requiring about 70 cubic yards of blasting,		
which, at \$50 per cubic yard, would cost	\$3,500	00
2. That the point of ledge contracting the channel at the Upper Hell Gate be blasted off so as to enlarge the water way, requiring		
about 1,500 cubic yards, at \$4 per cubic yard	6, 000	00
3. For deepening the bar about midway between the Upper Hell Gate and Arrowsic bridge, so as to afford a channel of 100 feet wide and 10 feet deep at mean low water, requiring 11,000 cubic	·	
yards of dredging, which, at 50 cents per cubic yard, would cost	5, 500	00
	15, 000	00
Add 10 per cent. for contingencies	1,500	00
•	16, 500	00
		==

This amount, it is estimated, is sufficient for the entire and permanent completion of the proposed work; and it could all be profitably expended upon it during the next fiscal year. The following information is furnished, in compliance with the requirements of the act of Congress providing for this survey, viz:

The locality of the proposed improvement is in the collection district of Bath, Maine. Bath is the only port of entry in this collection district, and is distant from the Upper Hell Gate three miles. The nearest light-houses are Seguin and Pond Island light-houses, near the mouth of Kennebec river; and the nearest fort is Fort Popham, at the mouth of the Kennebec river, distant about fifteen miles.

The amount of revenue collected at the port of Bath during the fiscal year

ending June 30, 1867, was \$40,820 56.

The amount of commerce and navigation that would be benefited by the proposed improvement is shown by the letters hereto appended, marked A and B. GEORGE THOM.

Lieut. Col. of Engineers, Brevet Brig. Gen. U. S. A.

A.

Custom-house, Bath, Maine, Collector's Office, July 27, 1867.

DBAR SIR: I am at length able to answer the second interrogatory contained in your letter of the 15th instant, in relation to the proposed improvement of the gut opposite the city of Bath, Maine, viz: "What amount of commerce and navigation would be benefited by the proposed improvement?"

The answer is given in the statement transmitted herewith, prepared by Messrs. Sampson & Riggs, merchants, who reside at Georgetown, a short distance below the Lower Gate, and are largely engaged in fitting out and supplying fishing and other vessels and in general trade. Their means of knowing about what they affirm are ample, and their estimates are worthy of confidence.

This statement has been procured by Wm. H. McLellan, esq., of this city, the principal owner and agent of the steamer Spray, to whom I am much indebted for his endeavois to have it made as accurate as possible.

I am, very respectfully, your obedient servant,

E. S. J. NEALLEY, Collector.

SAMPSON & RIGGS.

General GEO. THOM,

United States Engineer's Office, Portland, Maine.

В.

Gut passage-way from Kennebec river to Sheepscot bay open nine months in the year; lower or Sheepscot bay end never closed.

Number of row and sail-boats passing per day from ten to one hundred; number of persons on board from three to twenty; value of boats from ten to five hundred dollars; value of property transported each trip fifty to four hundred dollars. Number of scows, or gondolas, average five per day; number of hands on board from three to four; value of property transported each trip from one hundred to fifteen hundred dollars. Number of smacks, from the size of sail-boat up to twenty-five tons, from ten to fifteen per day; value of craft from four hundred to two thousand dollars; number of hands to each from four to eight; value of property transported each trip from six hundred to thirtyfive hundred dollars. Number of sailing and fishing vessels from twenty-five to eighty tons, four trips per day; value from one thousand to eight thousand dollars; value of cargo, fittings, and property on board each from eight hundred to three thousand dollars; number of hands to each vessel from four to twelve. Number of vessels from eighty to three hundred tons, one trip per day; value of vessels from eight to sixteen thousand dollars; value of cargo, nothing. mostly all light; number of hands on board from five to sixteen. One steamer. daily, (Sundays excepted.) up and back, valued at seven thousand dollars; number of hands on board three; average number of passengers twenty; value of freight each trip from one hundred to one thousand dollars. Some tug-boats, on the high-water rafts and tows of different kinds; logs in rafts to supply thirteen up and down saws in mills, say to the amount of forty thousand feet of lumber per month.

This short passage-way by tidal water from the Sheepscot bay to the Kennebec river, directly opposite the city of Bath, the whole length being from eight to nine miles, is, at the present time, and always has been, much used, though quite intricate from its many sunken rocks and quick runs. (Frontier Mission, Bartlet, chapter seventh, page 129; Ancient Dominions of Maine, pages 61, 62, and 63.)

By this passage we have access to some of the best tide-mill privileges in the United States. (Shattuck's Memorial, page 146.)

R 10.

Report on the examination of Union river, Maine, for the year ending June 30, 1867, by Brevet Brigadier General George Thom, lieutenant colonel corps of engineers.

U. S. ENGINEER OFFICE, Portland, Maine, September 10, 1867.

An act of Congress, approved March 2, 1867, provides for the examination or survey of Union river, Maine, but the object of the survey is not stated in the law. On a careful examination of the river, however, I became satisfied that it was for the improvement of the navigation of the river from its mouth up to the head of navigation, at Ellsworth, a distance of about four and a half miles.

The channel of this river is in some places very much obstructed by "slabs," "edgings," and "sawdust," which have come down from the numerous sawmills just above Ellsworth, and have become so completely interwoven as to have almost entirely closed the channel at low water near Ellsworth. These obstructions, together with numerous sunken rocks in the channel, and several rocky points projecting far out from the bank unmarked and unseen, except at low water, render the channel very intricate, and its navigation very difficult at all stages of the water.

Descending from the "upper landing" at Ellsworth, we find the river entirely filled at mean low water with "slabs," "edgings," and "sawdust," from three to four feet in depth, down to the "Sinker's wharf," one-third of a mile below "Nourse's Point;" nearly opposite to "Sinker's wharf" is a ledge covered with large boulders, extending from the right shore nearly half way across the river. It is entirely covered at high water, the rise and fall of the tide ranging from ten to fourteen feet. A stone beacon is here much wanted on its outer extremity at low water. The "Narrows," so called, commence about one-half mile below Ellsworth, (upper landing,) and extend for a distance of about half a mile downwards.

In the channel above the "Narrows" there are two or three boulders, each weighing from two to four tons, which can be seen at low water; these should be removed.

In the "Narrows" there are three boulders, and two more in mid-channel at the lower end, each weighing from one to four tons, all lying in two or three feet of water at low water; these should be removed.

Hall's Point (1½ mile below Ellsworth) is a ledge making out from the right shore and extending about one-third the way across the river. This ledge is bare at its outer extremity at mean low water, to mark which a beacon is necessary.

About 300 feet below Hall's Point a small boulder lies in the river, in about three feet of water at low water, and should be removed.

Sline Rocks are about two and a half miles below Ellsworth; they consist of two boulders, each weighing about two tons, lying in about three feet of water at low water; these rocks are very dangerous, and should be removed.

Fish Hole Point is about three miles below Ellsworth (upper) landing, and is a bold, rocky point, projecting about half way (say 300 feet) across the river, from its left bank, and then drops off suddenly into deep water; a beacon is much

needed to mark its outer extremity at low water.

Fullerton Point, three and a half miles below Ellsworth (upper) landing, is a bold ledge making out from the left shore, and extending one-third the way across the river. Its outer extremity is about four or five feet above mean low water. This is a very important point to be attended to, as vessels are liable to strike on it at high stages of water; a beacon should be placed on its outer extremity.

Horton Rocks, opposite to Fullerton Point, are also bare two or three feet at

low water, and should have a beacon to mark their outer extremity.

The lower bar, at the mouth of the river, about four and a half miles below Ellsworth, has about three feet of water on it at mean low water. This has been, it is said, somewhat increased of late years, by the accumulation of sawdust, which comes from the saw-mills above and near Ellsworth. Two boulders, bare at low water, lie in the channel, on this bar, and should be removed. At mean low water there are, except on the lower bar, about ten feet of water in the channel of the river from its mouth all the way up to near the foot of the Narrows; from the Narrows up to Ellsworth (upper) landing not more than four feet can be obtained.

The importance of this river as an outlet to the immense lumber trade and agricultural products of this section of the country is so great that the improvement of its navigation is most urgently recommended.

With a view to obtain a clear and unobstructed channel of not less than three feet in depth at mean low water, all the way up to the upper landing at Ellsworth, the following expenditures will be necessary, viz.:

or, the residence of the country, view,		
1st. Removing 30,000 cubic yards of slabs, edgings, and sawdust, so as to give a channel of 150 feet wide from Sinker's wharf up to		••
Ellsworth upper landing, at eighty cents	\$24,000	00
cost of \$250	3, 250	00
3d. For building five granite-cut stone beacons on the extremities of the ledges above named, the beacons to be eighty feet square and to rise about six feet above ordinary high-water mark, each containing about fifty cubic yards masonry, making in all 250		
cubic yards, at \$30 per cubic yard	7, 500 (00
Foundation for five beacons, at \$300	1,500 (
Total	36, 250)()
Adding ten per cent. for contingencies	3, 625)0
	39, 875)0

Say \$40,000.

This amount, it is believed, would be sufficient for the entire completion of this work, and for its permanent completion, if the State laws which have recently been enacted for the protection of this river from further injury by the deposit of slabs, edgings, and sawdust, be properly enforced. As a further security, however, I would respectfully suggest that the attention of Congress be called to this matter, with a view to such legislation as may be necessary for the protection of the navigation of its navigable waters.

Should Congress see fit to appropriate the above sum (viz., \$40,000) for the improvement of this river, which I strongly recommend, that amount can be profitably expended upon the work during the next fiscal year.

The following information is furnished in compliance with the requirements of

the law providing for the examination of this river:

This river is in "Frenchman's Bay district." Ellsworth is the port of entry, being at the head of navigation of the river.

The nearest light house is situated near the mouth of Union river, at Edgemoggin Reach. There is also one on "Bass Harbor Head."

The nearest fort is Fort Knox, in Penobscot river.

The collector of customs at Ellsworth is unable, for reasons given, to furnish a statement of the amount of revenue collected at that port during the past fiscal year. He states that there are about 20,000 tons of shipping owned in this district; there is manufactured on this river, annually, from 35,000,000 to 40,000,000 feet of lumber, nearly all of which is shipped from here. There is, of short lumber, of shingles, wood, &c., enough to make, say, 150 cargoes, annually, for vessels of about one hundred tons burden.

The yearly business on this river would give employment to say fifty vessels, running to Portland, Boston, New York, and intermediate places, these making from six to twelve voyages a season. The most of the lumber here is shipped to domestic ports, but the records of this office show that, for the quarter ending December 31, 1866, there were twelve American vessels which were cleared for foreign ports with lumber, the tonnage of which amounted to 3,983; value of cargoes, \$49,800.

GEORGE THOM,

Lieut. Col. of Engineers, Brevet Brig. Gen. U. S. A.

R 11.

Report on the improvement of the navigation of St. Croix river, above the "ledge," by Brevet Brigadier General George Thom, lieutenant colonel of engineers, for the year ending June 30, 1867.

An examination of this river from the "ledge" up to the head of navigation, at Calais, Maine, a distance of about four miles, showed the object of the appropriation to have been specially the deepening of the channel by the removal of the "slabs," "edgings," and "saw-dust," which obstructed the navigation of the river between the points named.

The tide at this place rises and falls from twenty to twenty-five feet, so that at high water there are about thirty feet of water on the eastern or St. Stephen's side of the river, all the way up to the bridge which crosses from Calais, Maine,

to St. Stephen's, in the province of New Brunswick.

At low water the channel is so much obstructed near Calais that vessels drawing four feet and a half can ascend only to the "lower wharf," also known as "McCollister's wharf," which is one mile and a half below the upper landing; while vessels drawing twelve feet can ascend, at low water, to a point about two miles above the "ledge."

From the upper landing down to McCollister's wharf the river is more or less filled with slabs, edgings, and saw-dust; while below McCollister's wharf, saw-dust, with very few if any edgings and slabs, have accumulated in the bed

of the river.

These edgings, slabs, and saw-dust come from the numerous saw-mills on both banks of the river above and near Calais, and have been accumulating. I was informed, for thirty years and more. I was also informed that twenty-five years ago vessels of large draught, say fifteen feet, could ascend to Calais at low water.

While the causes of these obstructions to the navigation are still in operation, it would not appear advisable to expend the appropriation in removing them, until a sufficient protection is given to the channel by adequate State laws, or else by such laws of Congress as may be necessary to prevent obstructions or other injury to the channels of its navigable water. I would therefore respectfully recommend that the attention of Congress be asked to this matter, at as early a day as practicable.

By an act of Congress approved March 2, 1867, the sum of fifteen thousand dollars was appropriated for this work: "Provided, The province of New Brunswick shall contribute and pay to the proper disbursing officer a like sum for said purpose; said payment being made on condition that in no event shall the province of New Brunswick be called upon for more than half the sum actually ex-

pended for said purpose."

With a view to commencing operations in this river at as early a day as practicable, I applied to the department on the 26th of March last for information as to what understanding, if any, the province of New Brunswick had with the United States government in this matter. So far as I have been able to learn,

that province has taken no action whatever in it, so that the matter will have to be brought to the attention of the new parliament, which is to convene in Ottawa, Canada, in September, for such legislation as may be necessary to give effect to the proviso of the law of Congress above referred to.

Operations in this river have, therefore, been necessarily suspended until the

provisions of the law making the appropriation can be complied with.

For an accurate estimate of "the amount that is required for the entire and permanent completion" of this work, a careful survey will be necessary. The examination made by me showed that, to open a channel one hundred feet wide and ten feet deep at low water would require the removal of not less than 100,000 cubic yards of slabs, edgings, and saw-dust; which,

At 80 cents a yard, would amount to	\$ 80,000 8, 000
Total	88,000
Deducting one half, if paid by the province of New Brunswick Amount appropriated by act of Congress approved March 2, 1867.	\$44, 000 15, 000
Additional amount required	29,000

Which amount could be profitably expended upon this work during the fiscal year ending June 30, 1869.

The following additional information is supplied in compliance with the requirements of the act of Cougress approved March 2, 1867, having been furnished by the deputy collector at Calais, Maine, viz:

The contemplated improvements of the "St. Croix river, above the ledge," are within the collection district of Passamaquoddy, and near the custom-house

in Calais.

There is no United States light-house now in use near this place, the nearest being in the vicinity of Eastport, Maine, about thirty miles below. There is no fort in the immediate vicinity, Fort Sullivan, also at Eastport, being the nearest

The amount of duties collected at the custom-house in Calais, for the fiscal

year ending June 30, 1867, was \$18,500.

The amount of commerce and navigation that would be benefited by these improvements consists of about one thousand vessels, probably averaging about one hundred tons each, which arrive at this port annually, being principally engaged in the coasting trade.

Hereto is appended a statement by the deputy collector, showing the amounts of exports and imports at this port during the year ending January 1, 1867, the number of arrivals and departures, and the number of vessels built, marked A.

GEORGE THOM,

Lieut. Col. of Engineers, Brevet Brigadier General.

U. S. Engineer Office,

Portland, Maine, September 10, 1867.

A.

PORT OF CALAIS.

Below we give the amount of exports and imports at this port during the year ending January 1, 1867, the number of arrivals and departures, and the number of vessels built:

Exports.—10,635,000 feet pine lumber; 17,394,000 feet hemlock lumber; 48,942,000 feet spruce lumber; 575,000 feet hard wood lumber; 210,000 feet hackmatack lumber; 100,000,000 lath; 4,000,000 pickets; 18,000,000 shingles;

35,000 ship knees; 14,000 cedar posts; 500,000 feet spool stuff; 400 cords birch wood; 370.000 hoops; 600,000 cedar sleepers; 53,000 clapboards; 200 cords bark; 139 ladders; 23,831 barrels calcined plaster; 8,065 cacks ground plaster; 8,000 sides leather.

Imports -82,000 bushels corn; 24,000 barrels flour; 1,800 barrels pork;

174 barrels beef; 13,000 hides; 1,518 tons coal; 5,000 tons plaster.

Built in Calais.—2 barks, 2 brigs, and 4 schooners, 2,350 tons.

Arrivals.—976. Clearances.—984.

B. M. FLINT, Deputy Collector.

APPENDIX S.

SAN FRANCISCO, August 5, 1867.

GRNERAL: I have the honor to submit the following report of my official

operations during the fiscal year ending July 30, 1867:

These operations have been conducted under the following appropriations, viz: survey of military defences; purchase and repair of instruments; removing obstructions to navigation in the Willamette river below Portland; and surveys and examinations on the Pacific coast. In addition to these, I am the engineer of the 12th and 13th light-house districts, and a member of the board of engineers for the Pacific coast

The operations under the above mentioned appropriations will be described in the order named.

SURVEYS OF MILITARY DEFENCES.

Early in July, 1866, I started with an escort of ten soldiers, by order of General Halleck, commanding the division of the Pacific, from Fort Churchill, Nevada, to examine the country between there and Ruby City, Idaho, with a view of finding a more direct and practicable route for teams between those two points. The report of this reconnoissance was made to the general commanding the department of California, and, being local in its character, is not sent in with this report, though a copy can be furnished at any time if called for.

The notes of this reconnoissance, together with those taken on a similar trip during a portion of the preceding year, have been plotted, resulting in a map of that comparatively unknown region, concerning a section of country never before mapped with any degree of detail. A reduced copy of this map, scale twelve miles to the inch, has been prepared for publication in a cheap manner, and, by authority of the general commanding the department, has been lithographed and published, copies of which have been forwarded to the engineer

department.

A map, on the same scale, of the whole of California, Nevada, Oregon, and a portion of Idaho, on one sheet, is nearly completed, and will be forwarded to the engineer department shortly, accompanied by a map of Arizona. These maps will represent, in much detail, all the known portions of those regions. Many portions are still unexplored, and, in consequence of the reduced number of troops in this department, the general is unwilling to spare troops for escort duty to topographical parties, and there seems but slight prospect of obtaining further topographical information with military assistance. One assistant has, by the suggestion of General McDowell, been for some months in Arizona, with instructions to accompany trains and scouting parties, and gather such topographical information as can be obtained in that way. Taking advantage of a geological party, directed by the geologist of this State, which is to travel through that portion of the State of Nevada near the 37th and 38th parallels, I have sent two other topographical assistants to accompany it. From this party very interesting results are expected, the country being nearly unknown.

My observations and investigations in meteorology and hypsometry have been prosecuted vigorously during the year. In February last I forwarded to you a report; complete in itself, of the results so far obtained, and believing they are of sufficient practical and scientific interest to warrant their publication, I have requested that they be published as a professional paper of the corps of engineers.

In the appendix to this paper are meteorological and hypsometrical tables, which, I think, will be considered superior to any heretofore prepared in English measures, and they contain all that an English or American meteorological computer may desire to take with him in the field on a reconnoissance.

On hand July 1, 1867..... 1,796 42

REMOVING OBSTRUCTIONS TO WILLAMETTE RIVER.

The order dated Washington, July 21, 1866, placing me in charge of this work, was not received until about the middle of October, in consequence of my being absent in the field on a reconnoissance.

From all the information I could collect, shortly after its receipt, I found it would be impracticable to do any work on that river during the seasons of winter and spring. I sent, however, my assistant, Lieutenant Heuer, United States engineers, to make an examination of the river, the report of which has shown you that the information before received was correct

The city of Portland had, during the two previous years, done work on this river by dredging, with the view of deepening the channel of the river, and had provided suitable dredging and other apparatus. Therefore the city authorities is the only party who can with economy do the work.

After making one bid for removing Swan Island bar, at a price exceeding the amount of the appropriation then available, the city have offered me the use of all the machinery free of cost, if I will keep it in repair and use it.

The act of Congress requires that the work be done by contract if possible. I was, therefore, directed to again invite proposals in the newspapers; and no bidders presenting themselves at the expiration of the time specified for opening the bids, I have accepted the offer made me by the city, and will commence work by hired labor in August next, which is the earliest possible time when dredging can be done.

I am directed to report on the following ten points, which I now do to the

best of my ability:

First. No survey or resurvey has been made by me, as, since I have been placed in charge of the work, the water has been too high to admit of work. The city, however, had made a survey of Swan Island bar, showing that

48,000 cubic yards of earth would have to be taken out to deepen the channel to eighteen feet of water. No such survey at the mouth of the river, where there is also a bar, has been made.

Second. The data to ascertain the amount required for the entire completion of the work is not at hand, as no actual work has been done by me for the above reason; but from the best information in my possession, based on rough estimates of work done and to be done, the sum of \$25,000 will be required for the completion of the work, in addition to that on hand.

Third. The sum of \$25,000 can profitably be expended during the next fiscal

year.

Fourth. The nearest collection district is Astoria.

Fifth. The nearest town is Portland. Sixth. The amount of revenue collected is unknown to me.

Seventh. The amount of commerce and navigation to be benefited by the completion of the work is large. Steamers drawing fifteen feet of water run from San Francisco to Portland regularly three times a month, except when stopped by extreme low water at the bars to be removed.

Eighth Proposals were invited, but no bidders appeared; hence the work

must and will be done by hired labor.

Ninth and tenth. Hence no contracts were made.

The following are the amounts received and expended during the last fiscal year on account of the appropriation for removing obstructions to navigation in the Willamette river, below Portland:

On hand July 1, 1866	\$000	00
Received during the year	10,000	00
Expended during the year	871	49
On hand July 1, 1867	9, 122	51

SURVEYS AND EXAMINATIONS ON THE PACIFIC COAST.

The order for this work was dated at the same time with the one just described, and for the same reason the order was not received until in October, Additional appropriation was made March 3, 1867.

The works designed to be executed under these appropriations are—

Survey of Blossom and Rincon Rocks, in the harbor of San Francisco, with the view of their ultimate removal.

Survey of the upper Columbia river, Oregon.

Survey of Crescent City harbor, with the view of making it a harbor of

refuge.

Experiments had previously been made on Rincon Rock, and as accurate surveys of both rocks had been made by the Coast Survey, it was deemed only necessary to make experiments on Blossom Rock to obtain data for estimating the cost of its removal. This has been done, and the results fully explained in the report of Lieutenant Heuer, who had immediate supervision of the work. The report was dated March 28, 1867, and forwarded by me to the engineer department the next day. From this report it appears that the probable cost of removing the rock will be \$60,000. The amount expended on the experiments was \$3,148 52, and the amount of stone removed 69 cubic yards. The amount of expenditure would have been much greater but for the kind assistance of the Coast Survey in furnishing a vessel, officers, and crew.

· On the upper Columbia no actual work has been done, for reasons similar to

those explained when speaking of the Willamette river.

An examination of this river has been made by Lieutenant Heuer during low water, and by myself during high water, showing that no work can be done until August. All preparations have been made to prosecute the work with vigor after its commencement.

The paper appended to this report, and marked A, is a report of the president of the Oregon Steam Navigation Company upon the commerce and navigation of the upper Columbia, and affords the fullest information on the subject.

A survey of Crescent City harbor has not been commenced by me, as I have been furnished from the Coast Survey office at this place a tracing of a chart of the harbor, sufficiently in detail to determine, after personal examination both by Lieutenant Heuer and myself, the position and extent of the proposed breakwater, and a rough estimate of its cost. This estimate, indefinite as it may be considered, would not be more exact if months were spent in another survey. The amount of stone required is sufficiently well known; the cost of putting it in position is what is indefinite. To build the breakwater will require about 410,000 cubic yards of stone. The same amount of materials thrown in the sea to build the breakwater at Plymouth cost \$7,000,000, as was fully explained in my report to the engineer department, of July 29, 1867. This, with the report of Lieutenant Heuer, of July 20, afford full information on the subject. For reasons given in my report, no appropriation is recommended for Crescent City harbor.

I will now endeavor to report on the following ten points referred to in the

act of Congress, viz:

1st. No survey or re survey has been made other than the one of Blossom Rock, since I have been placed in charge of this work, on account of high water.

2d. The data on which to determine the actual amount required for the completion of the survey of the upper Columbia is not at hand, as no actual work has been done, but from an examination of the river I think the amount on hand is enough to complete the survey, and that the sum of \$50,000 should be appropriated for work during the next fiscal year, in actually removing the obstructions.

For the removal of Blossom Rock, \$60,000 has been recommended. I do not recommend an appropriation for the removal of Rincon Rock until Blossom Rock has been removed. No appropriation for improving Crescent City harbor is recommended.

3d. The amount that can be profitably expended next year on the	
upper Columbia is	\$50,000
For Blossom Rock	60,000

4th and 5th. The collection district for the work at Blossom Rock is San

110,000

Francisco; and for the upper Columbia, Astoria.

6th. I have no means of ascertaining the amount of revenue collected at the nearest port of entry.

7th. The amount of commerce and navigation at San Francisco and on the upper Columbia is large. In 1864 there were 36,000 passengers and 21,834 tons of freight on the upper Columbia, and it is believed the amount has greatly increased since.

8th. 9th, and 10th. No work done by contract.

The following is the amount received and expended during the last fiscal year on account of the appropriation for "surveys and examinations on the Pacific coast:"

On hand July 1, 1866	\$0 00
Received during the year	10, 445 75
Expended during the year, Blossom Rock	3, 148 52
" " " Upper Columbia	661 42

On hand July 1, 1867..... 6, 635 81

My duties as a member of the board of engineers for the Pacific coast have

occupied a small portion of my time.

No light-houses have been erected during the year, except one at Cape Gregory, Oregon, together with the keeper's dwelling. The light is now in working order. Several light-houses have required and received repairs.

I have the honor to be, very respectfully, your obedient servant,

R. S. WILLIAMSON,

Brevet Lieut. Col. U. S. A., Major of Engineers.

Major General A. A. Humphreys, Chief of Engineers U. S. Army.

OFFICE OF OREGON STEAM NAVIGATION COMPANY, Portland, Oregon, July 20, 1867.

SIR: Yours of June 17 proximo, giving extract from an act of Congress making appropriations for examination and survey of certain works of improvement, and asking for information concerning the various lines of river transportation on the upper Columbia, of the obstacles to such transportation, &c., duly received.

In reply, I shall as briefly as possible touch upon the points named, and give

you such information as you desire, so far as I am able to do so.

What is known as the upper Columbia river is that section between Celilo, or the main Dalles of the Columbia, to Priest's rapids, a distance of two hundred mines, and is navigable with light-draught steamers at all seasons when not obstructed by ice. At present there are six stern-wheel steamers employed on this section of the river, three of which are large-sized boats, with carrying capacity of from two to three hundred tons each, and are used during the season of high water, say from the 1st of May to the 1st of September. The other three are small light-draught boats, with carrying capacity of from seventy-five to one hundred tons each.

From 1st of May to 1st of September, Snake river, from its mouth to Lewiston, a distance of one hundred and seventy miles, is added to the navigation of

the upper Columbia.

From Celilo to Priest's rapids the navigation is obstructed during the season of low water, by the following-named rapids, viz: "Five Mile rapids," "John Day's," "Indian rapids," "Squally Hook," "Rock creek," "Canoe Encampment," "Devil's Bend," "Umatilla rapids," and "Homely rapids."

The rapids presenting the most serious obstacles to navigation are "John Day's," "Umatilla rapids," and "Homely rapids," but all require more or less improvement to admit the use of large boats during the season of low water.

I shall not attempt to describe the character of these obstructions, or give an opinion as to the amount of work or means necessary to remove them. Lieutenant Heuer has examined, and has doubtless reported on this point all that is

necessary for your information.

I would add, however, that "Homely rapids" is situated just below the mouth of Snake river. The Columbia at this place is broad and full of bars. Channel obstructed with narrow ledges or boulders, which, if removed, would enable boats at low water to reach "Priest's rapids;" and as Snake river rises much sooner in the spring than the Columbia, the removal of this obstruction would enable boats to reach Lewiston at least three weeks earlier than at present.

From Celilo to "Priest's rapids," and from the mouth of Snake river to Lewiston, the banks of the river and the adjacent country are entirely destitute of timber; hence the article of fuel must either be carried in large quantities for so long a trip, or transported by steam or sail at great expense to convenient points along the river, for the accommodation of steamers, and more particularly for the smaller

class, which if compelled to carry wood for the round trip, would have little or

no capacity for freight.

Tributary to this section of the Columbia river is a large mining county bordering on the Columbia river, north of the forty-ninth parallel. An American steamboat has been successfully navigating the Columbia, from what is known as "Little Dalles." near Fort Colville, to "Death rapids." about two hundred and fifty miles north of the boundary line between the United States and British Columbia.

The Hudson Bay Company, who do a large business in that section of the country, and who, before this boat was built, supplied their posts from Victoria

via Frazer's river, now ship all their supplies via Columbia river.

Colville valley (in which Fort Colville is situated) is rich in agricultural and mineral wealth, and is rapidly filling up with permanent settlers. It can only be supplied via Columbia river. It is distant from Wallula about one hundred and seventy-five miles, and almost due north. There are now three steamboats navigating Clark's fork of the Columbia, from foot of Pend d'Oreille lake to Thompson falls.

The inducement offered to shippers by the establishment of this line has been such as to make tributary to the Columbia river all that rich mining country lying west and north of Helena in Montana Territory. The distance necessary to connect the boats of the upper Columbia with the boats of Pend d'Oreille lake

is one hundred and sixty miles, over a good wagon road.

It is the intention of the parties engaged in the enterprise to place a fourth boat on Clark's fork, to connect with the others, from Thompson's falls to the mouth of Jako, making the whole distance by steam on Clark's fork two hundred miles.

It is said that the mouth of Jako is only one hundred and twenty miles from Fort Benton on the Missouri river, and if so, the navigable waters of the Missouri and Columbia rivers are only separated by two hundred and eighty miles of land travel.

Already a large amount of freight consigned to different points in Montana has been and is being shipped up the Columbia. This trade is rapidly increasing, and must at no distant day be a large item in the business of the river. In short, all the country east of the Cascade range of mountains, from latitude 42° to 52° north, including all of eastern Oregon and Washington, all of Idaho, and a large portion of Montana, must of necessity be tributary to the Columbia river.

The following figures will show the increase of business on the Columbia

river from 1861 to 1864, inclusive:

	Number of passengers.	Tons of freight.
1861	10, 500	6, 290
1862	24, 500	14, 550
1863	22, 000	17, 646
1864		21. 834

I have not the figures before me to show the continued increase to the present time, but to show how rapidly the country is filling up with permanent settlers. I will state that previous to the present year, all the flour and most of the grain necessary for the supply of military posts in the department situated east of the Cascades was shipped from Fort Vancouver up the Columbia river to convenient points, and thence by teams at great expense to the different posts. This year the valleys of Colville, Walla-Walla, Grand Ronde, and Powder river, have produced a large surplus of all kinds of grain, the result of which is, that the government is now supplied at much less rates than was previously paid for transportation alone.

Two years ago, all the flour necessary for the supply of a large mining population in Idaho and eastern Oregon was shipped up the Columbia river.

In June of this year Walla Walla valley alone shipped down the Columbia, for San Francisco and New York markets, over five hundred tons of flour, and

has remaining for shipment at this date at least twenty thousand barrels.

Cheap freight is of the first importance to an agricultural country. secure this to the valleys tributary to the upper Columbia, it is absolutely necessary that the navigation be so improved as to make it practicable for the use of the largest class boats at all stages of water.

Average high water on the upper Columbia, from 1st of May to 1st of September; medium stage, from September to middle of November; low water, from

November to middle of March.

River obstructed by ice from thirty to sixty days of each year. Last winter there was no ice to interrupt navigation.

I have the honor to be, very respectfully, &c.,

J. C. AINSWORTH. President O. S. N. Company.

R. S. WILLIAMSON, Brevet Lieut. Col. U. S. A., Major of Engineers.

8 1.

SAN FRANCISCO, January 28, 1867.

GENERAL: I have the honor to acknowledge the receipt of your letter of December 29, 1866, concerning the operations on the Willamette and Columbia rivers.

In obedience to your instructions, I shall invite proposals for the removal of the obstructions at Swan Island bar, by advertising said proposals in newspapers in this city and in Portland, Oregon, according to regulations. bids will be sent on to Washington for approval.

Your directions also state that I am to ascertain the amount of rock to be removed in the upper Columbia, with sufficient accuracy to admit of a reliable estimate of the quantity to be blasted, &c. With that view a survey of the rapids of the upper Columbia will be made; but such work cannot be commenced to advantage, according to the best information I have collected, until the 1st of August next, and that information convinces me that that is the proper time to commence such work. I have corresponded with Captain Ainsworth, the president of the Oregon Steam Navigation Company, on this subject, with regard to the most economical and satisfactory manner of making this survey, and have suggested three plans of operation, one of which must be adopted. I will premise by saying that Captain Ainsworth owns or controls all the steamboats and nearly all the sailing vessels on the waters above the Dalles. I inquired if a small steamboat could be chartered for the surveying party, and in reply was informed it would cost \$4,000 in gold per month. This was for steamboat and crew, but exclusive of the pay and subsistence of the engineer party. This I considered out of the question. I have to resort to one of the other two modes of making the survey, viz : one by chartering a sailing vessel, which can be had at a much less price; but whether one can be had of sufficient size to admit of the party living and messing on board of her is a question to be still decided. The other plan is for the party to live in tents on shore, and be provided with boats, and move from point to point by aid of the passing steamers. In either case the operation will be expensive, on account of the thinly settled country, and no definite estimate can be had of the expenses until determined by experience. The obstructions being mostly needle rocks, their size is to be determined by accurate surveys. The expenses

of removing them will be best learned after the experiments on Blossom Rock, now to be commenced this week.

The part of your instructions most difficult to carry out is that relating to the mouth of the Willamette, and, fortunately, there is time to receive further instructions and explanations from you, as I cannot conceive it possible to prosecute to advantage such work as is directed in your instructions for the next two months. We have here, so far, a very severe winter, as the California newspapers will show you. (A strip from yesterday's Bulletin is enclosed.)

You state that "the conditions existing at the mouth of the Willamette should be studied with care, and observations upon the direction and force of the currents in the various channels should be made at those stages of that and the Columbia river which are believed to control the formation of shoals and the channels over them." It is supposed that the formation of shoals depends in a great measure on the relative force of the currents in the two rivers. At one time the one river may be high and the other low, and at another the reverse may be the case. They differ in different years, according to the varying amount of rain in the Willamette and snow in the upper Columbia.

Hence, to study fully the problem of the removal of the obstructions at the mouth of the Willamette, observations should be made at all seasons of the year and for several years. Meteorological records show that in one year the amount of rain may be four times as great as in another year. To obtain such observations as are described in the report on the Mississippi would require a party in constant employment and of considerable strength, and would proba-

bly entail an expense of \$1,000 in gold per month.

I did not understand that any portion of the appropriation for "removing obstructions" was intended to be applied to the study of the character of the rivers. The appropriation for these removes was inadequate for their removal, but much work by private parties had been done. A plan of operations had been adopted by those parties, and the very parties who had commenced the work (who, I am told, were the city authorities of Portland) had exerted themselves to procure the appropriation. The question then was, shall those parties, who have already expended more than the amount of the appropriation, continue the work by the aid of this appropriation, (which they could do much more cheaply, as they had the boats, &c., at hand,) or should the government act in an independent way and commence the work anew? Under these circumstances, and with the facts already given, I think it best to ask for more definite instructions as to how to prosecute the work at the mouth of the Willamette, particularly as all the work has been suspended in consequence of the season.

I will recall, in brief, the plan of operations they proposed, which is, inasmuch as the present ship channel is in quicksand, and when dredged out is soon filled up again, and whereas the south channel, which is said to have a hard bed, is believed to afford a chance for a permanent channel, when once made of the proper depth, it is proposed to make it the ship channel by dredging. Shall we expend the part of the appropriation available in this experiment, or shall we expend it in studying the character of the river?

I submit an estimate of the cost of removing the obstruction in the Willamette river, made by Lieutenant Heuer, which is to be accepted as a rude one

I have the honor to be, very respectfully, your obedient servant,

R. S. WILLIAMSON,

Brevet Lieut. Colonel U. S. Army, Major of Engineers.

Major General A. A. Humphreys,

Chief of Engineers, United States Army.

[Extract from San Francisco Bulletin.]

THE WEATHER.—The great question now being asked is, "When will it stop raining?" We have had an almost steady flow of rain for a month, and the chances of its drying up are apparently as slim as they were at the commencement of the deluge. According to Tennent's rain gauge .17 of an inch fell on the 22d instant, .40 on the 23d, .35 on the 24th, and .14 up to 8 o'clock this morning. The rain fall thus far is about 21 inches, and still it comes. Cold weather, too, accompanies the rain, and the hills in Contra Costa county were yesterday white with snow. To-day it is raining as hard as ever, and unless nature's reservoir becomes exhausted visions of dire calamities by flood will disturb the public mind.

S 2.

SAN FRANCISCO, CAL., April 17, 1867.

GENERAL: I have the honor to acknowledge the receipt this day of your letter of March 8, 1867, concerning the Willamette river. The statements made in the report of Lieutenant Heuer of November 27, 1866, were considered reliable, as he visited the mouth of the river and consulted with all the parties interested, particularly consulting with the engineer employed by the city, on whose judg-

ment every one there seemed to rely.

I have received also this day the enclosed letter from Mr. McCraken, chairman of the committee of the board of supervision on river improvement, in which is explained the discrepancy in the estimate made from the information gained from the civil engineer and the actual work to be still done on Swan Island bar. I think it probable that there was no reliable estimate made of the work done on that bar. The dredger would be used on it for a few days and then removed to another locality at the mouth of the river, where it was thought work would be most required, and no separate estimates were made.

I have every reason to suppose that the city authorities are sincere in stating that they did not expect the appropriation to cover the cost of the work, and I think that by accepting the proposition made the money will be judiciously spent.

If you concur with me in this view, I would respectfully suggest that you telegraph to me to that effect, as it seems some work can be done on the river before the freshets commence.

As the board of engineers think of visiting San Diego soon, to return about the 10th of May, I will be at leisure, and will, as soon as possible after an answer to this is received, visit Portland and the Willamette, and will conclude such arrangements as you may direct.

Very respectfully, your obedient servant,

R. S. WILLIAMSON,

Brevet Lieut. Col. U.S. A., Major of Engineers.

Major General A. A. HUMPHREYS,

Chief of Engineers U. S. A., Washington, D. C.

CALIFORNIA, HAWAIIAN, AND OREGON PACKET LINES, Portland, April 13, 1867.

DBAR SIR: Your valued favor came to hand and was placed before our board. We can explain the discrepancy readily by a plat or sketch of the river as surveyed by a competent engineer, which will show 46,000 cubic yards to be excavated to give the required depth. Our former engineer was too sanguine,

(an excellent man to conduct the mechanical part of the work,) and thought, at least represented, that he had done more and there was really less to do than the facts prove.

Another error of his: he gave the cost per month of running the dredge at \$1,000 in coin, when he omitted in that estimate \$25 per day, (Sundays included,)

or \$750 per month, for a steam tow-boat.

The city did not expect the appropriation to cover the cost of the work, but endeavored to avail itself of the amount to go as far as it would and pay the

balance out of its treasury.

I think I am warranted in assuring you that if the general government will undertake the work itself, and prosecute it vigorously, (as you are aware time is important to its interest,) the dredge will be placed at its disposal, to be returned in as good order, excepting ordinary wear and tear. But we will send the sketch when a copy is taken and write more fully on this subject by the next mail.

Thanking you for your kind interest in this matter, I remain, yours, truly.

JOHN McCRAKEN.

Colonel R. S. WILLIAMSON,

Engineer U. S. Army, San Francisco.

SAN FRANCISCO, May 30, 1867.

GENERAL: I have the honor to enclose herewith a copy of the advertisement which appeared for the first time in a morning paper here, as authorized by your letter of April 26. A similar advertisement will be sent to the Portland Herald.

Last evening I received a letter from the mayor of Portland, enclosing a resolution of the common council of that city, which is enclosed. The letter states that all facts relative to the nature and condition of the bars of the Willamette will be furnished me in a short time.

I have the honor to be, very respectfully, your obedient servant, R. S. WILLIAMSON,

Brevet Lieut. Col. U. S. Army, Major of Engineers.

General A. A. Humphrkys, Chief of Engineers, U. S. Army.

Sealed proposals in duplicate will be received by the undersigned at his office in Portland, Oregon, until noon of Thursday, the 27th of June, 1867. (when they will be opened, and bidders are invited to be present,) for deepening the Willamette river, between Portland and its mouth, to admit the passage of vessels drawing eighteen feet of water, by dredging over or through Swan Island bar, and the bar at the western mouth of the river, (the one between J. D. Percy's island and Laurie's island,) the channel of said depth to be at least 100 feet wide at the bottom.

Bidders will state their proposals in the government currency of the United States, and payment will be made on the satisfactory completion of the work, after examination by the undersigned or other authorized agent of the government, who will be on the spot at the time for the purpose. The payment is to be made in such funds as are furnished by the government.

Each bidder will be required, before the time fixed for the opening of the bids, to file in the office of the undersigned a bond with sufficient sureties, in a penalty of five thousand (5,000) dollars, conditioned that if his bid is accepted, he will execute a written agreement for the faithful performance of his contract, and give

such further bond for the performance of his written agreement as may be required; and said agreement is made subject to the approval of the Chief of Engineers United States army.

The contract will be awarded to the lowest responsible bidder, but the right

to reject any and all of the bids is reserved.

A copy of this advertisement must be attached to each bid.

Bids that do not comply with the above requirements will not be entertained. The expense of the advertisements must be paid by the successful bidder.
R. S. WILLIAMSON,

Brevet Lieut. Col. U. S. Army, Major of Engineers.

Be it resolved by the common council of the city of Portland, That the honorable the mayor be, and he is hereby, authorized and instructed to correspond with Colonel R. S. Williamson, United States engineer, and fully inform him of all particulars and facts at hand relative to the nature and condition of the bars of the Willamette river, both at Swan island and the mouth of said river, the amount of and cost of the work that has been done by the city, and the urgent necessity of prosecuting it at an early day.

And further, that the mayor be empowered to tender to Colonel Williamson (in the event he shall proceed to expend the appropriations made by the United States government for the purposes intended) the use of the dredging apparatus belonging to the city free of charge, provided he shall keep the same in thorough repair; and that he be further instructed to withdraw the proposal for dredging Swan Island bar, made under resolution of the common council passed March 8,

1867. Passed the common council May 16, 1867.

W. S. CALDWELL, Auditor and Clerk.

8 3.

SAN FRANCISCO, July 29, 1867.

GENERAL: I have the honor to report that on my return from the northern coast, (where I had been on light-house duty and had visited, among other places, Crescent City,) I found your letter of April 26, directing me to take charge of the survey of Crescent City harbor; and I submit a plan for its improvement, with a view of making it a harbor of refuge. My first step was to ascertain what had already been done towards a survey of that harbor, and on application at the Coast Survey office in this city I was kindly furnished with a copy of a Coast Survey chart of it—scale, 1-10,000—which is enclosed with this report. On its examination it appeared to me that it contains information sufficiently in detail to enable me to devise a plan for this improvement as well as if a more elaborate survey were made, which would require months of time and a considerable expenditure. It was necessary, however, that I should obtain some detail information on certain points, and as duty with the board of engineers for the Pacific coast, together with the operations on the Willamette and Columbia rivers, made it important that I should go at once to Oregon, I directed Lieutenant W. H. Heuer, United States engineers, to proceed to Crescent City for the purpose of obtaining the necessary information. He was instructed to examine the locality; ascertain the nature of the material to be found there; whether stone of great size and weight can be obtained; the means of transportation for them to the proposed place of deposit; whether a right of way for a railroad can be had from the place where the stone is found to the water near the place of deposit; the facilities in the vicinity for machinery and labor; the effects of previous

storms on the harbor, and all other information that would enable him to make a comprehensive report on the subject, with drawings and sections of the proposed work. His report is enclosed, and his estimate exceeds two millions of

dollars, (\$2,000,000.)

It is almost impossible to make anything like an accurate estimate of the cost of such a work, but by comparing the cost of other breakwaters which have been actually constructed or estimated for, some light may be thrown upon the subject, though it is very true that in each particular case elements come into the calculations which naturally affect the estimate. The case of the breakwater at Plymouth, in England, may be considered one not very dissimilar, and therefore I will state some of the facts concerning it. That work is 1,700 yards long, 210 feet broad at its base, and 30 feet at the top, which is 10 feet above ordinary low water of spring-tide. The amount of stone thrown in during the five years ending 1816 was one million tons, and the work was then considered about one half completed. The amount actually expended during that five years was £1,524.000, equal to \$7,563,173, and the estimate for the completion of the work was £1,562,639, equal to \$7,563,173. To compare the magnitude of that work with the one proposed at Crescent City we may consider that the amount of stone required, according to the estimate of Lieutenant Hener, is 409,914 cubic yards, which, if it weighs 170 pounds to the cubic foot, will weigh 940,753 tons, or nearly the quantity thrown in during the first five years at Plymouth At the same rate of cost as at the Plymouth breakwater the one proposed for Crescent City will cost \$7,000,000. The cost of labor, &c., in England when that work was prosecuted and their cost at this time in California is very different. I have no data with which to base an accurate estimate or comparison, but I think that it is certain we may reasonably increase the estimate several The work just spoken of and others in England and France have been constructed by throwing huge masses of stone into the sea at random, (pierre perdue.) The size of the stones used varies from one to five tons and upwards. The heaviest masses are required between the levels of low and high water, but their size must of course vary with the peculiarities of the localities; for in a place not subject to violent storms, as in sheltered bays, the stone may be comparatively small. They must, however, always be of sufficient size and weight not to be moved by the action of the waves. This is absolutely necessary to secure stability. We have every reason to suppose that at Crescent City none but the largest masses ought to be used, the violence of the winter storms there being very great. It is therefore doubtful if it will be practicable to procure stones of sufficient weight. This was found to be the case with the breakwater at Algiers, where the sea carried away nearly all the stones that were thrown in, though the blocks used were from 100 to 141 cubic feet, (eight to twelve tons.) To preserve the work care was constantly taken to replace them. When that place fell into the hands of the French a different system was adopted, viz., that of using huge masses of beton instead of natural stone, and of such size as to resist the action of the sea and remain inmovable. The minimum limit to the size of those blocks was 353 cubic feet. An interesting account of this work is to be found in the rudimentary treatise on foundations in Weale's series, by C. E. Dobson. That breakwater was completed in 1835, and, according to that writer, "gave it a stability which is proof against the severest test." Nevertheless, I find in the "papers relating to the report of the British harbor of refuge commission, 1846," the following: "The last fact to be noticed respecting the work at Cherbourg constitutes a very decided warning against the use of concrete, for the application of this material on a large scale has utterly failed, the blocks of concrete being broken to pieces." The "concrete" here spoken of may not have been made with hydraulic lime. We have every reason to suppose that large masses of artificial stone of a sufficiently durable character can be made. In the plan here submitted for a breakwater at Crescent City the width of it is

to be 30 feet, like the one at Plymouth. At a very exposed place, like Crescent City, a much greater width may be necessary. The jettee at Algiers had a top width of 118 feet. It would scarcely be worth while going into the examination of other works of this kind. We must come to the general conclusion that any breakwater at Crescent City must be a work of immense magnitude; that it is impossible to make any but a very rude approximate as to the cost of its construction, but that at least \$2,000,000 would be required for it—and it may require several times that amount. The argument in favor of a work of this kind at that place is, that it is the only place between San Francisco and Cape Flattery where a harbor of refuge can be made, and hence one should be made There is no great city there and no populous back country. Crescent City has but one hundred or two hundred inhabitants, if it has as many. All to the rear is a sparsely settled country or dense forest. The immense expenditure required for the construction of a breakwater at that place would be but the beginning of a large series. If a harbor were made there immense sums would be required for its fortifications, for otherwise it would be of valuable assistance to an enemy. Probably, also, constant work would be required in dredging to prevent its being filled up. With all these facts in view, I do not recommend an appropriation for the commencement of a breakwater at Crescent City at present.

I have the honor to be, very respectfully, your obedient servant, R. S. WILLIAMSON,

Brevet Lieutenant Colonel V. S. A., Major of Engineers.

General A. A. Humphreys, Chief of Engineers U. S. A.

SAN FRANCISCO, CALIFORNIA, July 20, 1867.

SIR: I have the honor to report, that I proceeded to Crescent City, California, and examined the vicinity with a view of obtaining information which would enable me to make plans and estimates for a breakwater to be constructed there. During the summer months the prevailing winds there are from the northwest; against such winds Crescent City harbor is now a comparatively good one, the only objection thereto being a few dangerous sunken rocks; but during the winter, when the winds are from the southwest or from the southeast, vessels can with the greatest difficulty lie there. Captains in command of vessels running up and down the coast inform me that a heavier swell occurs there (Crescent City) than anywhere else on the whole coast. During the winter months large logs, some of them twelve feet in diameter, are thrown violently on the beach by this swell, and several houses, against which these logs have been thus thrown, were battered down. Several years ago a wharf was constructed there on piles running from the main shore, near Battery Point, to Flat Rock, but some large drift-logs coming in contact with it during a gale, it was utterly demolished. Battery Point is the proper place from which to start the breakwater, although at first sight the southeast side of Light-house island appears the most eligible, but during gales the sea is forced in between this island and Battery Point with such a velocity as to create a very heavy swell in the harbor. Starting, then, with a breakwater from Battery Point, I would suggest that it be run in a south southeast (S.SE.) direction for a distance of 2,117.25 feet to a point marked A on the Coast Survey chart appended; thence in a direction slightly south of east for a distance of about 850 feet to B; thence in a direction slightly north of east for nearly 800 feet to C. This would be the most economical breakwater, giving the greatest extent of harbor that I think could be constructed there. It would be built on a natural shoal for its entire length; nearly all the dangerous rocks would be to the seaward of it; the reef outside would materially assist in

breaking the force of the waves before coming in contact with the breakwaterand it would be a good anchorage ground, with plenty of water inside for vessels Should the breakwater be constructed in the above direction there are two rocks namely, "Fauntleroy Rock," and one nearly east of it, and distant about 150 yards, which would require removal, as they are near the head of the breakwater. Both of these rocks are long and slender, with deep water on all sides of them. The expense of removing them would be slight. Metamorphic sandstone can be obtained in any quantity, and of sufficient size for building this breakwater, by quarrying from the spur of a mountain, distant from Battery Point about five miles. This spur is owned by a Mr. Gay, who has consented to allow the United States to take what stone they may need for the breakwater free of charge. A railway would have to be constructed from this quarry to Battery Point; it would require but little grading, as a good wagon road is already constructed between the two points, and there is no difficulty about obtaining the right of way. I examined the country within five miles of Crescent City, but could find no stone of suitable quality any nearer than that above mentioned. Whaler island, in the harbor, contains over 22,000 cubic yards of rock, but the rock contains too much silica and quartz to be used for breakwater purposes. There are no facilities in the vicinity of Crescent City for machinery or labor. Timber is found in abundance. It was suggested by you that the top of the breakwater be thirty feet wide, with an inward slope of about forty degrees, and a seaward slope of about twenty-five degrees from the horizontal. This would require for the entire breakwater, as suggested, 323,664 cubic yards of material; the first section, viz, that from Battery Point to A, being 2,117.25 feet in length, having an average depth of 28.3 feet; the other sections, from A to C, being 1,637.34 feet in length, having an average depth of 31.7 feet. This would place the top of the breakwater five feet above the highest known tides. I am led to believe, however, from the construction of previous breakwaters, (Cherbourg, France, and our Delaware breakwater,) that a slope of twenty-five degrees to the seaward will hardly suffice. In some places, on the outer face, it should be less, in others greater than twenty-five degrees, depending on the violence of the action of the waves on these points. From the "report on the British harbors of refuge," where is found a description of various breakwaters, including the Delaware breakwater, I have estimated the amount of material which would be required in the construction of the Crescent City breakwater, giving it a seaward slope, the same as that of the Delaware breakwater, (see drawings annexed,) and find that it would require 409,914 cubic yards of material. Rock can be quarried and placed at Battery Point at \$3 47 per cubic yard; this would On the supposition that it would require 100 men six years to complete the breakwater, working six months in the year, at

Total in currency

Respectfully submitted:

W. H. HEUER,

Lieutenant of Engineers.

2, 178, 009 08

Brevet Lieut. Col. R. S. WILLIAMSON.

Major of Engineers.

APPRNDIX T.

Report of Brevet Brigadier General N. Michler, Major of Engineers, United States Army, in charge of Public Buildings, Grounds, Works, &c.

OFFICE OF PUBLIC BUILDINGS, GROUNDS, AND WORKS,
CAPITOL OF THE UNITED STATES,
Washington, October 1, 1867.

GENERAL: On the 16th of October, 1866, I had the honor to submit my annual report for the fiscal year ending on the 30th of June of that year. At that time it was stated that plans of the military operations in front of Petersburg and Richmond, including detailed drawings of forts, redoubts, batteries, and mines, and topographical sketches of the various battle-fields from the Rapidan to Appomattox Court House, together with a large number of maps of the country exhibiting the lines of march of the contending armies, were in course of construction. The surveys cover an area of nearly fifteen hundred square miles. The fellowing brief recapitulation will show the number of sheets, their different scales, and the several sections into which the whole field of operations is subdivided:

I. The general maps, two inches to the mile, represent the country between

Cold Harbor and Appomattox Court House, comprising thirteen sheets.

II. The detailed maps are on a scale of eight inches to the mile, twenty-nine in number; eight illustrating the intrenched positions in front of Petersburg, and the remainder the lines of works around Richmond and along the James river above its junction with the Appomattox.

III. Ten sheets, scale of four inches to the mile, exhibit the most important battle-fields, comprising the Wilderness, Spottsylvania Court House, Tolopotomy, Cold Harbor, Five Forks, Jetersville, Sailor's Creek, Farmville and Appo-

mattox Court House.

IV. Thirteen sheets, one inch to the mile, show the original maps issued at the commencement of the campaign of 1864.

V. Three sheets of sections of the last mentioned issue, corrected and distri-

buted on the march.

VI. The index sheet, scale 250000, is a general map of the country lying east of the Alleghany mountains, and extending from the battle-field of Gettysburg

on the north, to the South Side Railroad of Virginia, on the south.

VII. One hundred and eleven drawings of forts, redoubts, batteries and mines, the scale of the respective plans being forty feet to one inch. The entire portfolio numbers one hundred and eighty sheets of antiquarian, embracing not only all the operations during the war of the army of the Potomac, but also of the army of the James and of the several detached commands engaged in Virginia.

At this date the maps are entirely completed, with but two or three exceptions, the surveys having been made with great accuracy and the topographi-

cal features of the country delineated on them with great care.

Whilst engaged in superintending the construction of these military maps, I was, in addition to that duty, detailed to carry out the directions of the Committee on Public Buildings and Grounds of the Senate of the United States relative to the selection of a suitable site for a public park and presidential mansion.

The views of the committee were imparted by its chairman, Hon. B. Gratz Brown, in several communications, of the 24th and 26th of July, addressed to the honorable Secretary of War; he therein requested that an engineer officer be placed in charge of the preliminary surveys of certain tracts of land adjoining or near the city of Washington, the preparation of the necessary maps and reports for the purpose above named, which, in the language of the Senate reso-

lution of the 18th of the same month, "shall combine convenience of access and healthfulness, good water and capability of adornment;" in addition to this to ascertain, if practicable, the price of said lands. After a careful examination of the many beautiful localities to be found in the vicinity of the capital, and having caused an accurate and detailed survey of its environs to be made, I had the honor to address a communication, dated January 29, 1867, to the chairman of the committee, submitting for his consideration the result of my investigations. In connection with that report two preliminary maps were presented, showing, more plainly than words can express, the required information and the respective advantages of the different sections surveyed. A copy of my report is herewith appended, (Senate miscellaneous document No. 21, second session, thirty-ninth Congress,) together with reduced photograph copies of the now complete topographical sketches accompanying it. It will not be necessary to accumulate words, after so much has been written and so ably spoken in advocating an improvement so essential to the comfort and pleasure of every enlightened community. The attentive perusal of the very comprehensive remarks and beautifully expressed sentiments delivered by the honorable chairman of the Committee on Public Buildings and Grounds in the Senate of the United States, on the 20th of February of this year, concerning the bill for the establishment and maintenance of a grand national park in the District of Columbia, at the expense of the United States government, will convince the most prejudiced judgment. It will not fail to urge the earliest action of Congress in carrying out the hopes expressed by the honorable senator, already almost unanimously concurred in by the Senate, as well as the earnest wishes of those most interested in the adornment of the capital of a great nation, that no unnecessary delay may occur in passing the needful laws for accomplishing such a grand and beautiful undertaking. A copy of Mr. Brown's speech, together with the bill reported from the Senate committee to establish a public park in the vicinity of Washington city, are herewith appended; the bill passed the Senate, but was laid on the table of the House of Representatives during the last hours of the session.

By the second section of an act of Congress approved March 2, 1867, "the duties heretofore imposed by law upon the Commissioner of Public Buildings are devolved upon the Engineer Bureau, as well as the Superintendent of the Washington Aqueduct, and all the public works and improvements of the United · States in the District of Columbia not otherwise provided by law." I had the honor, by engineer department orders dated March 13, 1867, to be assigned to execute the duties above stated, and at once to enter upon their performance. Immediately upon the receipt of these instructions, I called upon the Secretary of the Interior and the late Commissioner of Public Buildings, and received from them all the books, records, archives, and papers pertaining to the office of Commissioner of Public Buildings, and to the public works referred to in the act cited, and at once proceeded to the discharge of the duties appertaining to each of them. These are of such a diversified character that this report will only refer to the most important ones, and these in most general terms. I shall first consider such works in course of construction or undergoing repairs which are already authorized by different acts of Congress, and will then suggest some additional improvements in connection with the growth of the city as appear to be eminently necessary and ornamental. The different bridges across the Potomac and the Anacostia, or Eastern Branch, have either been rebuilt or placed in as good repair as the limited amount of the appropriations would admit. The one familiarly known as the Long bridge is, by actual measurement, four thousand six hundred and sixty-one feet. The sections are differently constructed, and of the following lengths:

Third, gravel road on causeway	1,967 feet.
Fourth, draw (south side)	148 feet.
Fifth, brace and frame work, supported by cribs	452 feet.
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It will be remembered that in consequence of the heavy masses of ice which lay against and crushed the bridge at different points, it had been rendered impassable for some months. The appropriation of fifteen thousand dollars enabled me to place it in such repair as to allow travel across it to be re-Much more work is still required to make it stand until a more substantial, suitable, and architectural structure be built to span this magnificent river. I have been ordered to prepare a special report to the bureau in regard to the improvements of the channel of the river, and surveys are now progressing to show what changes have recently taken place; the bridge question is so intimately associated with this subject, that it will necessarily form a part of the discussion. The first section of the bridge is in very good condition, having been recently placed so; the second (north draw) will have to be rebuilt, as the timbers are decaying, and a more solid support must be substituted for the present pile one; the third is in good repair—the brick side-walls are damaged in many places, and require attention both for safety and appearance; the fourth (south draw) is in good working order; the fifth requires so many repairs as to almost necessitate the entire rebuilding of that portion. Although in much better condition, it is said, than for many years past, still it is doubtful if it can stand against the ice and freshets of another winter. An early appropriation, an estimate of which is respectfully submitted, is earnestly urged, as the work should be commenced without unnecessary delay.

The bridge over the Potomac, near the Little Falls, is in a most dilapidated Having been greatly used during the war, with little or no care taken to keep it in good preservation, it is now in danger of giving way; the timbers of most of the spans, especially the three nearest the Virginia shore, are much worn and decayed. The bridge should have been covered to protect the wood, and the latter whitewashed to keep it sound. The flooring is much worn, and travel over it has become dangerous; only by the most careful attention on the part of the watchman can accidents be avoided. It would probably be economy to take the bridge down and rebuild it. An estimate of the cost of

repairing has been carefully prepared and submitted.

The lower or navy-yard bridge and the upper or Benning's bridge, over the Anacostia or Eastern Branch, are now both in good condition; the former has been recently renovated, and the latter substantially rebuilt. Neither of them for many years will require any extensive repairs; the flooring alone will have to be replaced from time to time. The large amount of the various commodities for market which pass over each of the several bridges enumerated, and the necessity for which so materially interests the largely increasing population of the city, will surely recommend favorable action upon the appropriations for their prompt and speedy improvements. Major T. Lubey, one of my assistants, has had the immediate supervision of these works; he has always given prompt attention and general satisfaction in carrying out my instructions.

The fence around the botanical garden has been advanced as far as the appropriation will admit; about eleven hundred and thirty feet are finished. The upper part is of iron, and the lower of brick; the latter rests upon a stone foundation, and is covered with a fine North river flag coping. Two additional faces of the grounds have still to be enclosed with a similar fence. The pavement on the north front (the south side of Pennsylvania avenue) should be regraded and repaved, as it is now much below the grade of the avenue. A pavement should also be laid on Maryland avenue, along the south front. These

improvements, which should be of flagging, will greatly add to the general appearance of the grounds. A very beneficial change is being effected by the construction of a culvert through the garden, which converts the exposed bed of the lower portion of Tiber creek into an extensive sewer. This work will not only remove a disagreeable feature from the sight of the many visitors who frequent the conservatories of rich and rare exotics as well as native plants there collected and arranged, (under the special care of Mr. W. R. Smith, for many years the superintendent of the botanical garden,) but will also aid in improving the sanitary condition of a section so nearly contiguous to the grounds surrounding the very Capitol of the nation. It would be well if the same system could be adopted in covering from sight and smell that pestiferous ditch of water styled the "Washington City canal," into which the Tiber empties. This canal will be referred to in a subsequent part of this report. It is to be very much regretted that the culvert cannot be completed this year, owing to the limited means on hand. Of the appropriation for the fiscal year ending the 30th of June, 1867, over seven thousand dollars was applied to other works by the late Commissioner of Public Buildings, and of that for the fiscal year ending June 30, 1868, the whole amount has been expended, without finishing An estimate for enabling me to do so is submitted. Owing to the large extent of land drained by the Tiber, a great accumulation of sand, gravel, and some very deleterious substances are washed down and deposited in the bed of the stream, throughout the length of the culvert, and into the canal. Some action should be taken to arch over this stream from where it crosses Pennsylvania avenue to the extreme northern limits of the city; the same ressons apply that have been given for covering the lower portion. In addition, some system of gravel pits and dams, for collecting the washings of the soil, should be arranged at different points, and the banks protected by sodding or masonry. The management of the Tiber must be treated in the light of a main sewer for a large and growing section of the city, and all improvements projected with that object in view. During the second session of the thirtyninth Congress a bill was introduced in the Senate of the United States to arch

The pavement of flag stones laid along Pennsylvania avenue and Seventeenth street in front of the War and Navy Departments, forms one of the best and most needed improvements in that portion of the city; it has given most general satisfaction, and should be still further extended along the street towards and around the south front of the President's grounds. Several very important renovations have been attended to during the summer at the Executive Mansion. and the building is now in very excellent order. The conservatory, a large part of which was destroyed by fire and the remainder of the framework decayed by the constant moisture of the heating apparatus, has been almost entirely rebuilt; the wing of the main structure requires to be thoroughly overhauled. It is to be regretted that a larger appropriation was not made in order that the old greenhouse might have been removed, and a more ornamental and tasteful one erected. Most of the valuable and rare plants, which so pleased and gratified the tastes of the many visitors from every part of the country, were destroyed by the fire which nearly consumed the building; to replace these, and to make important additions to them, will require an expenditure of several thousand dollars, which no doubt the liberality of Congress will furnish. The furnace of the Executive Mansion has been thoroughly examined and repaired. and some needed changes made in the apparatus for better regulating the distribution of the steam for heating. The roof of the mansion is also being placed in better condition, and many minor matters attended to. A larger appropriation is asked for annual repairs for the next fiscal year, as experience has taught me that the present one is entirely inadequate for so extensive a building.

PUBLIC SQUARES AND RESERVATIONS.

The plan adopted by General Washington for laying out the city of Washington consists of wide streets and avenues intersecting each other; the former, running from north to south, designated by numbers, and from east to west, called by letters, cross each other at right angles, and are again cut diagonally by avenues bearing the names of the different States of the Union. In consequence of this system many public places have been formed, consisting of circles, triangles, and squares; in different parts of the city sections are set apart as reservations for the benefit of citizens and for public buildings. Many of these have already been beautified, and the remaining ones should be improved as early as possible; while adding so much to the appearance of the city, they at the same time largely contribute to the health, pleasure, and recreation of its inhabitants. The grounds immediately about the President's house are in excellent condition, and have been a favorite resort for many. The reservation to the south, and extending to the canal, generally known as the "White Lot"the scene of so many games of ball-should now be laid out and included in the above. As the work on the Treasury extension is drawing to a close, the workshops, which have heretofore interfered with this improvement, can be removed, and the plans of adornment be extended. One of the most charming places for recreation is Lafayette square; it has been made very beautiful, but owing to the ground being so level there is defective drainage; by adopting a system of under-ground drains leading towards the southwest angle, and thence connecting by a sewer through the avenue with that on Seventeenth street, this objection can be remedied. A very marked change has taken place in the appearance of Franklin square, but still more has to be done. During the fall months it is proposed to set out a large number of deciduous trees, to further ornament the beds: a large number of evergreens have already been planted, and are in a flourishing The square has already been under-drained, and the paths have been substantially laid. An iron fence is still needed to properly enclose it. Circle at the intersection of Pennsylvania and New Hampshire avenues and K street west, also presents a most pleasing feature to the gaze. The grounds west of the Capitol are in excellent order, and form one of the great points of attraction both for citizens and strangers; those to the east have been very generally resorted to, large crowds collecting from time to time to listen to the music of the band which weekly performs there. In all of these comfortable lodges have been built for the watchmen. The enlargement of the grounds north and south of the Capitol is being made under the immediate direction of the architect of the Capitol extension; the roadway and paths on the east front should be handsomely paved. A recent and very interesting letter from General Meigs, now travelling in Europe, describes, in great detail, the style of pavement in use for public squares in several of the large cities, and should be adopted in our own capital; a copy of the letter is appended.

Most of the triangular places along the main avenues have been enclosed, and some of them very prettily ornamented with trees and shrubbery: a great deal more in the way of similar improvements should be attended to as soon as practicable, as they not only very much enhance the value of property, but afford for both rich and poor the means of enjoyment, Particular attention is called to Lincoln square, which has been simply enclosed by a paling fence; the beds and paths have still to be laid out, trees and shrubbery planted, and other important changes to be made. It is the only square in the eastern part of the city, and it is due to the increasing growth of that section, that steps should be taken at once to ornament the grounds. The vacant places at the intersections of Pennsylvania avenue with North and South Carolina, those where Georgia and Virginia avenues cross each other, and the square formed by the

meeting of Maryland and Massachusetts avenues, should also be enclosed and improved. These are all east of the Capitol, a hitherto much neglected portion of the city as far as the general government is concerned. Extensive changes have taken place there in the last few months; streets have been opened and graded, gutters paved, curbs set, pavements laid, and many houses are being built.

Passing to that part of the city to the west of the Capitol the following suggestions are offered in regard to future improvements: The two triangular spaces on Vermont avenue between I and K streets have been united and now form one square, making a most agreeable alteration. It is proposed to make a corresponding square on Connecticut avenue between the same streets as soon as the frame buildings of the Freedman's Bureau, now occupying the public space, shall be removed. Circles should also be laid out on Vermont avenue where it intersects Massachusetts and Rhode Island avenues, and also at the intersection of Massachusetts, Connecticut and New Hampshire avenues. The value of these improvements can be scarcely realized. The reservation known as Judiciary square is sadly in need of embellishment; it can only be attended to when the frame buildings used by the medical department of the army are taken down. It is to be hoped that they can soon be dispensed with Several prominent citizens, among others the General commanding the United States army and the mayor of the city, have urgently requested that a new square be opened on New Jersey avenue between H and I streets north. This is a very beautiful and commanding part of the city, and many valuable buildings have been erected, and others are still in course of construction. It is earnestly urged upon Congress to make an appropriation for purchasing the necessary ground. The triangular space located at this point belonged not many years ago to the government, but was sold by authority of one of the previous Commissioners of Public Buildings. In planning the city, a large reservation, known as Mount Vernon place, was laid out at the intersections of K street north with New York and Massachusetts avenues. Most unfortunately for the ornament and health of that part of the city, the original design has not been perfected. Eighth street has not only been opened through it, separating it into two parts, but on one of these divisions has been erected a most unsightly building for a market house. The latter, with its attending annoyances, forms an intolerable nuisance, which should be abated at once. On market days the most offensive matter accumulates in the adjoining streets, greatly detrimental to the health of the residents in the neighborhood. The refuse vegetable matter thrown from the wagons of the hucksters, and the offal from the stall of the butcher, mingle with the filth created by the many animals which are brought and allowed to stand around the place, causing a most disagreeable stench, especially in summer, and thereby engendering sickness. By what authority the market is located on this public reservation cannot be ascertained. It should be removed, and arrangements similar to those in all our large cities be adopted to supply the wants of the community. The grounds could then be improved and become what they were originally intended to be.

A nuisance similar to this has been created by the dilapidated and unsighly buildings on Pennsylvania avenue known as the "Centre Market." This too should be abated; it is not only a reflection upon the good taste of the community that such an old and objectionable structure should meet the gaze upon the principal avenue of the capital, but it is a disgrace to see this main artery, connecting the Capitol with most of the public buildings, obstructed by such a diversified and by no means pleasing collection of commodities as are usually offered for sale on every market day. Cannot a more suitable locality be found and one equally convenient of access? And cannot an ornamental as well as a commodious market be erected which will be a credit to the city? Something

should be done towards improving the public spaces formed by the intersection of Pennsylvania and Louisiana avenues. As long, however, as the market wagons are allowed to occupy them it is useless to attempt any needed reforms. If the corporation does not take the necessary steps to remove the cause of this great nuisance by erecting a suitable and imposing structure, the government should resume control of the reservation for the purpose of improvement. It is still a matter of controversy whether the government has yielded its claim to it, and whether the corporation holds it by any other title or authority than that of actual possession.

But to proceed to the consideration of another of the reservations directly under my charge brings me to the large section immediately south of the canal, and reaching from near the Capitol west to the banks of the Potomac; these grounds are generally denominated "the mall." The extent of this reservation is greater than any other within the city limits, and is centrally located for the establishment of a limited park for a large part of the city. Only one portion of it has been tastefully laid out in accordance with the plan proposed by Mr. Downing in 1851, and approved by Mr. Fillmore, then President of the United States. A great deal of additional work has to be executed before perfecting the system adopted. Unfortunately the grounds are subdivided by several transverse streets running north and south; formerly there were but three of them-Seventh, Twelfth, and Fourteeenth-but lately, by direction of Congress, another, Sixth street, has been opened. Could these streets be made subterranean by tunnelling, the different parts of the mall could be thrown into one, and thereby furnish ample space. As this is not practicable, owing to the level nature of the ground, another plan might be adopted. Let the whole extent of the reservation be laid out in carriage ways, paths for equestrians, and walks for pedestrians, as if the different parts formed a unit; gates with their lodges could be placed where the different walks would pass from one section to another, the crossing of the streets between them being bandsomely paved with flagging. The keepers of these gates, selected from the many dependent and worthy soldiers who have been disabled during the war by loss of an arm or leg, could, by some mechanical arrangement, manœuvre, while seated in their lodges, the gates, opening or closing them for the passer by. The government experimental farm would have to be removed to some more suitable place without the limits of the city, and the various frame buildings, used by the army during the war for hospitals, would have to come down. The basin at the mouth of the Tiber might be partially filled, thereby giving an increased number of acres, and suspension bridges could be built across the canal, uniting the grounds to those south of the presidential mansion. By some such plan beautiful and continuous drives could be had between the latter building and the Capitol, free from all the dust and noise and bustle of the busy streets of the city. The accompanying sketch will illustrate the ideas which I have endeavored to convey. It is to be hoped, too, in the event of such improvements, that action be taken by the association having the matter in charge towards the completion of the monument being raised to the memory of Washington.

Fountains.—In all the contemplated improvements of the public grounds of the Capitol, one very ornamental, as well as sanitary, feature should not be forgotten; the free introduction of water, as jets d'eau, fountains, miniature lakes, into each and all of them, will greatly enhance their charms by refreshing and cooling the heated atmosphere of summer, and by the preservation in all their natural beauty of the various shrubs and trees which adorn them. The abundant supply furnished from the great falls of the Potomac by the Washington aqueduct will be a never-failing source from which to obtain all that may be needed for such purposes. Groups of statuary should be artistically arranged throughout the grounds as another evidence of enlightened taste. Additional

propagating houses should be built to furnish and replace the various plants needed for embellishment.

The Washington canal is a work upon which much has been written, and many suggestions offered in reference to its permanent improvement. All admit that in its present condition it is a great nuisance, extremely disgusting to the senses of both sight and smell. It is nothing more than an open sewer, constantly generating noxious gases, which are most deleterious to those not only residing immediately along its banks, but to the inhabitants of the entire city. Many plans have been proposed for cleaning it, the most impracticable and expensive having generally been adopted. An inspection will show how signally the last project—executed at an expense to the city of some sixty thousand dollars—has failed in accomplishing the much-needed reform. It is the main artery of the sewerage of the largest part of the city, it being the receptacle not only of the excrement and sediment of the sewers, but also of the surface drainage. In addition to this an immense quantity of material is washed into it at every heavy rain by the Tiber. No one can appreciate the large amount of deposits thus formed unless by actual examination. After carefully examining the subject, the following conclusions have been arrived at as the most feasible:

In the first place, the course of the canal should be altered at certain points, so as to remove all sharp angles, and its bed be excavated by dredging to such a depth as to bring the sole at least below the line of low tide of the river. A portion of the width of the canal along the north side should be then enclosed by a substantial wall, and arched over, converting it into a regular sewer. The river at high tide would, to a certain extent, aid in cleaning it; but a more efficient wav would be to make free use of the aqueduct water for thoroughly flushing it at frequent and regular periods. The remaining section of the canal should be kept constantly dredged after the first labor of deepening it is executed; but very little additional work will be annually required. It could then be used for the ordinary purposes of trade, and also become an ornament to the city, instead of remaining the filthy water-course that it now is. As the sewers of all the public buildings empty into it, a proportional part of the expense of placing it in order should be borne by the government, and annual appropriations be made to keep it open and clean. A general system of sewerage should be adopted. In all other cities this subject commands more attention than any other. Whether the government, the corporation, or private enterprise undertake the work, it is absolutely necessary that it should be performed at once, and in the most thorough manner. An improvement, such as is now contemplated, will not only benefit the health of the city, but increase the value of property along it. Instead of the present unsightly structures and heaps of rubbish and dirt, magnificent mansions would spring up, embellished by beautiful gardens. What a great advantage it would be to the mall, the approaches to which are over the canal, the latter extending along its whole length on the north.

Being a member of a board of engineers instructed to examine an improved ship-lock, recently patented, in relation to "its adaptation to aiding in the construction of a ship-canal through the city of Washington," the subject of certain improvements in the present canal will probably be discussed at considerable

length.

In addition to the public grounds, the officer in charge has the care and improvement of all the avenues, twenty-one in number, and certain of the streets passing through or adjoining them. Many of these have been opened, and it is respectfully recommended that authority be given to improve the remaining ones. They form the direct line of communication between different sections of the city, and between many of the public buildings and grounds. In justice to property holders along them, and for the public convenience, the grades should

be established as soon as practicable, that each may know the condition of his property before planning any new improvements; the roadways should be placed in good travelling condition. Massachusetts, New Hampshire, Connecticut, New Jersey, Vermont, Delaware, New York, and Maryland avenues should be graded. But two appropriations were made during the second session of the thirty-ninth Congress for this purpose. That "for grading and repairing Virginia avenue" has been applied to the repairs lying between Sixth and Ninth streets east. By an arrangement made with General O.O. Howard, Commissioner of the Bureau of Freedmen, the work has been extended; the grading and gravelling has been completed to Tenth street, and the grading as far as Twelfth street. Part of the following square has also been graded, the necessary excavation furnishing a very superior gravel to form the superstructure of other portions of the avenue. When the appropriation was nearly exhausted General Howard offered to furnish a certain amount to enable me to employ laborers to continue the work, paying the men out of funds donated for the support and relief of indigent freedmen, and intrusted to him for distribution; the balance of appropriation remaining on hand was then applied to the hire of carts. This work has been most substantially done, and very creditably to Captain T. A. Stone, who has had the immediate superintendence of it. The advantage of opening and improving this avenue is already experienced; a large amount of the travel crossing the lower bridge, over the Eastern Branch, is now diverted from Pennsylvania avenue, east of the Capitol, finding other ways of approach to the markets of the city. Forming with Massachusetts and Pennsylvania avenues parallel roads throughout its entire extent, between the Eastern Branch and Georgetown, the improvements should be continued, and an estimate has been submitted for that purpose.

The oldest and greatest thoroughfare from one limit of the Capitol to the opposite one, connecting the most important public buildings and grounds, is Pennsylvania avenue; a glance at the map shows that it is the most central. In consideration of the great importance of this avenue, the relative position it bears to all the avenues and streets, which either diverge from it in every direction or intersect it along its entire length, some most marked improvements should be inaugurated to render it as great and beautiful in appearance as it has proved to be necessary and accessible. The very limited appropriation has enabled me to make only such repairs as were most needed; some sections had become almost impassable, either from the effects of the weather, or from having been cut up by the immense amount of travel over them, or from both causes combined. It is now only in tolerable order, although it is said to be in much better condition than for many years past. The seat of government of a nation should be able to boast of at least one magnificent avenue; that part extending from Rock creek, its boundary on the west, to that splendid pile of architecture, the Capitol, should receive the advantages of the most improved road-way. There is scarcely a street or avenue in the city over which one can drive with ease and comfort; it is only lately that the Belgian pavement has been laid to any extent, and the agreeable contrast experienced between it and that of old-fashioned cobble-stone cannot be but appreciated by all. The numerous deep gutters, which cross the streets of Washington in every block, cause constant wear and tear to both horse and vehicle; it is to be hoped that at no very distant day the drainage of the entire city will be underground, and that a more even surface for driving may be obtained. The streets of a city are public property, in which all citizens are more or less interested, and in point of necessity no other public work can equal them; why, then, is not more attention paid to paving and ornamenting them? "The paving of streets is of early date, and is in fact necessary to any considerable degree of civilization and traffic. The Romans paved their streets in the same elaborate and solid manner in which they paved their highways." Portions of the ancient pavement of the streets of that city are in use

at the present day, and that of Pompeii remains entire. In one of the pamphlets on the subject of pavements it is well remarked that, "considering the present development of the arts and sciences, there is no reason why the city streets should not be as agreeable for walking and pleasure riding as the roads in the Central Park; and, at the same time, be so substantially paved as to meet all other requirements." In consideration of all these facts, and the great importance of having at least one good pavement traversing the city, the recommendation s made that the present cobble-stone pavement be removed, and that Pennsylvania avenue, west of the Capitol, be relaid with either one or the other of the improved pavements now coming into general use. If stone is to be used, the Belgian pavement possesses the greatest advantage; but should one of wood be preferred, which is delightful, easy and not noisy, either the Nicholson, Ream, Fayette, or Stafford will prove to be as near perfection as it is possible to construct them. Then again the Asphaltum, the Concrete, and other similar pavements have their advantages in certain climates. Each one of those named has its advocates, and their relative merits, in both a practical and economical view, should be severely tested by competitive comparisons, made under the same and equal trials, both as regards climate and use. Pamphlets have been printed describing the benefits of each, and all can find an opportunity of testing them by actual experience. It is unnecessary to describe them in this report. The length of that part of the avenue which it is proposed to improve is over two and a half miles, and averaging in superficial measurement about one hundred and ninety-six thousand square yards. The cobble-stone pavement to be removed can be used on other and less frequented avenues.

Although the nation at large is deeply interested in the general improvement of the metropolis of the republic, and especially in the accomplishment of such a great and much needed work, still the property-holders along the avenue, as well as the citizens generally, are mostly benefited by it; the expense of constructing an improved pavement might, therefore, be equitably proportioned between the municipal and general government. In addition to laying a good carriage way, other advantageous changes might be introduced for the beautifying of this avenue. The great widths of the avenues and streets in Washington offer many reasons, both in an ornamental and in an economical consideration of the matter, for imitating the taste and utility displayed in some of the larger cities of Europe. The "Unter-der-Linden," as described in the letter from General Meigs, previously referred to, "is a street of great celebrity in Berliu, and the people are still praising the electors who laid it out two hundred years ago. It is the principal street of a city of six hundred thousand inhabitants; upon it are the royal palaces, those of most of the princes, the principal shops and hotels. It has a wide gravel walk in the centre, four rows of trees which give shade, wide sidewalks next the houses, and yet it is never encumbered. central walk is sometimes filled in the evening by citizens and strangers enjoying the long summer twilight of this northern latitude, in which darkness does not come on until 10 p. m." The general gives a sketch of it, and hopes that Pennsylvania avenue may yet be arranged like it. The following are the dimensions of the different parts into which it is divided: First, foot-path, paved, adjoining houses, 15 feet; second, carriage-way, paved, 33 feet; third, line of stone poets, 3 feet; fourth, row of trees; fifth, carriage-way and equestrian-way, paved, 24 feet; sixth, row of trees; seventh, promenade, gravelled, 60 feet; eighth, row of trees; ninth, ride, gravelled, 24 feet; tenth, row of trees; eleventh, line of stone posts, 3 feet; twelfth, carriage-way, paved, 33 feet: thirteenth, foot-path, paved, 15 feet. These measurements give the width of the Unter-der-Linden at two hundred feet; Pennsylvania avenue is one hundred and sixty. The trees are not very large, having perished in the occupation of the city by hostile armies, and repeatedly renewed. If the thirty-three feet carriage-way happens to be

full or obstructed, carriages take the twenty-four feet lines between the trees; but these twenty-four feet lines are ordinarily used only by equestrians and by parties who drag their wagons assisted by their dogs. One of them, that on the north side of the promenade, is gravelled, to be used as a summer road. This street is the resort for business and recreation of all Berlin, and of all strangers.

Is it not possible, in view of any contemplated improvement of Pennsylvania avenue, to adopt some of the plans proposed for ornamenting this prominent thoroughfare, and relieving it from its present unfinished appearance? Besides the improvements already suggested, there is another which should receive prompt action. From an examination which was made by my direction, Mr. Theo. B. Samo, engineer of the Washington aqueduct, reports that "between First street west and Fifteenth street west, on Pennsylvania avenue, there are fifteen fire-plugs; thirteen are similar in construction to those known as 'New York plugs; 'each one is connected with a pipe, designated as the 'four-inch' or 'spring-pipe.' The other two are Philadelphia plugs, and are connected with the twelve-inch government mains." "The four-inch pipe is an old pipe nearly worn out, and is connected with the twelve-inch main only at Third street west and Thirteen-and-a-half street west. Not more than two plugs can be supplied by it at once, owing to its small diameter and the distance between its connections." It is respectfully suggested that the New York plugs, which are too small and have been constantly out of repair, should be replaced by those of more improved make and facilities; a new six-inch pipe should be laid in the place of the four-inch, and more frequent connections made with the twelve-inch main, in order that a sufficiency of water may be had in case of fires. At present, the steam engines have to obtain their supply from other streets. The cost of replacing the old plugs is comparatively small, considering the great amount of interest at stake, and an estimate is herewith submitted.

The relation of the general government to the city of Washington is very ably set forth in two very interesting pamphlets, the one addressed by the mayor of the city, in November, 1865, to the Hon. James Harlan, then Secretary of the Interior, and the other is a copy of the remarks which originally appeared in the editorial columns of the Union. The fifteenth section of an act of Congress approved the 15th day of May, 1820, "incorporating the inhabitants of the city of Washington," and the third section of an act approved May 5, 1864, amending the previous one, directs that "in all cases in which the streets, avenues, or alleys of the said city pass through or by any property of the United States, the Commissioner of Public Buildings shall pay to the duly authorized officer of the corporation the just proportion of the expense incurred in improving such avenue, street, or alley which said property bears to the whole cost thereof, to be ascertained in the same manner as the same is apportioned among the individual proprietors of the property improved thereby."
Under the authority of these acts a large sum is due for the many improvements which have been completed, or are in course of construction, by order of the corporation; the late Commissioner of Public Buildings presented claims amounting to more than ninety thousand dollars, for which an appropriation has never been made; and only recently the mayor has officially informed me of the cost of additional works throughout the city; the latter have been personally inspected and approved. As it is contemplated to continue these improvements during the coming year, it is important that provision be made in advance to have ready in the hands of the officer in charge sufficient funds to pay the government proportionate part. Unless this is done, the advance of improvement is obstructed, and great suffering is sometimes caused by the inability to pay the wages of the laborer. Instead of appropriating money for the immediate care of the indigent and poor, it would perhaps be better policy to inaugurate improvements so as to enable them to obtain work and gain their own support.

In the hands of honest officers this plan will work well; public works should be, in more senses than one, public benefactors. Before concluding the brief suggestions offered in reference to the improvement of the different avenues, especial attention is called to Boundary street, towards which many of them lead; this street forms a connecting link between them, and at the same time skirts a greater part of the limits of the city. It is capable of great embellishment; by increasing the width and planting along it rows of shade trees, it will become a most delightful and much-frequented drive.

Many nuisances have been abated throughout the city by authority of the acts of Congress referred to, for the payment of which several public lots have

been offered for sale, and some small appropriations are desired.

During the last winter an act passed to increase the supply of water in the Capitol building. For accomplishing this object in a proper manner the sum appropriated was not sufficient. By means, however, of certain funds at the disposal of the architect in charge of the extension, and which are applicable for the purpose, the work is progressing, and will be completed before the assembling of Congress. Owing to the great quantity of water daily used in the machine-shops of the navy yard, supplied only by the twelve-inch main many complaints have been made of the great scarcity, during working hours throughout the eastern section of the city. This can be easily remedied by connecting the twenty-inch main, now being laid in North B street for the benefit of the Capitol, with the twelve-inch main on First street east. An additional appropriation is asked for this much needed improvement. To meet the demands of the constantly increasing growth of the northern part of the city, another main will soon have to be brought into that section from the distributing reservoir near Drover's Rest. In his last report the mayor calls attention to the necessity of introducing an additional supply of water to meet this increased want.

In addition to the duties appertaining to the office of the late Commissioner of Public Buildings, the superintendence of the Washington aqueduct also devolved upon me. This most important work has for several years been under the general control of the Department of the Interior, Mr. Theodore B. Samo, the engineer in charge, having had for the last two years the immediate direction of all engineering operations. His very coucise report for the year ending September 30 furnishes in detail the progress of the several supple mental works in course of construction, and also makes such recommendations as are necessary for the thorough completion of the entire aqueduct. It is very gratifying to have it in my power to report the very able and conscientious manner in which he has discharged his duties, and, after very careful inspections of the entire work, to coincide with him in the views entertained in relation to its progress, and to approve of the estimates submitted for the completion of many important parts upon which labor has been suspended for want of the necessary appropriations. Certain tracts of land are occupied by the government for aqueduct purposes, for which, in some cases, only nominal rents are paid; in others no rents have been paid at all, and claims are now being made by the owners to become either repossessed of their property, or to be paid for its use. As the United States must retain the few acres for the benefit of this national work, it is earnestly urged that authority be given to purchase them. But a few thousand dollars is needed to meet all claims and to purchase the The great importance of introducing into the capital an unlimited supply of pure and wholesome water cannot be overestimated. The large amount of valuable public property concentrated in the city, amounting in the aggregate to millions of dollars; the immense accumulation of important government archives, many of them stored away in buildings which are not fire-proof; the large number of public buildings, will certainly show the necessity for the

Washington aqueduct, and that no expense should be spared in completing it. The water thus supplied has become a great motive power at the different government works throughout the city, and should the capital of the nation become what every enlightened citizen should desire to see it, a still larger demand will be made for both useful and ornamental purposes. a member of a board of officers to select a site and prepare plans and estimates for a new War Department building, it was ascertained that this department alone occupied a great many buildings, and, with the exception of one, all are unsafe and not constructed for protection against fire; it will require a very large fire-proof building to preserve free from accident its valuable archives. In the safety and preservation of these almost every family in the land is inter-

The repairs of Fort Foote, on the Potomac, which are progressing under the mmediate direction of Mr. S. T. Abert, civil engineer, were also placed under my temporary superintendence. One of the most charitable and disinterested appropriations which the officer in charge of public buildings is called upon to disburse is that for the care of such transient paupers as are in need of medical advice and treatment; a home is also furnished, where proper care and nursing, and suitable nourishment, can be given at all times. Arrangements have been made with Providence Hospital to admit a limited number of patients; although at times the quota is exceeded, still they are always kindly received and the best of attention given them. To know and to appreciate that such is the case it needs only to be mentioned that the hospital is under the efficient control of the Sisters of Charity. Before concluding this already extended report it is respectfully recommended that additional clerical assistance be allowed in this office, and that the number of assistants and laborers under the public gardener be increased; the former should be excellent accountants, and the latter men who thoroughly understand the care and beautifying of the public squares.

As an inducement to obtain experienced and reliable persons compensation commensurate with their services should be afforded them; the present pay as authorized by law is not deemed sufficient, and it is earnestly urged that an increase may be granted. Major James Nokes, the public gardener, and his assistants and laborers, have faithfully performed the labors required of them in the care of the different squares and reservations, the appearance of which will also testify to the skill displayed by them. The assistants in the different offices connected with public buildings, grounds, and walks all deserve great credit for the manner in which they have severally performed their duties.

Most especially to my general superintendent, Mr. B. F. Burns, much praise is due for the efficient, faithful, and competent manner in which he has at all times carried out my instructions.

I am, general, very respectfully, your obedient servant,

N. MICHLER.

Major of Engineers, Brevet Brig. Gen. U. S. A., in charge.

Major General A. A. HUMPHREYS,

Chief of the Corps of Engineers, U. S. A.

APPENDIX T-1.

REPORT IN RELATION TO "PUBLIC PARK AND SITE OF PRESIDENTIAL MANSION," APPENDED TO ANNUAL REPORT DATED OCTOBER 1, 1857, OF BREVET BRIGADIER GENERAL N. MICHLER, IN CHARGE OF PUBLIC BUILDINGS, GROUNDS, AND WORKS, WITH ACCOMPANYING SKETCHES.

Communication of N. Michler, major of engineers, to the chairman of the Committee on Public Buildings and Grounds, relative to a suitable site for a public park and presidential mansion, submitted to accompany the bill (S. 549) for the establishment and maintenance of a public park in the District of Columbia. February 13.

WASHINGTON CITY, January 29, 1867.

SIR: In compliance with the contents of your letters of the 24th and 26th of July, 1866, addressed to the honorable Secretary of War, I was detailed by the Chief of Engineers, with the consent of the General-in-chief, to carry out the views of the committee in regard to the special duty assigned me. In the letters referred to you requested that an engineer officer be detailed to make the necessary preliminary surveys and maps of certain tracts of land adjoining or near this city, for the purposes of a public park, and also a suitable site for a presidential mansion, and which, in the language of the Senate resolution of the 18th of the same month, "shall combine convenience of access and healthfulness, good water, and capability of adornment;" in addition to this to ascertain, if

practicable, the price of said lands.

After a careful examination of the many beautiful localities to be found in the vicinity of the capital, and having caused an accurate and detailed survey of its environs to be made, I now have the honor to submit for your consideration the conclusions to which I have arrived. In connection with this report two preliminary maps have been prepared, which will show more plainly than words can express the required information and the respective advantages of the different sections. The Senate resolution would seem to imply that one and the same tract of land should be designated for a site for grounds for a presidential mansion as well as for a public park; but as it is not definitely so stated, it has been judged best by me to separate the subjects. Should such not be the intention of your honorable committee it will be easy to combine the two, where so many splendid situations present themselves from which to make As it is designed to build a home for the President to which he can retire from the active cares and business of his high office, and where he can secure that ease, comfort, and seclusion so necessary to a statesman, it would seem best to locate it away from the constant turmoil of a city life, at such a distance where his privacy cannot easily be intruded upon, and still sufficiently accessible for all practical purposes.

In the first place let me consider the subject of a public park. Where so much has been written on so interesting a feature to any large city as that of a park, and where the necessity of public grounds, either for the sake of healthful recreation and exercise for all classes of society, or for the gratification of their tastes, whether for pleasure or curiosity, has become apparent to every enlightened community, it would seem to be unnecessary for me to dilate further upon the matter, to say nothing of the natural or artificial beauties which adorn a park, and so cultivate an appreciative and refined taste in those who seek its shades for the purpose of breathing the free air of Heaven and admiring nature. It certainly is the most economical and practical means of providing all, old and young, rich and poor, with that greatest of all needs, healthy exercise in the

open country.

To accomplish these ends there should be a spaciousness in the extent of the

grounds, not merely presenting the appearance of a large domain, but in reality possessing many miles of drives and rides and walks, all independent of each other, and either open or protected so as to be suitable for the different seasons. There should be a variety of scenery, a happy combination of the beautiful and picturesque—the smooth plateau and the gently undulating glade vying with the ruggedness of the rocky ravine and the fertile valley, the thickly mantled primeval forest contrasting with the green lawn, grand old trees with flowering Wild, bold, rapid streams. coursing their way along the entire length and breadth of such a scene would not only lend enchantment to the view, but add to the capabilities of adornment. While nature lavishly offers a succession of falls, cascades, and rapids to greet the eye, as the waters dash through some romantic dale, the hand of art can be used to transform them into ponds and lakes as they gently glide through the more peaceful valleys, thereby rendering them the means of pleasure and recreation for boating or skating. What so useful as an abundance of water, or so ornamental when converted into fountains and jets to cool the heated atmosphere? It furnishes, also, opportunities for the engineer and artist to display their taste in constructing ornamental and rustic bridges to span the stream.

An attempt has been made in a few words to describe the purposes and beauties of a public park. In no place has nature been more bountiful of her charms than in the vicinity of this city, and all can be found so near and accessible; the valley of the Rock creek and its tributaries, the Broad and Piney branches and the several minor rivulets, with the adjoining hills overlooking these beautiful streams, present the capital of the nation advantages not to be lightly disregarded in providing a park worthy a great people. All the elements which constitute a public resort of the kind can be found in this wild and romantic tract of country. With its charming drives and walks, its hills and dales, its pleasant valleys and deep ravines, its primeval forests and cultivated fields, its running waters, its rocks clothed with rich fern and mosses, its repose and tranquillity, its light and shade, its ever-varying shrubbery, its beautiful and extensive views, the locality is already possessed of all the features necessary for the object in view. There you can find nature diversified in almost every hue and form, needing but the taste of the artist and the skill of the engineer to enhance its beauty and usefulness; gentle pruning, and removing what may be distasteful, improving the roads and paths, and the construction of new ones, and increasing the already large growth of trees and shrubs, deciduous and evergreen, by adding to them those of other climes and countries. of the various trees and shrubs, and vines and creepers, to be found already flourishing in the region described, and also the nature of the soil, will be appended to this report. A glance at the map will show the topographical features of the country, and its accessibility to both Washington and Georgetown.

The valley of Rock creek occupies a central position to both, as it lies between the Tennallytown road on the west, one of the most prominent thoroughfares leading out of the one city, and the Fourteenth street road and Seventh street turnpike on the east, two of the finest communications running in a northerly direction from the other. From these main highways many branches cross the valleys or follow along the banks of the stream; these transverse roads already form beautiful drives. Rock creek winds along for more than four miles through the centre of the proposed grounds, receiving at convenient points the waters of the Broad and Piney branches, and several smaller tributaries. For a short distance it courses through a narrow but beautiful valley, then wildly dashes for a mile over a succession of falls and rapids, with a descent of some eighty feet, the banks on both sides being bold, rocky, and picturesque; then passes again through narrow valleys or between high, bluff banks. At many points the creek is capable of being dammed, thus forming a series of lakes and ponds for useful and ornamental purposes. The many deep ravines setting in towards

it can furnish romantic walks and quiet retreats for the pedestrian. The larger part of the ground is thickly wooded, and capable of great adornment. Here we find the several varieties of oak, the beech, the locust, the mulberry, the hickory, the sassafras, the persimmon, the dogwood, the pine, with a great many shrubs, vines, and creepers, growing, climbing, and trailing throughout the woods. Beautiful vistas, artistically arranged, can be cut through them, exhibiting distant points of landscape, while charming promenades can invite the wanderer to seek cooling shades. Nature has been so rich in her vegetable creation that the plan of transplanting trees of large growth, which has been adopted in most of the modern parks, will be unnecessary. There are some few country seats, such as Blagden's, Pearce's, and Walbridge's, which have been highly cultivated; should it be found desirable to erect the presidential mansion within the enclosure of the park, the first mentioned site possesses many advantages, both ornamental and valuable. Here and there some prominent point offers commanding views of the surrounding country, where observatories can be located, conservatories built for exotic plants, and geometrical flower-gardens planted. Back from the stream some level plateaus extend, which can be appropriately employed for zoological and botanical gardens, grounds for play and parade, and many other useful purposes.

The map shows the most desirable localities, the surveys having been made in great detail. The lay of the land is such that it admits of thorough drainage, and the nature of the soil offers all the facilities for building good roads; the granite and limestone rocks which are found outcropping at different points will furnish the materials for their superstructure, In fact, every facility is offered

for laying out and constructing a grand national park.

The questions now arise as to what should be the extent of the proposed work, and the probable price of the land. As it should be one worthy the capital of the nation, and as the ground can be secured at a reasonable price before being occupied by costly suburban villas, it is respectfully recommended to the honorable committee to purchase at once a sufficient number of acres bordering on Rock creek to anticipate the future growth of the city and its increased population. With the view of retaining as much of the pictures que scenery along the stream. and of also embracing the sites of some few of the forts on the north constructed for defence of the city, which have become historical, and from the parapets of which extensive views can be had, I have marked on the maps such lines as may be satisfactorily taken as approximate bounds of the park. In case my recommendations should be considered too extravagant, I have caused a second series of lines to be drawn for grounds of more moderate dimensions. The first tract would contain about two thousand five hundred and forty acres, more or less; and the second, one thousand eight hundred, more or less. As there is so much difference of opinion as to the price of the land, the quality and improvements varying so much, it is a difficult matter to offer anything more than an approximate appraisement. As the right of eminent domain empowers the government to take property, and as such power is the necessary incident to sovereignty, the question would finally have to be settled by a commission appointed by some competent court. The price ranges from \$50 to \$1,000 per acre; a mean of \$200 should amply cover the entire cost. It will be noticed that the southern limits, as drawn, of the proposed park, do not approach more closely than necessary the city limits, leaving out where possible such sites as would greatly enhance the cost. Avenues leading along Rock creek to the southern limits of the park should be opened. According to the above figures the larger tract would amount to \$508,000, and the smaller one to \$360,000. As the work of constructing a park will consume many years, no longer delay than is absolutely necessary should be consumed in the preliminary arrangements for the passage of the necessary laws and the purchase of the lands.

The dimensions of the most celebrated European parks are as follows:

London.—All parks in and near London, including gardens, squares, and parade grounds, 6,000 acres. Hyde Park, 380 acres; Regent's Park, 372 acres; Windsor Little Park, 300 acres; Kensington, 227 acres; Windsor Great Park, 3,500 acres; Richmond Park, 2,250 acres.

Dublin.—Phœnix Park, about 2,000 acres.

Garden at Versailles, 3,000 acres; Bois de Boulogne, 2,158 acres; Munich, Englischer Garten, about 500 acres; Vienna, Prater, 1,500 acres; Birkenhead

Park, near Liverpool, 180 acres.

The Central Park of New York, the most important work of the kind undertaken in America, is over two and a half miles in length by one-half mile in breadth, and contains over eight hundred and forty acres. There are about nine and a half miles of drives, five and a half miles of bridle road, and some twenty-five miles of walks. The annual sum provided for the expense of maintaining it, to wit, \$150,000, is reported to be insufficient. The number of visitors continue to increase with each year; in 1865, 7,593,139 persons entered. Hunting Course Park, near Philadelphia, and the Druid Park, near Baltimore, have also been constructed. The establishment of parks is exciting great attention throughout the land, and adds vastly to the enjoyment of the people.

I would now call the attention of your honorable committee to the remaining subject of this report—the selection of a site for a presidential mansion. In the memoranda submitted to the Secretary of War by letter of July 26, 1866, you requested "that the ground known as Meridian Hill," and "the estate of the late Washington Berry," should be particularly examined, as they are thought to contain all the requisite advantages for such a site; "also such other localities as may, in the judgment of the engineer," present eligible positions for such a purpose. In compliance with your wish I made special reconnaissances and surveys of the above-named places, as well as some others, which offer great

inducements, and will now discuss each separately.

I. Meridian Hill—(Colonel Messmore's estate.)—This site is located due north from the present White House, on the first range of hills bounding the limits of the city of Washington. It is of easy access, several avenues and streets leading in that direction. On the east it adjoins the lands of Columbia College, and on the west those of Mr. Little. The number of acres contained in this tract is one hundred and twenty, which added to that of Mr. Little's thirtyeight acres gives a total of one hundred and fifty-eight. The latter offered to sell to the government at about six cents a square foot, or \$2,613 per acre. these estates are eligible building sites; the view towards the south, overlooking the city and the valley of the Potomac, being particularly fine. At one time some large forest trees added beauty to the scene, but most of them were destroyed during the war. There are no improvements, the old mansion-house having been destroyed by fire, and the walls are alone standing. North of the site the land is nearly level, only slightly undulating. Although possessed of considerable advantages, there are several objections to this selection, in connection with the object in view. Lying just above the plateau of the city, and not screened by any belt of timber, it is exposed to the miasmatic influences rising from the marshes of the Potomac. Again, it is too near the city to afford any retirement and repose for the Chief Magistratc. Already the street railroads approach, and numerous houses are being built on all sides of this site.

II. Metropolis View—(Homestead of the late Washington Berry.)—This estate lies northeast of the Capitol, between the old Bladensburg road and Lincoln avenue, the latter a continuation of North Capitol street. It is distant from the Capitol about two and a half miles, and from the White House about three miles. It contains some three hundred and fifty acres, valued by the trustees at \$500 an acre, with the improvements, including a very fine spring; the whole is offered at \$200,000. To the east of it lies Brentwood, the fine estate of Mrs.

Pearson, and to the west Glenwood Cemetery, and Harewood, the beautiful grounds of Mr. Corcoran. In front spreads out Eckington, so many years the homestead of the late Mr. Gales, which contains 130 acres; the price of this land is placed at \$1,000 per acre, with \$25,000 additional for improvements. Metropolis View is beautifully situated, having a high and commanding position; it is partially covered with groves of fine old trees, deciduous and evergreens, and possessed of an abundance of timber. A fine spring rises in the place, and two small streams, tributaries of the Tiber, course through it. In nearly every direction the eye meets with charming landscape scenes, and it overlooks the Capitol and the broad valley of the Potomac. This locality possesses many attractions, and is susceptible of great improvement. It is easy of access by some of the finest avenues and streets leading out of the city, and is at a very convenient distance from the most prominent public buildings.

Eckington is a delightful place, but it is not sufficiently high to afford any extensive views. It should, however, be purchased in addition to the Berry estate, should Metropolis View be selected as the site. The two tracts of land united would furnish ample grounds to surround the mansion, and also open a fine park to connect with the city on the direct line with the Capitol. The sum total of the valuations of both estates amounts to \$355,000. In regard to the healthfulness of this locality, the opinions of those with whom I have consulted differ very materially. Some think that the miasma carried up the valley of the Tiber from the Eastern branch is very deleterious to health, while others, who have long inhabited these old homesteads, pronounce them to be perfectly

salubrious.

III. Harewood—(Mr. Corcoran's estate.)—Among the many delightful drives around the city of Washington none can compare with those to be found within the enclosures of this delightful retreat. The grounds are most artistically arranged, and no expense has been spared in adorning them by all the appliances

at the command of taste and wealth.

The grounds are naturally beautiful and undulating, and all that skill can accomplish has been applied to render them most charming and picturesque. In addition to the natural growth of vegetation, many trees and plants of other nations and climes have been introduced to impart their luxuriance to the scene. The estate covers some two hundred acres, but as you follow the gentle windings of the drives and walks, the imagination is led to believe it to be of much greater extent. Good roads lead to it from the city, making it perfectly accessible. A fine spring furnishes a plentiful supply of water, and in point of health it is all that can be desired. This spot, originally selected by the proprietor upon which to erect a princely mansion, is one of the most delightful situations among the many fine ones in the environs of Washington; it would be a most eligible site

for a presidential mansion.

IV. The homestead of Mr. Moncure Robinson.—This estate is now occupied by a brother of the proprietor, the latter residing in the city of Philadelphia. It lies adjoining the lands belonging to the United States Military Asylum; the road which leads out of the city on the prolongation of North Capitol street and passes near the Home for the old soldiers, almost divides the place into equal parts. There are about seventy acres in all, which the owner proposes to sell at \$1,000 per acre, without the improvements; these he values at \$30,000. The mansion occupies one of the most elevated positions in the neighborhood of Washington. An extensive panorama of the surrounding country lies before the beholder; from every point of the compass the eye can dwell upon magnificent landscapes extending far into Maryland and Virginia, and combining all that is beautiful and picturesque. In one direction the gaze rests for miles on the waters of the majestic Potomac, and in another there are mountains and hills mantled with forests, and plains and valleys highly cultivated. The place con-

tains a large portion of heavy timber, and is so situated as to offer numerous advantages for improvement. From its great height it will be far above all malarious influences. There are fine springs in the neighborhood, which furnish an abundance of water for useful and ornamental purposes. The locality is convenient to both cities. Through Washington several avenues and streets lead towards the road above referred to as connecting with North Capitol street; by this drive a straight-line communication can be had with the Capitol, the distance between the two being less than four miles. By the avenues and streets connecting with the Fourteenth street road and Seventh street turnpike, thence by Rock Creek Church road, a very direct drive of a little over four miles can be had with the White House and the public buildings adjoining it. From Georgetown almost an air-line can be had via Broundary street, Taylor's lane, and Rock Creek Church road, distance of about four miles.

Directly in front, or south of Mr. Robinson's beautiful locality, lie the very pretty grounds of Mrs. R. S. Wood, consisting of forty acres. The two must be considered inseparable should the Robinson site be selected for a presidential mansion. They are valued at about \$1,000 per acre, not including the improvements. The two places can probably be purchased for \$150,000. Mrs. Wood's tract joins Harewood on the south, and on the west that of the Military Asylum. The lands of the latter do not belong to the government, but are in trust for the old soldiers, and contain some two hundred and fifty-eight acres. A reference to the map will show the honorable committee the peculiarly attractive features, both of position and general convenience of access, offered by the locality described above, containing in all about one hundred and fourteen acres, sufficient for the necessary purposes of embellishment and utility; and lying contiguous to the already ornamented grounds of Harewood and the Military Asylum, enjoying all the charms and advantages of those delightful places, it would be difficult indeed to find a spot more admirably adapted as a retired, pleasant home for the President of the United States.

A table of distances from the Capitol and Executive Mansion to prominent

points of interest is also added for the information of the committee.

Table of distances.

	From Capitol.	From Executive Mansion.
To Mrs. Hobbie's, (southern limit of proposed park)	4# 41 3 4 5 3 2 3 3 3 3 3	Miles. 2 3 4 3 2 5 1 3 4 3 4 3 4 3 4
Entrance to Harewood	3 4	3 1 3 1 3 <u>1</u>

In concluding this report I would respectfully suggest to your honorable committee the necessity of commencing the construction of the national park as soon as practicable. It is a grand and beautiful undertaking, and should be prosecuted with the greatest energy. A sufficient appropriation for enclosing the grounds purchased, for improving and keeping in repair the drives and walks already constructed, and for the laying out of others, should be made. For this purpose one hundred thousand dollars would be sufficient for present expenditures.

I am, sir, very respectfully, your obedient servant,

N. MICHLER,

Major of Engineers, Brevet Brigadier General U. S. A.

Hon. B. GRATZ BROWN.

Chairman of the Committee on Public Buildings and Grounds, United States Senate.

Remarks on the vegetation of the District of Columbia, by Dr. Arthur Schottof Georgetown; appended to the report of General N. Michler.

In order to specify the vegetable growth of the District of Columbia, in relation to the topographical features of the same, the subjoined lists of plants are suggestively proposed, as they principally tend to give shape to the general appearance of the landscape. As the geological aspect of the area in question presents considerable variety in its details, it is thought proper to bring the numerous floral types, not less varied in size, shape, and individual development, under separate heads, corresponding to their particular habits. For the sake of facilitating their survey, the various species are here arranged in the following order, commencing with the trees, larger and smaller, and these followed by shrubs, vines, creepers, and the undergrowth of herbs and weeds. The same disposition is observed in mentioning their topographical distribution, commencing on the hill tops, and then gradually descending to the low grounds, river banks, marshes, and bogs. As to the selection of the species hereafter named, there is no hesitation in giving the trees, shrubbery, and vines pretty much in full, as all these forms by their size make themselves more prominent, while with plants, herbs, and weeds, either their rich floral, or more foliaceous development, or clustering mode of growth, invite special notice. Commencing with the more elevated portions of the District, particularly towards its western limits, the following are to be found:

White oak: Large, lofty; rich woods.

Post oak: Rough or post white oak; less tall; sandy soil.

Red oak: Good sized; rocky woods.

American beech: Large and very ornamental; rich woods on hill slopes bordering on water courses.

Hop hornbeam: Large and handsome; rich woods. Mockernut hickory: Large and graceful; rich woods.

Shell-bark or shag-bark hickory: Tall, handsome; rich woods.

Thick shell-bark hickory: Good size, with the former. Poplar tulip tree: One of the loftiest forms; rich woods. Honey locust: Good sized, very ornamental; rich woods.

Spanish oak: Large or small; rich woods.

Quercitron or black oak: Large tree; dry woods.

Common persimmon: Middle sized, but handsome; sporadically all through.

Sassafras: Fifteen to fifty feet high; rich woods.

Red mulberry: Middle sized; rich woods.

Red bird: Small tree; hill slopes.

Tupelo, black or sour gum tree: Middle sized; hill-sides.

Locust tree: Tall, ornamental; borders of woods, hill and road sides.

Staghorn sumac: Twenty to thirty feet high; open hill-sides.

Smooth sumac: Of lesser size, on rocky, barren soil.

Box elder: Small, handsome tree; hill-sides; borders of thickets descending to river banks.

Flowering dogwood: Small tree; in rocky places.

Jersey or scrub pine: Fifteen to twenty-five feet high; on barren and sterile hills. Red cedar: Fifteen to thirty feet high; dry, rocky, or sterile hill-sides.

Japan paper mulberry: Small tree or shrub; open hill and road-side; pretty well naturalized.

Panicled dogwood: Thickets and hill-sides; shrub four to eight feet high. Black haw, shoe-leaved arrow-wood: Treelike shrub; shady hill-sides.

Maple-leaved arrow-wood: Three to five feet high; rocky woods.

Spice bush: Five to ten feet high; damp woods.

Poison oak; poison ivy: Climbing or trailing everywhere.

Bladdernut: Ten feet high; moist thickets.

Burning bush: Sporadically everywhere through the woods; tall and upright shrub.

Summer grape: Thickets.

Virginia creeper: One of the chinquopin; six to twenty feet high; sandy woods; boldest climbers; in the woods generally.

Maryland andromeda: Sandy woods; very ornamental.

Deerberry, squaw huckleberry: Dry woods and hill-sides; neat and graceful

Common swamp blueberry: Four to five feet high; moist, shady places and copses.

Purple Pinxter flower: All through the woods.

Indian hemp: Three to five feet high; valleys and hill-sides.

Woodbine: Trailing; rocky woods. Trumpet honeysuckle: Woodlands.

Of the undergrowth, herbage, and weeds, the following may be mentioned:

Creeping wintergreen: Damp woods, in the shade of evergreens.

Ground laurels: Sandy woods and rocky soil.

Common greenbrier: Moist thickets.

Halbert-leaved greenbrier: Thickets in sterile or sandy soil.

Bouncing bet, common soapwort: Thoroughly naturalized, in waste places, and rugged, open hill-sides.

Wild pink: Rocky or gravelly places.

Purslane: Sunny sites in rocky or sandy soil.

Spring beauty: Moist, open woods.

Velvet-leaf: Escaped from gardens, naturalized in waste places.

High mallow: Naturalized like the former; waste places.

Wild crane's bill: Open woods.

Wood sorrel: Three species; rocky places.

Wax-work, climbing bittersweet: Along streams and thickets.

New Jersey tea: Undershrub; dry woodlands.

Milkwort: Woods, in light soil; besides three or four more species of the genus. Meadowsweet: Two or three species, among them F. tomentosa, L. hardtrack, steeplebush; low grounds and meadows.

Indian physic: Also called bowman's root; rich woods.

Purple flowering raspberry: Ornamental; hill-sides and rocky banks.

High blackberry: Border of thickets. Running swamp blackberry: Wet woods.

Sand blackberry: Sandy woods.

Hawthorn; whitethorn: Dry rocky banks.

Shadbush; serviceberry: Hillsides and river banks.

Deer grass; meadow beauty: Sandy swamps.

Loosestrife: Wet meadows.

Common evening primrose: Everywhere. River sundrops: River banks and swamps. False loosestrife; seedbox: Swamps. Yellow passion flower: Damp thickets.

Early saxifrage: Exposed rocks. Bishop's cap: Hillsides, in rich soil.

Panicled cornel, four feet to eight feet high: Thickets and hillsides.

Bluets; delicate little herb: Grassy banks and commons.

Of the very large family of composite, especially fully represented in North America, the District has its correspondingly good share. The species are distributed over every kind of locality, but to mention them all separately would lead too far; it is therefore preferred to present here an extract of such genera, which show the largest and most showy species. They are as follows:

Ironweed, button snakeroot, thoroughwort, mistflower, coltsfoot, starwort, double-blistered starwort, golden rod, golden aster, groundseltre, elecampane, rosin plant, oxeye, coneflower, sunflower, tickseed, burr marigold, crownbeard, cudweed, everlasting, Indian plantain, plumed thistle, hawkbit, hawkweed, rattlesnake root, &c., &c.

Cardinal flower: Low grounds. Great lobelia: In similar localities. False wintergreen: Damp sandy woods.

Small pyrola: Open woods.
One-sided pyrola: Rich woods.
Pipsissewa: Dry woods.

American cowslip: Rich soil.

Common mullein: Fields and roadsides. Turtlehead; snakehead: Damp thickets. Bushy false foxglove: Dry copses.

Painted cup: Low grounds.

Of the mint family well represented in the District the following genera count the more showy forms; Horsebalm, dittany, horsemint, sage, catmint, mountain mint, scullcap, false dragonhead, germander.

Of the borage family, the following genera are more prominently represented:

Vipers bugloss, false cromwell, smooth lungwort.

Of the waterleaf family: Waterleaf.

Of the polemonium family: Phlox, several showy species.

Of the convolvulus family: Bracted bindweed, morning glory. Of the nightshade family: Jamestown weed, thorn apple, nightshade. Of the gentian family: American centaury, gentian, American columbo.

Of the milkweed family: Milkweed, gonolobus.

Of the birthwort family: Wild ginger.

The class of the endogens is equally well represented by members of the following genera: Adam and Eve, or putty-root, false orchis, Arethusa, ladies' tresses, rattlesnake plantain, lady's slipper, star grass, blue-eyed grass, yam, three-leaved nightshade, Solomon's seal, lily, dog's-tooth violet, bellwort, devil's bit. Of the remaining genera of this class, the following ought to be mentioned: Dayflower, spiderwort; besides a very extensive list of grasses and sedges which close the series of endogens.

Especially noteworthy for the wood flora are of the cryptogams, the fern, and mosses. The former, of course, appear more prominent on account of size and higher development. The following genera are characteristics for the District: Polypody, brake, bracken, maiden hair, Woodwardia, spleenwort, Dicksonia, bladder fern, woodsin, wood fern, shield fern, sensitive fern, flowering fern,

moonwort.

Of the clubmoss family, (lycopodium, the ground pine,) lycopodium dendroi

deum, Micht., and L. complanatum, L., are the more conspicuous.

The list of herbs and undergrowth plants in general has been given here more extensively on account of the broader distribution of the genera and species, which descend almost equally from the hill-sides over clearings and open flats, down to the water's edge. It now remains to name such species of trees, shrubs, and suffrubicose forms, not mentioned heretofore on account of their being more

The folexclusively bound to the vicinity of water-courses, swamps, and bogs. lowing deserve particular notice:

Swamp white oak: Pretty large; low, moist woods.

Swamp chestnut oak: Low woods and river banks; a large tree.

Willow oak: 30 to 50 feet high; sandy, low woods.

Black jack or barren oak: 8 to 20 feet high, dry, sandy barrens.

Sweet gum: Moist woods; large and beautiful tree.

Hornbeam, ironwood: 10 to 20 feet high; along streams.

River or red birch: Large tree; gracefully overhanging river banks.

Smooth alder: 6 to 12 feet high; bordering water-courses.

Silk-leaved willow: Shrub 4 to 10 feet high; sandy river banks.

Brittle willow: Tall and handsome. Poplar, aspen: 20 to 50 feet high. Large toothed aspen: Large tree.

Planetree, buttonwood: also called sycamore: Large tree, alluvial river banks.

Sweet bay, small or laurel magnolia: 4 to 15 feet high; swamps. North American papaw: Small tree; bands of streams in rich soil.

Swamp rose mallow: 5 to 6 feet high; borders of marshes; flowers very showy.

Halbert-leaved mallow: River banks, swamps.

Maple, white or silver maple; Fine ornamental tree; river banks.

Red or swamp maple: Small tree; swamps and wet woods.

Strawberry bush: Wet places. Winter or frost grape: Thickets and river banks.

Wild yellow plum: Bush or tree 8 to 15 feet high; river banks.

Chokecherry: Tall, overhanging shrub; river banks.

Swamp rose: Low grounds. Witch hazel: Damp woods.

Common elder: Border of thickets and low grounds.

Buttonbush: Wet places; forming thickets.

Clammy or white honeysuckle: Swamps and boggy places.

Mountain laurel; also called cabin bush: Rocky hills, damp soil and banks of rivers.

Trumpet bower: Bold climber, with very showy flowers.

Leather or moose wood: Damp, rich wood swamps; 2 to 7 feet high.

American or white elm: Large ornamental tree; moist woods and alluvial river banks.

Fringe tree: Low tree or shrub, very ornamental; river banks.

Red ash: Woods along streams; large tree.

Most of the herbs and undergrowth plants of the lower sections of the District, such as swamps and overflows, will be found already embodied in the re-. spective list above. Truly aquatic forms, though some of them peculiar to this locality, are not deemed necessary to be considered here.

As a general remark it may be stated that the western portion of the District, forming an outlier of the eastern slope of the Alleghanies, shows over its hillsides and valleys a far greater diversity of species than east, on the other hand, in the flats towards the Eastern Branch; these exhibit a tiring monotony on

account of the more or less clustering habits of its vegetable forms.

In conclusion, attention may be drawn to the sudden changes in the qualities of soil, irrespective of the section it may belong to. For example, there can be observed on the top of a hill an excessive sterility, when on its slope, upon a horizon only a few yards below, the richest piece of ground imaginable may be encountered. The same occurs upon the flats and alluvious of the middle and eastern section. In most cases this will be found to be the result of the deteriorating agency of some kind of denudation, wherever the original vegetable cover of the soil had been impaired either by nature or man. Through the course of civilization in the immediate vicinity of a large city, the deteriorating causes by the hand of man are going on almost incessantly. It is therefore that we here find very extensive patches of land which, after having been cleared of their original cover, are deprived of every particle of vegetable soil; hence the multitude of desolate hillslopes and flats, alternating upon the same level with pieces of the richest forest lands imaginable. This observation may serve as subject for reflection in regard to inconsiderate clearing, and to the robbing of nature's treasures by a few seasons of speculative cultivation of the soil without ever making some just returns.

There is no doubt the surface crust of this District has been, previous to its disintegration, rich and genial. This is sufficiently proved by the above list of vegetable forms. After having been divested of its natural protection, denuding agencies, both natural and artificial, thoroughly impoverished it in many

instances.

WASHINGTON, D. C., January 16, 1867.

A public park for the capital.

The following is the speech of Hon. B. Gratz Brown, of Missouri, delivered in the Senate on the 20th ultimo, on the bill for the establishment and mainten-

ance of a public park in the District of Columbia:

Mr. Brown. Mr. President, the bill which I have just called up is one to establish a public park in the vicinity of Washington city. It is reported from the committee as a substitute for a similar bill offered by myself at an early stage of the session, and differs from that chiefly in the modes designated for acquiring ownership of the designated grounds. The locality is the same, the general area contemplated the same, and the purpose of procuring and ornamenting such a public place of resort, at the expense of the United States and not of the corporations of this District, is the same.

It will be remembered, Mr. President, that at the last session of Congress, when this matter was first mooted, objection was taken to the lack of definite information on which to proceed in making selection of grounds; and to obviate that difficulty as far as possible, a resolution was referred to the Committee on Public Buildings and Grounds, of which I have the honor to be chairman, instructing them to have surveys made of various eligible sites around the city of Washington adapted to the purposes of a public park. In pursuance of that resolution of the Senate, an application was made to the Secretary of War, which secured a detail of engineer officers, who, during the recess of Congress, proceeded to make the surveys desired. I hold in my hand an elaborate map, containing the result of those surveys, accompanied by an admirable report from the officer in charge of the work, which report has been printed and laid on our tables. I have before me also a minor plat, illustrative of the surveys, which. it may be well for senators to scrutinize. It was found, on making these examinations, that there was but one location in the neighborhood of this city eminently adapted to the purposes of a public park. I suppose all members of the Senate are familiar enough with the environs of the city to know the beautiful and romantic valleys of Rock creek. The character of the ground around and adjacent to that stream is exactly suited to the purposes we desire. It has running water; it has rugged hills; it has picturesque scenery; it has abundance of varied forest timber; it has a native undergrowth seething with beauty. It has the tangled vine and the clustering wild flower, and the quiet mosses, gray with age, and indeed a thousand imprints of native adornment that no hand of art could ever equal in its most imitative mood. Moreover, with so much of attractiveness in its present uncultured state, it has likewise every capscity for adornment and development, and can be made, with less expense than almost any spot of equal area I have ever seen within reach of a great city, one of the most beautiful resorts in the world. The amount of ground which was surveyed embraced twenty-seven hundred acres. It will not be necessary,

however, to take in all of that ground in order to secure what is desired for the purposes of a park, in the shape of drives, alcoves, recesses, and places capable of adornment. Fortunately the amount to be embraced is almost entirely optional, as the situation is such that large additions may be made without abating much the extent of the drives or the beautiful diversity of views.

The committee, after having made a very careful examination of the plans and surveys submitted by Brigadier General Michler-and, by the way, I must be permitted here to compliment him in the highest terms for the zeal he has manifested in this work and the admirable manner of its execution—the committee, I say, finding that the number of owners was so great as to preclude any joint offer for sale to the United States, thought it best to establish a commission authorized to negotiate in behalf of the government, and subjecting their action to the approval of the next Congress. It was believed that if we were to order a condemnation of the ground there might be improper combinations to secure a verdict, and the interest of the government might be sacrificed in the premises. It was believed, furthermore, that if an opportunity was had for conferring authoritatively with those who owned the ground, and if it was known that the purchase would depend in a great measure upon the reasonable character of the offers that were presented, there would be an opportunity of getting what was needed at a fair price, and probably of making a better selection of that which was desired than by any other mode.

For this reason the committee have reported a bill which simply provides a commission, and that commission embraces the officers who have had charge so far of the surveys, together with General Meigs, who is placed at the head of it, and authorizes them to confer with the owners of the property, to see what terms can be had, what grounds may be held by minor heirs or others that it will be necessary to condemn, and to find what portions may be left out and still not impair the desirability of the grounds for the purposes of a public park, and report the whole of their investigations and all their propositions at as early

a day as may be found practicable.

I do not think there is anything in the bill that can be objected to, if it is desirable to enter upon the work of securing for the city of Washington a public park. I think it is of such a character that will probably conduce more to the security of the government than any other which can be drawn. As to the necessity and the desirability of initiating such a proposition and obtaining the necessary location now, while the ground can be had on reasonable terms, I do not think there is much room for question. The ground in the vicinity of this city must soon become immensely valuable, now that the uncertainty with which sectional discord and disunion so long threatened the stability of the capital has passed away. At the present time Rock creek and its adjacent heights has few residences upon it, and those of comparatively trifling value, and the whole area surveyed can be had, as is estimated by General Michler in the report he has submitted, at an average price of perhaps less than \$200 an acre. This would make a total cost of less than half a million dollars; a mere trifle of expenditure for "a thing of beauty" which will prove "a joy forever."

Mr. President, I had intended, when presenting this subject to the consideration of the Senate, to have remarked somewhat freely upon the influence such surroundings are calculated to exert upon those who come hither from all parts of this great nation to bend their minds to the dismal science of law-making and of government. Those who, for any length of time, have undergone the wear and tear of such life as this, who have all their energies run to brain, and all their souls fused into politics, need not be told that anything which holds out hope of either mental or passional relief is seized upon with avidity. How necessary, then, that all the ennobling influences of nature—the scenic splendor of shifting views, the life and animation of gay concourse, the uprisen majesty of the forest, the intoxicating gladness of spring flowers, the laugh of the heavens through playing branches, the shimmer of the waters, the song of birds, grace-

ful forms, inspirations—should be so abundantly grouped around this nation's capital. There is no expenditure that can be made which shall add to the grandeur or adornment of the public buildings that fill so largely the eye of admiration of the world, or of the vast libraries that are accumulating so rapidly the treasures of all languages within your reach, or of the conservatories and gardens and cabinets that minister to your tastes, that will not freely be sanctioned by the people; for such in itself is the establishment of a nation's university, whither all may come to wonder and to learn, and in which all may feel a rightful patriotic pride. Only let it be worthy. Let your doing be on a scale commensurate with the pride to which you minister and the people you are sent hither to represent. And it is in the same spirit that I would have you, senators, inaugurate a public park that shall have no rival anywhere for beauty or extent or ornamentation, as it will have none for the illustrious characters gathered from a whole continent in the after time to wisely rule our republic from this

centre of its power.

Mr. President, the experience of foreign nations has been worth much on this subject of the extent of their places of public resort, and I do not think it would be wise in us altogether to disregard it. There it has been found that size was an invaluable feature, and even in the present day continued effort is being made to enlarge those now in existence. The latest data I have been able to lay my hands upon gives the dimensions of the more celebrated European parks as follows: Hyde Park, 380 acres; Regent's Park, 372 acres; Windsor Little Park, 300 acres; Windsor Great Park, 3,500 acres; Richmond Park, 2,250 acres; Phœnix Park, at Dublin, 2,000 acres; Garden of Versailles, 3,000 acres; Bois de Bologne, 2,158 acres; Englitche Garten, at Munich, 500 acres; and the Prater, at Vienna, 1,500 acres. The Central Park, in New York city, contains 840 acres, and authority has just been granted for the laying out of a park at Chicago of 2,000 acres. It will thus be made to appear that the amount of territory embraced in the present survey, 2,700 acres, does not exceed many of the more famous of those I have cited. And yet what would be thought of the proposition to reduce the area of either Windsor Park or Phœnix Park, or the Garden at Versailles? It would be simply set down, sir, as a barbarism. Let us, then, profit by the accumulated experience of so many metropolitan cities and so many great nations, and secure, while we may, here at the city of Washington, ample scope for a national park worthy of our people and our country.

But I perceive it is unnecessary to prolong any speech in behalf of what the Senate is evidently so willing to concur in voting. I will close, therefore, with expressing the hope that no delays attendant upon the close of the session may cause this measure to fail in the other branch of Congress, and that if it shall become a law the commission organized under it will not be too contracted in their views as to the extent of ground that should be embraced in this national park.

APPENDIX T-2.

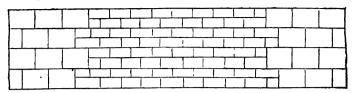
Copy of a letter from General M. C. Meigs, appended to the annual report, dated October 1, 1867, of Brevet Brigadier General N. Michler, in charge of public buildings, grounds, and works.

DRESDEN, July 27, 1867.

DEAR GENERAL: You are, I believe, now in charge of the paving of the avenues, for which appropriations have been made by Congress, and of the expenditure of the funds appropriated for the public places in Washington.

The streets of the cities of Germany, Denmark and Prussia, which I have lately seen, are so well paved that I am induced to send you a few notes thereon. The pavement in general use is like what we call Belgian. The blocks used are not large, but they are laid in regular courses across the line of travel. I

noticed that larger blocks are often used on the margin of carriage roads than in the more frequented middle of the way.



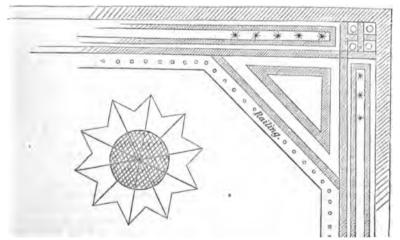
Care seems to be taken to lay the courses at right angles to the line of travel. In a few streets they are laid diagonally, but in only a few. The stones seem to be laid on a bed of gravel or coarse sand. In the repairs I have seen making I saw no other preparation as foundation, but I am not sure that in first paving a street some greater and more elaborate preparation is not made. The

French Ponts et Chaussées reports will doubtless give full details.

Sidewalks here are never paved with brick. There is generally a line of flags admitting of walking in single file; in the wider and more important streets two such lines. The rest of the sidewalk is either paved as the street, sometimes worse, i. e., with cobble-stones, which are torture to the feet, or else it is paved with small stones about the size used for concrete or macadamizing, which are laid in sand or gravel as close as they can be packed, points down and flat bases up, and then rammed to a smooth and even surface. These make a pavement easier to the foot than either flags or bricks, and as the stones are sorted and laid in patterns, far more pleasing to the eye than either. A space around every fountain or statue in the public places and streets of Berlin is paved with this mosaic. The colors used are red, gray, black, brown, which seem to be granites and sandstones, and white, which is of marble fragments.

In the pavement about the monument to Frederick the Great, the pieces of stone average two square inches surface each, or fifty of them fill a space of

10 by 10 inches. Below is one corner of this pavement:



Stars 10-rayed—white, with red centres. Color of pavement—red, gray, blue, white.

This pavement never gets muddy like our gravel walks. It dries after a shower quickly, even more quickly than brick, and far more rapidly than flag-stone.

A shower brings out the colors more vividly and improves its appearance. It is very pleasant to the foot and very ornamental to the street and squares.

Your resources in Washington are red and gray sandstone from the Seneca quarries, brown from the stone-yards, blue limestone veined with white from the Potomac, gray granite, red fragments of brick, white spauls from the marble yards of the city, and doubtless other colors would be found if the attempt to introduce this pavement around the Greenough's Washington, the Jackson, the

Mills's Washington, the fountain at the Capitol, &c., was made.

Shop-keepers in Berlin lay the whole pavement in front of their shops in this rude and cheap mosaic, sometimes. The name of the store or the number is sometimes introduced in block letters—white or black. The experience of these old towns leads them to pave the whole of their public squares which are not parks. They are used as market places—tables or wagons standing all over them. They are paved as the streets—nearly level—with very shallow undulations serving to carry off the water to the gully holes of sewers passing under them. The streets, which come in irregularly, seem to continue their pavement across on the direct lines of travel. The intermediate spaces are paved in irregular lines, or laid off into circles, triangles, &c., in which sometimes stones sorted of different tints are used with good ornamental effect, and sometimes the decoration depends upon the coursing above the block.

The space in front of such buildings as the Capitol is, in Europe—as far as I have yet seen—always paved. The court-yards of palaces are also paved like streets; not with flags or regularly cut stones, but with rough rectangular blocks like those used in Belgian pavements, and with mosaic of the small two-inch

stone like Berlin trottoirs.

Rain and the broom keep all free from dust. The waste of gravel and sand east of the Capitol would be much improved by such a pavement; Belgian on all the lines much used by carriages—small mosaic on those parts used princi-

pally by footmen.

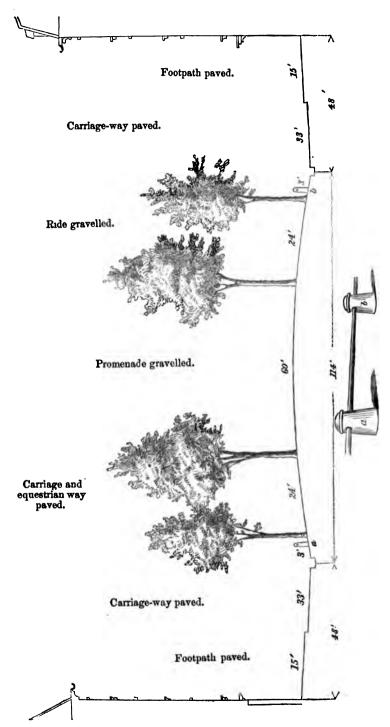
Asphalte is also much used for sidewalks in the German cities, as in Paris. With us it is too expensive, and I do not find it as pleasant to the foot as the small stone mosaic pavement, whose irregularity is sufficient to prevent the soreness caused by treading always upon a flat, hard stone surface, which presses the same parts of the sole at every step without any relief or change. My feet have been in a good condition to test the quality of pavement since I have been here,

for since my sickness a little walking makes them very sore.

The Unter-den-Linden is a street of great celebrity in Berlin, and the people are still praising the electors who laid it out two hundred years ago. It is the principal street of a city of 600,000 people; upon it are the royal palaces, those of most of the princes, the principal shops and hotels. It has a wide gravel walk in its centre, four rows of trees which give shade, wide trottoirs or sidewalks next the houses, and yet it is never encumbered. The central walk is sometimes filled in the evening with promenaders enjoying the long summer twilight of this northern latitude, in which darkness does not come until 10 p. m. I give you a sketch of it, and I hope that Pennsylvania avenue may yet be arranged like it. It would, in our hot climate, be a great improvement, and the economy in paving it and keeping it in repair would be very great. In Berlin droschkies are the common carriages. They are large carriages, to open or close. The top lets down and they carry four to six persons, all behind one horse. The streets are very level and the pavements are excellent. I have seen a man and dog pulling many a four-wheeled wagon with as much furniture or truck on it as a good horse and cart draw with us. Dogs are in universal use by the street porters in place of horses—generally only one dog harnessed along a pole. The man or boy takes hold of the pole and pulls by a "bricole" over gutters; the dog does the whole work in smooth places.

But to the Unter-den-Linden, whose trees are not very large. They have, I suppose, perished in occupations of the city by hostile armies and been repeat-

edly renewed.



a and b represent lines of stone posts about 15 feet apart, connected by iron rods about 11 inch.

If the thirty-three feet carriage way happens to be full or obstructed, carriages take the twenty-four feet line between the trees, but these twenty-four feet lines are ordinarily used only by equestrians and by porters, who drag their wagons, assisted by their dogs. One of them, that on the north side of the promenade, is gravelled to be used as a summer road.

This street is the resort for business and recreation of all Berlin, and of all

strangers.

Pennsylvania avenue is capable of a similar improvement. You see ninety feet are given exclusively to footmen, sixty-six feet to carriages, forty-eight feet are common property of footmen, horsemen, and carriages.

Hoping that these details will interest you, and serve, perhaps, as authority

in introducing improvements as yet novel in Washington,

I am, with regards to our friends at the club, yours truly,

M. C. MEIGS.

General MICHLER, Washington, D. C.

A true copy:

N. MICHLER, Major of Engineers, But. Brig. Gen. U. S. A.

APPENDIX T-3.

Report of the engineer of the Washington aqueduct, appended to the annual report, dated October 1, 1867, of Brevet Brigadier General N. Michler, in charge of public buildings, grounds, and works.

OFFICE OF THE WASHINGTON AQUEDUCT,

Washington, D. C., October 1, 1867.

GENERAL: I have the honor to submit the annual report of the operations upon the Washington aqueduct during the past year, and an estimate of the amount required for its completion.

POTOMAC DAM.

At the date of the last annual report, October 1, 1866, work had been resumed on the Potomac dam at Great Falls, and it was confidently predicted that by the beginning of December the foundation masonry would be completed across the Maryland channel to Conn's island. A heavy freshet occurring on the 16th of October caused high water for the remainder of the season and a suspension of work upon the foundation masonry. The superstructure masonry was continued until December 20, when all operations were suspended for the winter.

This year the spring freshets were unusually high, accompanied by large masses of ice. Although the dam was unfinished, the masonry sustained very little damage. The water did not subside sufficiently until the 20th of June, when a large force of masons and laborers resumed operations, and although the season has been unusually wet and the work often interrupted by freshets, yet, owing to the energy and perseverance of the contractors, Messrs. Charles H. Sherrill and Anson Bangs, the foundation and superstructure masonry are now completed across the Maryland channel to Conn's island. A large portion of the temporary dam was washed away during the spring freshets, and several times during the summer. It was repaired after each freshet, and kept in repair until the present time.

GATEHOUSE AT GREAT FALLS.

The work on this gatehouse has been resumed, and it will be completed before he beginning of winter. The floor and the timbers supporting the iron gates are very much decayed, and should be replaced with flooring and girders of cast-iron.

BRIDGES.

The stone bridges on the aqueduct are all unfinished. An estimate of the cost of completing them will be found at the end of this report, and also in each of the annual reports for the years 1864, 1865, and 1866.

The importance of completing these bridges cannot be overestimated. In their present state they are rapidly deteriorating, and if we have a succession of winters as cold and changeable as the last, their usefulness for aqueduct purposes will soon become seriously impaired.

CONNECTING CONDUIT AT THE RECEIVING RESERVOIR.

The work on the connecting conduit was resumed August 13, 1866, and vigorously prosecuted until its completion. . Dalecarlia tunnel, eight hundred feet in length, was continued day and night until March 4th, when it was pierced through. On August 8th the waters of Powder Mill Branch and of the recciving reservoir were shut off, and the water of the Potomac (which since the 5th of December, 1863, had emptied into the receiving reservoir) was turned

into the new connecting conduit.

In making the excavations for this conduit, more rock was encountered than was estimated for; nearly its entire length was built on rock foundations, but the most expensive and difficult portion of the work was Dalecarlia tunnel, a large part of which is constructed through soft and loose rock that is not selfsustaining. This part of the tunnel, as it progressed, was carefully shored with heavy timbers and every precaution used to protect the lives of the miners and to prevent the roof and sides from caving, yet extensive slides took place and several accidents happened to the workmen, though only one life was lost. Over one hundred feet of the south heading caved in and became an open cutting.

The cost of the connecting conduit has consequently exceeded the appropriation made by Congress in July, 1866, and there is a balance due the contract-

ors, Messrs. Sherrill and Bangs.

Three hundred feet of the tunnel will have to be arched, and the water slopes of the embankment will have to be lined with ripraps to protect them from the waves of the receiving reservoir.

THE RECEIVING RESERVOIR.

On August 8th, when the Potomac water was turned into the connecting conduit, this reservoir was shut off from the couduit and has not been used since. The water in it, which had become very impure, was emptied out; it was refilled again and now contains about four days' supply, which can be used in the event of an accident happening to the conduit above.

This reservoir could be improved and made very useful for storage and settling of water, by deepening the shallow parts and lining the slopes with ripraps. Eventually this improvement will be found necessary. The lands in connection with this reservoir might be improved and beautified, and made into a park which would be easy of access and a desirable place of resort.

DISTRIBUTING RESERVOIR.

Work on the distributing reservoir was suspended in June, 1864. Since then it has been used for storage and settling purposes. On the completion of the connecting conduit, the Potomac water was introduced directly into this reservoir. Owing to its unfinished condition it is necessary to keep the water at a low elevation. When it is finished the water can be raised several feet higher, which will give a greater pressure in the pipes and a largely increased supply of

water to Capitol Hill, and other high points in the cities.

This reservoir has been in use for over three years and no repairs have been made on any part of it. The estimate for completing it is, consequently, considerably increased, and the longer it is used in its present state the greater

must be the final cost of completing it.

In the estimate I have included the cost of laying an iron main from the distributing reservoir pipe vault to Foundry branch pipe vault, to be used either for a supply or drain pipe. If, from any cause, it becomes necessary to empty this reservoir, it can be done only by shutting off one of the mains and using it for a drain pipe. The necessity of having separate drainage and supply pipes is sufficiently obvious. The several gate-houses in connection with this reservoir are in an incomplete condition, and during the cold weather of last winter were a constant source of expense and anxiety, and a watchman had to be employed, day and night, to keep the water in them from freezing over.

HIGH SERVICE RESERVOIR AT GEORGETOWN.

The high service reservoir is also unfinished, but has been in constant use since June, 1865. Its present appearance is very unsightly; either it should be finished according to the original design, or the dome should be cut down to the level of the gravel walk and protected by an ornamental iron railing.

WATER-PRESSURE ENGINE.

The Worthington water-pressure engine is located in the west abutment of bridge No. 6. It has supplied the high service with water for nearly eight years. It was first put in operation in November, 1859, and worked till October 20, 1862, when it was stopped for repairs. The pistons were taken out and sent to New York, where new cylinders were cast and fitted to them, and the engine was put in motion November 11, 1862. It was stopped again for repairs in August, 1863, and the valves, which had become worn, were taken out and planed. Since then it has been stopped but a few hours at a time for slight repairs until the 16th of last month, when it was taken apart and fitted with new pistons and completely cleaned and repaired. This occupied seven days, and the heights were supplied with water by the Georgetown steam fire-engine, which was loaned by the city government for that purpose. The water-pressure engine is now as good as new, and is again in constant operation.

IRON BRIDGES.

Bridge No. 5 over College pond is in good condition.

Bridge No. 6 over Rock creek is now being repaired. The forty-eight inch tubes have been thoroughly scraped and painted; the ornamental wreaths which encircle the joints of the tubes had become loose, and many of them dropped off; several fell into Rock creek and cannot be found; the others, and those which were loose and liable to fall, have been replaced and securely bolted to the tubes. The sidewalks are nearly worn out, and the roadway requires a new floor to bring it even with the rails of the Washington and Georgetown railway.

GOVERNMENT MAINS.

The government mains are all in good condition. The twelve-inch main in Pennsylvania avenue between Sixth and Eighth streets east was lowered three feet during the past summer, in order to conform to the grade of the avenue; and a twelve-inch stop valve was placed in the main near Sixth street east. In North B street a twenty-inch main is now being laid, and will be connected by a twelve-inch main with the pipes in the Capitol. This will increase the supply of water to the Capitol, but it will not be abundant until the distributing reservoir is completed, and the water raised to a higher elevation.

LANDS.

The United States occupy for aqueduct purposes the following described parcels of land:

At bridge 6, Georgetown, the lot on which are located the pipe yard, work shop, and office.

At the high service reservoir, a lot fronting on High street, and partly cov-

ered by the reservoir embankment.

In Montgomery county, the roadway across the farm of William Brooke, and near Great Falls the roadway across the farms of Jackson, Collins, and Anderson.

At Great Falls $5\frac{44}{100}$ acres of the estate of the late Hall Neilson.

Each of the above described pieces will always be required for aqueduct use, and as the United States do not own them, I respectfully recommend that they be purchased.

FINANCIAL STATEMENT.

At the date of the last annual report the balance in the treasury		
applicable to this work was	\$117,198	04
Appropriated by Congress December 20, 1866	12,000	00
Appropriated by Congress March 2, 1867	20,000	
Total	149,198	
The expenditures to date are as follows:		
For construction of connecting conduit	\$69,248	86
For construction of Potomac dam	33,057	
For engineering, superintendence, and repairs	21,208	
For office rent	187	
For gas and fuel	188	
For stationery	96	
For rent of land	215	
For printing and advertising	128	
For internal revenue	204	
For paving approaches to bridge No. 6	817	
For repairs to water-pressure engine	300	00
Balance in treasury October 1, 1867	23,544	56
Total	149,198	04
Summary of appropriations made by Congress for the Washing	ton aquedr	ict:
For April 30, 1852	\$5,000	
For March 3, 1853	100,000	
For March 3, 1855	250,000	
For August 18, 1856	250,000	
For March 3, 1857	1,000,000	
For June 12, 1858	800,000	
For June 25, 1860	500,000	
For July 4, 1864	150,000	
For July 28, 1866	142,584	
For December 20, 1866	12,000	
For March 2, 1867	20,000	
Total	3,229,584	00

lands as far south on both sides of the lake as the shore line could be surveyed during the season; also to execute the off-shore hydrography of the northern portion of the lake.

The method of checking the work by fixing the latitudes of points, and the azimuths of the lines connecting them, was adopted as the best that was attain-

able under the circumstances.

There being no points of sufficient elevation on the shore to command the distance across the lake, (which at the narrowest part is about sixty miles,) and the comparatively flat country and dense forests rendering a triangulation inland extremely difficult, if not impossible, and the direction of the shore lines being nearly north and south, the difference of latitude between points with the azimuth of the line connecting them gives a closer approximation to the distance than could otherwise be obtained.

The off-shore hydrography was continued in the usual manner, south of previous work, for an average width of ten miles; on the east shore, from a point abreast of Point Aux Becs Scies (Old Point Betsey, as known by sailors) light-house, to Little Point au Sable, a distance of 78 miles; and on the west shore from near Bailey's harbor to Deansville, a distance of 48 miles.

This party returned to Detroit on the first of November, having accomplished

the following details of work, viz:

•	
Number of secondary triangulation stations	18
Number of sextant angles	
Number of theodolite pointings	1,669
Number of theodolite readings	2, 728
Number of lines of soundings run with steamer	
Number of miles of soundings run with steamer	
Number of square miles of off-shore hydrography	1,008
Number of miles of shore line run in connecting stations, &c., with shore	
work	
Number of readings of levelling rod	72
Number of casts of lead from steamer	1,070
Number of miles run by steamer on general duty	4, 728

Five lines, separated from each other about nine miles, were sounded across

the lake. The greatest depth of water found was 141 fathoms.

During the winter this work was plotted upon a scale of one sixty-thousandth. covering three sheets of antiquarian paper, with an area of 1,206 square inches of hydrography, and one sheet on a scale of one two-hundred-thousandth, covering an area of 473 square inches of hydrography.

Assistant Henry Gillman with his party sailed May 25, and was landed at White Fish bay, on the west side of Lake Michigan.

He was instructed to take up the survey on that side of the lake at the most southern point reached by previous surveys, which was near Bailey's harhor, and continue the work, embracing the coast line with the adjacent hydrography and topography, as far south as possible.

At the close of the season the survey of this coast had been completed to a

point about two miles south of Two rivers.

The amount of work accomplished by this party was as follows:

Number of secondary triangulation stations built	10
Number of sounding stations built	134
Number of buoys placed and located	155
Number of sextant angles	
Number of theodolite pointings	
Number of theodolite readings	6, 177
Number of compass readings for magnetic declination	659
Number of lines sounded with small hoats	1, 050

APPENDIX U.

Annual report on the survey of the north and northwest lakes for the year ending June 30, 1867, Brevet Brigadier General W. F. Raynolds, lieutenant colonel of engineers, superintendent.

> OFFICE U. S. LAKE SURVEY. Detroit, October 8, 1867.

SIR: At the commencement of the year ending the 30th of June last the lake survey parties in the field were prosecuting their several duties in the

localities specified in my last annual report, as follows:

One steamer and two shore parties in Lake Michigan; two steamers and three shore parties in Lake Superior. Astronomical party divided into three sections, working in both Lake Superior and Lake Michigan.

The duties of these several parties will be reported upon separately.

LAKE MICHIGAN.

The surveys of former years in this important lake had been prosecuted with special reference to producing charts of localities, of which the following had

been published:

The Straits of Mackinac, the Beaver Island group, the Fox and Manitou Island group, including Grand and Little Traverse bays. And the survey of Green bay had been finished, and charts of the same were in preparation, when it was decided to bring out a connected chart of the whole of the north end of the lake on a small scale, which, in connection with the published chart of Lake Huron, would cover a continuous district from the head of the St. Clair river, through the straits of Mackinac, and southward in Lake Michigan, past all the islands and known prominent dangers.

This decision rendered it necessary to extend the survey of both shores of the lake to the southward, including the off-shore hydrography in connection therewith; to carry soundings across the lake, and, as a triangulation was impossible, to connect the shore line by long azimuth lines and astronomical ob-

servations.

These important duties were divided as follows:

The astronomical party under charge of First Lieutenant M. R. Brown, corps of engineers, with divisions under assistants O. B. Wheeler and S. W. Robinson.

A hydrographical and primary triangulation party under Assistant O. N. Chaffee, on board the lake survey steamer Ada.

A topographical and hydrographical party on the east shore of the lake under Assistant A. F. Chaffee.

A topographical and hydrographical party on the west shore of the lake,

under charge of Assistant Henry Gillman.

The astronomical party was first sent into Lake Superior, and was not transferred to Lake Michigan until the 27th of September. The instructions given were to occupy prominent points on both shores of the lake, south of the limits of triangulation as far as the shore line could be surveyed, determining carefully the latitude of as many prominent headlands as possible.

Eight points were occupied, two of which were on the east and six on the

west shore.

Before these observations were computed Lieutenant Brown was relieved from duty on the lake survey, and First Lieutenant James F. Gregory was placed in charge of this work, the details of which will be found in Lieutenant Gregory's report, herewith attached, marked A.

Assistant O. N. Chaffee was instructed to continue the triangulation as far south as practicable; then to determine carefully the azimuths between headlands as far south on both sides of the lake as the shore line could be surveyed during the season; also to execute the off-shore hydrography of the northern portion of the lake.

The method of checking the work by fixing the latitudes of points, and the azimuths of the lines connecting them, was adopted as the best that was attain-

able under the circumstances.

There being no points of sufficient elevation on the shore to command the distance across the lake, (which at the narrowest part is about sixty miles,) and the comparatively flat country and dense forests rendering a triangulation inland extremely difficult, if not impossible, and the direction of the shore lines being nearly north and south, the difference of latitude between points with the azimuth of the line connecting them gives a closer approximation to the distance than could otherwise be obtained.

The off-shore hydrography was continued in the usual manner, south of previous work, for an average width of ten miles; on the east shore, from a point abreast of Point Aux Becs Scies (Old Point Betsey, as known by sailors) light-house, to Little Point au Sable, a distance of 78 miles; and on the west shore from near Bailey's harbor to Deansville, a distance of 48 miles.

This party returned to Detroit on the first of November, having accomplished

the following details of work, viz:

Number of secondary triangulation stations	18
Number of sextant angles	
Number of theodolite pointings	1,669
Number of theodolite readings	
Number of lines of soundings run with steamer	111
Number of miles of soundings run with steamer	1, 203
Number of square miles of off-shore hydrography	1,008
Number of miles of shore line run in connecting stations, &c., with shore	
work	21
Number of readings of levelling rod	72
Number of casts of lead from steamer	
Number of miles run by steamer on general duty	4, 729
Time lines are said from such about the site will a such as a line will be such as a line w	

Five lines, separated from each other about nine miles, were sounded across

the lake. The greatest depth of water found was 141 fathoms.

During the winter this work was plotted upon a scale of one sixty-thousandth. covering three sheets of antiquarian paper, with an area of 1,206 square inches of hydrography, and one sheet on a scale of one two-hundred-thousandth. covering an area of 473 square inches of hydrography.

Assistant Henry Gillman with his party sailed May 25, and was landed at

White Fish bay, on the west side of Lake Michigan.

He was instructed to take up the survey on that side of the lake at the most southern point reached by previous surveys, which was near Bailey's harbor, and continue the work, embracing the coast line with the adjacent hydrography and topography, as far south as possible.

At the close of the season the survey of this coast had been completed to a

point about two miles south of Two rivers.

The amount of work accomplished by this party was as follows:

Number of secondary triangulation stations built	10
Number of sounding stations built	134
Number of buoys placed and located	155
Number of sextant angles	68
Number of theodolite pointings	4, 131
Number of theodolite readings	6, 177
Number of compass readings for magnetic declination	659
Number of lines sounded with small hosts	1.050

Number of miles sounded with small boats	1,050
Number of casts of lead from small boats	47, 645
Number of square miles of hydrography, in shore	65
Number of square miles of topography	58
Number of miles run with stadia for topography	66 1 73 1
Number of shore lines run	
Number of observations on Polaris for determining meridian	8

During the winter the above was plotted on a scale of one sixteen-thousandth, requiring five antiquarian sheets and covering 759 square inches of topography

and 944 square inches of hydrography.

Assistant A. F. Chaffee sailed with his party on the 19th of May for Frankfort, on the east side of Lake Michigan, with orders to commence work at Point Aux Becs Scies, where previous surveys ended, and work to the southward, if possible, as far as Little Point au Sable.

Assistant Chaffee returned to Detroit on the 5th of November, having finished

the duty assigned him.

The following are the details of the work accomplished by this party:

Number of primary triangulation stations built	1
Number of sounding stations built	222
Number of buoys placed and located	164
Number of theodolite pointings	4,822
Number of theodolite readings	4, 818
Number of compass bearings for magnetic declinations	43
Number of lines sounded with small boats	1, 310
Number of miles sounded with small boats	959
Number of casts of the lead from small boats	47, 428
Number of square miles of hydrography, in-shore	65
Number miles run with stadia for topography	71
Number of miles of shore line run	99
Number of level of sights taken	56

The above was plotted on a scale of one sixteen-thousandth, and covered seven sheets of antiquarian paper, including 941 square inches of hydrography and 375 square inches of topography.

The following is the total amount of work executed in Lake Michigan during

the season of 1866:

Number of primary triangulation stations built	1
Number of secondary triangulation stations built	28
Number of sounding stations built	356
Number of buoys placed out and located	319
Number of sextant angles	448
Number of theodolite pointings	10,622
Number of theodolite readings	13,723
Number of compass readings for magnetic declination	702
Number of lines sounded with steamer	111
Number of lines sounded with small boats	2, 360
Number of miles sounded with steamer	1, 203
Number of miles sounded with small boats	1, 925
Number of casts of lead from small boats	95, 073
Number of casts of lead from steamer	1,070
Number of square miles of hydrography off-shore	1,008
Number of square miles of hydrography in-shore	130
Number of square miles of topography	1371
Number of miles of shore-line run	175
Number of level sights taken	128
Number of observations on Polaris for determining meridian	8

Number of miles run by steamer on general duty	4,729
Number of astronomical stations occupied	8
Number of pairs of stars observed for latitude	1, 131

The triangulation in Lake Michigan is shown in the accompanying sketch, marked B.

LAKE SUPERIOR.

The objects aimed at in Lake Superior were:

- 1. To effect a junction between the local surveys of previous years, and procure a continuous survey from west of Ontonagon, around Keweenaw Point, to Chocolate river.
 - 2. The completion of the hydrographical survey of Keweenaw bay.

The preparation and measurement of a base line for triangulation.
 The reconnoissance for the primary triangulation of the east end of the lake.

- 5. To read as many of the angles of the larger triangles as was practicable with the instruments that were available.
- 6. To determine the latitude and longitude of the principal points of triangulation.

To attain these ends the following parties were sent into the field:

First Lieutenant M. R. Brown, corps of engineers, was placed in charge of the astronomical and primary triangulation party, with instructions to divide it into three sections, placing two of them under charge of Assistants O. B. Wheeler and S. W. Robinson, and to station the different sections at the vertices of the larger triangles, and to observe for latitude, using the differential zenith telescope, and for differences of longitude, using the method of instantaneous signals, to be made at suitable points, and also to read the angles at the points occupied with the instruments furnished him.

In the latter part of September Assistant Robinson was relieved, at his own request, to enable him to accept the position of assistant professor of engineering in the University of Michigan, and Assistant G. Y. Wisner was assigned to his duties.

First Lieutenant James F. Gregory reported for duty after all the parties had left for the field. He was assigned temporarily to assist Lieutenant Brown. Subsequently he was placed in charge of the steamer Search, and in April last, Lieutenant Brown being relieved from the lake survey by order from the department, Lieutenant Gregory was placed in charge of the astronomical computations then being made. His report of the entire operations of the party will be found herewith, marked A.

Assistant D. F. Henry was assigned to the charge of the party on board the steamer Search, and was directed to resume the reconnoissance for points of primary triangulation; to build stations; to sound lines across the lake; to make surveys of distant shoals; and to aid the astronomical party by landing them at the points to be occupied, keeping them supplied with provisions, &c.

He left this city with his party on the 23d of May, and continued on the work assigned him until August 31, when he was transferred to the base line at Portage entry, in order to commence its measurement, turning over the steamer Search to Lieutenant Gregory.

Number of the delite werdings	296
Number of theodolite readings	
Number of compass readings for magnetic declination	3
Number of lines sounded with steamer	33
Number of lines sounded with small boats	124
Number of miles sounded with steamer	785
Number of miles sounded with small boats	43
Number of casts of lead made from steamer	194
Number of casts of lead made from small boats	1,484
Number of square miles of hydrography in-shore	3
Number of miles of shore line chained and sketched	2}
Number of barometer readings	113
Number of heights measured by barometer	13
Number of miles run by steamer on general duty	4,839
A suit tent II amond a son out of his googen's weak will be found becomish most	

Assistant Henry's report of his season's work will be found herewith, marked B. Assistant A. C. Lamson was assigned to the charge of the party on board the

steamer Surveyor, and left this city on the 22d of May.

The instructions given were to build stations for primary triangulation at points selected the season before; to cut out lines of sight; and to make a hydrographical survey of Keweenaw bay, and to the eastward along the south shore of Lake Superior, as far as possible.

The Surveyor returned to Detroit on the 23d of October, having accomplished

the following work, viz:

Number of primary triangulation stations built	8
Number of sounding stations built	8
Number of sextant angles measured	60
Number of theodolite pointings	2,794
Number of theodolite readings	5.585
Number of lines sounded with steamer	117
Number of miles sounded with steamer	1.1781
Number of casts of lead from steamer	1,232
Number of square miles of hydrography off-shore	1.0511
Number of miles run with stadia for topography	1
Number of miles of line of sight opened	3
Number of miles run by steamer on general duty	2.4901
	_
Completing the off-shore hydrography from Keweenaw Point to La	ugning
White Fish Point, twenty miles to the eastward of Marquette, a dist	ance of
about 150 miles.	

The above work was plotted during the winter on a scale of one one-hundredand-seventy-thousandth, and embraced 147 square inches of hydrography.

Assistant J. R. Mayer was placed in charge of a party to survey the shoreline and adjacent hydrography and topography, commencing at Keweenaw bay and working to the eastward until he joined the party of Assistant Albert Molitor, who was assigned to a similar duty beginning at Marquette and working to the westward.

The field assigned to this party comprised a portion of the east side of Keweenaw bay, and the important indentation of Huron bay extending inland about twelve miles, and affording one of the most perfectly sheltered harbors on the entire lakes, the whole of which was minutely surveyed; also, the Huron islands, which lie in the track of vessels plying between Marquette and Portage entry.

Assistant Mayer left this city with his party on the 23d of May, and returned in the latter part of October, having performed the following amount of work:

Number of secondary triangulation stations built	32
Number of sounding stations built	105
Number of buoys placed and located	133
Number of lines sounded with small boats	1,029

Number of miles sounded with small boats	763
Number of casts of the lead with small boats	28,864
Number of square miles of hydrography in-shore	53
Number of theodolite pointings	3,678
Number of theodolite readings	4,224
Number of secondary base lines measured	2
Number of meridian line observed	1
Number of miles of shore-line run with stadia	58
Number of miles run with stadia for topography	201
Number of square miles of topography sketched	25
	_

The above was plotted on a scale of one sixteen-thousandth, on two sheets of antiquarian paper, and embraced 584 square inches of minute hydrography and

635 square inches of minute topography.

Assistant Albert Molitor, in charge of a similar party, left Detroit at the same time as Assistant Mayer, with instructions to commence his work at Marquette and continue to the westward until he met Assistant Mayer, which he did on the 18th of August, at Huron river, when his party was transferred to the base line

at Portage entry, to assist in its measurement.

The field surveyed by Assistant Molitor included Granite island, a small but dangerous island lying in the track of commerce, and about five and three-quarter miles from the mainland, also several islands lying near the shore, which, with the indentation of the main land, afford reasonably good anchorage for vessels in almost any storms, all of which were carefully surveyed, and will be fully shown on the charts to be prepared.

A reef was also discovered by this party between Partridge island and the main shore, one rock lying but two feet beneath the surface of the water.

During the latter part of the season, after his party had been sent to the base line, Assistant Molitor was detailed to make a survey of the mouth of the Au Sable river, Lake Huron, for harbor purposes, which he accomplished, and returned to Detroit on the 17th of September.

The following are the details of the work performed by this party during the

season, viz:

Number of sounding stations built	252
Number of buoys placed out and located	160
Number of lines of soundings made with small boats	1, 138
Number of miles of soundings made with small boats	924
Number of casts of the lead made with small boats	34, 786
Number of theodolite pointings	6, 467
Number of theodolite readings	8,650
Number of sexant angles	269
Number of secondary base lines	5
Number of miles of shore-line surveyed	64
Number of observations for true meridian	3
Number of compass readings for magnetic declination	24
Number of square miles of hydrography in-shore	33 [
Number of square miles of topography	25 <u>i</u>
Number of vertical angles for topography	338

The above work was plotted on a scale of one sixteen-thousandth, covering four sheets of antiquarian paper, embracing 534½ square inches of hydrography and 790½ square inches of topography; and in addition a map of Granite island. Lake Superior, on a scale of one five-hundredth, embracing 99½ square inches

of topography.

Number of theodolite readings

47

217 225

Number of buoys placed out and located	7
Number of lines sounded with small boat	123
Number of casts of lead with small boat	
which was plotted on a scale of one twenty-four-hundredth, covering a sh	eet of
antiquarian paper, and embracing sixty square inches of topography, ar	id one
hundred and four square inches of hydrography.	

SCRVBY OF THE HARBOR AT SUPERIOR CITY.

In accordance with letter from the department dated July 23, 1866, directing that a survey of the above-named point be made for the purpose of making an estimate for the improvement of the harbor, Assistant E. S. Wheeler was detailed from Assistant Henry's party for the duty on the 10th of September. He accomplished the survey in eight days. The following are the details of the work:

Number of buoys placed out and located	23
Number of theodolite pointings.	309
Number of theodolite readings	309
Number of miles of shore-line chained and sketched	2
Number of square miles of hydrography in-shore	3
Number of square miles of hydrography in-shore	193
Number of miles of soundings with small boats	
Number of casts of the lead	

The above was plotted on a scale of one five-thousandth, and embraced six and one-half square inches of topography, and fifty-seven and one-half square inches of hydrography.

The total amount of work accomplished in Lake Superior during the season

of 1866 is as follows:	10 000001
Number of primary triangulation stations built	11
Number of secondary triangulation stations built	32
Number of sounding stations built	376
Number of water stations built	2
Number of base line transit stations built	4
Number of buoys placed and located	348
Number of sextant angles	630
Number of theodolite pointings	13, 544
Number of theodolite readings	19,064
Number of compass bearings for magnetic declination	´ 3
Number of lines sounded with steamer	150
Number of lines sounded with small boats	2, 484
Number of miles sounded with steamer	1,9634
Number of miles sounded with small boats	1,773
Number of casts of the lead from steamer	1, 426
Number of casts of the lead from small boats	68, 310
Number of square miles of hydrography off-shore	1,0511
Number of square miles of hydrography in-shore	90
Number of square miles of topography	50 <u>}</u>
Number of miles of shore line	126]
Number of miles run with stadia for topography	21 1
Number of observations for true meridian	3 -
Number of vertical angles for topography	33 8
Number of secondary base lines	7
Number of miles of line of sight opened	3
Number of barometer readings	113
Number of heights measured by barometer	13
Number of miles run by steamer on general duty	7, 329 <u>1</u>

Field operations for season of 1867.

RIVER ST. CLAIR SURVEY.

Navigation being open longer by several weeks, both at the beginning and end of the season, in the St Clair and Detroit rivers than in Lake Superior, I recommended to the department to use this time for making a survey of these localities, and thus connect the surveys in Lake Erie with those in Lake Huron.

The programme for the season's operations was approved as presented. In order to carry it out, the three steamers belonging to the lake survey were at once prepared for service and despatched to the St. Clair river, with instructions

to work in that locality until navigation was open into Lake Superior.

The organization was as follows: Lieutenant James Mercur, corps of engineers, was placed in charge of the steamer Search, with instructions to commence at Port Huron, at the foot of Lake Huron, and to carry the survey as far as the town of St. Clair, from which point Lieutenant B. D. Greene, corps of engineers, with the party on board the steamer Surveyor, was to carry it on to meet the work of Assistant O. N. Chaffee, who, with the party on the steamer Ada, was instructed to begin his survey near the town of Algonac, at the head of the delta of the St. Clair river, and continue the survey over the entire area covered by the delta, or as much thereof as possible, before the opening of navigation in Lake Superior.

The steamer Search left Detroit for her field of duty on the 17th of April, Lieutenant Mercur having as his assistants Messrs. Albert Molitor, Flint and

Towar.

On the 31st of May the party returned to this city, having completed the duty assigned them.

The following is the amount of work accomplished:

Number of secondary triangulation stations built	41
Number of sounding stations built.	
Number of theodolite pointings	6, 387
Number of theodolite readings	
Number of lines sounded	645
Number of miles sounded	254
Number of casts of the lead taken	10,726
Number of miles levelled	131
Number of miles of shore-line run	31 1
Number of miles measured with stadia	274
Number of square miles of topography	25
Number of vertical angles for topography	1, 165
Number of compass bearings for magnetic declination	11
Number of observations for true meridian	1
The state of Commence and Just the Court (April 18) The	

The steamer Surveyor sailed with the Search (April 17,) Lieutenant Greene having as his assistants Messrs. Lamson, Paul Mayer and Marr.

She returned to Detroit on the 24th of May, the party having finished the work assigned them, the details of which were as follows:

71
161
9
4,681
4, 366
32
179 1
321
16

883

29£

23

REPORT OF THE SECRETARY OF WAR.	561
Number of readings for levels. Number of bench-marks established. Number of feet measured with chain. Number of lines sounded with small boat. Number of miles sounded with small boat. Number of casts of the lead. Number of compass bearings for magnetic declination. Number of observations for true meridian.	126 4 5, 240 408 190 7, 733 7
The steamer Ada left on the 16th of April, having Assistant O. N. in charge, with Messrs. A. F. Chaffee, Foote and Wallace as assistance turned to this city on the 24th of May to reorganize for the season's Lake Superior, having surveyed 24½ miles of river course. The following are the details of work of Assistant Chaffee's party:	nts, and
Number of miles sounded with small boats. Number of square miles of topography Number of miles run with stadia. Number of miles of water line sketched. Number of feet chained for secondary base line. Number of observations for true meridian. Number of water gauges erected. Number of square miles of hydrography.	79 280 23 75 4, 627 7 1, 014 19, 262 408½ 32 84½ 25½ 12, 701 2 8
The following is a general summary of the work performed in carry survey from the head of the St. Clair river to the deep water of Lake St	ing the
Number of secondary triangulation stations built Number of sounding stations built Number of buoys placed out and located Number of sextant angles	191 571 32 75
Number of theodolite pointings	15, 695 16, 361 25
Number of lines sounded with small boats	2, 067 853 37, 721 89 1
Number of square miles of topography	538

The above comprises a complete survey of the St. Clair river from Lake Huron to the deep water in Lake St. Clair.

The following table exhibits at a glance the amount of work executed in Lakes

Number of miles of shore line run

Number of miles levelled.....

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Superior and Michigan during the season of 1866, and in the river St. Clair previous the 30th of June, 1867:

Character of work.	Lake Michi- fran.	Lake Supe-	River 8t, Clair.	Total for year ending June 30, 1807.
Number of primary triangulation stations built	1	11		12
Number of secondary triangulation stations built	28	32	191	251
Number of sounding stations built	356	376	571	1,303
Number of water stations built		2		2
Number of base-line transit stations built		4		4
Number of buoys placed out and located	319	348	32	639
Number of sextant angles	448	630	75	1, 153
Number of theodolite pointings	10, 622	13, 544	15, 695	39, 861
Number of theodolite readings	13, 723	19,064	16,361	49, 148
Number of compass bearings for magnetic declination	702	3	95	730
Number of lines sounded with steamer	111	150		351
Number of lines sounded with small boats	2, 360	2, 484	2, 067	6, 911
Number of miles sounded with steamer		1, 963}		3, 166
Number of miles sounded with small boats	1, 925	1,773	853	4, 551
Number of casts of the lead from steamer	1, 070	1, 426		2, 496
Number of casts of the lead from small boats	95, 073	68, 310	37,721	201, 194
Number of square miles of hydrography off-shore	1,008	1,051		2,059
Number of square miles of hydrography in-shore	130	90	23	243
Number of square miles of topography	56	50}	891	198
Number of miles of shore line	175	126	881	390
Number of miles of shore line. Number of miles run with stadia for topography	137 1	21 }	538	696
Number of observations for true meridian	8	3	5	16
Number of level sights taken	128		149	277
Number of Vertical angles for topography		338	1, 165	1,503
Number of secondary base lines measured		7	1	8
Number of miles of line of sight opened		3		3
Number of barometer readings		113		113
Number of heights measured by barometer		13		13
Number of miles run by steamers on general duty	4,728	7, 329		12,057
Number of astronomical stations occupied		7		15
Number of pairs of stars observed for latitude	1, 131	1, 104		5 33
Number of flashes for longitude		527	- 	527

LAKE SUPERIOR.

In accordance with the programme for the season, preparations were made for taking the field in Lake Superior as soon as possible after the opening of navigation. The plan proposed was to carry the survey from the head of the St. Mary's river westward as far as Marquette, to continue the primary triangulation and observations for latitude and longitude, and to survey as much of Isle Royale as possible. For the accomplishment of these objects the following disposition of parties was made and the field occupied in the following order:

Brevet Licutenant Colonel F. U. Farquhar, captain corps of engineers, was placed in charge of the steamer Search, which left this city on the 7th of June, having on board the parties of Lieutenant Greene and Assistant Lamson. Colonel Farquhar was instructed—1st. To make the off-shore soundings from Grand island eastward as far as possible, or until he met the work of Assistant O. N. Chaffee. 2d. To carry supplies to the three shore parties between Marquette and the St. Mary's river, moving their camps, and giving such instructions as may be required to secure uniformity and the completion of their work. 3d. Making the necessary reconnoissance to determine the most suitable points to be occupied by the astronomical parties, and locating them at the points selected, keeping them supplied with provisions, &c., &c. 4th. Making reconnoissance for points of primary triangulation in the eastern portion of the lake. 5th. To survey all shoals or small islands beyond the limits of the shore parties' work. 6th. To have a general supervision of all the parties in Lake Superior, and, in my absence, give such instructions as might be necessary to secure satisfactory discharge of their duties.

First Lieutenant James F. Gregory, corps of engineers, was placed in charge of the party on board of the steamer Surveyor, which also sailed for Lake

Superior on the 7th of June. The duties assigned him were as follows: 1st. Making the off-shore soundings around Isle Royale, including the channel between it and the north shore. 2d. The reconnoissance for primary triangulation between Isle Royale and the north shore, erecting stations, and reading the angles of the same. 3d. The survey of all shoals or small islands beyond the limits of the work of the shore parties. 4th. Moving the two shore parties on Isle Royale, keeping them supplied with provisions, &c., &c. 5th. In the absence of myself and Colonel Farquhar, to give such instructions to the shore parties on Isle Royale as might be requisite to secure a hearty co-operation and faithful performance of the duties committed to them.

Lieutenant Gregory reports that the work has been greatly impeded by dense The shores of the island are also very bad. No dependence can be placed upon soundings, the water frequently shoaling up between soundings from fifty or sixty fathoms to a few feet, and in many places a lead-line dropped

from the rocks reaches from eight to twenty fathoms.

Assistant O. N. Chaffee sailed June 8, in charge of the steamer Ada, under instructions to take up the primary triangulation of Lake Superior east of Keweenaw Point, and push it forward as rapidly as possible as far as the triangulation could be carried to the eastward. Having completed this duty, he was directed to take up the off-shore hydrography at the head of the St. Mary's river, and continue it to the northwest, around White Fish Point, until he met and connected with the work of Colonel Farguhar. He was also directed to make the necessary reconnoissance and triangulation to determine the general contour on the Canadian side of the bay south of White Fish Point.

First Lieutenant B. D. Greene, corps of engineers, was placed in charge of a party which left Detroit on the 7th of June, on board the steamer Search, for the purpose of making a topographical and hydrographical survey of the south side of the Isle Royale. He was landed on the most eastern point of the island, and instructed to continue the survey to the westward as far as possible.

Assistant A. C. Lamson also sailed on board the steamer Search, with his party, June 7, for Isle Royale, having been instructed to survey the north side of the island, commencing at the most eastern point and work to the westward

as far as practicable.

The field of duty assigned these two parties comprised the numerous bays and islands forming the eastern portion of Isle Royale-as complicated and difficult portion of the coast as any in the whole extent of the lakes. The shore is almost invariably bold and rocky, and in most places so steep as to make landing impossible. Numerous deep and narrow bays, separated by rocky islets, form a succession of harbors unsurpassed in excellence and beauty. The bottom, however, is very irregular, the depth changing from many fathoms to a few feet between casts of the lead, and requiring the most minute survey to render the harbors available.

Assistant Henry Gillman sailed with his party on June 7, entrusted with the duty of making a topographical and hydrographical survey from Grand island, and continue to the westward until his work joined the survey previously made of Marquette harbor, after which his party was to be transferred to the base-

line at Portage entry, to assist in its measurement.

Assistant J. R. Mayer, with his party, also sailed June 7. Assistant Mayer was instructed to commence his surveys at Grand island, and continue to the eastward until his work joined that of Assistant Molitor. The field assigned Assistant Mayer includes the well-known locality of the "Pictured Rocks," where it is unsafe to be in a small boat except in a perfect calm.

Assistant Albert Molitor was placed in charge of a similar party, and instructed to commence his surveys at the most northern point of the surveys heretofore made in the St. Mary's river and Taquamenon bay, and to continue his work from that point north and westward, around White Fish Point, until he met and closed upon the work of Assistant Mayer. This party sailed for their field of duty on June 8, and arrived at their first camping ground in Taquamenon bay on the 10th of the same month.

ASTRONOMICAL DUTY.

First Lieutenant James Mercur, corps of engineers, was placed in charge of

the astronomical party.

He was directed to separate his party into three divisions, placing two of them in charge of Assistants O. B. Wheeler and G. Y. Wisner, and instructed to occupy the same stations as last season, more powerful instruments having been borrowed from the United States Coast Survey, and it is hoped that better results may be obtained.

Lieutenant Mercur left this city with his party June 20, and landed at Copper Harbor, from which point the different sections of his party were carried

to their destinations by the steamer Search.

THE "SUN TELEGRAPH."

The operations of the astronomical and triangulation parties for the past three years have incidentally developed an exceedingly interesting application

of the heliotrope, which is worth mentioning.

While using this instrument in Green Bay, in 1865, Assistants O. B. Wheeler and Robinson commenced communicating with each other by cutting off the light from the heliotrope in such a way as to make the telegraphic alphabet to be read by sight instead of by sound, the distance between them being about twenty miles. Practice soon enabled them to do so with facility. In 1866 the same thing was done over a distance of between fifty and sixty miles; and during the past season Assistant Wheeler sent an order for me from station Tip-Top to station St. Ignace, a distance of ninety-two miles, which order was received and obeyed.

This method of communicating intelligence must have an important bearing in military operations, for by the simple aid of a small looking-glass, or even the light of a lamp, information could be sent to the enemy almost without the

possibility of detection.

On the 30th of June, the date to which a report is called for by the regulations, the several parties were engaged in the discharge of their respective duties, and were making good progress. The details of the work done by them during the season will be given in my next annual report.

OUTFLOW OF THE LAKES.

The subject of the supply of water in the chain of lakes is one that has never been examined, and as no discussion of the phenomenon of the lakes can be complete without it, I proposed to the department to commence the investigation the present season.

The suggestion having been approved, the duty was assigned to Assistant D. F. Henry, who was directed to gauge carefully the rivers forming the con-

necting links in the chain.

For this purpose parties were to be stationed as follows: In the St. Mary's river, below the falls; in the St. Clair river, near the town of St. Clair; in the Niagara river, below the falls; and in the St. Lawrence river, near the town of

Ogdensburg.

Assistant Henry, with Assistants Foote and Flint, (who had charge of the small parties,) left this city on the 8th of June for St. Clair, where preliminary experiments were made; after which, on the 29th of June, Assistant Foote, with his party, was transferred to Sault Ste. Marie, while Assistant Flint was left to continue the work at St. Clair.

The method pursued is substantially the same as that adopted on the Mississippi river survey. (See Report on the Physics and Hydraulics of the Mississippi river survey.

sissippi River, page 222, et seq.)

These investigations are still going on, and of course cannot now be reported upon.

METEOROLOGICAL DEPARTMENT.

On the 1st of January last Assistant D. F. Henry was placed in charge of the meteorological department of the survey. Observations have been made throughout the year at twelve stations, covering the entire chain of lakes, and the records transmitted monthly to this office, where they are reduced and tabulated.

These observations have now been continued through a period of nearly seven years, and have already produced highly interesting and important results.

The existence of a lunar tide has been clearly shown. The influence of the lakes upon the climate demonstrates, and recent investigations seem to indicate, the existence, during calm weather, of a land and lake breeze.

Indeed, the further all the investigations are carried, the more fully it is shown that all the phenomena of the ocean pertain to the lakes, and that they

are justly entitled to their common appellation of "inland seas"

Assistant Henry's report will be found herewith, marked C. He has not as yet been able to discuss fully the data that have been obtained, and there yet remains much to be done.

OFFICE WORK.

In addition to the duties of the parties in the field, whose work is reduced and plotted during the winter by the assistants in charge, the details of which have been given in each case, and the reduction of meteorological observations, which has been mentioned as constantly in progress, there are other duties carried on in the office throughout the year. These consist of the

REDUCTION OF MAPS FOR THE ENGRAVER.

All the field-notes are projected on a scale of not less than one sixteen-thousandth, or about four inches to one mile.

These maps show in detail all the work that has been done, and where a larger scale is required to do this it is used. These detail maps, though invaluable as records and for reference, are, however, entirely too unwieldy for general use. They are therefore reduced to an appropriate scale for publication.

During the past year Assistant J. U. Mueller has finished a map of the north end of Lake Michigan, including the Straits of Mackinac and Green bay, on a scale of one four-hundred-thousandth, which was forwarded to the bureau

on the 5th of October, 1867.

This map involved the reduction of one hundred and ten of the maps of detail, and, while it shows all the leading features of the district included within its limits, gives no idea of the amount of labor expended to produce it. Shoals that required days to survey, and upon which thousands of casts of the lead have been taken, are represented by a single figure showing the least depth. Large bays that have been minutely surveyed are represented only by shading, and a few figures to show the depth of water. It is only by an examination of the maps of detail that the value of the survey can be appreciated.

The execution of the map in question is highly creditable to Mr. Mueller.

Assistant Edward Molitor has completed the reduction of a map of the south end of Green bay, from the entrance thereof to Fox river, which was forwarded to the bureau on the 19th of August, 1867. This map, which is on the scale of one one-hundred-and-twenty-thousandth, shows in considerable detail all the dangers to navigation, as well as the harbors, anchorage, &c.

It is on the same scale as similar maps heretofore published of the Straits of Mackinac, the Beaver Island group, Grand and Little Traverse bays, and the

north end of Green bay.

Mr. Molitor has executed his work with great care, and in a style fully equal to previous charts on the same scale.

The following is a list of the lake survey charts already published, and which

are distributed free of charge to the vessels engaged in lake commerce:

Lake Erie. West end Lake Erie. Kelly's and Bass Islands. Head of Green Bay, Saint Clair Flats, Tawas Harbor, Eagle Harbor, River Ste. Marie, No. 1, Maumee Bay. Ontonagon Harbor, Thunder Bay, Presque Isle and Middle Island, South end Lake Huron, West end Lake Superior, Grand and Little Traverse Bays. L'Anse, Keweenaw Bay,

Straits of Mackinac,
East Neebish Rapids,
Saginaw River,
Buffalo Harbor,
Beaver Group,
Agate Harbor,
River Stc. Marie, No. 2,
Eagle River,
Saginaw Bay,
Marquette Harbor,
Lake Huron,
Grand Island,
Copper Harbor,
North end Green Bay,
Portage Lake.

The data for the following charts are now in the office, and the preparation of them will be at once commenced, and as soon as they are prepared will be forwarded to the bureau for engraving.

East end of Lake Superior to Grand island. (Preliminary.)

Central portion of Lake Superior from Grand island to Ontonagon. (Preliminary.)

East end of Isle Royale, Lake Superior. Huron bay and islands, Lake Superior.

Big and Little Sturgeon bays, Lake Michigan.

The accompanying sketch, marked A, shows at a glance the charts that have been published as well as those in preparation.

CHART DISTRIBUTION.

In addition to attending to the current business of the office, in assisting me making out vouchers and the payment of accounts, Assistants Henry Clague and H. M. Wright are charged with the duty of superintending the distribution of the published charts of the survey.

Under the regulations now in force, charts are issued to all vessels plying upon the lakes without regard to nationality. Each chart is distinctly marked with the name of the vessel and the date of presentation; also a memorandum of the understanding that "it is to be considered as the property of the vessel on change of either owner or master," and that "it will not be duplicated unless satisfactory reasons can be furnished for its loss." The distributions are made at this office, and by a special agent in Buffalo. A record is kept at both places, which are exchanged monthly, and no vessel is supplied without first examining the record to see if she has previously been furnished.

No provision is made for procuring charts in any way other than the above. The owner or master may be ready, as they frequently are, to pay any price for the charts, but we are not allowed to furnish them. The difficulty of deciding what are "satisfactory reasons" for the loss of charts has been found so great, that we have established a rule not to duplicate without proof that the vessel has been sunk or otherwise so seriously injured as to render the preservation of the charts impossible. The practical result is, that there are very many greatly needing the charts who cannot get them.

I have in my previous report called the attention of the department to this matter, and I would again most respectfully urge that some provision may be

made for selling charts to all who may desire them, at the cost of printing and paper.

The demand for charts is constantly increasing, the number distributed during

the past year being 5,464, or 1,829 more than the year previous.

A statement is given herewith, marked D, showing the number of each kind distributed, and the total distribution up to 30th June last.

FUTURE OPERATIONS.

The present season's operations will complete the shore-line of Lake Superior as far westward as Ontonagon, including the eastern portion of Isle Royale.

A base line five and one-half miles long will be measured, and the triangulation of Keweenaw bay completed. The scheme of triangulation is shown on the sketch submitted herewith.

The propriety of continuing the survey of Lake Superior until it is completed cannot be questioned. All that is now required previous to the preparation of charts of the eastern and middle portions of the lake is the completion of the survey of Isle Royale, and of the off shore hydrography and the triangulation. A portion of our force in a single season will accomplish the two duties named first. The completion of the triangulation will depend upon the facilities that are furnished.

As soon as it became probable that a triangulation could be carried over a large portion, if not the whole of Lake Superior, I reported to the bureau September 6, 1865, that the instruments belonging to the survey were entirely inadequate for the work, and that the method previously adopted for reading the angles of the triangulation—that is, by visiting the stations, expecting to read them in a few hours—should be discontinued; the lines of sight being so long that they could only be seen over during the best of weather. I therefore recommend procuring instruments suitable for the purpose, and stationing small parties at the different points and leaving them until the opportunity offered for reading the angles.

The reconnoissance showed with considerable certainty that if the angles of a single triangle could be read, the triangulation could be carried over the entire lake. This was triangle "Vulcan—St. Ignace—Northeast." (See Sketch B.) During a portion of the season of 1866, parties supplied with the best of instruments that could be spared for the purpose were stationed at the vertices of this triangle, with instructions to read the angles if a favorable opportunity offered. After waiting over a month without success, the attempt was for the time abandoned.

The effort proved that the project was not impracticable; the difficulty being, first, that the instruments used had not sufficient power; and, second, that the

reflectors in the heliotropes were too small.

Under these circumstances I asked for, and obtained, permission to borrow from the Coast Survey such instruments as were needed. I also had six looking-glasses of the best plate-glass, ten by fourteen inches, prepared, with rough equatorial mountings, which were to be used for throwing the reflection of the sun through an opening in a screen placed in the proper position.

The following instruments were kindly loaned me by Assistant J. E. Hilgard, in charge of the coast survey: One twenty-four inch theodolite by Troughton, reading by three micrometer microscopes to single seconds; one fourteen-inch Brünner repeating theodolite, reading by two verniers to five seconds; and one twelve-inch Gambey repeating theodolite, reading by two verniers to five

seconds.

The examination made last season also showed that by changing the station northeast about four miles, an additional elevation could be obtained of about five hundred feet.

It was decided to occupy this point the present season, and, with our improved facilities and increased knowledge, make another and final attempt to read the

angles required, and I am happy to report that the effort has been an entire success.

The sides of the triangle in question are respectively 92, 93.6, and 100.6 miles; its area varies but little from four thousand square miles. The angles have all been read satisfactorily, being respectively as follows:

Stations	A	ngle	
•	0	,	"
St. Ignace—Vulcan—Tip-Top	56 66	30	39. 45 6. 57
St. Ignace—Vulcan—Tip-Top. Vulcan—St. Ignace—Tip-Top. Vulcan—Tip-Top—St. Ignace	57	15	6.66
Total	180	00	52.69

Showing a spherical excess very nearly what it should be.

Our success in reading the angles of this unusually large triangle by the aid of instruments borrowed from the Coast Survey demonstrates, I think, the necessity of procuring, as soon as possible, the instruments that have been asked for. I have now scarcely a doubt that the triangulation can be carried over the entire lake. There yet remain several triangles with sides nearly as long as the ones that have been obtained, and I most sincerely hope that before the commencement of another season the instruments that are necessary to execute this important and interesting work may be furnished.

The proposed scheme of triangulation is shown in the accompanying sketch,

marked C.

ESTIMATES.

I have the honor to submit herewith estimates for continuing the survey for the fiscal year ending June 30, 1869, on the same scale and general plan that has heretofore been adopted.

Very respectfully, your obedient servant,

W. F. RAYNOLDS,

Lieut. Col. of Engineers, Brevet Brig. Gen. U. S. A.

Brevet Major General A. A. HUMPHREYS,

Chief of Engineers U.S. A., Washington, D. C.

Summary of the report on the survey of the north and northwest lakes for the year ending June 30, 1867.

I have the honor to submit the following summary of the operations of the lake survey for the year ending June 30, 1867:

One steamer and two shore parties were engaged during the summer of 1866 in Lake Michigan, carrying the survey of both shores to the southward, and

in executing the primary triangulation, off-shore soundings, &c.

The work was carried on the west side to a point near Two Rivers, Wisconsin, and on the east side of the lake to Little Point au Sable, giving a connected survey from these points to the foot of Lake Huron and head of Green bay. The amount of work done in Lake Michigan was as follows:

Number of primary triangulation stations built	1
	28
Number of secondary triangulation stations built	ZO
	356
Number of sounding stations built	300
	319
Number of buoys placed out and located	213
	448
Number of sextant angles	440

Number of theodolite pointings	10,622
Number of theodolite readings	13, 723
Number of compass readings for magnetic declinations	702
Number of lines sounded with steamer	111
Number of lines sounded with small boats	2, 360
Number of miles sounded with steamer	1, 203
Number of miles sounded with small boats	1, 925
Number of casts of the lead from steamer	1, 070
Number of casts of the lead from small boats	95, 073
Number of square miles of hydrography, off-shore	1,008
Number of square miles of hydrography, in shore	130
Number of square miles of topography	137 <u>4</u>
Number of miles of shore line run	175
Number of level sights taken	128
Number of observations on Polaris for determining meridian	8
Number of miles run by steamer on general duty	4, 72 8
Number of astronomical stations occupied	8
Number of pairs of stars observed for latitude	1, 131

Two steamers and two shore parties were engaged in Lake Superior in connecting between previous surveys in Keweenaw bay and at Marquette, and in making reconnoissance for primary triangulation over the entire lake, and in making an hydrographical survey from Keweenaw Point to Grand island.

The amount of work done in Lake Superior was as follows:

Number of primary triangulation stations built	11
Number of secondary triangulation stations built	32
Number of sounding stations built	376
Number of water stations built	2
Number of base line transit stations built	• 4
Number of buoys placed out and located	348
Number of sextant angles	630
Number of theodolite pointings	13, 544
Number of theodolite readings	19,064
Number of compass bearings for magnetic declination	3
Number of lines sounded with steamer	150
Number of lines sounded with small boats	2, 484
Number of miles sounded with steamer	1, 963 🜡
Number of miles sounded with small boats	1,773
Number of casts of the lead from steamer	1, 426
Number of casts of the lead from small boats	68, 310
Number of square miles of hydrography, off-shore	1, 051 1
Number of square miles of hydrography, in-shore	90
Number of square miles of topography	50 1
Number of miles of shore line	126]
Number of miles run with stadia for topography	21
Number of observations for true meridian	3
Number of vertical angles for topography	338
Number of secondary base lines	7
Number of miles of line of sight opened	3
Number of barometer readings	113
Number of heights measured by barometer	13
Number of miles run by steamer on general duty	7, 3291

Three astronomical parties were engaged, during the first of the season, in Lake Superior, and subsequently in Lake Michigan, in determining the latitude of points by the aid of the differential zenith telescope, and differences of longitude by means of instantaneous signals; and also in reading the angles of

primary triangles at some of the points occupied. Seven points in Lake Superior and eight on Lake Michigan were occupied, and the following work done:

2, 235 pairs of stars observed for latitude.

2, 192 stars observed for instrumental correction, time, &c. 1, 222 pointings of theodolite for primary triangulation.

In the months of April and May, 1867, parties on board three steamers were engaged in the survey of the St. Clair river, and the survey was completed from Port Huron to include the greater portion of the "Delta," comprising the following amount of work:

Number of secondary triangulation stations built	191
Number of sounding stations built	571
Number of buoys placed out and located	32
Number of sextant angles	75
Number of theodolite pointings	15, 695
Number of theodolite readings	16, 361
Number of compass readings for magnetic declination	7
Number of lines sounded with small boats	2, 067
Number of miles sounded with small boats	853
Number of casts of the lead	37, 721
Number of square miles of topography	894
Number of miles run with stadia for topography	538
Number of miles of shore line run	887
Number of miles levelled.	291
Number of vertical angles for topography	1, 165
Number of square miles of hydrography	23
and the state of t	

The following table shows the total amount of work done in the different localities during the season ending June 30, 1867:

Character of work.	Lake Michi- gan.	Lake Supe- rior.	River St. Clair.	Total for year ending June 30, 1867.
Number of primary triangulation stations built. Number of secondary triangulation stations built Number of sounding stations built Number of base-line transit stations built. Number of base-line transit stations built. Number of base-line transit stations built. Number of buoys placed out and located. Number of theodolite pointings. Number of theodolite pointings. Number of compass bearings for magnetic declination. Number of lines sounded with steamer. Number of lines sounded with steamer. Number of miles sounded with steamer. Number of miles sounded with small boats. Number of casts of the lead from steamer. Number of casts of the lead from steamer. Number of square miles of hydrography off-shore. Number of square miles of hydrography in-shore. Number of square miles of hydrography in-shore. Number of miles of shore line. Number of miles run with stadia for topography. Number of observations for true meridian. Number of secondary base lines measured. Number of miles of line of sight opened. Number of miles of line of sight opened. Number of heights measured by barometer. Number of miles run by steamers on general duty. Number of astronomical stations occupied.	1 28 356 319 448 10, 622 13, 793 11, 203 1,	111 322 376 4 348 369 13, 544 19, 664 1, 963 1, 426 68, 310 10, 051 196 21 13 3 3 3 3 3 1, 426 68, 310 1, 201 1, 2	232 75 15,695 16,361 25 25 2,067 853 894 881 538 149 1,165 1	12 251 1, 303 2 4 4 693 39, 861 4, 148 730 6, 911 1, 166 4, 551 2, 496 901, 104 2, 695 1, 503 390 6, 966 1, 104 2, 105 1, 104 2, 105 1, 104 1, 104 1, 105 1,
Number of pairs of stars observed for latitude. Number of stars observed for instrumental correction, time, &c Number of pointings of theodolite for primary triangulation		1, 104 2, 192 1, 224		1, 230 2, 198

The above work in Lakes Michigan and Superior was plotted during the past winter, and comprised 26 sheets of antiquarian paper, embracing 2,725½ square inches of topography and 2,290¾ square inches of hydrography. The St. Clair work has not as yet been projected, the parties having gone into the field immediately upon its completion.

Five thousand four hundred and sixty-four lake survey charts have been dis-

tributed during the year, or 1,829 more than the year previous.

One detail chart of the south end of Green bay, on a scale of one one-hundredand-twenty-thousandth, and one of the north end of Lake Michigan, including the straits of Mackinac and Green bay, on a scale of one four-hundred-thousandth, have been completed and forwarded to the bureau for publication.

At the date to which this report is made the following force was engaged in

prosecuting the operations of the survey:

- 3 steamers in Lake Superior.
- 5 shore parties in Lake Superior.
- 3 astronomical parties in Lake Superior.
- 2 gauging parties, measuring the outflow of the rivers St. Mary's and St. Clair.
- 13 meteorological observers at different points.
 - 2 draughtsmen, reducing maps for publication.
 - 2 assistants, in office and attending to chart distribution.
- 3 assistants, engaged in reducing meteorological observations, copying, &c. Respectfully submitted:

W. F. RAYNOLDS,

Lieut. Col. of Engineers, Brevet Brig. Gen. U. S. A.

Brevet Major General A. A. HUMPHREYS, Chief of Engineers U. S. A., Washington, D. C.

Estimate for continuing the survey of the north and northwest lakes, including Lake Superior, for the fiscal year to commence July 1, 1868, and end June 30, 1869.

For three parties for hydrographical and general triangulation reconnoissance, one for each of the lake survey steamers Search, Surveyor, and Ada, the cost of each party will be as follows, viz:

One assistant, 183 days, at \$4 80 per day	\$880	40
One assistant, 183 days, at \$3 60 per day	658	80
One assistant, 183 days, at \$3 per day	549	
One applicable, 100 days, at 60 per day		
One recorder, 183 days, at \$1 25 per day	228	
One sailing-master, 183 days, at \$2 75 per day	503	25
One mate, 6 months, at \$50 per month	300	00
One steam engineer, 183 days, at \$2 50 per day	457	50
One assistant steam engineer, 183 days, at \$1 50 per day	274	50
One carpenter, 183 days, at \$2 per day	366	00
One steward, 183 days, at \$1 50 per day	274	50
One cook, 183 days, at \$1 50 per day	274	50
One second cook, 183 days, at \$1 per day	183	00
Four firemen, 183 days, at \$1 25 per day	915	00
Fourteen seamen, 183 days, at \$1 per day	2,562	00
Subsistence for the above thirty persons, 183 days, at 50 cents each	•	
per day	2,745	00
550 tons coal for fuel, at \$10 per ton	5,550	
•		

\$50,016 **6**0

•		
For three astronomical, magnetic, and primary triangulation parties, as follows:		
One assistant, 183 days, at \$4 80 per day	\$880 40	
One assistant, 183 days, at \$2 50 per day	457 50	
One cook, 150 days, at \$1 50 per day.	225 00	
One cook, 150 days, at \$1 50 per day. Five men, for boats' crews and laborers, 150 days, at 30 cents per day. Transportation of parties, provisions, camp-equipage, &c., to and	. 600 00	
	500 00	
from the field, at \$250 each way	600 00	
Total for one partyFor three parties	3, 262 90	\$9,78 8 70
For five topographical and hydrographical parties to survey shore- line and adjacent hydrography and topography, the cost will be as follows:		
One assistant, 183 days, at \$4 80 per day	\$880 40	
One assistant, 183 days, at \$3 60 per day	658 80	
One recorder, 183 days, at \$1 50 per day	274 50	
One foreman, 165 days, at \$1 40 per day	231 00	
One steward, 165 days, at \$1 40 per day	231 00	
One cook, 165 days, at \$1 40 per day	231 00	
One waiter, 165 days, at \$1 per day	165 00	
Two leadsmen, 165 days, at \$1 per day	330 00	
Two chainmen, 165 days, at 90 cents per day	297 00 1,848 00	
Subsistence of above twenty-five men, 165 days, at 50 cents each	1,040 00	
per day	2,062 50	
materials for stations, camp and mess equipage, &c	600 00	
Transportation of twenty-five men to and from the field, at \$20 Expense of moving provisions, camp equipage, &c., each way, at	1,000 00	
\$250	500 00	
Total for one partyFor five parties.		46,546 00
For two parties for gauging the outflow of the lakes, the cost will be as follows, viz:		22,
One assistant, 183 days, at \$3 50 per day	\$640 50	
One assistant, 183 days, at \$2 per day	366 00	
Six men, 165 days, at \$2 each per day, including board	1,980 00	
Board of two assistants, 165 days, at \$1 each per day	330 00	
Transportation to and from the field, at \$150	300 00	
Total for one partyFor two parties	3,616 50	7, 233 00
•		•,
Office and miscellaneous expenses:		
Office rent and fuel for one year		
of 365 days, at \$5 30 each per day. Four assistants, heads of parties, 162 days, at \$5 30 each.	3,869 00	
Four assistants, heads of parties, 182 days, at \$5 30 each	3,858 40	
Two assistants, neads of parties, 102 days, at \$4 70 each	1,710 00	
Two assistants, heads of parties, 182 days, at \$4 10 each	1,492 40	
One accountant, 365 days, at \$4 10 per day	1,496 50	
One accountant, 365 days, at \$3 50 per day	1,277 50 3,822 00	
Six assistants, 182 days, at \$3 50 per day One assistant, 182 days, at \$2 90 per day	527 80	
Four copyists and recorders, 365 days, at \$2 50 per day	3,650 00	
One office servant, 365 days, at \$1 50 per day	547 50	
Three steamers in ordinary, at \$1,000 each	3,000 00	
gineers	5,000 00	
Travelling expenses of superintendent and assistants while attending to the duties of the survey	750 00	

Expenses in office, drawing-paper and materials, stationery, Nautical Almanacs, &c)
Total estimate for office and miscellaneous expenses for one year	\$37,401 90
camp equipage, &c	15,098 62
Total estimate	166, 084 82
Danastulla submitted .	

Respectfully submitted:

W. F. RAYNOLDS,

Lieutenant Colonel of Engineers, Bvt. Brig. Gen. U.S. A.

Brevet Major General A. A. HUMPHREYS, Chief of Engineers U. S. Army, Washington, D. C.

A.—Lieutenant Gregory's report on the astronomical work for the season of 1866.

OPFICE UNITED STATES LAKE SURVEY, Detroit, Michigan, May 31, 1867.

SIR: I have the honor to submit the following report of the operations in the astronomical department of the lake survey since the date of the last annual

report:

To First Lieutenant M. R. Brown, corps of engineers, was assigned the charge of the astronomical party last season; all the field work was therefore performed under his direction. He also supervised and aided in the performance of office work until March 12, 1867, when, he being removed from the lake survey, I was

assigned to duty in his place.

Lieutenant Brown took the party into the field with instructions to separate it into three divisions, and to locate the divisions at the best practicable points for the astronomical determination of the large primary triangles of Lake Superior. The first triangle selected was Vulcan, Northeast, and St. Ignace. This triangle covers an area of so great an extent that it may be well to give it a more minute description than it is necessary to accord the others, which, though large, were determined with comparatively little difficulty.

Vulcan station is situated on the highest available eminence on Keweenaw Point, the first platform of the station being eight hundred feet above the lake

and seventy-five above the ground.

The point selected on St. Ignace island is twelve hundred and eighty-nine feet above the lake, and at Northeast an elevation of nine hundred and forty-

five feet above the lake was reached.

Michipicoten island was at first considered the proper location for the station, but, on examination, it was found that the greatest, height was but nine hundred and twenty-nine feet above the lake, and the horizon of this point and that of Vulcan being twenty-five miles apart, it was decided best to occupy the location on the Northeast coast.

The lengths of the sides of this triangle, as since determined by the latitudes and difference of longitudes of the stations at the apices, are: Vulcan-St. Ignace, 93.0 miles; Vulcan-Northeast, 96.6 miles; and St. Ignace-Northeast, 88.46 miles.

Neither the powder flashes of several nights, nor the heliotrope's light for several days, having been seen, the latitudes of the stations were approximately determined, and also the differences of longitude, by transportation of chronometers, in order to determine the azimuths of the stations from each other. The land not being visible, it was necessary that the observers should be able to direct their instruments towards the points where the signals were made, a slight deviation having heretofore thrown the flashes out of the field of view of

the telescopes. After this determination, the flashes were plainly visible by the aid of the telescope, and were frequently seen by the unaided eye, when it was known exactly where to look for them. This triangle being the largest, was therefore the most difficult to determine.

The three others, viz., Vulcan-St. Ignace and Isle Royale, Vulcan-Isle Royale-Whealkate, Vulcan-Whealkate and Huron Mount, were similarly determined, and a description of the means employed for the determination of one will therefore

apply to the others.

The differences in longitude were determined by means of a series of powder flashes made on several nights by the observers at the three stations, the times of the individual flashes being arranged according to a programme previously prepared. The instant of each flash was recorded at all those stations by chronometers, of which the errors and rates were determined nightly. The difference of the local times of each flash, as recorded by the several observers, is the approximate difference in longitude between the stations. The mean of the difference of local times of each night's flashes is the mean result for that night. A mean of all the nights' flashes is given as a final result.

The method for the determination of latitude was that of opposite and nearly equal meridian zenith distances. The zenith telescope was the instrument em-

ployed for this purpose.

Åt all the stations azimuths of prominent objects were determined by frequent observations of Polaris at elongation and culmination. At Vulcan station connection was made with the topographical station located there in 1865.

At the close of the season's work in Lake Superior the party was removed to Lake Michigan, where observations were made for the latitudes of eight stations. The results obtained during the past winter from the observations made last

season I have arranged in tables as follows:

Tables 1 to 8, inclusive, contain the general and final results, with probable error of single, mean, and final results, for the latitudes of eight stations in Lake Michigan, viz: Cana Island, Whitefish Point, Clay Banks, Kewaunee, Sheboygan, Big Point au Sable, Rawley's Point, and Little Point au Sable.

Tables 9 to 15, inclusive, contain general and final results, with probable errors of single, mean, and final results, for the latitude of seven stations in Lake Superior, viz: North East, Vulcan, St. Ignace, Whealkate, Isle Royale,

Huron Mount, and Marquette.

Table No. 16 contains the latitudes of primary triangulation stations, as determined from the astronomical stations; and when the observations were made near no triangulation station the latitude of the nearest shore-line stake is given.

Table No. 17 contains results for differences in longitude between the stations at the apices of the four large primary triangles in Lake Superior. In the table will be found the number of flashes recorded at each station for each night, the mean results for differences in longitude for each night, and the final result, or mean of all the nights, as reduced to the primary triangular stations.

The observers and the instruments used by them at the various stations are as follows: Lieutenant M. R. Brown and Assistant G. Y. Wisner observed at Vulcan station for latitude and difference of longitude, and for latitude at Kewaunee, Whitefish Point, and Sheboygan. I was with the division under Lieutenant Brown a portion of the time during which observations were made at

Vulcan station.

The instruments used by this division were Würdemann transit No. 15, of

32 inch focal length, and zenith telescope No. 1, of same focal length.

Assistant O. B. Wheeler, aided by Mr. G. H. Benzenbergh, observed for latitude and difference of longitude at St. Ignace and Whealkate stations, and for latitude at Big Point au Sable and Little Point au Sable stations. The instruments used were Würdemann transit No. 1, 32-inch focal length, and zenith telescope No. 15, same focal length.

Assistant S. W. Robinson and Mr. A. A. Robinson observed for latitude and difference of longitude at North East, Isle Royale, and Huron Mount stations, and for latitude at Cana Island, Clay Banks, and Rawley's Point stations. The instrument used was a transit, prime vertical and zenith telescope, combined, of 26-inch focal length, manufactured by Ristor & Martin, Berlin.

To the astronomical party was also assigned the duty of reading the angles of the four large primary triangles before mentioned, as well as could be done with the instruments furnished. This was not satisfactorily accomplished. The instruments furnished to the observers were unreliable and entirely unfit for accurate work; and on two of the lines, viz., Vulcan—North East and Vulcan—St. Ignace, the distances were so great—being respectively 96.6 miles and 93 miles—that the heliotropes were never seen. The other lines of these triangles have all been seen over, and the angles approximately measured by aid of the heliotrope; it is, therefore, confidently expected that, with good instruments and the employment of some apparatus to enable the observers to read the angles at night, all the angles can be successfully measured. That steady, brilliant lights; used after dark, could be seen, and angles measured between them at distances at which the heliotrope cannot be seen, I think beyond a doubt. The powder flashes made at North East and St. Ignace stations, for differences in longitude, were frequently distinctly visible at Vulcan station to the unaided eye, on those nights when extraordinary refraction appeared the greatest, and, as before stated, the heliotropes were never seen over these lines.

The extraordinary refraction, or mirage, which is at times so wonderfully great in Lake Superior region, can be made of so much importance in aiding the long lines of sight, that a brief account of some of its exhibitions may be useful and interesting. The following extracts are from Lieutenant Brown's field

report for the season 1866. This was at Vulcan station:

"In order to test the amount of refraction at night, and to ascertain how much advantage would be found in attempting to obtain readings to lights at night, instead of using the heliotrope, I kept watch, or had some one stationed to watch, on favorable nights, for the Grand Island light—one of the fourth-order, distant about 85 miles, and elevated 287 feet above the lake. It was seen for short intervals several times, indicating an addition of height of over a thousand feet, given in effect to the light by extraordinary refraction; the first platform of Vulcan station being 800 feet above the lake.

"On one occasion during the past summer I saw land in the vicinity of Portage entry, not over 150 feet in height, elevated to an apparent height of over 2,000 feet, judging by comparison with the known height of Whealkate, which was not much affected at the time, and this without much distortion of outline.

The appearance lasted over a half an hour."

This phenomenon is no rare occurrence, scarcely a clear day passing without greater or less exbibitions of it. It is usually noticeable from the middle of the afternoon until after sunset, the greatest effects having been noticed just before sunset. The strange and varied appearances presented by land, as seen at great distances over the lake, and sometimes at short distances, have been witnessed

by nearly all the persons connected with the lake survey.

Assistant Wisner states that when engaged at Vulcan station, he saw, on one occasion, three distinct apparitions, one above the other, of the heliotrope, at Isle Royale station; the lowest of these was of a ruddy orange-color, the middle one a pure white, and the top one a very pale white. At another time he saw two apparitions of the same heliotrope, both clearly distinct and of the same color. The distance between the heliotropes appeared to be about the elevation of the island above the water-line. The island itself was also greatly elevated in appearance on both of these occasions. The distance between Vulcan and Isle Royale stations is 59.3 miles, Vulcan being 800 feet and Isle Royale station 400 feet above the lake.

When at the station on Point Abbeye, in September last, I saw the Huron islands elevated in appearance to at least twice their real height, and above them appeared their inverted images; the station on the top of West Huron island

being in contact with the station on its inverted image.

The distance from Point Abbeye station to that on Huron island is not over seven miles. On the same afternoon the whole of Keweenaw Point, as seen from Point Abbeye, was greatly elevated, and also with an inverted image above it. These appearances frequently assume many varied forms in the course of a few minutes, and again appear fixed for sometimes nearly an hour at a time.

I append, also, some sketches, the originals of which were made by Assistant Robinson, when at North East station, illustrating the appearances presented by Isle St. Ignace to an observer at North East station, 88.46 miles distant, on the

afternoons of August 13 and September 10, 1866.

The office-work of the past season has been performed by Lieutenants M. B. Brown, James Mercur, B. D. Greene, corps of engineers, Assistants O. B. Wheeler and G. Y. Wisner, and myself, Assistant S. W. Robinson having left the survey last fall, on the arrival of the party from the field.

Each reduction has been computed by two persons, each working entirely independent of the other. Their results have afterwards been carefully compared

and corrected.

All who have been connected with this department of the survey since I assumed direction of the work have performed their parts with assiduity and zeal; and to Assistants Wheeler and Wisner I am especially indebted for their constant attention to the work, for which their ability and experience has peculiarly fitted them.

I am, sir, very respectfully, your obedient servant,

JAMES F. GREGORY, First Lieutenant Corps of Engineers.

Brevet Major General A. A. Humphrbys, Chief of Engineers U. S. A., Washington, D. C.

Latitude of lake survey astronomical station, Cana island, Lake Michigan.

TABLE No. 1.—General Results.

Date.	pairs ved.	Mean results for			Probable error of a-	
Date.	No. of pairs observed.	each	nig	h t.	Single result.	Mean result.
1866.		٥	,	"	"	"
September 30	40	45	05	16.73	土 0.706	士 0,112
October 1	129	45	05	16, 58	0.897	0.079
October 2	31	45	05	16, 69	1.290	0, 232
Sum	200	Probable Final res			result = 45° 05′ 16.	$=\pm 0.065$ $=\pm 0.065$

Latitude of lake survey astronomical station, White Fish Point, Lake Michigan.

TABLE No. 2.—GENERAL RESULTS.

Date.	pairs ved.	Mean results for			Probable error of a-	
Detc.	No. of obser	each	nig	ht.,	Single result.	Mean result.
1866. October 1 October 2	33 64	o 44 44	, 52 50	,, 50, 88 50, 26	± ·1.996 1.618	± 0.347 0.202
Sum	97	Probable Final res			l result = 44° 52	$ = \pm 0.172$ $50.''47 \pm 0.172$

Latitude of lake survey astronomical station, Clay Banks, Lake Michigan.

TABLE No. 3.—GENERAL RESULTS.

Date.	pairs rved.		esul		Probable error of a—		
Dan o.	No. of pairs observed.	each night. Single result		Single result.	Mean result.		
1866. October 4 October 5	20 127	o 44 44	41 41	" 15. 72 15. 2 5	± 0.968 1.119	± 0.216 0.099	
Sum	147	Probable Final res	erro ult	or of fina	l result = 44° 41	= ± 0.090 1' 15."31 ± 0.090	

Latitude of lake survey astronomical station, Kewaunee, Lake Michigan.

TABLE No. 4.—GENERAL RESULTS.

Date.	pairs ved.	Mean results for	Probable error of a-			
Dair,	No. of pair observed.	each night.	Single result.	Mean result.		
1866. October 7 October 8	52 58	0 ' " 44 27 12.27 44 27 11.63	" ± 0.932 0.783	± 0.122 0.109		
Sum	110	Probable error of final result				

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Latitude of lake survey astronomical station, Sheboygan, Lake Michigan.

TABLE No. 5.—GENERAL RESULTS.

Date.	pairs ved.	Mean results for	Probable error of a-		
Date.	No. of pairs observed.	each night.	Single result.	Mean result.	
1866. October 13	· 44	0 / " 43 45 54.00 43 45 54.39	± 0.970 0.972	± 0.146 0.158	
Sum	82	Probable error of fina Final result	l result = 43° 45	= ± 0.410 54.413 ± 0.416	

Latitude of lake survey astronomical station, Big Point au Sable, Lak-Michigan.

TABLE No. 6.—GENERAL RESULTS.

Date.	pairs ved.	Mean 1	esul	ts for	Probable error of a-		
Date.	No. of pair observed.	each night.				Mean result.	
1866. October 4	25 68	o 44 44	, 03 03	30, 36 30, 70	± 0.670 0.865	± 0.134	
October 6	48	44	03	30. 20	0.678	0.095	
Sum	141				l result = 44° 03′		

Latitude of lake survey astronomical station, Rawley's Point, Lake Michigan.

TABLE No. 7.—GENERAL RESULTS.

	pairs ved.	Mean results for each	Probable error of a-		
Date.	No. of obser	night. Single result. Mea			
1865. October 8	93 54	0 ' " 44 11 34.86 44 11 34.67	± 0.839 0.793	± 0.087 0.107	
Sum	147	Probable error of final Finale result	results44° 11′	= ± 0.′00 34.″755 ± 0.tr	

Latitude of lake survey astronomical station, Little Point au Sable, Lake Michigan.

TABLE NO. 8.—GENERAL RESULTS.

	of pairs		esul		Probable error of a-		
Date.	No. o	each	nig	ht.	Single result.	Mean result.	
1866. October 14 October 15 October 16 October 17 Sum	10 73 65 59 207	Probable			± 0.626 0.839 0.851 0.739		

Latitude of lake survey astronomical station, north east coast, Lake Superior. TABLE No. 9—GENERAL RESULTS.

	pairs ved.	Mean results for each night.			Probable error of a-		
Date.	No. of pairs observed.				Single result.	Mean result.	
1866.		0	,	"	11		
June 29	15	48	15	36, 95	士 0.555	± 0.143	
July 7	26	48	15	38, 83	1.369	0.268	
July 8	30	48	15	38, 29	0.593	0.108	
July 10	31	48	15	38.89	0.768	0.138	
July 17	4	48	15	39, 60	0, 407	0. 203	
August 11	3	· 4 8	15	20.07	0, 405	0, 234	
August 15	25	48	15	35, 47 24, 92	0.762	0.152	
Sum	134	Probable Finale re	erro	or of fina	l result	$ = \pm 0.0070$ 38.519 ± 0.0070	

Latitude of lake survey astronomical station, Vulcan, Lake Superior. Table No. 10.—General Results.

	Mean results for		Probable error of a-				
Dute.	No of pairs observed.	each	nig			Mean rés	ult.
1866.	200	0	1	,,	"	.1.	.,
August 15	29 20	. 47 47		44, 07 44, 32	± 1.153		0.210 0.316 •
August 17	30	47	26	44. 10	0. 883		0. 161
August Id	5	47	26	44, 58	0.764		0. 341
Sept. 16	7	47	26	44.81	0.613		0, 232
Rept. 17	4	47	26	44. 13	0, 639		0.349
Sept. 18	6	47	26	44. 95	0.942		0.334
	101	Probable Final res	erro ult.	r of fiue	l result	= ± 44.251 ±	0, 104 0, 104

Latitude of lake survey astronomical station, Saint Ignace, Lake Superior.

TABLE	No.	11	-GENERAL	RESULTS.
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Dete	pairs ved.	Mean	Mean results for		Probable error of a-		
Date.	No. of pairs observed.	each night.			Single result.	Mean result.	
1866.			,	11	"	"	
August 3	14	48	47	26, 99	± 0.856	± 0.228	
August 4	23	48	47	28, 12	0, 451	0, 009	
August 9	25 8 41	48	47	28.49	0. 041	0.008	
August 14	8	48	47	28, 56	1.293	0. 459	
August 15	41	48	47	28, 33	0. 672	0. 104	
August 16	1	48	47	28, 20			
August 24	23	48	47	23. 36	0.890	0.185	
August 28	4	48	47	29.45	0.883	0.440	
August 29	9	48	47	27.98	0. 6 50	0. 216	
Sum	148				result 48° 47	= ± 0".063 ' 28".23 ± 0".058	

Latitude of lake survey astronomical station, Whealkate, Lake Superior. TABLE NO. 12.—GENERAL RESULTS.

Date.	pairs	Mean results for	Probable e	Probable error of a-		
	No. of pairs observed.	each night.	Single resuit.	Mean resu.:.		
1866.		0 1 . "	"	,,		
September 18 September 21	3	47 04 19.27	± 0.445	± 0.314		
September 21	29	47 04 18.48	0.706	0. 131		
September 24	25	47 04 18.68	0,576	0, 115		
September 25	46	47 04 18.24	0. 330	0,048		
8um	103	Probable error of fin Final result		= ± 0".059 18".44 ± 0".059		

Latitude of lake survey astronomical station, Isle Royale, Lake Superior. TABLE No. 13.—General Results.

Date.	pairs ved.	Mean results for	Probable error of a-		
, 240.	No. of pairs observed.	each night.	Single result.	Mean result.	
1866. • August 26 September 1 September 2 September 3 September 16	65	6 7 7 55.925 48 07 54.586 48 07 54.644 48 07 55.291 48 07 64.807	が ± 0.741 0.736 0.846 0.764 0.629	± 0.053 0.095 0.105 0.169 0.100	
Sum	190	Probable error of final Final result	result 48° 07′ 5	= ± 0".056 64".807 ± 0".056	

Latitude of lake survey astronomical station, Huron mountain, Lake Superior.

Table No. 14.—General Results.

Date.	pairs ved.	Mean 1	esul	ts for	Probable e	rror of a-
Date.	No. of pairs observed.	each	nig!	ht.	Single result.	Mean result.
1866.			,	"	"	"
September 20	4	46	52	49.3	± 0.196	± 0.098
September 21	33	46	52	51.7	0, 236	0.041
September 24	151	46	52	51.6	1, 035	0.084
September 25	124	46	52	51.9	1,217	0. 107
Sum	312	Probable Final res			l result 46° 52′	= ± 0". 064 51". 719 ± 0". 064

Latitude of lake survey astronomical station, Marquette, Lake Superior. Table No. 15.—General Results.

Date.	pairs	Mean results f		able error of a—
Date.	No. of pobserv	each night.	Single res	ult. Mean result.
1865. July 10 July 25 July 26 July 30	19 30 32 35	46 31 59.1 46 32 00.4 46 31 59.2 46 32 00.6 Probable error of Final result	07 70 02 02 0. 02 0. 0.	

Latitudes of lake survey primary triangulation stations, or shore line stakes nearest to astronomical stations.

TABLE No. 16.—DETERMINED 1866.

Nam	es of primary triangulation stations, or shore line stakes.	No	rth la	titude.	Difference in lati- tude from astro- nomical stations.
LAKE MICHIGAN.	Little Point au Sable, stake No. 161 Big Point au Sable Kewaunee, observing post No. 3. Clay Banks Cana island White Fish Point, stake No. 11	3 44 44 44 44 45 44	40 03 27 41 05 52	22. 76 27. 76 14. 00 15. 793 16. 475 50. 63	- 01.64 - 02.71 + 03.00 - 00.483 00.00 + 00.21
LAKE SUPERIOR.	Northeast Saint Ignace Vulcan Whealkate Isle Royale Huron mountain	47	15 47 26 04 07 52	24. 27 28. 87 44. 251 18. 125 54. 794 52. 495	" - 14.249 + 00.64 - 00.00 - 00.315 + 00.29 + 00.776

Results for differences in longitude reduced to primary triangulation stations, Lake Superior—Season, 1866.

TABLE No. 17.

Date.	ber bes.	Difference gitude		ber hes.	Difference gitud		ber hes.	Difference of	
Date.	Number of flashes.	St. Ignace Vulca		Number of flashes.	Vulcan w North E		Number of fisshes.	St. Ignace w North E	rest of
1866.		,	"			"	İ	,	<i>u</i> ·
Aug. 4	2	00	13.98	2	6	46. 36	3	7	00.38
Aug. 6	8	00	14. 22 13. 89	7	6	46. 18 46. 23	8	7 7	00.53 00,23
Aug. 14	10	.00	1.5. 09	11	0	40. 23	1 10		00, 23
Mean			• • • • • •		6	46. 26	ļ	7	00, 38
					Isle Royal of Vulc			Isle Royale of St. Ign	
			•	1		"		. ,	μ
Aug. 26				9	3	04.63	1		
Aug. 27	6	00	13, 80	10	3	04.76	6	2	51.06
Aug. 28	6	00	14. 17	10	3	04.94	7	2 2	50.80
Aug. 29 Aug. 30	9	00	13.84	9		04.61	7 9	2	50.75 50.78
Mean		· 00	13.98					2	50.85
•		Whealkate of Vulce						Whealkate of Isle Ro	
		,	"					,	"
Sept. 9	6	3	27.36	7	3	04.66	11	00	22,77
Sept. 10	21	3	27, 26	21	3	04.74	21	00	22,53
Sept. 13	20	3	27.44	20	3	04.70	21	00	22.76
Mean					3	04.72		00	22, 69
• .					Huron mo west of V			Whealkate	
					. ,	"		• ,	"
Sept. 19			27.24	17	00	30.56	18	2	56, 64
Sept. 20 Sept. 21	1		27.65	26 24	00 00	30, 73 30, 42	27 26	2	56.91 56.65
Sept. 21			27. 07 27. 12	24		JU. 42	20	2	eo. 00
Mean		3	27.31		00	30.57		2	56.73

B.—Assistant D. F. Henry's report for season of 1866.

Office United States Lake Survey,

Detroit, November 1, 1866.

SIR: I have the honor to submit the following report of work accomplished

during the past season:

After having finished the winter's work, and made comparisons of the base apparatus to test the compensation of the tubes, on the 14th of May left Detroit on the steamer Search, to look again for the shoals at the mouth of the Detroit river. In my report for the month of May, I gave a detailed report of my search, and success in finding one about three miles from Bar Point.

On the morning of the 24th of May, I again left Detroit on the steamer

Search, for Lake Superior, in accordance with your orders, a copy of which I

enclose, having on board the parties of Assistants Mayer and Molitor.

Being detained by adverse winds, I was not able to land them until the 29th:
Assistant Molitor about ten miles above Marquette, near Partridge island, and
Assistant Mayer on the east side of Huron bay.

I next visited the base line on l'Anse bay, and found that the progress made

in the grubbing was very satisfactory.

I ran a transit line through the cut on the side. I then went to Copper Harbor and looked at the location of Vulcan station, which Assistant Lamson was

then building.

On the 4th of June I took Lieutenant M. R. Brown on board, and ran over to the north shore. After much detention on account of bad weather, I selected the points for the main triangulation, on Isle St. Ignace, Michipicoton island, and one on the northeast coast.

The land on that side is very much broken, and the hills mostly bare on top. The rocks are almost entirely of the igneous formation. At St. Ignace my barometrical determination of the height differed only eight feet from Bayfield's

estimate.

I made, while at St. Ignace, a partial survey of the harbor in which we lay. This differed so little from Bayfield's sketchings as to again make me wonder at the wonderful accuracy with which he did his work, considering the time and means at his disposal.

At Michipicoton I had little trouble in finding the highest point, a hill about

930 feet above the lake.

On the northeast coast I was more troubled, the hills six or eight miles from the shore being much higher than those near by; but I selected a point which you afterwards approved of—a bare hill about 940 feet high, and about two miles back. Assistant Robinson has since found the height of the hills back to be about 1,500 feet, and therefore much better point for the station.

On the 20th of June I returned to Copper Harbor, where I found the parties of Assistants Wheeler and Robinson. I took them on board and ran to Portage entry, where I found you and Brevet Brigadier General O. M. Poe, corps of engineers. We then ran back to the north shore, and landed the above parties at stations Northeast and St. Ignace, and returned to Copper Harbor by way of Fort Williams and Isle Royale, on the 26th. After visiting the camp of Assistants Mayer and Molitor we ran to Marquette, where you and General Poe left for Detroit.

On the 29th I visited the base line, to examine the progress of the grubbing, &c.

On the 30th I sounded out to and around Stannard's Rock.

On the 2d of July I made a minute topographical sketch of Little Gull isle, near Mauitou island, for light-house purposes, and commenced repairing and whitewashing the primary triangulation stations at Kewenaw bay.

On the 5th I took Mr. Harding down on the base line, and inspected and ac-

cepted three miles of it.

On the 7th I ran to Isle Royale and commenced building a 30-foot station on the point selected by you. This station occupied me until the 12th, and then I was detained by the fogs (which have hung over the lake, particularly near the north shore, for the greater part of the season) until the 16th, when I ran for

Copper Harbor.

On the 18th and 19th, I built two water stations on Stannard's Rock, and made a minute survey of it. What is known as Stannard's Rock is the highest part of the shoal, and is a mass of porphyritic trap about twenty feet across, and rising about three feet above the present level of the water. The bad part of the shoal extends about one-half mile north-northwest from the rock, at which distance there is only two and one-half feet of water over the rocks. The ridge

extends about four miles north by east from the rock, where it falls off from 26 fathoms to 70 fathoms.

On the south and east sides the water is very deep, while on the west and north it runs off very gradually. One of the stations I built on the north shoal and the other on the rock itself. The latter still remained the last time I visited the rock, the 28th of August, although we had had two or three severe storms.

I think there is no doubt a day-beacon might be constructed which would stand through all the seasons. I got a base line for the location of the buoys by stretching a small cord, of which the length was known, over a series of floats, from the rock nearly at right angles with the line between the stations. Simultaneous angles were then read from each station, between the other and a flag at the end of the cord.

I also at this time sounded several lines with the steamer connecting the shoal

with the shore work of Keweenaw.

On the 20th I took Lieutenant Brown and supplies for the north shore parties on board, and run over to the northeast. The next day we run to St. Ignace, and then back to northeast, carrying a chronometer with us to determine the difference of time between the stations, in order to get their approximate azimuth from Vulcan.

Running back to Copper Harbor, I spent the next few days in repairing stations and steamer soundings around Stannard's Rock, and after going to Marquette for coal, I run over to the north shore in a dense fog extending all across the lake, to move the astronomical parties up on the hill, according to your instructions.

On the 3d of August I ran back to Marquette, where you came on board. The 6th we ran to Houghton, visiting on the way the camps of Assistants

Mayer and Molitor, and the base line.

The next day we ran to Copper Harbor, sounding a line out to Stannard's Rock on the way, and then tried to do the sounding off Copper Harbor, but the weather would not permit. Made a survey of the improvement at Lac La Belle, and then ran to Portage entry and Marquette, visiting Assistants Mayer and Molitor on the way.

On the 10th Assistant E. S. Wheeler was by your orders sent to Superior

City, to make a resurvey of the mouth of St. Louis river.

On the 12th, you having left for Detroit, I ran to Copper Harbor.

On the 16th I ran over to the north shore, and moved Assistant Robinson's party to Isle Royale, having to go to Fort William on the way for powder.

Having run back to Copper Harbor on the 21st, I was from that time until the 25th trying to get Assistant Molitor's party on board, a strong wind blowing all the time from the northwest. At the latter date I moved him over to the base line to commence the grading.

On the 30th Assistant E. S. Wheeler rejoined me.

After having run to Marquette for coal, on the 31st I landed the base apparatus, &c. I turned over the steamer Search to Lieutenant J. F. Gregory, and went into camp on shore to measure the base line.

Whenever practicable, I have sounded lines when running across the lake. The deepest water (978 feet) was found about thirty miles off Manitou island.

The water seems to be generally deeper on the south side of the lake, though

· the higher hills on the north side made me think it would be different.

The bottom of the lake, where the depth is from sixty to ninety fathoms, is red sand and clay. This clay is of all shades of yellow and brown. Where the depth exceeds ninety fathoms the bottom is almost invariably clay. I have a number of specimens, and I hope they will be examined by a good microscopist, to ascertain whether it contains any of the infusoria found at the bottom of the ocean.

In sounding a line from Manitou island to Pie island, I found a very remark-able ridge, on which the water was less than 100 fathoms in depth, though there is 150 fathoms on each side of it.

During the season I also visited the Batteau Rock, about twenty miles from Isle Royale, which forms a shoal very similar to Stannard's Rock, and on which I think a day beacon would be of great advantage; for, although there is very little commerce as yet with the north shore, yet during the summer months steamers with pleasure parties are frequently running to Isle Royale and Thunder Bay, and might in thick weather get far enough from their course to run on these rocks.

For a month after going into camp it rained almost every day, and in fact we had not more than two weeks good weather during the whole time I was there.

I built a comparator house twenty-two feet long, six feet wide, and eight feet high, in which I put up the comparator posts, and made comparisons of the standard bar and tubes.

Assuming the bar to be the same length as formerly, then these comparisons show that tube No. 1 is 0.0027 inches longer than it was in 1864, and that tube No. 2 is 0.0057 inches shorter. This is probably due to the putting in the new agates, and general repairs made by William Würdemann last year.

Last fall there were but about one-half dozen wet spots on the whole base line, but now it is nearly covered with water, and requires side ditches the greater part of the length of it to carry off the surface water. The subsoil being hard clay upon sandstone, having an almost horizontal stratification, there is no natural drainage, and but little absorption of water falling on the surface.

About eight thousand feet was prepared for measurement, and after the side

ditches had taken off most of the water I commenced measurement.

I sank a stone five feet long, having a brass frustum of a pyramid let into its upper surface under the south base station, and two other reference stones about 100 feet on each side, at right angles to the line.

Contact was made with a plumb line of fine copper wire, let down from the centre of a trivet on the centre post, the bob of 4½ weight being over a point in

the brass.

I measured 284 tubes, but the ground was so shaky in places as to make even that unreliable. I closed the measurement on a point in a brass, similar to the one at the commencement, let into a stone four feet long sunk beneath the surface of the ground.

On the 14th, having received orders to return to Detroit, I put my party n board of the steamer Search, ran to Houghton to communicate your orders to Assistant Lamson, and on the 16th went to Assistant Mayer's camp and took

his party on board.

After coaling at Marquette I ran to Grand island, where I left three shore

party boats.

I then went to Middle island, Lake Huron, to find a reported shoal between the island and mainland. As there was a vessel still on the shoal, I had but little trouble in finding and locating it. Having made a minute survey of it I ran to Detroit, arriving here on the 18th.

Since that time I have been engaged in laying up the steamer, and in making

a plot of the shoal found.

I beg leave, here, to thank Assistant E. S. Wheeler and Recorder David Wallace, for the able, efficient, and willing manner in which they have performed all their duties during the past season.

Very respectfully, your obedient servant,

D. F. HENRY,
Assistant Lake Survey.

Colonel W. F. RAYNOLDS,

Corps Engineers, Superintendent Lake Survey, Detroit.

C .- Meteorological report.

OFFICE UNITED STATES LAKE SURVEY. Detroit, May 31, 1867.

Siz: I have the honor to submit the following report on the reductions of

the meteorological records kept at the several lake survey statious.

On the 1st of January, by your orders, I took charge of the meteorological department, and have therefore had but little time to make the necessary reductions, and have been obliged to omit some important parts of the observations.

The situation of the several stations are as follows:

Superior, at the head of Lake Superior, near the mouth of St. Louis river. Latitude (approximately) 46° 46' 30" north; longitude 92° 03' 28" west; height of observatory above the lake, forty feet. Observer, E. H. Bly. It is protected on the west side by a range of hills rising about 600 feet above the lake, and on the east side by a lower range forming a valley through which the prevailing winds, northeast, have a free sweep from the lake.

Ontonagon, at the mouth of Ontonagon river, on the south side of Lake Superior; latitude 46° 52' 30" north; longitude 89° 30' 30" west; height of observatory above the lake, ten feet. Observer, Hampton B. Smith. A range of high hills runs from five to twelve miles back of the town, which is thus protected from the southerly winds, but fully open to those from the lake.

Marquette, east of the middle of the south side of Lake Superior; latitude 46° 32' 51" north; longitude 87° 22' 57" west; height of observatory above the lake, ninety-two feet. Observer, H. S. Bacon. Hills from 400 to 600 feet high are on the east and south of the town. The height of Lake Superior above the sea level is about 620 feet.

Milwaukec, on the west side of Lake Michigan, opposite the widest part of the lake; latitude 43° 03' north; longitude 87° 55' west; height of observatory above the lake, fifteen feet. Observer, J. A Lapham. A range of low hills are situated to the northwest and southwest of the city, which is fully exposed to the winds from the lake, but those from the land are much broken in their

Grand Haven, on the castern side of the lake, at the mouth of Grand river, directly opposite Milwaukee; latitude 43° 05' north; longitude 86° 12' 33" west; height of observatory above the lake, twelve feet. Observer, Heber Squier. A range of sand dunes breaks the winds from the north and east. Lake Michigan is approximately 576 feet above the sea.

Thunder Bay island, Lake Huron; latitude 45° 02' 13" north; longitude 83' 11' 26" west; height of observatary above the lake, forty feet. Observer, J. J. Malden. A low rocky island about two miles from the shore, which is of the same character. It is situated north of the centre of the lake, and is fully exposed to all winds.

Tawas City, on a bay of the same name, near the mouth of Saginaw bay. Lake Huron, opening to the south and west; latitude 44° 15' 57" north; longitude 83° 30' 54" west; height of observatory above the lake, thirteen feet. Observer, C. H. Whittemore. A range of high lands to the north and east of the town protects it from winds from those quarters.

Lake Huron is about 570 feet above the sea.

Detroit, on a river of the same name; latitude 42° 19' 58" north; longitude 83° west; height of observatory above the river, twelve feet. Observer, John Brennan. Country low and flat all around the city. Approximate height of the river above the sea, 550 feet.

Monroe City, at the western end of Lake Erie; latitude 41° 53' 36" north; longitude 83° 19' 26" west; height of observatory above the lake, seven feet. Observer, John Lane. Country much the same as at Detroit, but the town is exposed to the northeast winds from the lake.

Cleveland, near the middle of the southern side of Lake Erie; latitude 41° 30′ north; longitude 81° 47′ west; height of observatory above the lake, eighty-five feet. Observer, Benj A. Stanard. The city is built on the bluffs overlooking the lake, and has but little high land back of it.

Buffalo, at the eastern extremity of Lake Erie; latitude 42° 53' north; longitude 78° 55' west; height of observatory above the lake, twenty-five feet. Observer, Edward Dorr. Country rather low and flat, but exposed to the southwest and west winds from the lake. The approximate height of Lake Erie above

the sea level is 544 feet.

Fort Niagara, at the mouth of the Niagara river, at the western end of Lake Ontario; latitude 43° 15' north; longitude 79° 55' west; height of observatory above the lake, 27.5 feet. Observer, Lewis Leffman. Fully exposed to winds from the lake, but somewhat protected from the low hills to the south, which form the Falls of Niagara.

Charlotte, at the mouth of Genesee river, near the middle of the southern side of Lake Ontario; latitude 43° 12′ 34″ north; longitude 77° 51′ west; height of observatory above the lake, 37.5 feet. Observer, Andrew Mulligan. The town lies on both sides of the valley formed by the Genesee river; low hills to

the west and south.

Sackett's Harbor, on a bay at the eastern end of Lake Ontario; latitude 43° 55' north; longitude 75° 57' west; height of observatory above the lake, 30.6 feet. Observer, Henry Metcalf. Country around the town low and flat, exposed to all winds from the westward. Lake Ontario is about 235 feet above the sea.

GENERAL METEOROLOGICAL REDUCTIONS.

In tables A will be found the reductions of the observations at the several stations, from July to December, inclusive, for 1866. The reductions for the rest of this year were given in the last report.

In these the barometer has been reduced to the freezing point; the elasticity, the humidity, and the gaseous pressure of the atmosphere, computed from the

psychrometer readings; and the winds resolved and reduced.

Tables B give the maximum, minimum, and mean of the barometer and thermometer, and the amount of rain-fall for the months, seasons and years, since the commencement of the observations.

Tables C, D, and E, are compiled from the above, and show the stage of the barometer and thermometer, and amount of rain-fall for the several years.

By comparing the mean yearly mean of the barometer given in the last column but one of table C, with the mean for the several years given in table F, it will be seen that the greatest amount of mean of any year differs from the general mean is 0.083 inch, and that the average variation is less 0.05 inch. Therefore, the means given in the tables differ from the means of the several stations, probably less than 0.01 inch.

In the same manner comparing tables D and G, we find the mean temperature for each year differs from the general mean temperature by less than a

degree.

The range of the barometer given in table C is only that of the extremes of the tri-daily observations, and is therefore smaller than the true range.

The range of temperature is obtained from the extreme readings of the maxi-

mum and minimum thermometers.

In the following tables the stations are grouped according to their latitudes, and the general mean and range of the temperature given. The temperature generally increases as the latitude decreases, though there are some anomalies. Milwaukee is nearly two degrees colder than Grand H aven, thoughthe latitude is about the same.

Superior has a lower temperature than Ontonagon or Marquette, though there

is little difference in the latitudes of the three places. This may be due to the valley formed by the hills on each side of the former place, through which the northeast winds of the lake draw with great violence. Again, the temperature of Tawas City is a degree and a half higher than that of Thunder bay. This is also due to the position of the two stations, the latter being fully exposed, and the former only open to southwesterly winds.

The range also varies greatly, but this is probably due to one station not being affected by some warm or cold spell, which is experienced at another, as the

observations are for a series of years.

Table showing the change of temperature for different latitudes

Stations	·	atite	ide.	Lor	gitz	ıde.	Temper	ature.
				L			Mean.	Range.
			"	0	,	",	۰	
Ontonagon				89	30	30		135
Superior				92	3	28	38.5	137
Marquette				87	22	57	41.5	136
Thunder Bay island				83	8	34	42.6	118
Tawas City				83	30	54	44.2	115
Sackett's Harbor				75	57	0	46.9	13:
Port Niagara				79	55	0	46.6	108
Clarlotte				77	51	0	48.1	119
Grand Haven				86	12	33	47.3	107
Milwaukee				87	55	0	45.8	127
Buffalo				78	55	0	47. 0	115
Detroit				83	0	0	48. 1	117
Monroe City				83	19	26	49. 5	122
Cleveland		1 30	0	81	47	0	49.8	111

In the following table the stations are grouped according to their height above the sea level, and the mean and range of the barometrical readings given

This also shows some rather curious anomalies, but none of much consequence

Table showing the relation of the height of the barometer to the height of the station above the sea.

Stations.	Height above	Barometer -	+ 28 inches
	ine sea.	Mean.	Range.
	Feet.	-	1
Marquette	710	1.262	2 109
Superior		1, 301	1.978
Ontonagon			1,799
Cleveland		1, 321	1,997
Thunder Bay island		1.341	2.079
Milwankee		1.358	2 056
Grand Haven	588	1. 353	1.6%
Tawas City		1. 359	2.552
Buffalo		1. 359	2 136
Detroit		1.368	2 078
Monroe City		1.392	3 041
Charlotte		1.699	2 330
		1. 692	2 182
Sackett's Harbor			2.10
Fort Niagara	962.5	1.732	700

HALF-HOURLY OBSERVATIONS AT THUNDER BAY ISLAND.

A series of half-hourly observations was taken during two years, from December 1, 1863, to November 30, 1865, at Thunder Bay island, Lake Huron. These have been reduced in the same manner as the general meteorological abstract given in table A.

Table I gives the monthly means of the above, and table K the means of the several half hours for spring, summer, autumn, winter, and for the two years.

The latter tables are graphically represented in diagram 1. In the first line of the diagram are given the curves of the total and gaseous atmospheric pressure represented by a full and broken line respectively. To save room the curves were not plotted according to their relative height, and the gaseous is placed above the total curve, though it properly should be below.

In the second line are given the curves of temperature, the dry-bulb being represented by a full, and the wet by a broken line; in the third the relative amount of humidity in the atmosphere; in the fourth, the relative amount of rain-fall for each hour; in the fifth, the velocity of the wind; and at the bottom are given the resultant directions from which the wind blew at each hour.

The times of the maxima and the minima of the barometer, thermometer, and of the humidity, are given in the following tables, the first of which is arranged according to the state of the atmosphere, and the second according to the seasons.

	BARO Spring.	ROMETER.	BAROMFTER, TOTAL PRESSURE + 28 INCHES, pring. Summer. Auto	TESSURE +	28 INCHES	TES. Autumn.	Winter.	ter.	Year.	
Бініс.	Hour.	Amount.	Hour.	Amount.	Hour.	Amount.	Hour.	Amount.	Hour.	Amount.
let minimum Jet maximum 24 unnimum 24 maximum	7. m. 5 CO a. m. 10 CO a. m. 4 CO p. m. 10 30 p. m.	1.1.1.1 32.2.2.1 32.4.2.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.	A. m. 10 00 a. m. 600 p.m.	1.417	2.30 s.m. 10 30 s.m. 3 00 p.m. 9 00 p.m.	1. 370 1. 381 1. 354 1. 376	7 30 P. H. 7 30 P. H. 7 30 P. H. 7 30 P. H.	1.326 1.314 1.314 1.342	7. m. 4. 30 s. m. 10 00 s. m. 10 30 p. m.	1.352 1.367 1.341 1.356
Меш.	6 30 a.m. 1 00 p.m. 8 60 p.m.	1.317	6 30 a.m.	3 1.408	{ 3 30 a.m. 12 00 m. 7 00 p.m.	\$ 1.570	7 30 s.m. 11 30 s.m. 5 30 p.m.	1.332	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.354
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lst maximum lst mirimum 2d maximum anistanam	0 30 s.m. 3 30 p.m.	1.163	3 30 s.m. 4 30 p.m.	1,000	12 00 mid. 2 30 p.m.	1. 108 1. 058	9 30 a. m. 5 30 a. m. 7 30 p. m.	1. 246 1. 226 1. 246 1. 246	3 00 p. m.	1. 125
Mean	6 30a.m. 7 30p.m.	3 1.147	\$ 8 00 a.m. \$ 8 30 p.m.	\$ 0.966	{ 10 00 a.m. { 7 00 p.m.	3 1.092	4 00 a.m. 5 30 a.m. 5 30 p.m.	1.234	\$ 10 00 a.m. \$ 7 30 p.m.	1.110
		THE	THERMOMETER,	, DRY-BULB.	,В.					
Mirimum Maximum Menn	3 30 a.m. 2 00 p.m. 8 00 a.m. 7 30 p.m.	32.5 41.9 37.1	3 30 a.m. 2 30 p.m. \$ 8 60 a.m. 7 30 p.m.	57.4 69.7 5 63.4	5 00 s.m. 2 30 p.m. \$ 9 00 s.m. \$ 8 00 p.m.	45.0 52.6 47.7	3 30 a.m. 2 30 p.m. 5 10 00 a.m. 8 30 p.m.	% 26.2 26.2 23.0	2 30 p.m. 8 30 p.m. 8 00 p.m.	39.0 47.6 42.8
		THE	THERMOMETER,	WET-BULB	,B.					
Kinimim Maximun Mena	3 00a.m. 2 00 p.m. 7 30 u.m. 8 30 p.m.	30.5	3 30 a.m. 2 00 p.m. 7 30 a.m. 7 30 p.m.	54.5 61.8 38.9	4 30 a.m. 2 30 p.m. 9 00 a.m. 7 30 p.m.	42.7 47.6 \$ 44.6	3 30a.m. 2 30p.m. 9 30p.m. 8 30p.m.	18.8 20.0	3 30 p.m. 7 30 p.m.	36.6 42.6 39.4
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State.	Total.	ol.	Gaseons.	ous.	Dry.bulb.	alb.	Wet-bulb.	bulb.		. Kur
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2d maxi			\$ 8 00 a.m. 8 30 p.m.	3 0.966	\$ 8 00 a.m.	~~ 8 4	{ 7.30 s.m.	~~ %	8 300 m	7.25
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Mean		~~	{ 10 00 a.m.	3 1.092	\$ 9 00 a.m. 8 00 p.m.	47.7	2 30 p.m.	4.6	\$ 9 30 a.m.	ssr. {

	. 662	. 85		35	\$2
	E E	ай ~~~			: 1
	20 00 53 30 P. H.	₹10 30 m.m. 8 00 p.m.		3 00 p.m. 5 30 p.m.	9 30 a.m.
	18.8 23.7	\$ · 20.9		8. 9. 8. 6. 8. 6.	% %
	20.7 3 30 a.m. 18.8 2 00 p.m. 26.2 2 30 p.m. 23.7 5 30 a.m.	{10 C0a.m. } 23.0 { 9 30 a.m. } 20.9 { 10 30 a.m. } .696		3 30 m. 2 00 p.m.	{ 8 30 p.m. } 39.4 { 9.30 p.m. }
	28. 7. 84.	83 ~~		39.0 47.6	 42.8 42.8
	3 30 a.m. 2 30 p.m.	10 CO m.m.		4 30 a.m. 2 00 p.m.	} 8 30a.m. } 42.8
ER.	1. 232	1.234	ď	1.083	3 1.110
WINTER	300 min	5 20 20 20 20 20 20 20 20 20 20 20 20 20	YEAR	200 p. ii.	{ 7.30p.m. } 1.110
	1.336			1.359	1. 38
	10001	<u> </u>		450	0000 0000 0000 0000 0000 0000 0000 0000 0000
	lst miolinum 1st maximum 2d miolinum 2d miolinum	Moan		let minimum. Jei mestinam. 2d mi innam.	Ad maximum. Mean

It will be noticed that the barometric curve shows the usual double daily maxima and minima, except during the summer season, when, from a cause which I cannot explain, we only have the usual first maximum and second minimum.

The gaseous pressure curve shows a double maximum and minimum during

the winter, and has an indication of the same in the spring.

This shows how little dependence can be placed on the readings of the psychrometer when the temperature of the air is much below freezing, even in the hands of a careful observer, and I would, therefore, recommend the discontinuance of the reading of that instrument when the temperature is below freezing.

The first minimum of the total pressure varies from 3.30 to 5 a. m.; the second, from 2 to 4 p. m., (excluding the summer minimum as anomalous;) the first maximum from 9.30 to 10.30 a. m; and the second from 7.30 to 10.30 p. m.

Of the gaseous pressure the maximum is from midnight to 3 a. m., and the minimum from 2.30 to 4.30 p. m., the winter curve being incorrect on account of the lowness of the temperature.

In the thermometer curves, the almost perfect parallelism of the curves of the dry and wet bulbs during the winter shows, as before noticed, the want of accuracy in the instrument during cold weather.

The minimum of the dry bulb occurs at 3.30 a. m. during the spring, summer, and winter, but in autumn it is at 5 a. m., while the maximum occurs at 2 p. m. in the spring, and at 2 30 p. m. during the rest of the year.

The minimum of the wet bulb varies from 3 to 4.30 a. m., and the maximum from 2 to 2.30 p. m., being a little in advance of the maximum and minimum of

the dry bulb.

I intended to compute from the observations, tables for the hourly connections for the periodic and non-periodic variations of temperature, but have been obliged

to postpone it on account of want of time.

The humidity curves have their maxima from 4.30 to 6.30 a.m., or from one to two hours subsequent to the time of minimum temperature, and their minima from 2 to 3.30 p. m. being after the maximum of temperature during the spring and summer, and before during the autumn and winter.

The very depressed winter curve is probably due to the incorrectness of the

psychrometer before mentioned.

The curve of downfall of rain and snow generally shows a maximum at about 4 in the morning, and again about 4 in the afternoon, the minimum being about 10 in the morning, and again in the night, thus being opposed to the curve of barometric pressure.

I have never seen this noticed elsewhere, and these records are too limited to establish a general rule of correspondence between the curves, though I think

the subject is worthy of further investigation.

The velocity of wind curves shows a maximum shortly after midnight, and a minimum after noon; this is in accordance with the general impression that the wind blows harder during the night than the day, which is commonly true during warm weather, but I did not expect to see it so marked during the storms of winter. During the spring the course of the wind varies quite regularly, considering the shortness of the time of observation, from north 5° west at midnight to north 24° east at 11 a. m.; in the summer from north 47° west at 4 a. m., to north 80° east at 2 p m.; in the autumn from north 87° west at 5 a. m., to north 22° west at 3 p. m.; in the winter from north 71° west at 9 a. m., to north 50° west at 6 p. m.; thus showing that the course of the wind varies during the day opposite to the apparent motion of the sun even during the winter storms, and the differing directions in the several seasons. This subject will be more fully discussed under the head of land and lake breezes.

In the lower lines of tables K, I have given the means of twenty-four hours, and also the means of one hour's tri-daily observation, viz: 7 a. m. and 2 and

9 p.m., for the seasons and years. These will be found, together with their differences, in the following tables:

ANEMOGRAPH RECORDS.

Table showing the means of the barometer, &c., for 24 hours, and for 7 a.m. and 2 and 9 p.m.

Ø4	Spr	ing.	Sun	met.	Aut	umn.	₩b	nter.	2 ye	METS.
State.	24 hrs.	7, 2, & 9.	24 hrs.	7, 2, & 9.	24 hrs.	7, 2, & 9	24 hrs.	7, 2, & 9.	24 hrs.	7, 2, 49.
Barometer+28 inches.										
Total pressure Gaseous pressure	1. 317 1. 147	1. 317 1. 145	1.408 0.966	1. 400 0. 962	1. 370 1. 092	1. 368 1. 088	1.332 1,234	1. 328 1. 228	1. 354 1. 110	1.353 L 106
Thermometer.						1				
Dry bulb	37°, 1 33°, 4	37°. 7 34°. 5	63°. 4 58°. 2	64°. 2 58°. 8	47°. 7 44°. 6	48°. 4 44°. 9	23°. 0 20°. 9	23°. 5 21°. 3	42°, 8 30°, 4	43º.4 39º.9
Cloudiness	6.9	6.9	5. 4	5.5	7.3	7.3	8.9	8.1	7.0	7.0
Wind.										
Direction	N. 6º E. 1. 9	N. 6º E. 1. 9	N. 24°W 1. 0	N. 9°W 0.7	N. 520W L. 1	N. 58°W 0. 9	N. 59°W 2. 3	N. 61°W 2. 1	N. 34°W 1. 4	N. 31°W 1. 2

Table showing the differences between the quantities in the above table.

[+ signifies that the 7, 2, and 9 means are greater than those for 24 hours; — the reverse.]

Seasons.	Baron	neter.	Tempe	rature.	Cloudi-	Wind,	
cyclevan,	Total.	Gascous.	Dry bulb.	Wet bulb.	ness.	Direction.	Force.
Spring	0. 000	0.002	+ 0.6	+ 1.1	0.0	0.0	0.0
Summer	0. 008	- 0,004	+ 0.8	+ 0.6	+ 0.1	- N. 16° W.	_ a 3
Autumn	- 0.002	- 0.004	+ 0.7	+ 0.3	0.0	+ N. 60 W.	- 0.2
Winter	0.004	- 0.006	+ 0.5	+ 0.4	0.1	+ N. 20 W.	-0.8
Two years,	0. 001	0, 004	+ 0.6	+ 0.5	0.0	— N. 3º W.	- 0.2

It will be seen from the foregoing that the differences of the barometic means at the hours of the tri-daily observations and for the whole day amount to almost nothing, while the temperature varies about a half degree.

The cloudiness is the same, and even the force and direction of the wind cor-

respond very nearly.

Tables L give the hourly records of the direction and relative velocity of the wind at Milwaukee for 1861. These were omitted in the discussion of the winds in the report for 1865, and are added here to complete the records at Milwaukee.

I have not had time to discuss the winds at the several stations for the two years; they are behind, but hope to be able to do so another season.

These records were taken from the sheets of Burnell's Anemograph, kept at Milwaukee by Observer J. A. Lapham.

This instrument is a late invention, and as its mode of working is peculiar, a description of it may not be out of place.

The paper, which is about six inches wide, passes over herisontal rollers moved

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			DEBING	G.						
		Barometer	Barometer + 28 inches.			Therm	Thermometer.		1	4
State.	Total.	ë	Gaseous.	104.	Dry-bulb.	alb.	Wet-balb.	oalb.	ramona.	ns.
	Hour.	Amount.	Hour.	Amount.	Hour.	Amount	Hour.	Amount.	Hour.	Amount.
let mi- fun'm Inf muximus 20 ion loons	10 00 mm. 10 00 mm. 10 00 mm.	1.331	A 78. 3 30 p.m. 0 30 p.m.	1.195	A: #. 3 30 m. 2 03 p.m.	32.5 41.0	3 (0) E. E. 20 (0) E. 20 (0)	30.5	5.30 p.m.	764
Mean.	6 2 3 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		\$ 33 a.m. \$	1.147	8 00 B	37.1	{ 7 30 a.m. } 33.♣	g ~~	8 8 90 mm. }	§17. {
			SUMMER.	ER.						
let minitorum Jei mend num 2) ini futum	10 00 a.m. 6 00 p m.	1,417	4 30 p.m. 3 30 s.m.	0. 931	3 30 s.m. 2 30 p.m.	57. 4 60. 7	3 30 a.m. 2 00 p.m.	54. 5 61. 8	2 30 a.m. 4 30 p.m.	. 630 . 783
2d maximmin	6 30 a.m.		\$ 8 00 a.m. \$ 8 30 p.m.	3 0.966	\$ 8 00 s.m.	25. 4.0	\$ 7.30 p.m.	56.2	\$ 9 00 m. 8 30 p.m.	7.25
			AUTUMN.	MN.						
let u inbunen. Det mest mung 2d mil tanun.	10 30 m. 30 20 m. 30 70 m.	 38.33 37.33	29 30 p.m. 12 00 m.	1, 058	5 00 p.m. 2 30 p.m.	45.0 52.6	4 30 p.m.	49.7 47 .6	8 00 p.m. 6 30 p.m.	
Mean	\$ 33.00 p.m. 7 00 p.m.	~~	{ 10 00 a.m.	3 1.092	\$ 9 00 s.m. 8 9 00 p.m.	47.7	\$ 9 00 km.	~ 4.6	\$ 9 30 s.m.	\$cr. {

The records are taken from the sheets by passing them under a glass scale of the width of the hour spaces, graduated between the maxima and minima lines of the four cardinal points. By placing the centre of the scale over the hourmarks on the paper, the mean direction of the wind for the half hour preceding and the half hour following may be read, and the number of double strokes (or curves) seen underneath the scale will give the relative velocity per hour.

This anemograph was set up in the latter part of February, 1861, under the direction of Captain G. G. Meade, topographical engineer, but it did not get into working order until the first of April following, at which time the hourly records

commence.

During the month of May the velocity cups were broken, and had to be taken off for repairs. They were replaced about the first of June, at which time the machine was taken down from a high tower, on which it was first placed, and removed to the observer's house for greater convenience, but with not so good an exposure.

The effect of this is apparent in the greater mean velocity of the wind during the month of April, and I have, therefore, omitted this month in many of my

calculations.

To find the mean direction of the wind, for a month or year, we must first find the sums of all the winds that have blown during that time from the different points of the compass. This has been done for each month, according to the form shown in table M.

The velocity numbers are taken from the hourly records, in table L, and placed in their appropriate columns corresponding to the direction of the wind. The number of entries in these columns gives the number of hours the wind has blown from the several points of the compass, or the duration of the wind. The sum of each column gives the whole body of wind which has passed over the place from each point.

I call this the amount of the wind, as it is the product of the duration by the

velocity.

In the last column are given the hours of calm.

In table N are placed the duration and amount, numbers, and the relative

velocity found by dividing one by the other.

In place of interpolating for the hours omitted in the records, and for the purpose of comparing one month with another, I have reduced all the months to a mean month of 730 hours. These reductions will be found in the sixth and seventh columns of table N.

In tables O are given the observed duration, mean relative velocity, and the amount of wind for the several points for each month, and for the whole time; and in tables P the comparative duration and amount for the same time.

In tables Q are shown the mean comparative duration, and amount and mean

relative velocity, for each season and for the year.

These quantities are graphically represented in the wind roses, diagrams 2-13. The shaded parts show the duration, &c., for the several points, being plotted from the outer towards the inner circle. It will be seen that in the earlier months the N.NE. and SE. winds prevail, and the W. and SW. in the latter.

In every month but December the greatest velocity is during the winds from

the N.NE., and in that month from the E.SE. and SW.

The amount and velocity roses for the month of April cannot be compared with those of the other months, on account of the change in the position of the anemograph before mentioned.

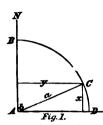
The wind roses for the seasons and year need to be compared with those of subsequent years to draw any conclusion from them in regard to the prevalence

of different winds.

RESULTANT DIRECTION AND VELOCITY.

Lambert's formula for finding the resultant direction of the wind is for only eight points, and does not take into account the velocity.

I modified it somewhat, as follows:



In Fig. 1, let A B represent the meridian A D, an east line, and A C the direction from which the wind we wish to reduce is blowing. This wind must be resolved into two forces, one N and one E, so by taking all the winds blowing around the point A during a certain time, and resolving into their resultant forces in the direction of the four cardinal points, they can be summed up and the resultant direction obtained. To do this we can use either the duration or amount of the wind. Then letting a = the duration or amount of the wind, from a given point; b = the angle made by that point with the meridian; x = the resultant, north or south, and

y = the resultant east or west;

then we have

$$x = \frac{a \cos b}{\sin \theta} = \cos b.$$

$$y = \frac{a \sin \theta}{\sin \theta} = a \sin \theta.$$
sine 90°



Making a = 1, and b severally equal to the angle the first four points make with the meridian, we will have a series of constant multiples for the quantities in table N.

In table R are given the duration and amount thus reduced for the month of

The sums of the quantities in the duration and amount columns are then

E-W- = the tangent of the angle made substituted in Lambert's formula; -

by the resultant directions of the wind with the meridian.

The proportionate part of the wind blowing in the resultant directions is found by multiplying the numerator of the fraction by the sine of the angle of the resultant direction, and dividing by the duration.

The resultant relative volocity is found by dividing the sum of all the amount

columns by the sum of all the duration columns.

In table S is given the resultant direction, progress, (or proportionate part,) and relative velocity of the wind, computed from both the duration and amount for each month, and for the whole time.

In most of the months it will be noticed that the resultant direction computed from the duration and the amount differs but little, though in the month of Sep-

tember the difference is nearly sixty-two degrees.

In the early months of the year it will be seen that the resultant directions are from the north and northeast, and in the later months from the south and southwest, while that for the whole time is nearly west.

I have also given the resultant direction, omitting the month of April, which, as computed from the amount, differs not quite eleven degrees from that of the

whole time, including that month.

I have called storms all those continuous winds whose relative velocity is four and one-half miles per hour and over, that being being nearly one-half the greatest relative velocity (9.2).

In table U are given the storms which occurred during the different months,

and in table V the same grouped according to their directions.

Nearly one-half of the storms it will be seen come from the north to northeast, and about one-third from the southwest to west.

The greatest velocity was in June, with a southwest wind, and the longest storm was in November, with a southeast wind.

RAIN-FALL WITH DIFFERENT WINDS.

Tables W give the direction of the wind during the time of downfall of rain or snow at Milwaukee for the years 1861-'62-'63 and 1864.

The following table, compiled from the above, shows the number of times of rain or snow (amounting to more than 0.1 inch) when the wind blew from sixteen points of the compass, and the proportionate amount of downfall for each point during the four years:

Points.	No. of times of downfall.	Amount of downfall in U.S.inches.	Proportionate amount of downfall.
North	2	0. 91	0.45
North-northeast.	22	12. 32	0.56
Northeast.	28	16.81	0.60
East-northeast	7	4.63	0.60
East	3	2, 32	0.16
East-southeast.	13	8.07	0.61
Southeast.	26	10.98	0.42
South-southeast.	10	7.07	0.71
South	8	4.61	0.58
South-southwest.	15	7.52	0, 50
South-southwest	29	11.87	0.41
West-southwest	8	2. 14	0.27
West	4	2.04	0.51
West-northwest	6	1.48	0.25
Northwest	11	5.90	0.54
North-northwest	6	2, 28	0.36
Calm	16	7.29	0.46

From this it will be seen that though rain fell about the same number of times when the wind blew from the northeast, southeast, and southwest, yet the amount of rain was nearly half as much again with a northeast wind as with the wind from the other points, and that nearly one-third of the whole amount of downfall occurred when the wind was north northeast to northeast.

The least amount of downfall was when the wind blew from the cardinal points. This may be partly accounted for by the fact, as shown by all the anemograph records, that the wind blows more seldom from those points than from any other of the compass.

COMPARISON OF HUMIDITY, EVAPORATION, TEMPERATURE, ETC.

For the purpose of showing the relation between the humidity, evaporation, temperature, cloudiness, and the direction and velocity of the wind. I have given in table X their several daily means for the summer months of the years 1862. 1863, and 1864 at Milwaukee, Wisconsin, and in table Y have eliminated the quantities separately and found the proportionate value of the others for each year.

Table Z is a summation of the last for the three years taken.

Thus we have in the last mentioned tables, first, the mean value of the temperature, humidity, &c., for the direction of the wind, and the number of days observed; second, the mean value of the several States corresponding to the different velocities of the wind; and so on.

Looking at the final table, Z, it will be seen that we have the highest temperature with a wind from the south-southwest to west, and the lowest with one from the north to north-northeast; the greatest velocity of the wind from the north to northeast, the least from the east to southeast.

The direction of the wind seems, however, to have but little effect on the evap-

oration and humidity.

Again, we have the highest temperature when the relative velocity of the wind is from 0.5 to 1.0, and the least when it is blowing the strongest. The humidity is not regularly and the evaporation but slightly affected by its increase.

Thirdly, the temperature is seen to be but little effected by the changes in the amount of humidity in the atmosphere, but the evaporation and cloudiness are

almost in an inverse proportion to its increase.

Fourthly, the cloudiness does not show much effect on the temperature except when the sky is overcast, but the evaporation decreases and the humidity increases as we pass from a clear to a clouded sky.

Fifthly, the humidity decreases slightly and the evaporation increases with a rise of temperature, and the sky becomes somewhat more clouded and the wind

less strong.

Sixthly, the elimination of the evaporation but reasserts the foregoing; with its increase the temperature rises and the humidity and evaporation decrease.

Most of the above facts might seem to be so plain as to need no proof, but it is interesting to be able to demonstrate them, and some of them are rather curious; for instance, the slight effect the velocity of the wind has upon the humidity and evaporation, and also the effect of the cloudiness upon the temperature.

ATMOSPHERICAL PRESSURE UPON THE WATER.

The effect of the atmospheric pressure upon the surface of the water is shown in tables A A, where the mean height of the water for every six hours from Saxton's self-registering tide guage, the mean relative velocity and direction of the wind for every six hours from Burnell's anemograph, and the tri-daily observations of the barometer and thermometer from the general meteorological abstract, are given for the years 1861–'62-'63 and '64 at Milwaukee, Wisconsin.

These tables are graphically represented in diagrams 14-21.

Milwaukee, as will be seen by a reference to the description of the stations, is situated on a small bay on the west side of Lake Michigan, south of the centre and opposite the broadest part of the lake. It is protected by low hills from the full effect of the winds from the south to north by west, but exposed to those from other quarters. The effect of easterly winds is to raise the water in the bay, and of westerly winds to depress it. Northerly winds will raise and southerly winds will lower the level of the water in the whole lake after they have blown for some time. Therefore the water in a southeast wind will be first raised by the easterly and afterward lowered by the southerly tendency, while a northeast wind will raise the water continually. In southwest and northwest winds this action is reversed. In some cases however the first effect of a northeast wind is to blow the water away from the mouth of the bay, and thus, for a short time, to lower it. These facts must be taken into consideration before we can intelligently examine the diagrams to find the effects of atmospheric pres-In many cases in the diagrams the water will rise or fall concrary to the influence exerted by the winds. It will then be seen that the barometer curve is opposed to the water curve, and that the direct pressure of the air is greater than that of the air in motion. And when the wind is strong enough to raise or lower the water, contrary to the weight of air, the rapidity with which it

returns to its equilibrium shows the effect of the pressure upon it. The maximum rise in the water will occur when a strong easterly wind is blowing at the time of a low barometer, and the minimum when a westerly wind and a high barometer are simultaneous. These facts show that the atmospheric pressure on the water of the lakes is the same, though in a less degree, as that often seen on the coast of South America during the great fall of the barometer before a hurricane, and also why the fishermen along the coast can often predict the approach of storms by the rise or fall of the water. In order not to confuse the diagrams the temperature curves were omitted.

LAND AND LAKE BREEZE.

To ascertain whether we have upon the lakes a land and lake breeze similar to the land and sea breezes on the ocean, I have compiled from the hourly anemograph records at Milwaukee and Cleveland and from the half-hourly observations at Thunder Bay island tables B B. In these tables the number of days are given in which the wind blew from the different points of the compass during the several hours in the months of June, July and August, all stormy and cloudy days being first thrown out.

A summation of these tables for each place is given in tables C C, and the latter are graphically represented in the diagrams 22 and 25, according to the principle of indexed planes used by M. Koeuptz in his meteorology, in which the curves represent the outlines of planes passing through every fifth number in the tables, the larger number being taken as hills and the lesser as valleys.

Diagram 22, for Milwaukee for four years, shows a decided breeze off the lake from the east-southeast and southeast from 10 or 11 o'clock a. m. till 6 or 7 o'clock p. m.; from noon till 5 p. m. on nearly one-half of the days taken, the wind blew from these two points. This would be from the broadest part of the lake.

The land breeze is not as well marked, its effect being probably diminished

by the hills back of Milwaukee.

In diagrams 23, for Cleveland, on the other hand, we have but little indication of a lake breeze, but have a land breeze from 9 o'clock in the evening till 9 o'clock in the morning.

The Cleveland anemograph being on a high tower upon the bluff overlooking the lake, the lake breeze rarely rises high enough to effect it, while it feels the

full influence of all winds off land.

There is, however, but one year's observations here, as the anemograph broke down and has never been replaced.

Diagram 24, being very curiously banded, I reduced the number of points

used to eight and replotted it in diagram 25.

This banding shows the difference between a careful observer and a machine, the observer having only recorded the wind as blowing from the eight principal points.

The latter diagram shows a breeze from the south and southeast, in which direction the main body of the lake lies, and from the north from 10 a. m. to 10

p. m., and from the north and northwest from midnight to 8 a. m.

The island lying a distance from land, and having a large body of water around it from north to south by east, the lake breeze assumes a rotary character opposed to the apparent motion of the sun, and the land breeze much weakened in its effect. This was also shown in diagram 1 to be true for all the seasons as well as for the summer months we are now investigating. The island is not large enough to have a land breeze of its own, and only received a part of that from the mainland.

Whether this lake breeze blows from the centre of the lake towards each shore, is a question we have not the means at present of investigating, but hope to have at some future time.

The last lines in the tables give the number of days taken in each month and season and the number of hours of calm.

In tables C C it will be seen that the calmest hours were from 10 o'clock at night to 4 o'clock in the morning at Milwaukee, and at Thunder Bay island from 8 o'clock in the evening till midnight, while at Cleveland the most windy hours were from 10 at night to 3 in the morning, which is a confirmation of the supposition that the day breeze does not reach the elevation of the observatory.

The records of the levels of the water at the several stations have not been reduced since 1863, nor have I been able to give enough attention to them to

report upon them this year.

I hope another year to be able to bring them up.

Very respectfully, your obedient servant,

D. F. HENRY, Assistant Lake Survey.

General W. F. RAYNOLDS,

Corps of Engineers, Sup't Lake Survey, Detroit.

	Baroma	ter reduc	ad to tam			TI	MPE	RATI	JR B -	-7ан	RENHI	UT.			VAP	OR.	
Date.	of 32°	Fahr., ex inches a	pressed i	n United	P	t bul oint pora	of	1		are (tem- of the	B	l d	in	ticity ches :		
	7 a. m.	2 p. m.	9 P. II.	Mean.	7 a. m.	2 p. m.	9 p. B.	7 B. IB.	8 p. B.	9 7. 13.	Mosn.	Maximum	Mhiwum	7 a. m.	2 p. m.	9 p.m.	Mean.
1866.					•	•	•			•		•					
July 1	29, 262 29, 257	29. 298 29. 155	29. 259 29. 112	29. 250 29. 175	54 57	60 72	54 64	55 59	63 80	56 68	58. 0 69. 0	65 82	50 50	. 404 . 439	. 478 . 677		. 49
3	28. 923 28. 960	28. 914 28. 952	28. 960 28. 970	28. 932	62 63	67 69	60	64 66	79 86	67	70.0	81	59 55	. 529	. 635	425	. 53
4 5	28. 917	28. 958	28. 999	28. 961 28. 958	69	53	65 49	73	55	71 51	74.3 59.7	88 70	50	. 53 6 . 65 5	. 376	. 521	43
6	29.018	29. 051	29.085	29. 051	49	51	61	50	52	63	55.0	65	48	. 335			
7 8	29, 146 29, 502	29, 203 29, 534	29. 219 29. 546	29. 189 29. 527	62 58	64	58 57	63 60	74 65	60	65. 7 61. 7	67	55 50	. 542 456	. 462 . 451		
9	29.610	29. 532	29.472	29. 538	54	60	53	55	62	54	57.0	66	50	. 404	. 49	. 389	. 4
10 11	29. 430 29. 330	29, 256	29. 339 29. 241	29. 385 29. 276	59 70	73	68 74	60 74	91	76 82	68.0 82.3	89 94	43 65	. 487 670	. 569	231	
12	29, 282	29. 218	29. 243	29. 248	75	80	76	80	98	87	88.3	99	75	. 800	. 779	. 748	ļ. 77
13 14	29. 286 29. 218	29. 213 29. 176	29. 125 29. 120	29. 208 29. 171	63 60	69	68 58	64 61	71 62	69 59	68.0 60.7	87 70	59 55		. 682 . 491		
15	29. 141	29. 122	29. 255	29. 173	63	80	66	65	90	67	74.0	90	56		. 897		
16	29. 225	29. 251	29. 754	29.410	69	79	65	70	89	75	78.0	89	65	. 695	. 855 . 608	. 483	1.6
17 18	29. 468 29. 421	29. 435 29. 378	29. 431 29. 365	29. 445 29. 388	66 63	67 68	62 72	67 65	71 76	64 75	67.3 72.0	76 81	64 52		. 577		
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20 21	29. 311 29. 131	29. 220 29. 206	29, 205 29, 234	29. 245 29. 190	65 59	71 58	65 59	67 60	82 60	67 60	75. 3 60. 0	68 68	54 55		. 610 . 45 6		
22	29. 199	29. 235	29, 276	29, 237	63	68	55	64	70	56	63.3	74	55	. 562	. 658	490	F. 5
23 24	29. 302 29. 294	29. 306 29. 302	29. 274 29. 325	29. 294 29. 306	53 58	57 75	54 67	54 59	58 86	55 76	55. 7 73. 7	60 89	52 48		. 45 <u>2</u> . 719		
25	29. 411	29, 392	29. 504	29, 436	63	58	58	65	60	60	61.7	74	60	. 549	. 456	. 456	. 4
26 27	29. 406 29. 348	29. 336 29. 302	29. 356 29. 338	29, 366 29, 329	59 65	64 79	66	60 69	68 86	67 64	65. 0 73. 0	68 91	62 56		. 543 . 8 95		
28	29, 396	29. 310	29. 290	29, 332	67	77	68	68	78	70	72.0	80	58		. 693 . 914		
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August 1	29. 135	29, 231	29. 326	29. 231	60	68	65	63	72	68	67.7	73	58		. 631		
3	29, 367 29, 336	29. 355 29. 341	29. 315 29. 489	29. 346 29. 389	58 66	70 63	65 57	60 68	76 64	68 58	68.0 63.3	78 75	48 56		. 652 . 562		
4	29. 522	29. 604	29. 584	29. 570	58	65	55	60	67	56	61.0	67	43	456	. 591	420	. 40
5	29. 597 29. 479	29. 578 29. 438	29. 584 29. 406	29. 586 29. 441	58	60	60 60	60	65 63	61	62. 0 61. 7	67	48	456	. 451	505	. 4
6 7	29. 379	29. 369	29. 342	29, 363	60 59	62 59	58	61	60	61 59	59.7	64 62	58 53	. 487	. 542 . 487	469	. 44
8	29. 317 29. 294	29, 271 29, 278	29. 268 29. 306	29. 285	59	63	60	60	65	61	62.0	68	55	. 487	549	. 505	. 5
9 10	29. 336	29. 308	29. 319	29. 293 29. 321	59 56	75 60	60 55	60 57	81 6i	61 56	67. 3 58. 0	81 63	50 55	. 487 . 436		. 420	. 4
11	29. 304	29, 279	29. 252	29, 278	56	58	58	57	60	59	58.7	60	54	. 436	. 456	469	4.4
12 13	29. 242 29. 319	29, 276 29, 250	29. 299 29. 230	29. 272 29. 266	59 59	60 75	58 72	60 60	61 82	61 74	60.3 72.0	62 84	55 55	. 487 . 487	. 5∪5 . 773	. 430 . 757	. 6
14	29. 354	29. 422	29. 498	29. 425	67	72	63	68	82	65	71.7	83	62	. 648	. 650	. 549	. 6
15 16	29. 650 29. 512	24.617 29.468	29, 612 29, 423	29. 626 29. 468	57 59	60 65	56 64	58 60	62 67	57 70	59. 0 65. 7	67 79	55 51		. 491 . 591		
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18 19	29. 454 29. 442	29. 421 29. 361	29. 391 29. 318	29. 422 29. 374	53 57	70 61	63 56	55 58	73 73	64 63	64. 0 64. 7	75 73	48 54	. 376 . 452	. 693 510		
20	29. 300	29. 264	29. 279	29. 281	54	64	57	56	67	62	61.7	70	59	. 391	556	399	. 4
21 22	29. 330 29. 295	29, 332 29, 345	29. 391 29. 372	28. 351 29. 337	51 45	52 51	47	54 46	59 52	49 47	57. 3 48. 3	63 56	48 39	. 335	296 361	. 297 907	3
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94 25	29. 383 29. 373	29. 354 29. 233	29. 362	29. 366 29. 278	48	56 61	47	50	63	49	54.0	64 78	44	309	356	. 997	. 33
25 26	29, 373	29. 233 39. 032	29. 227 29. 108	29. 278 29. 057	38 55 54 55 55 55 55 55 55	70	58 55	40 58	75 76	62 56	59. 0 63. 3	80	39 53	. 393 .	483 659	420	. ·
27	29. 208	29. 234	29, 234	29, 225	54	58 65	55 54 57 55 57	58 56 54 56 56	60	56 59	57. 3	62	59	. 391	456	. 391	. 41
28 29	29. 237 29. 272	29. 236 29. 232	29. 256 29. 227	29. 243 29. 244	52 54	65 60	57 55	56	70 62	59 56	61. 0 58. 0	75 62	43 54	369 391	550 491	420	. 43
30	29. 212	29. 178	29. 153	29. 181	54	56	57	56	58 56	58	57.3	60	54 L	391.	429	452	. 않
31	29. 006	28. 961	28. 891	28. 953	54	55	56	55	56	57	56. 0	59	52	404	420	4.00	. 42
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the northern and northwestern lakes at Superior City, Wisconsin.

	VAI	POR.				w	IND.						oun udin	P88.	and deci-	or melted inches and
8•	Hum	idity. n == 1,0	000.		Direction om when		111	ocity iles, our.	, in per	tant velocity,	Resultant direction.	(10:	sky.) = sky ly ov cast.	en-	of evap	of rain n U. S. ls.
7 . B.	2 p. B.	9 P. III	Mean.	7 a. m.	2 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Resultan in miles,	Resultan	7 a. m.	2 p. E.	9 p. m.	Amount U. S. mals.	Amount or snow, in decimals
. 934 . 878 . 888 . 833 . 807 . 927 . 940 . 934 . 940 . 782 . 943 . 941 . 948 . 948 . 949	. 831 . 661 . 896 . 387 . 989 . 930 . 551 . 884 . 432 . 892 . 898	. 872 . 793 . 642 . 708 . 859 . 886 . 880 . 822 . 933 . 544 . 947 . 939 . 946 . 893 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 943 . 944 . 945 . 945 . 946 . 947 . 948	.879 .777 .609 .614 .445 .599 .914 .917 .762 .599 .921 .823 .921 .829 .925 .685 .883 .820 .823 .829 .829 .829 .829 .829 .829 .829 .829	NE	NNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNNN	NEWWIN AND AND AND AND AND AND AND AND AND AN	1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00	22 9 4 8 0 0 0 1 1 3 6 6 1 1 3 6 6 1 1 3 6 6 1 1 3 6 6 1 1 3 6 6 1 1 3 6 6 1 1 3 6 6 1 1 3 6 6 1 1 1 1	1.06 2.00 3.06 1.00 4.20 2.06 0.10 0.06 1.00 0.06 1.00 0.06 1.00 0.06 1.00 0.06 1.00 0.06 1.00 0.06 0.00 0.00	1.03.3 1.3.00 1.002.66 1.003 4.03 4.03 1.77 0.66 0.77 1.30 1.07 1.07 1.07	N.39 E N.45 W N.12 E South S.45 W N.45 E N.4	7 6 10 0 2 10 0 0 0 0 0 0 10 8 8 0 0 9 0 10 6 10 10 0 8 0 10 10 10 10 10 10 10	5 4 3 4 0 10 6 3 0 0 3 3 3 10 7 10 3 6 6 3 3 3 2 2 8 5	8 3 2 100 8 6 3 0 0 6 2 100 100 100 100 100 100 100 100 100 1		
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. 831 . 889 . 895 . 890 . 991 . 940 . 940 . 946 . 946 . 947 . 947 . 947 . 947 . 948 . 949 . 947 . 949 . 947 . 949	. 904 727 943 893 731 942 940 890 891 941 744 941 768 893 893 817 894 894 894 895 897 897 898 897 898 898 898 898 898 898	. 843 . 947 . 941 . 941 . 941 . 941 . 943 . 949 . 949 . 949 . 940 . 944 . 790 . 943 . 723 . 723 . 723 . 923 . 853 . 943 . 853 . 943 . 944 . 945 . 946 . 946 . 947 . 947 . 948	. 826 817 925 903 851 941 940 924 875 875 889 889 889 849 849 849 849 849 849 849	NW SW NE NE NW NW NW NW NW NW NW NW NW NE NE NE NW NW NW NW NW NW NW NW NW NW NW NW NW	NW	NW NW NE	8.40 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1	1.0 0.6 1.0 2.0	3.66.046.046.046.00 5.20.05.00 6.00.060 6.00.046.00 6.20.05.00 6.00.00	C. 633-4404-4-33-4-4-0-7-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	N. 45 E N. 45 E	36630071001001001001001001001001001001000000	5 4 8 6 6 8 10 10 3 2 10 10 10 3 4 3 3 5 6 3 3 4 4 0 3 3 5 5 10 5 2	0 5 6 8 10 10 10 5 8 10 10 10 10 10 10 10 10 10 10 10 10 10		.12 .34

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3	29. 247	29. 228	29. 251	29, 242	50	60	56	53	73	62	62.7	75	47	. 321	. 345	. 364	. 3
4 5	29. 210 29. 237	29. 222 29. 258	29, 219 29, 311	29. 217 29. 269	52 52	57 62	52 54	53 53	58 67	53 56	54. 7 58. 7	63 70	50 44		. 542 . 489		. 4
6	29. 322	28. 206	29. 280	29. 301	53	02	53	55	01	54	54. 5	60	46	. 376	. 103	369	
7	29. 247	29. 196	29, 247	29. 230	52	60	49	53	62	50	55.0	63	50		. 491	. 335	
8 9	29. 363 29. 563	29. 404 29. 522	29. 487 29. 494	29. 418 29. 526	49	58 59	49 52	50 48	62 64	51 54	54. 3 55. 3	63 66	43 38		. 429 . 433		. 3 !. 3
10	29. 441	29. 346	29. 264	29. 350	46	60	50	47	66	52	55.0	69	40	. 297	. 438		. 3
11 12	29.099	28. 962	28. 881	28.987	46	61	58	47	65 62	59 52	57. 0	68 63	40 50	. 297 . 335	. 483 . 340	. 469	
13	29. 881 29. 259	28. 974 29. 242	29, 098 29, 225	28. 984 29. 242	49	55 53	51 54	50 43	58	56	54. 7 52. 3	62	39	. 254	. 336		
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15 16	29. 426 29. 280	29, 264 29, 350	29. 207 29. 425	29, 299 29, 352	38 43	58 53	50 40	40 44	65 55	51 41	52.0 46.7	67 56	29 40	. 203	. 389 . 376		
17	29. 482	20, 415	29. 383	29. 427	36	53	45	37	54	46	45.7	55	32		. 369		
18	29. 392 29. 494	29. 343 29. 505	29. 385	29.373	37	47	40	38	54	41	44.3	55 53	30 35		. 231 . 232	. 235	
19 20	29. 494	29.518	29. 525 29 425	29. 508 29. 505	42	46	43 34	43 41	52 48	45 35	46. 7 41. 3	49	35		. 310		
21	29. 510	29, 456	29.418	29. 461	33	48	43	34	50	45	43.0	51	23	. 175	. 309	. 251	:
22 23	29. 188 29. 219	29, 151 29, 186	29, 187	29, 175 29, 202	42	52 50	49 51	43 48	54 51	50 52	49. 0 50. 3	55 53	40		. 362		
24	28. 941	29. 038	29. 285	29. 088	49	54	40	50	56	43	49.7	56	40		. 391		
25	29, 338	29. 294	29. 274	29. 302	42	55	53	43	65	56	54.7	66	35	. 254		. 363	
96 27	29, 250 29, 380	29, 362 29, 377	29. 374 29. 282	29, 329 29, 346	45 50	58 56	48 47	48 52	70 60	50 48	56. 0 53. 3	74 60	45 43	. 334	. 323	. 310	
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2	29. 463	29. 447	29. 470	29. 460	42	56	48	45	64	50	53,0	64	42	. 228			
3 4	29, 423 29, 603	29, 390 29, 568	29, 485 29, 575	29, 433 29, 582	38 40	53 50	48 45	40	55 52	50 47	48. 3 47. 3	60 57	35 37	. 203 . 208			
5	29. 553	29, 460	29. 440	29. 484	42	54	48	44	55	50	49.7	57	40	. 241	. 404	. 309	. :
6 7	29, 358 29, 287	29, 261 29, 150	29. 286 29. 153	29. 302 29. 197	40 57	68 69	58 54	43 58	81 84	62 56	62. 0 66. 0	84	38 54	. 208 . 452		. 429	
á	29, 287	29. 108	29. 109	29. 197	48	50	47	51	52	48	50.3	64	45	296		310	
9	29. 258	29, 310	29. 375	29. 314	47	54	43	48	56	44	49. 3	60	44	. 310		. 964	
10 11	29, 450 29, 512	29. 517 29. 464	29, 543	29. 503 29. 453	40 31	52 50	39 48	41 32	56 52	40 50	45.7 44.7	60 52	39 29	. 235		. 225	
12	29. 453	29. 301	29. 257	29. 337	46	53	50	47	54	52	51.0	57	40	. 297	. 389	. 334	(:
13 14	29, 321 29, 593	29. 322 29. 580	29. 372 29. 618	29. 338 29. 597	41	54 55	55 50	42 46	56 56	56 51	51.3 51.0	57 57	36	244 269	. 391	. 430 348	
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16	29. 791	29. 652	29. 599	29.681	42	63	50	43	74	56	57.7	74	40	251	. 356	. 982	
17 18	29. 465 28. 882	29. 285 28. 881	29. 197 29. 001	29. 316 28. 921	47 50	55 52	50 43	48 54	57 53	51 44	52.0 50.3	64 61	40	. 310	. 407	. 346 964	
19	29. 165	29. 196	29. 213	29, 191	34	58	38	35	61	39	45.0	61	33	113	. 443	216	. :
20 21	29. 228 28. 902	29. 152 28. 810	29. 127 28. 788	29. 169 28. 833	46 42	46	46 35	47	47	47 36	47. 0 40. 7	50 48	35	. 297 254	. 297	. 29 7	
22	28. 703	28. 692	28. 945	28. 780	34	36	29	35	37	30	34.0	40	30	. 183	. 190	. 149	. 1
23	29, 230	29. 452	29. 542	29.408	29	31	29	30	32	30	30.7	34	28	. 149 . 142 . 168	. 100	. 149	. 1
24 25	29. 543 29. 283	29. 407 29. 207	29, 395 29, 210	29. 448 29. 233	32	31	28 32	33 33 32	32 35	29 33	30.0 33.7	33 35	26	168	113	ies	. :
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the northern and northwestern lakes at Superior City, Wisconsin.

	VAI	POR.				w	LND.					clo	ouni udin == ele	288.	ation, in	in or melted S. inches and
Sa		idity. n = 1,0	100.		Direction om when		110	ocity iles, our.		mites, per hour.	Resultant direction.	(10: tire	sky.) = sky ly ov cast.)	er-	t of evaporation inches and	20.4
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.872 .872 .872 .932 .832 .832 .927 .923 .923 .923 .923 .916 .916 .916 .916 .916 .916 .916 .916	. 525 . 425 . 425 . 937 . 740 . 884 . 773 . 783 . 685 . 561 . 686 . 631 . 698 . 561 . 592 . 872 . 441 . 727 . 441 . 727 . 441 . 727 . 441 . 727 . 441 . 727 . 441 . 727 . 441 . 727 . 441 . 728 . 844	. 884 . 880 . 665 . 932 . 933 . 937 . 859 . 861 . 939 . 912 . 912 . 912 . 912 . 927 . 930 . 898 . 840 . 927 . 750 . 856 . 939 . 856 . 856 . 939 . 856 . 856 . 939 . 856 . 856 . 939 . 856 . 856 . 939 . 856	. 760 . 629 . 934 . 901 . 853 . 848 . 891 . 853 . 892 . 796 . 823 . 892 . 793 . 900 . 919 . 912 . 864 . 928 . 853 . 862 . 864 . 862 . 862 . 862 . 862 . 864 . 862	NW NW NW NW NW NW NW NW NW NW NW NW NW N	NW	Calm Calm NW NW NE NW Calm	2.06 4.20 1.60 1.00 1.00 1.00 1.00 1.00 1.00 1.0	5.46.0 3.66.0 1.99.6 1.20.0 1.00.0 1.00.0 2.00.0 1.66.0 2.00.0 1.66.0 2.00.0 1.66.0 2.00.0 1.66.0 2.00.0 1.66.0 2.00.0 1.66.0 2.00.0 1.66.0 1.		3.23 3.33 1.77 1.00 1.33 1.09 0.33 4.77 4.77 2.60 0.10 0.90 0.90 0.90 0.90 0.90 0.90 0.9	N. 45 W. N. 45 W. N. 45 W. N. 45 E. N. 45 E. N. 45 E. N. 45 W. S. 45 E. N. 45 W. N.	0 0 3 10 0 0 7 7 3 4 0 8 8 5 6 5 0 8 8 8 0 10 10 10 4 0 0 0 0 0 0	0 3 4 5 5 3 2 7 7 4 5 6 5 6 7 5 5 4 10 10 5 3 0 0 0 0 0 0 0 0	10 4 7 10 7 8 5 2 5 0 6 6 2 9 0 0 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		. 49 64 13 .03 .80 .10
. 892	.714	. 859	. 832				<u> </u>	<u></u>	<u> </u>	1.1	N. 40 W.	4. 3	3.9	4.3		4.08
. 867 . 762 . 820 . 750 . 836 . 750 . 937 . 912 . 912 . 923 . 914 . 927 . 914 . 927 . 918 . 928 . 928 . 928 . 928 . 928 . 928 . 928 . 928 . 928 . 928 . 928 . 928 . 928 . 928 . 928 . 928 . 938	. 407 . 575 . 869 . 861 . 483 . 483 . 483 . 861 . 872 . 786 . 933 . 833 . 833 . 833 . 835 . 835 . 993 . 896	.708 .856 .847 .856 .847 .872 .915 .910 .856 .861 .929 .929 .929 .929 .929 .929 .929 .930 .887 .887 .898 .898 .898 .898 .898 .898	. 661 . 848 . 819 . 875 . 668 . 859 . 907 . 907 . 928 . 673 . 928 . 853 . 908 . 863 . 874 . 892 . 893 . 894 . 895	SW NE SW NE Caim Caim NE Caim	SYESON SYESON SYNTHESIS ON SYNT	NE NE Calm NE NE NE NE NE NE NE NE NE NE NE NE NE	2.69 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0	3.6 3.6 2.0 7.6	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.07 1.06 1.76 1.03 1.03 1.03 1.03 1.03 1.03 1.03 1.03	N. 46 E. N. 45 E. N. 45 E. R. 45 E. R. 45 E. N.	0 0 8 0 10 0 0 10 10 10 10 10 10 10 10 10 10	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	10 3 10 3 0 5 8 10 0 0 0 3 10 10 10 10 10 10 10 10 10 10 10 10 10		

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	7 p. II.	2 р. п.	9 0 19	Mean.	7 a. m.	2 p. E.	9 p. m.	7 a. m.	2 p. m.	9 p. B.	Meen.	Meximan	Minimum	7 a. m.	2 p. m.	9 p. m.	Menn.
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Mov. 1	29. 498	29, 182 29, 544	29. 314 29. 580	29. 161 29. 541	33 30	36 36	31 21	34	37 38	32 22	34. 3 30. 3	37 39	28	. 155	. 199 . 186	. 101	. 14
3 4	29. 726 29. 945	29. 705 29. 962	29. 763 29. 986	29. 731 29. 964	94 31	30 35	19 31	25 32	32 36	32	25. 7 33. 3	35 37	11	. 162	. 144 . 191	163	1.17
5 6	30, 001 29, 592	29, 845 29, 474	29.770 29.384	29. 872 29. 483	15 37	35 40	31	16 36	37 42	32 42	28.3 40.7	41	12 29		. 178 . 22 1		
7 8	29. 286 29. 187	29. 141 29. 164	29. 102 29. 194	29, 176 29, 182	40 38	44 54	40	41 40	45 58	41	42.3 48.7	46 58	29 39	. 235	. 275 . 365	. 235	. 2
9	29. 372	29. 393	29. 413	29. 393	33	50	35	36	58	37	43.7	58	35	. 149	. 255	. 17:	. 19
10 11	29, 345 29, 288	29. 254 29. 297	29. 272 29. 327	29. 290 29. 304	36 29	35 33	34 26	37 30	36 34	35 27	36. 0 30. 3	38 36	28 25	. 199 . 149	. 191 . 1 75	. 183). 19
12 13	29, 343 29, 262	29. 447 29. 209	29. 434 29. 092	29. 408 29. 188	26 35	33 37	31	27 37	35 39	32 40	31.3	35 41	92 35	. 129		. 169	. 1
14	28, 932	28.900	26.979	28. 937	37	40	36	38	41	37	38.7	41	35	. 907	. 235	. 199	. 2
15 16	28. 942 29. 033	28. 788 29. 102	28.770 29.120	28, 833 29, 085	32 33	32 33	33 30	33	33 35	34 31	33.3	39	30	. 175	. 168 . 162	. 155	. 10
17 18	29. 018 29. 135	28. 935 29. 100	29. 025 29. 105	28, 993 29, 113	29	38 42	31	30 30	39 47	32	33. 7 37. 0	41	92 97	. 149 . 149	. 216	. 169	1. 17
19	29. 020	29. 080	29. 297	29. 132	29	38	29	30	40	31	33.7	41	28	. 149	. 903	. 137	. 10
90 21	29. 446 29. 268	29. 435 29. 160	29. 398 29. 175	29. 496 29. 201	21 26	27 26	21	92 27	28 30	99 28	24. 0 28. 3	32 31	18 20	. 101 . 1 2 9	. 130	. 136	. 13
22 23	29. 300 29. 553	29. 410 29. 550	29. 480 29. 623	29. 430 29. 575	28 21	27	22 11	30 223	28	24 12	27. 3 21. 0	30	20 10		. 136 . 134		
24 25	29. 619 29. 084	29. 510	29. 453 29. 040	29. 527 29. 045	14	25 33	11	15	26 36	12 25	17. 7	29 37	8	. 071	. 123	. 061	. 0
26	29.061	29. 010 29. 190	29, 240	29, 164	18 16	33	24 35	20 18	36	36	27. 0 30. 0	37	15	. 067	. 149 . 149	. 191	. 13
27 28	28. 988 28. 998	22 828 29, 128	28. 831 29. 188	28. 882 29, 105	35 21	36 23	33 21	36 22	37 24	34 22	35.7 22.7	39 35	33		. 199 . 11 3		
29 30	29. 319 29. 430	29, 248 29, 358	29. 321 29. 383	29. 296 29. 390	17	24 26	17	18	25 28	18 10	20.3 13.7	28 31	15 0	. 083	. 117 . 117	. 083	.09
Means	29. 299	29. 281	29. 302	29. 294	=	==	-	28. 4	36. 2	29. 4	31. 4	==		. 140	. 180	. 149	. 15
Dec. 1	29, 319	29, 161	29, 060	29, 190	7	98	25	8	31	26	21.7	36	5	048	. 119	. 123	. 097
2 3	28. 996 28. 821	28. 910 28. 770	28.917 28.808	28.941 28.800	14 34	35 38	32	15 35	37	34 33	28.7 37.3	38 47	12 30	. 071	. 178	. 155	. 13
4	28.950	29.060	29. 225	29.078	22	35	24	23	38	25	28.7	39	20	. 107	. 151 . 1 65	. 117	. 13
5 6	29. 468 29. 423	29, 530 29, 237	29. 560 29. 229	29. 519 29. 296	17	32 35	23 30	18	35 40	32	23.7 29.0	36 43	15		. 142 . 139		
7 8	29. 128 29. 064	29, 029 28, 966	29. 177 29. 031	29. 111 29. 020	22 13	38	25 13	25 14	46 30	26 14	32.3 19.3	49 30	25 10	. 084	. 125 . 130	. 193	. 11
9	29. 103	29, 102	29.088	29.098	- 1	9	6	0	10	7	05. 7	15	- 2	. 030	. (.54	. 046	. W
10 11	29. 190 29. 334	29, 169 29, 323	29. 196 29. 397	29. 185 29. 351	- 6	10	- 2	- 3	11	- 1	06. 0 02. 3	14 12	- 5		. 048 . 057		
12 13	29. 544 29. 622	29, 2£3 29, 640	29, 557 29, 709	29. 461 29. 657	-11 - 9	13 10	- 3 10	-10 - 8	14	- 2 11	00. 7 05. 0	16	-12 -10	. 014	. 067 . 046	. 026	. CC
14	29.803	29, 781	29, 791 29, 554	29. 792	- 1	18	17	10	20 21	5	11.7	20	5	. 054	. 076	. 041	. 05
15 16	29 723 29.449	29, 645 29, 396	29. 371	29. 641 29. 405	19	18 23	21	20	24	18 22	13.0 22.0	23 25	- 5 15	. 092	. 065 . 112	101	. 11
17 18	29. 409 29. 237	29, 361 29, 220	29. 275 29. 330	29. 348 29. 262	7 91	24	23 26	92	26 28	24	19.3 23.7	26 29	5 90		. 106 . 136		
19 20	29, 639 29, 753	29, 737 29, 506	29. 811 29. 441	29. 729 29. 567	11	15 20	14	12	16 21	15 20	14.3	28 22	10	. 061	. 074	. 071	.06
21	29. 261	29, 110	29, 010	29. 127	19	98	28	90	30	29	26.3	31	18	. 092	. 130	. 142	12
22 23	28. 646 28. 628	28, 605 28, 885	28.675	28. 642 28. 890	32 23	38 94	33 18	33 24	40 25	34 20	35. 7 23. 0	41 35	28 18		. 303		
24 25	29. 080 29. 223	29. 097 29. 160	29. 199 29. 169	29. 125 29. 184	18 - 6	15 13	13	90 - 5	16 14	2 14	12.7 07.7	01 16	ì	. 076	. 074 . 067	.083	000
26	29. 212	29. 329	29. 473	29. 338	8	9	- ĭ	10	10	0	06.7	14	- 6 - 1	. 040	. 054	. 030).	045
27 28	29. 595 29. 780	29. 684 29. 744	29. 764 29. 851	29. 681 29. 792	-17	7 7	- 1 - 4 - 9	-13 -16	8 8 2	- 3 - 8	- 2.7 - 5.3 - 7.3 - 4.3 - 4.7	10	-18 -17	. 008	. 048 . 048	. ලක්. 017 .	024
29 30	29. 790 29. 420	29. 643 29. 142	29, 591 29, 238	29. 675 29. 267	-13	1 0	-13 3	-12 -18	2	-12	- 7.3 - 4.3	5	-14 -20	L 012	. 034	012.	019
31	29. 475	29, 452	29. 683	29. 537		5	- 9	-12	6	- 8	- 4.7	8	-14	019	. 043	617	024
Means	29. 331	29. 280	29. 327	29. 313	_		_	08. 0		_			_	1	. 098		_

the northern and northwestern lakes at Superior City, Wisconsin.

	VAI	POR.			DID.			ı. . .	- 	clo (0	oun udin = cl	086. 682)	poration, in	or melted S. inches		
8a	Hum turatio	idity. n == 1,0	100.		Direction om when	124	ocity iles, our.		t velocity,	Resultant direction.	tire	ly o	AGL-	t of eva	A up	
Ë	ē.	Ė	Mean.	렴	a i	B	ä	B	E A	Resultan in miles,	mitee	gi gi	B	E ci.	mount U. 8. mala	Amount mow,
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. 895	. 903	. 896	. 898	N	N	Calm	3.6	4. 2	0	2.6		6	4	10		
. 893 . 872	.811	. 860 . 850	. 855 . 839	NW	8W	NE	2.0 2.4	2. 4 1. 6	0, 6 5, 4	1.0 2.7	N. 79 W N. 28 W	9	3	0		
. 896	.900	. 896	.897	NB	NE	E	1.0	3. 6	0.6	1. 7	N. 50 E	5	ŏ	4		
. 829	. 807	. 896	. 844	Calm	NB	NE	0	2.0	2.0	1.3	N. 45 E .	0	0	0		
. 905 . 912	. 829	.914	. 883	SW	Calm	NE	0.6	2.0	1.0 0.6	0.7	N. 45 E .	9	6 10	10		····;
. 890	.756	.777	. 784	8W	8W	SW	2.0	4. 2	0.3		N. 45 E . N. 45 W .	4	10	10		. 15
. 705	. 529	. 807	.690	8W	8W	Calm	4. 2	3. 6	0	2.6		0	0	0		
. 903	. 900	. 898	.900	NE	NE	NE	3.6		3. 6	5.0	N. 45 E .	10	10	10		
. 890 . 880	. 895 . 797	. 890	. 888 . 858	NW	Calm NW	Calm	2.0	0.6	2.0	0.7	N. 45 W	10	10	0	1	.30
. 907	.816	.910	. 844	NE	NE	NE	1.0	0.6	7. 2	3.0	N. 45 E .	7	10	10		
. 905	. 912	. 903	. 907	NE	NE	8W	1.0	1.0	1.0	3.3	N. 45 E	10	10	10		. 24
. 893 . 895	. 893 . 797	. 895 . 893	. 894	NW	8W	Calm	1.0 2.0	2.0	0			10	10	10		.0€
. 890	.907	. 896	. 898	Calm	8W	Calm	20		0			10	8 7	6		
. 890	. 624	. 895	. 803	8W	Calm	Calm	1. Ŏ	0	ŏ	0.3		ŏ	3	ŏ		
. 890	. 820	. 788	. 833	Calm	NW	NW	0		5.4	2.6	N. 45 W.	3	8	4		
. 860 . 880	. 883	. 960 . 983	. 868	NW Calm	SW Calm	Calm	6.0	1. 0 0	0	2.0 0.0		10	10	10		
. 782	.883	. 738	. 801	NW	NW	NW	2.0	1. 2				8	6	3		36
. 860	. 775	.8/14	. 813	NW	NW	Calm	1.0	0.6	ő	0.6		10	3	ŏ	:	
. 823	. 876	. 804	. 8:14	8W	8W	Calm	1.0	0. 3	0	0.3		6	8	0		
. 702 . 682	. 705 . 705	. 872	. 760 . 962	8W	8W	Calm	2.0 1.0		8. 4	1.7 3.3		0	3	10		
.900	.903	. 895	.899	NF	NE	NE	40. 0		6.0			10	10	10		
. 860	. 868	. 860	. 863	NW	NW	NW	2.0	1.6	4. 2	2.7	N. 45 W	10	10	10		1.30
. 849	. 872 . 768	. 849 . 791	. 857 . 763	NW	NW	NW Calm	2.0	1.0	2.0	1.7	N. 45 W	10	0	3		
. 730	. 708	. 191	. 763	NW	N W	Caim	1.0	0.6		0.7	N. 45 W	0	3	0	•••••	
. 850	. 824	. 864	. 846					<u></u>		1.7	N. 24 E	6. 2	5. 6	4. 8		2. 55
	1	ŀ	•									İ	l			
.777	. 685	. 876	. 779	8W	Calm	Calm	1.0	0	0	0.3	S. 45 W	0	0	0	1	l
. 823	. 807	. 792	. 807	Calm	Calm	Calm	0	0	0	0.0	Calm	0	4	10		
. 898 . 864	. 522	. 893	.771	Calm	NE	NW Calm	0 1. 2	1. 2 0. 9	1.6	0.7	N. 13 W	10	3	0		.30
. 840	. 698	. 868	.802	NW	NE	Calm	i. 2 1. 0	1. 2	0	0.7 0.3	N. 45 W N. 16 E	0	0	0		
. 475	. 557	. 792	. 608	8W	8W	8W	1.0	5.4	1.2	2.7	8. 45 W	ŏ	3	ŏ		
. 622	. 400	. 876	. 633	8W	8W	8₩	0.6	4. 2	7. 2	4.0		0	3	10		
. 816 . 69 5	. 782 . 791	. 816 . 769	. 805 . 752	NW	8W	S₩	2.0	2.0 3.0	0.6 2.0	0.7 1,7	8. 45 W 8. 65 W	10	10	10		· • • • • • • • • • • • • • • • • • • •
. 769	1777	.730	. 759	8w	8w	s₩	5. 4	3.6	2.0	3.7	S. 45 W	10	10	3		
. 658	. 797	. 683	.713	NW	NW	NW	2.4	5. 0	3.6	3.0	N. 45 W.	3	0	4		
. 539 . 577	. 816 . 610	. 671	. 675 . 661	8W	8W	Calm	1. 0 0. 6	1.2	0	0.7	8. 45 W	0	0	10		
.791	.702	. 750	.748	SW	NE	Calm	1.0		0	0. 2 0. 3	N. 45 E N. 45 E	10	8	10		. 0
. 695	. 570	. 840	. 702	8W	Calm	NE	1.0	0	1.6	0.3	N. 45 E .	0	0	4	l:	.
. 850	. 868	. 860	. 859 . 800	NE	NE	NE	2.0	1.0	1.0		N. 45 E	10	10	10		
. 777 . 860	. 883	. 868	. 874	8W	NE	Calm	1. 0 1. 0	0. 9 0. 6	2.0	0. 1 0. 7	8. 45 W. N. 77 E.	10	10	10		
. 804	. 8929	. 823	. 819	NE	NE	N 16	8. 4	6.0	4. 2	6.0		10	10	10		
. 760	. 855	. 850	. 842	NE	NW	NE	2.0	1.6	2.0	1.3	N. 25 E .	6	10	10		.ic
. 850 . 893	. 792	. 895	.840	SW Calm	8W	Calm	0. 6 0		0			10	10	10		
. 868	. 872	.702	. 814	8W	8₩	8W	3.0		3.6 2.0		8. 45 W . 8. 45 W .	10	10	8		
. 702	. 839	. 460	. 664	NW	йw	NW	4. 9	3.6	2.0	3. 2	N. 45 W.	io	10	ŏ		
. 627	.816	.816	. 753	NE	NW	8W	1.0	0. 6	2.0	0.3	8.73 W	0	10	10		
. 5 0 2 . 474	. 791 . 777	. 695 . 658	. 699 . 636	Calm	NW	NW	1.0	4. 2 5. 4				10	3	10		
. 395	777	. 577	. 583	NW	NW	8w	1.0	0.6	1.0	0.7	N. 67 W	0	0	ő		
. 498	.718	. 498	. 571	8W	8W	8W	2.0	2.4	1.0	1.7	8. 45 W.	10	0	0		
. 335 . 498	. 707 . 760	.740	. 594	SW	Calm NW	NE	1.0	0	1.0		N. 45 E .	10	10	10		···· <u>·</u>
. 150	. 700	.311	. 012	D₩	M W	NW	1.0	Z. U	2.4	1.3	N. 53 W	3	0	0		. 1
. 697	. 744	. 768	. 736			l				6. 2	N. 83 W	5. 1	4.8	5. 1		.0

						TE	MPE	RATU	re-	-FAH	RENHI	IT.			VAP	OR.	
Date.	of 32°	Fahr., ex	ed to temp pressed in ad decima	United	P	t bull oint o	of	1		lb, or ure o air.		i	d	at	ticity ches :		
	7 a. m.	2 p. m.	9 p. m.	Mean.	7 a. m.	2 p. m.	9 p. B.	78.11.	8 p. H.	9 p. m.	Mosn.	Maximum.	Minimum	7 a. m.	2 p. m.	9 p. m.	1
1866.					•		•	•	•	•	•		•				Ī
fuly 1	29. 313 29. 290	29. 262 29. 212	29. 300 29. 102	29. 292 29. 201	61 61	64. 5 65		67. 5 65	90 71	68 67. 5	71.8 67.8		63. 2 60. 5		. 399 . 537	. 411 . 439	
3	28. 947	28. 884	28. 966	28. 932	60	66	57	64.	69. 5	62.5	65. 5		60. 5	. 458	. 592	. 392	٤.4
4	28. 966 28. 943	28. 915 28. 986	28. 959 29. 018	28. 947	56 65	67. 5		58	75 68. 5	69. 5	67. 5 64. 0	29 74	50. 4 57	. 422	. 571 . 519	. 594 . 439	
5 6	29, 059	29, 027	29.023	28. 982 29. 036		63. 5 57	62	59	60	62. 5	60.5	68	56	. 454	. 426		
7]	29. 159	29, 173	29. 267	29. 200	61. 5	61	53. 5	64	66	58	62.7	71.5	56. 5	. 512	. 470	. 345	5.
8	29. 456 29. 643	29, 548 29, 535	29. 579 29. 480	29. 528 29. 553	50 44	55 58. 5	53	52. 5 45. 5	61.5	55 55. 5	56. 3 55. 3	65. 5 73	49 39. 5	. 328	. 347 . 405	. 376	5. L
10	29, 475	29. 379	29. 362	29. 405	59	68	88	64	86. 5	75	75. 2	58	64. 4	. 433	. 436		. .
11 12	29, 391 29, 271	29. 280 29. 280	29. 291 29. 270	29. 321 29. 274	67		71 75	72, : 80	93 82	81. 5 82	82.3 81.3	93 94. 5	64. 5 75. 5	. 588 759	. 501 . 667	. 617	
13	29. 296	29, 258	29. 285	29, 280	74 63	70	65	65. 5	78	67	70. 2	81. 5	62	. 542	. 625	. 591	١.
14	29. 188 29. 127	29. 222 29. 119	29. 136 29. 217	29. 182	65	70	73	66	76	80. 5	74. 2 78. 0		63	. 604 . 744			
15 16	29. 233	29. 257	29. 335	29. 154 29. 275	73 64		70 66	78 65	84 83. :	72 70	72.8	88 85	68 64	. 583		580	
17	29. 457	29, 463	29. 558	29. 293	61	65	56	54.	559	59	57. 5		60		- ; <u>. :</u>	. 409	
18 19	29. 441 29. 414	29. 403 29. 367	29, 396 29, 327	29. 413 29. 369	54 50	63 65, 5	61 55	55. 5 60. 5	5 78	63. 5 70	63. 7 69. 5	77 84. 2	47. 5 53	. 398	. 455 . 595	. 50	
20	29. 346	29. 291	29. 249	29. 295	61	73	65. 5	64	79	70	71.0	80. 5	58	. 497	. 73u	l. 556	
21 22	29. 196 29. 198	29. 185 24. 313	29. 178 29. 339	29. 186 29. 283	61.5	67	78 57	64 63	69 64. 5	66 58. 5	66.3 62.0	73. 5 66. 2	57. 2 157	. 512 . 526	. 635 . 458	. 440	 5 .
23	29. 348	29, 269	29. 309	29. 309	53. 5	6l	56	52	63	60	56.3	67	53		. 510	. 396	6.
24 25	29. 307 29. 414	29, 320 29, 384	29. 344	29. 324 29. 405	55 61	69 68. 5	63. 5 62	56 64	77 92	67 70	66. 7 75. 3	86. 5 84	56	. 420 . 497	. 601 . 515		
26	29. 336	29, 300	29. 335	29. 324		68	64	69	76	68	71.0	78	57 62	. 581	. 577		
27 28	29.370 29.371	29. 371 29. 309	29. 342 29. 276	29. 361 29. 319	62 62	66 68	62 64	62 64	72 79	66 68	66. 7 70. 3	76 82	61 58	. 556 . 529		. 50a	
29	29. 307	29. 278	29. 106	29. 230	63	62	58	60	68	63	63.7	76	60		. 476		
30 31	29. 378 29. 087	29. 304 28. 987	29. 205 28. 990	29. 296 29. 021	58 64	63 75	64 60	60 65	70 83	70 62	66. 7 70. 0	82 83	57 62	. 456 . 583		. 516 . 4 91	
Means	29. 281	29. 254	29. 257	29. 264				62.	74.9	66. 7	67. 9			. 496	. 550	. 502	
lugust 1	29. 063	29, 099	29. 283	29, 148	55	56	54	60	60	56	58. 7	65	55	. 367	. 396	391	J.
2	29. 390	29. 297	29. 227	29. 305	55	62	58	56	70	65	63.7	79	54	. 420			
3 4	29. 348 29. 614	29. 328 29. 571	29. 434 29. 583	29. 370 29. 589	58 49	54 58	53 56	60 50	56 64	55 59	57. 0 57. 7	65 72	54 42	. 456	. 391		
5	29.616	29. 576	29. 546	29. 579	50	61	56	51	66	57	58.0	72	45	. 348	. 449	. 369	١.
6 7	29. 554 29. 437	29. 451 29. 363	29. 446 29. 332	29. 484 29. 377	51 55	62 64	56 58	52 58	70 73	62 66	61.3 65.7	74 83	46 56	. 361	. 449 . 476		
8	29. 304	29, 288	29. 267	29. 286	57	56	56	59	58	58	56.3	68	56	. 439	. 422	. 42	2,
9 10	29, 287 29, 368	29. 344 29. 340	29. 364 29. 333	29.332 29.347	54. 5 57	62 66	57 64	56 59	68 76	61 68, 5	61.7 67.8	73 81. (52 55. 5	. 405 . 439			
11	29. 345	29. 280	29. 263	29. 296	60	65	65	63	70	68	67. 0	81	56	. 478		. 577	71.
12 13	29. 216 29. 361	29. 234 29. 325	29.310 29.205	29, 253	62	62 64	58 71	63. : 50. :	5 65. : 5 6 9. :	59.5	62.8	58	56. 5 60	. 536 . 463			
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the northern and northwestern lakes at Ontonagon, Michigan.

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the northern and northwestern lakes at Ontonagon, Michigan.

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the northern and northwestern lakes at Ontonagon, Michigan.

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the northern and northwestern lakes at Marquette, Michigan.

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the northern and northwestern lakes at Marquette, Michigan.

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the northern and northwestern lakes at Marquette, Michigan.

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21	29. 253	29, 160	29. 180	29. 198	65	67	65	69	74	68	70.3	75	65	. 564	. 560	. 57	7.
22	29. 243	29, 228	29. 218	29. 230	65	66	67	70	73	71	71.3	81	63		. 545		
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7 8	29, 413 29, 156	29. 376 29. 050	29. 346 29. 271	29. 378 29. 159	58 62	61 65	61 55	62 64	65 71	63 60	63. 3 65. 0	66 75	51 60	1, 429	. 480 . 537		
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11 12	29. 430 29. 181	29. 351 29. 170	29, 281 29, 197	29. 354 29. 183	63 65	62 74	65 68	66 68	65 84	68 72	66. 3 74. 7	68 85	62 65		. 516 704		
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14	29. 321	29. 360	29. 453	29. 378	63	68	60	67	77	67	70.0	82	60		. 564		
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the northern and northwestern lakes at Milwaukee, Wisconsin.

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7 a. m.	9 p. m.	Mean.	7 a. m.	2 р. ш.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Resultant in miles, p	Resultan	7 a. m.	2 р. ш.	9 р. ш.	Amount U. S. mal*.	Amount snow. decima
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	Barome	ter reduc	ed to tem	erature		TE	MPER	ATU	r e —	FAHR	ENHE	T.			VAP	OR.	
Date.	of 32°	Fahr., ex	pressed in id decima	United	P	bult oint o	of	P			tem-	ď		ine	rticit; ches s		
	7 a. m.	2 p. m.	9 p. m.	Mean.	7 в. т.	2 p. m.	9 p. m.	7 B. m.	2 p. m.	9 р. т.	Mean.	Moimum	Maximum	7 a. m.	2 р. т.	9 p. m.	Menn.
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29 30 Means	29. 559 29. 639 29. 368	29. 544 29. 584 29. 349	29, 568 29, 566 29, 375	29. 557 29. 596 29. 364	50 49	55 	55 51	53 53 54. 6	70 61 62.3	60 53 55, 5	61. 0 59. 0	63	50 53	<u> </u>	. 337		3
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the northern and northwestern lakes, at Milwaukee, Wisconsin.

	VAF	OR.		ļ		W	IND.					clo	ount udin	eas.	tion in d deci-	or melted inches and
84		nidity.	000.		Direction om when		in n	locit iles, hour,	per	tant velocity,	Resultant direction.	(10=	sky.) = sky ly ov cast.	en-	of evaporation inches and de	ag.
7 8. 10.	2 p. m.	9 p. m.	Mean.	7 а. ш.	2 p. m.	9 р. т.	7 a. m.	2 p. m.	9 p. m.	Resultan in miles	Resultan	7 a. m.	2 p. m.	9 p. m.	Amount U. S. mals.	Amount of snow, in t decimals.
. 843 . 602 . 638 . 602 . 633 . 816 . 694	. 848 449 658 716 638 636 405 673 828 790 432 432 783 783 783 783 783 783 783 783 783 783	843 680 683 756 653 7753 805 761 882 831 805 762 831 805 763 805 763 805 763 805 763 805 763 805 763 805 763 805 763 805 763 805 805 805 805 805 805 805 805 805 805	845 626 736 666 612 666 742 863 854 686 756 673 633 453 648 634 453 648 649 649 646 646 647 648 648 649 649 649 649 649 649 649 649 649 649	Calm NW NW S.SE W. NW Calm SE W. NW Calm SE Calm NA Calm SE Calm SE SW N. NE E. NE Calm SW Calm SW Calm SW Calm SW Calm SW Calm SW Calm SW Calm SW Calm SW Calm SW Calm SW Calm SW Calm SW Calm SW Calm	8.8W W.NW S. 8E W.NW N.NW Calm S. 8E N.NW E Calm W.NW W.8W NW W.8W W.8W W.8W W.8W W.8W W	Calm	0 0 5 5 5 0 0 1 1 0 0 0 5 7 7 0 0 0 3 3 1 1 2 2 0 0 0 0 0 7 7 7 0 0 0 0 0 0 0 0 0 0	0.5 1.5 1.0 1.2 8 0 0.4 1.4 1.0 0 0.4 1.3 2.2 2.2 1 0 0.3 0.0 1.6 0.3 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0 .8 6 0.5 0 2 3 0.2 0 0 0 0 0 0	1.03 0.22 1.01 0.75 0.05 0.00 0.02 0.12 0.11 0.13 1.00 1.33 1.00 0.66 0.06 0.06	N. 75 W. S. 29 E S. 19 W. N. 75 W. N. 75 W. N. 45 W. S. 19 E S. 45 E. Calm N. 67 W. S. 45 E. Calm N. 65 E N. 45 E Calm S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 21 W. S. 22 E S. 24 W. S. 24 W. S. 24 W. S. 24 W. S. 24 W. S. 24 W. S. 24 W. S. 24 W. S. 24 W. S. 24 W. S. 24 W. S. 24 W. S. 24 W. S. 25 W. S. 2	5 0 0 10 0 5 9 0 1 10 0 10 0 10 10 0 10	10 1 0 2 3 10 6 1 3 10 10 10 10 10 10 10 10 10 10 10 10 10	7 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		1.34
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. 798 . 756 . 638 . 722 . 598 . 767 . 765 . 667 . 781 . 786 . 833 . 698 . 850 . 833 . 698 . 816 . 790 . 672 . 672 . 672 . 719 . 672 . 719 . 679 . 719 . 679 . 719 . 679 . 719 . 679 . 719 . 689 . 689 . 790 . 689 . 790 . 790 . 791 . 795	.790 .627 .561 .613 .577 .636 .504 .636 .519 .606 .641 .688 .599 .511 .487 .713 .647 .713 .647 .713 .647 .713 .726 .557 .610 .598 .509 .509 .509 .509 .509 .509 .509 .509	.675 .653 .777 .798 .785 .792 .765 .790 .850 .802 .864 .843 .777 .667 .772 .816 .337 .765 .856 .606 .527 .790 .607 .790 .607 .790 .790 .790 .790 .790 .790 .790 .7	754 679 659 644 654 723 715 683 731 698 672 799 764 707 617 765 764 823 822 407 747 662 757 643 532 658 658 658 659 659 659 659 659 659 659 659 659 659	Calm Calm Calm E. SE Calm S. SE Calm Calm N. NE N. NE N. NE Calm Calm Calm Calm Calm Calm Calm Calm SW Calm Calm SW Calm SW W. NW N. NW Calm S. SE SW N. NW Calm S. SW Calm S. SW N. NW Calm	8	w:nw	0 0 4 0 9 9 7 7 5 2 5 5 4 4 0 8 2 0 2 0 5 1 1 4 2 0 2 2 0 5 1 1 4 2 0 2 2 0 5 1 1 4 2 0 2 2 0 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0.56 1.0 19 0.14 1.0 1.0 1.0 1.0 1.3 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0	0.2 0 0 0 0 0 5 0 0 0 5 0 0 0 0 0 0 0 0 0 0	0.20.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.	N. 45 E. S. 21 E. S. 21 E. S. 17 E. S. 15 E. Calm. S. 73 W. S. 71 W. S. 71 W. S. 19 E. N. 43 E. N. 73 E. S. 19 E. S. 17 E. S. 17 E. S. 17 E. S. 17 E. S. 17 E. S. 17 E. S. 18 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 57 W. N. 58 W. N. 57 W. N. 58 W. N. 57 W. N. 58 W. N. 57 W. N. 58 W. N. 57 W. N. 58 W.	0 0 1 3 5 0 8 9 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 9 3 10 0 0 0 1 10 10 5 9 8 10 10 10 17 7 9 10 5 0 10 10 5 5 0 10 10 5 5 0 10 10 5 5 0 10 10 5 5 0 10 10 5 5 0 10 5 5 0 10 5 5 0 10 10 5 5 0 10 5 0 10 5 0 10 5 0 10 5 0 10 5 0 10 5 0 10 5 0 10 5 0 10 5 0 10 5 0 10 5 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0.7 0.7 888 45 .02 .70

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Date.	of 32°	Fahr., ex	ed to temp spressed in ad decima	n United	P	bul oint pora	of	l g	y bul erate pen	ire o	tem-	a	ď	in	rticity ches		
	7 a. m.	2 p. m.	9 p. m.	Mean.	7 a. m.	2 р. п.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Mean.	Maximum	Minimum	7 a. m.	2 p. m.	9 p. m.	Mean.
1866.		`				0			۰	٥	0		۰				
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3	29. 659	29. 648	29.713	29, 673	34	40	41	37	47	43	42.3	48	33	. 157	. 156	. 231	. 1:
4	29. 821	29. 897	29. 955	29. 891	36	34	32	40	39	38	39.0	45	38		. 131		
5 6	30. 040 29. 890	30. 027 29. 752	29. 973 29. 675	30, 013 29, 772	35 29	36 40	36	40 32	41	40 38	40. 3 38. 7	53 53	38		. 147 . 169		
7	29. 575	29. 440	29. 357	29. 457	31	45	44	35	52	52	46. 3	56	36	. 128	. 207	. 183	
8	29. 365	29. 372	29. 417	29, 385	48	48	40	52	59	45	52.0	60	44		. 190		
9 10	29. 480 29. 420	29. 485 29. 265	29. 505 29. 179	29. 490 29. 289	38 33	45 43	37 40	43 37	58 47	45 42	48. 7 42. 0	59 50	43 36	. 164	. 260 . 225	. 116 . 221	
ii	29. 144	29, 261	29, 400	29. 268	34	36	35	37	40	40	39.0	45	36	. 157	. 160	. 139	9.1
12	29. 537	29. 573	29. 593	29. 568	29	38	35	31	42	37	36.7	45	32	. 137	. 177 . 182	. 178	
13 14	29. 420 29. 063	29. 347 29. 133	29. 225 29. 290	29. 331 29. 162	3 9	40 37	42 38	43	45 43	47 42	45. 0 44. 0	48	34		. 142		
15	29. 123	28. 969	28.898	28.997	33	35	32	37	39	35	37.0	49	35	. 136	. 152	. 14:	
16	28.904	28. 989	29. 154	29.016	30	34	34 42	33	40	38	37.0	41 47	32		. 118		
17 18	29. 211 29. 202	29, 154 29, 179	29. 136 29. 148	29. 167 29. 176	34	39 41	41	37	45 45	45 44	40. 7 42. 0	49	37		. 160 . 205		
19	29. 083	29, 036	29, 086	29.068	35	40	39	38	44	42	42.0	46	38	. 165	. 195	. 173	3. 3
20	29. 235	29. 291	29. 378	29. 301	34	36	35	38	42	38 37	39.3	46 42	38		. 134		
21 22	29. 373 29. 114	29. 118	29. 074 29. 424	29, 188 29, 269	31	34	34 28	34	36	31	35. 7 32. 0	41	34 31	. 151	. 170	119	
23	29. 463	29. 491	29. 459	29.471	26	29	25	30	32	29	30. 3	40	29	. 095	. 126	0.0	• . !
24	29, 606	29. 562	29, 564	29. 577	20	24	24	22	29	27	26.0	31	22		. 072		
25 26	29. 521 29. 273	29. 343 29. 253	29, 288 29, 248	29. 384 29. 258	20 33	34 39	32	22 36	40	36 43	22. 7 40. 7	41 45	21 36		. 118 . 186		
27	29. 116	28. 979	28, 989	29, 028	39	45	43	43	48	46	45.7	53	42		. 260		
28	29. 056	29. 066	29. 166	29. 096	29	29	30	33	33	32	32. 7	50	31	. 114			
29 3 0	29. 134 29. 271	29. 032 29. 388	29. 187 29. 431	29, 118 29, 363	27 22	30 25	30 25	30 25	33 29	31 29	31.3 27.7	36 35	31 25		. 132 . 089		
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Dec. 1	29. 604 29. 288	29. 471 29. 182	29. 418 29. 244	29. 498 29. 238	16 27	28 38	31 40	17 30	33	33 45	27. 7 39. 7	39 49	17 30	. 078 . 113	. 096 . 151		
3	28. 966	28. 873	28. 926	28. 922	42	41	39	45	47	42	44. 7	49	43	. 228	. 249	. 199	. 2
4	29.046	29. 046	29. 295	29. 129	30 29	38 39	33	33	43	36 43	37. 3 39. 0	46	33		. 164 . 186		
5 6	29. 465 29. 408	29. 488 29. 400	29. 543 29. 460	29, 499 29, 423	40	42	38	42	45	40	42.3	47	41	221	228		
7	29. 375	29. 250	29. 011	29, 212	37	38	37	39	41	39	36. 3	45	39		. 190	. 194	١,
8	28. 615	28. 898 29. 195	29, 169 29, 248	28. 894 29. 193	32 11	24	19	34 12	27	22 11	27. 7 10. 7	42 25	22		. 095 . 037		
9 10	29. 135 29. 302	29. 193	29. 246	29. 306	15	ıí	12	17	13	14	11.3	17	5		. 049		
11	29. 335	29. 385	29. 475	29. 398	4	5	3	5	8	5	06.0	17	4		. 021	. 027	
12 13	29, 600 29, 276	29. 608 29. 525	29, 618 29, 640	29. 609 29. 480	- 2 11	9 15	10	- 1 14	12 18	13 11	08. 0 14. 3	15 21	- 2 10	. 037	. 031 . 052	. 034	
14	29. 780	29. 782	29, 830	29. 797	16	21	22	18	24	25	22. 3	28	iĭ	. 067	. 079		
15	29. 694	29. 513	29. 351	29. 519	20	23	23	25	26	27	26. 0	29	23		. 089	. 072	
16 17	29, 100 29, 371	29. 105 29. 448	29. 237 29. 494	29. 147 29. 438	25 12	25 24	19 22	27 14	27 28	22 25	25. 3 22. 3	30 31	21	. 052	. 112 . 063	. 069	
18	29. 469	29. 358	29.411	29. 413	16	27	30	18	30	32	26.7	33	15	. 067	. 113	. 144	- 1
19	29. 431	29. 562	29.720	29. 571	31	25	20	33 22	29 23	23 25	28.3 23.3	35 27	22	. 151 . 083	. 069 . 058	. 074 . 067	
20 21	29. 842 29. 504	29. 792 29. 362	29. 689 29. 284	29. 774 29. 383	20 25	19 26	21 31	27	29	33	29.3	33	25		. 106	. 162	0
22	28. 999	28. 874	28. 826	28. 900	35	37	37	37	40	39	38. 7	44	32		. 181		. 1
23	28. 775	28. 846	28. 904	28. 842	35	31	27	37 28	34 25	32 18	34.3	33	32 16	. 178 . 083	. 139	090	. 1
24 25	28, 942 29, 248	28. 999 29. 381	29, 152 29, 243	29. 031 29. 291	24	20 16	14 21	11	21	25	23. 7 19. 0	27	10	. 022	. 033	. 067	. 0
26	29. 236	29. 217	29.317	29, 257	12	14	7	14	17	10	13.7	29	8	. 052	. 048	. 096	. 0
27	29, 478	29. 520	29. 623	29. 540	0	5	3	2 2	7	5	04. 7 07. 7	16 17		. 021	. 032	.014	. 0
28 29	29. 688 29. 695	29. 666 29. 600	29. 713 29. 610	29. 689 29. 635	ő	5	- 3	2	8	- í	03.0	12		. 021	. 021	. 015	. 0
30 31	29. 575 29. 325	29. 454 29. 415	29. 385 29. 640	29. 471 29. 460	- 6 18	11 15	15	- 5 20	9 18	19 6	07. 7 14. 7	20 23	- 5	. 021		. 040	. 0
٠.	29. 341	29. 339	29. 380	29. 353	<u> </u>		<u> </u>	20. 6					!	. 091		095	_

the northern and northwestern lakes at Milwaukee, Wisconsin.

	VAP	or.				w	IND.					clo	nount udine = cle	ess.	tion, in d deci-	melted inches
Sat	Hum turation	idity. n = 1,0	00,		Direction om when		m	city, iles, our.	per	nt velocity,	Resultant direction	(10:	sky.) = sky ely ov cast.)	/ en- / er-	of evaporation inches and	of rain or in U. S. ecimals.
7 a. m.	2 p.m.	9 p. m.	Mean.	7 a. m.	2 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Resultant vel	Resultan	7 a. m.	2 p. m.	9 p. m.	Amount U. S. mals.	Amount of snow, in and decim
.547 .712 .645 .537 .694 .698 .727 .619 .712 .778 .669 .669 .690 .791 .721 .721 .721 .721 .721 .721 .696 .696 .696 .696 .696 .696 .696 .69	411 446 483 547 567 543 381 777 698 645 661 607 511 636 476 677 500 802 694 449 476 669 777 606 669 777 608 669 677 500 800 800 800 800 800 800 800 800 800	847 712 833 450 645 719 807 807 629 557 621 661 698 762 756 622 628 633 767 794 833 863 863 863 863 863 863 863 863 863	.602 .676 .547 .592 .652 .512 .583 .752 .638 .752 .696 .661 .602 .696 .616 .617 .742 .743 .646 .597 .688 .759 .743 .743 .743 .743 .743 .743 .743 .743	SW Calm E. NE SE SW W. NW Calm S. SE S. S. S. S. S. S. S. S. S. S. S. S.	W. SW W. NW Calm NE S.SE S.SE W. NW E. NE S. SE S. SE W. NW W. NW N. NW N. NW N. NW N. NE N. N. N. N. N. N. N. N. N. N. N. N. N. N	Calm E. NE. E. NE. S.SE Calm SW	0 2. 2 0. 5 0. 3 0. 1 1. 2	2.00 3.41 2.00 5.51 4.05 4.05 5.51 6.06 6.06 6.06 6.06 6.06 6.06 6.06 6.0	1. 5 0 0. 4 0 2. 0 6 0 0. 5 0 0. 5 0 0. 3. 6 1. 1 0 0 0 0. 3 0 0. 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.7 0.1 0.3 0.3 1.3 1.7 1.2 0.2 0.2 0.7 0.3 0.6 0.2 0.7 0.7 0.2 0.7 0.2 0.7 0.2 0.7 0.3 0.3 0.3 0.3 0.3 0.3 0.3 0.3	N. 75 W. N. 71 E S. 17 E S. 17 E S. 20 W. N. 71 W. N. 74 W. N. 74 W. N. 74 W. N. 45 W. S. 21 E S. 29 E S. 29 W. N. 15 W. N. 15 W. N. 13 W. N. 13 W. N. 13 W. N. 14 W. N. 14 W. N. 15 W. N. 14 W. N. 15 W. N. 15 W. N. 17 E S. 45 W. N. 11 W. N. 17 W. N. 11 W.	5 1 1 0 9 10 0 0 2 0 5 5 0 10 8 3 3 5 10 0 0 0 10 0 10 10 10 10 10 10 10 10 1	0 3 10 9 7 8 0 0 10 10 0 3 7 1 1 8 10 10 10 10 10 10 10 10 10 10 10 10 10	5 0 0 0 0 0 0 0 10 10 10 10 10 10 10 10 1		
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. 834 675 762 703 816 829 816 635 538 457 682 378 800 721 807 543 800 721 543 803 804 438 438 438 438 438 627 702	510 522 772 587 687 684 568 623 334 455 525 610 634 761 753 87 87 87 87 87 87 87 87 87 87 87 87 87	. 800 . 607 . 744 . 705 . 820 . 816 . 584 . 596 . 622 . 794 . 622 . 794 . 496 . 496 . 496 . 496 . 371 . 500 . 496 . 371 . 500 . 377 . 253 . 369 . 392 . 393	. 715 601 759 . 665 736 . 673 . 656 . 599 . 513 . 528 . 514 . 702 . 670 . 707 . 774 . 785 . 672 . 410 . 397 . 410 . 272 . 320 . 510	Calm S.SW S.SW W.NW Calm W.SW W.NW W.NW W.NW W.NW M.NE NE Calm Calm Calm E.SE S.SW Calm NW NW NW NW NW NW NW NW NW NW NW NW NW	W. NW Calm	W. NW N.NE E N.NW Calm Calm S.SE S.SE S.SW Calm NW NW NW NW NW NW	0.7 1.5 1.1 2.0 0.9 0.2 0.2	0.6770.442.592.2.590.662.2.800.5771.5301.2.11.672.2.11.0.11.672.2.11.0.11.672.2.11.0	1. 8 0. 44 0. 3 0. 2 2. 8 0. 5 0 0. 1 3. 0 0 2. 1 0. 2 2. 1 0 0. 2 2 0. 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1. 0 0 0 0 0 0 1 1 0 0 0 0 0 0 1 1 0	S. 11 W. S. 41 W. S. 41 W. N. 61 E N. 75 W. N. 61 E N. 75 W. N. 71 W. N. 79 W. N. 73 W. N. 73 W. N. 73 W. N. 73 W. N. 73 W. N. 73 W. N. 73 E S. 65 W. N. 75	0 6 0 0	0 0 10 10 10 10 10 10 10 10 10 10 10 10	0 5 10 0 0 10 10 10 10 10 10 10 10 10 10 10		

	Barome	ter reduc	ed to tem	nerature		TE:	MPER	ATU	RE—	Fahi	UKYH	EIT.			VAF	·0.	•
Date.	of 32°	Fahr., ex sinches a	pressed i	n United	P	t bul oint porn		P	y bull eratu pen s	re o		É	e e			in U. i	
	7 а. ш.	2 p. II.	9 p. m.	Mean.	7 a. m.	2 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Mean.	Maximum	Mioimum	7 а. ш.	2 p. m.	9 p. m.	Mean.
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4	29. 250	29, 221	29. 198	29. 223	71	70	68	73	74	71	65. 5	i)	53	. 731	. 679	. 644	. 6
5	28, 992	29. 678	29.388	29. 078 29. 190	57	66	68	60	68	71	68. 0 65. 5		60	. 426	. 612	. 614	. 61 . 53
7		29. 159		29, 159		78			81		81. 0				.918		91
8 9	29, 266 29, 569	29. 315 29. 574	28. 913 29. 584	29. 165 29. 576	65 55	67	64 58	68	71 65	67 62	68. 7 61. 7		66 55	. 577	. 60€ . 516	. 556	. 5-
10	29. 567	29. 518	29. 450	29. 512	52	62	56	58 56	72	58	62. 0		46	. 335	. 595		
11	29. 492	29.418	29. 420	29. 443	52	70	72	55	75	74	68. 0		46	. 349	. 666	. 757	. 52
12 13	29. 418 29. 270	29. 333 29. 218	29. 262	29, 378 29, 250	68 73	77 81	72	72	83 88	76	77. 5 80. 3		70	. 631	. 860 . 962	. 731	. 74
14	29. 312	29. 249	29. 287	29. 283	71	75	79	75	81	83	79. 7		66. 5	. 704	. 787	. 936	
15 16	29, 309 29, 363	29. 268 29. 345	29. 293 29. 390	29. 290 29. 363	74	80 72	84 70	78 78	86 77	89 73	84. 3 76. 0		72 73	. 785	.942	1. 096 . 693	
17		29, 283	29. 271	29, 277		64	61		67	64	63. 5		63		. 556	. 497	
18 19	29. 321 29. 297	29, 125 29, 243	29, 241 29, 203	29, 229 29, 248	55	66	70	58	71	74	67. 7		63. 5 57	. 393	. 572 . 679	. 679	
20	29. 159	29. 228	29, 235	29, 207	60 55	70	64 68	64 59	74 74	70 72	69. 3 68. 3		60	. 380	. 718	. 516 . 631	
21	29. 342	29, 332	29. 469	29, 381	83	85	78	85	86	83	84. 7	·]	76	1. 101			La
22 23	29, 333 29, 230	29. 375 29. 162	29. 330 29. 193	29, 346 29, 195	73 63	72 73	70 71	78 65	77 77	73 74	76. 0 72. 0		73 62	.744	. 718	. 693 I . 718	
24	29, 325	29, 328	29. 331	29, 328	60	67	62	63	70	65	66. 0		60	. 478	. 622	. 516	. 5
25 26	29. 376 29. 381	29, 298 29, 388	29. 335	29. 336 29. 421	55	72	69	58	78	74	70. 0		60. 5 50		. 704 . 693	. 641	
27	29. 484	29. 420	29, 493 29, 392	29. 421	64 61	70 63	67 70. 5	65 63	73 68	70	68. 3		57	.583	. 509	. 698	
28	29. 413	29. 375	29. 318	29, 369	64	71	70	68	79	75	74. 0		67. 5	. 543	. 651	. 666	. 62
29 30	29, 290 29, 225	29. 270 29. 230	29, 264 29, 230	29. 275 29. 228	64 65	73 71	73 71	69 68	77 75	77 74	74. 3 72. 3		63 58	. 529	. 757 . 704	. 757	
31	29. 311	29. 311		29. 311	61	64		64	71		67. 5			. 497	. 503		.35.
Means	*29. 342	t29. 363	;29. 314	29. 308				66. 7	41.7	69. 3	71. 2		<u> </u>	. 556	. 706	. 650	. 67:
Aug. 1	29. 386	29. 227	29. 085	29. 233	52	67	69. 5	55	73	71	66. 3	ļ	51	. 349	. 581	. 701	.54
2 3	29, 009 29, 173	28. 969	29. 084 29. 400	29. 021 29. 287	68 57	72	69 67 5		75	71 70	72. 8 65. 0		50. 5 54. 5		. 744	. 623	
4	29. 288	29. 296	29. 256	29. 280	53	65	67. 5	60 56	68		62.0			. 363	. 577		. 47
5 6	29. 487	29. 388 29. 497	29. 448 29. 523	29.418		60	65	l	66	69	68. 0		52	400	. 438	. 564	
7	29. 101	29. 342	29. 480	29. 502 29. 411	58	64	65 65, 5	62	69. 5 72	68 68	66. 5 70. 0		55 58	. 429	. 631	. 577	
8	29, 481	29. 445	29. 453	29. 460	52	67	60	56	71	63	63. 3		58 51. 5		. 600	. 478	. 47
9 10	29. 148 29. 479	29, 255 29, 456	29. 353	29, 252 29, 468	55 54	62 64	62	58 56, 5	65 68	65	62. 7 62. 3		55	. 393	. 516 . 543	. 516	46
11	29. 479		29. 438	29, 458	65		69	62	J	71	66. 5		61	. 577		. 622	. 6.
12 13	29. 291 29. 140		29, 270 29, 240	29, 280 29, 190	63	• • • •	68 65	65 67. 5		70 68	67. 5 67. 8		63 62, 5	. 549	• • • •	. 658	.00
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15 16	29, 628 29, 709	29, 688	29. 731	29, 682	57	55	52	60	59	55	58.0		53	. 426	. 380	. 349	
17	29. 538	29, 655 29, 491	29. 628	29, 664 29, 514	42	59 64	56	45 52	64 69	60	56. 3 60. 5		32	.228	. 433 . 529	. 396	
18	29.413		29.316	29, 264	57		62	60	ļ. .	63	61. 5		57. 5	. 426		. 542	. 45
19 20	29. 332	29. 256	29. 304 29. 203	29, 280 29, 268	46	61	58 57	47	63	61 59	63. 0 53. 0		56. 5 44	.297	. 483	. 443 . 439	. 410
21	29. 233			29, 233	51		ļ	53			53. 0	l		. 348			. 34
22	29. 248	29, 226 29, 213	29. 266	29. 247	44	52	19	46	55		51. 2		52	. 262	. 349		
24	29. 265 29. 311	29. 302	29. 325	29. 166 29. 311	44	43	51		57 46	54	51. 5 47. 7	1	31. 5	. 202 . 278	. 350	. 335	
25	29. 443	29. 356	29. 312	29. 370	46	54	53	49	59	56	54. 7		44. 5	. 271	. 351	. 363	. 52
26 27	29, 209	29. 092	29. 164 29. 218	29, 187 29, 155	51	65		53	67	64 66	58. 5 66. 5		19 52	. 348		. 497	
28	29. 254	29. 291	29, 316	29. 287	54	59	55	57	62	58	59. 0		58	. 378	. 460	. 393	. 41
29 30	29. 363	29. 323	29. 306 29. 206	29, 331	50	58		57	62. 5	44. 5	54. 7		43	. 268		. 911 . 591	
31	29. 241	29. 253	29. 200	29. 206 29. 247	64	64	65	67. 5	67. 5	01	67. 5			. 549	. 549	. 391	
Means	29. 343	29. 339	29. 334	29. 320	_	<u> </u>	-	<u> </u>	i—		61.7	<u> </u>	-	. 390	497	. 509	. 46
	20. 1713	25. 559	~3. Uk71	23. 320	١			JO. 4	03. 1	03. 4	01.7			. 350	. 431		

^{*} Mean of 27 observations.

REPORT OF THE SECRETARY OF WAR

the northern and northwestern lakes at Tawas City, Michigan.

	V.A	POR.				v	/IND		-			cle	noun oudii		N A	melted
Sa	Hun sturation	nidity. on = 1,	000.		Direction com when			locity niles, nour.	y, in per	Resultant velocity, in miles, per bour.	Resultant direction.			y en ver-	of eve	of rain or in U. S.
ej Ej	p. ii	p. ii	Mean.	e e	P. B.	p. H	P E	P.	p. m.	esultar n miles	esultan	9	p. m.	p. m.	Imount U. S. mals.	Amonnt of range, in
	- 64	- 6-				_ &	1	_ CR	6	==	<u> </u>	-	Cs	- 6		<u>-</u>
. 765 . 731	. 564	. 843 . 898	. 724 . 804	w sw	8 8	s sw	2 8	8 5	3 13	4.0	8. 37 W	3 10	5 9	10 10		
. 902	. 854 810	. 850	. 852 . 854	sw	sw	8W	12	12	2 2	1.3 8.7	S. 21 W S. 41 W	4	10	10 4		
. 822	. 895	. 850	. 895	sw	8		3	12	2	4.0 1.7		8	. 9			
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. 805	.768	. 903	. 859	SE	8 W	sw	2 2	10	2	4.3		3	1 4	0		
. 804 . 817	. 787 . 727	.815	. 846	8W	w	w	6	18 27	4	6.7	West	li	7	: 8		
. 812	. 744	. 830	. 795	8	8	8	4	8	2	4.7	South	2	5	3		
. 819 . 776	. 758	. 803	. 793	sw	sw w	8W	8 2	20	3	9.3		10	10	10		
. 770	. 774	.833	. 837	311	NE	w		2	2	0.7			10	10		
. 816	. 754	.810	. 793	w	E	W	2	2	3	0.7	West	6	4	4		
. 780 . 761	. 810 . 856	. 704	. 765	8 W	8 8E	W	2 2	4 2	3	2.3 1.0		5 8	3 5	5		• • • • • •
. 915	. 836	. 789	.847	8	S	8w	3	ıĩ	3	5. 7		10	10	5		
776	. 774	. 854	. 801	8W	w	W	8	2	3	4.3	S. 83 W	6	10	1 10		
890 831	.817	.856	.854	sw	SW	SW	2 2	15	2 2	4.0		10	8	3		
. 816	. 734	. 764	.771	SE	8	w	2	14	3	2.0	S. 15 W.	ŏ	2	3		
. 944	. 854	. 848	. 882	NW	E	W	6	15	3	3.0	N. 62 E .	0	l l	3		
. 846 . 793	. 743	. 833	. 821	E	E	SW	2 2	4	2 2	1.7	S. 74 E . N. 40 E .	1 0	6	10		
. 747	.817	. 817	. 794	w	E	w	2	2	4	1.3	West	7	7	8		
. 843 . 833	.812	. 856	. 804	W	N	N	2 2	16	3	4.0 5.7	N. 10 W. N. 83 E	0	10	8		
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.818	. 785	. 827	. 814	===				=	==	2. 1	8.51 W	4.3	6. 1	4. 9	<u></u>	. 30
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the northern and northwestern lakes at Tawas City, Michigan.

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the northern and northwestern lakes at Tawas City, Michigan.

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675 800 .794 .756 8 8 8 10 5 7.7 South 10 10 10 900 .907 .820 .876 8 8 5 8 6 6.3 South 10 10 10 900 .800 .768 .799 NW 8W N 4 4 2 2.0 N.71W 10 10 10 .702 .730 .610 .681 W W W 6 5 8 6.3 West 10 10 10 .702 .730 .610 .681 W W W W 6 5 8 6.3 West 10 10 10 .702 .730 .610 .681 W W W W 4 8 1.7 West 10 10 10 .702 .730 .623 .551 NW		607			NW									:			. 05
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. 702 . 730 . 610 . 681 W. W. W. 6 . 5 . 8 . 6. 3 West. 10 . 10 . 10	. 900	. 907	. 820	. 876	8	8	8	5	8	6	6. 3	South	10	10	10		. 52
. 702 .730 .610 .681 W W W 6 3 8 5.7 West 10 5 0 .475 .525 .623 .541 NW NW NW 4 8 14 8.7 N.45 W 0 6 0 .492 .623 .558 NW NW NW 16 9 8.3 N.45 W 4 1	. 600 709	730	768	789		8W	N		5	2	6.3	N. 71 W. West	10			•••••	
	. 702	. 730	. 610	. 681	w	w	w	6	3	8	5. 7	West	10	5	0		
. 610 .475 .648 .578 NW NW N 3 15 5 7.3 N.36 W . 10 10 10 10 438 .492 .465 NW W 5 4 2.7 N.64 W 9 5		. 525	. 623	. 541	NW	NW	NW	4	8	14	8.7	N. 45 W		6			
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. 677 . 685 . 668 . 679	. 677	. 685	. 668	. 679					ļ		5. 3	8. 72 W	4. 4	7.3	6. 5		1. 45

	Barome	ter reduc	ed to tem	perature		TX	MPE	RATU	RE-	FAH	RENHE	IT.			V AI	POR.	
Date.			pressed in nd decim		p	t bul oint o poras	D Í	p	bul bratu pen s	ure o	tem-	ä	٠	ine	ticity ches	, in I	J. S dec
	7 P. III.	2 P. B.	9 p. m.	Mean.	7 a. m.	2 p. m.	9 p. m.	7 8. 10.	2 p. m.	9 p. m.	Mean.	Maximum	Minimum	7 a. m.	2 р. ш.	9 p. m.	Mean.
1866.					-	-	-	-	-	-	•	-	•	_		_	_
July 1	29. 644	29. 619	29, 644	29. 636	60	63	59	63	73	63	66. 3	74	ន	. 478	. 442	- 776	. 56
3	29. 582 29. 381	29. 525 29. 375	29. 449 29. 191	29, 519 29, 316	61 62	66 62	64 58	66 68	77	71 63	71.3 68.3	78 77	61 62	476	. 492	503 116	49
4	29. 105	29. 289	29, 197	29. 197	60	67	63	63	76	70	69. 7	75	60	. 478	. 542	. 482	. 5
5 6	29. 315 29. 535	29. 321 29. 481	29. 324 29. 324	29. 320 29. 447	61 71	73 72	69 67	70 77	84 84	75 73	76.3 78.0	86 85	64 69		. 663 . 623	. 028 . 521	
7	29. 371	29. 347	29. 389	29. 369	68	74	69	71	83	71	75.0	84	67	. 644	715	. 682	
8 9	29. 327 29. 594	29. 485	29. 463 29. 621	29. 425 29. 611	69 57	68 61	55 56	71 60	74 71	64	69.7	78	62 55		. 604	- 314	
10	29, 594	29. 619 29. 612	29. 521	29. 611	58	63	62	61	72	61 66	64. 0 66. 3	71 74	50	. 443	. 403 . 455	183 502	
11	29, 692	29. 588	29. 533	29, 604	61	67	64	64	78	70	70.7	79	52	. 497	. 514	516	. 5
12 13	29, 581 29, 483	29. 528 29. 391	29. 454 29. 331	29. 521 29. 402	66 70	72 75	71 74	70 75	84 90	75 80	76.3 81.7	85 91	61 61	. 586 . 666	. 623 . 665	. 704 . 75ê	
14	29. 419	29. 410	29. 400	29. 410	71	75	73	77	91	79	82.3	92	70	. 678	. 651	730	
15	29. 458	29, 537	29. 425	29. 473	71	75	74	78	90	81	83. 0	92	71	. 664	. 665	. 745	
16 17	29, 523 29, 520	29. 464 29. 442	29, 459 29, 376	29. 482 29. 446	74 73	76 72	74	81 79	91 77	80 73	84. 0 76. 3	92 87	73 71	. 745 . 730	. 694 . 718	. 758	
18	29, 306	29. 312	29. 379	29. 332	71	65	60	72	69	64	68.3	78	62	. 745	. 543	. 465	. 5
19 2 0	29. 497 29. 478	29. 499 29. 441	29. 486 29. 393	29. 494 29. 437	61 62	65 64	62 62	64 66	70	65 67	66. 3	71	56 58	. 497	.550	- 516	
20 21	29. 478	29, 441	29, 239	29. 437	66	69	64	68	70 77	68	67. 7 71. 0	73 78	64	. 502 612	. 601	489 43	
22	29. 255	29, 219	29, 210	29, 228	67	72	67	70	81	72	74.3	83	62	. 622	. 663	. 95	. 6
23 24	29. 323 29. 439	29. 322 29. 399	29. 337 29. 380	29. 327 29. 406	63 65	66 68	66	68 67	76 80	67 72	70. 3 73. 0	72 83	62 58	. 369 . 591	. 505 . 524	489 559	
25	29. 436	29. 459	29. 464	29. 453	68	69	62	72	80	69	73.7	82	64	. 631	. 56	62	
26	29. 492	29. 477	29. 423	29. 464	64	70	68	66	80	74	73.3	82	58	. 569	. 598	. 604	. 5
27 28	29. 436 29. 324	29. 396 29. 283	29. 336 29. 148	29. 389 29. 252	70 69	73 65	69 65	73 73	82 70	72 69	75. 7 70. 7	83 84	69 62	. 693 . 655	. 690 . 550	668 564	. 60 . 57
29	29. 286	29, 221	29. 215	29, 241	63	70	66	65	80	74	73.0	83	61		. 598	. 532	. 56
30 31	29, 325 29, 444	29, 305 29, 385	29. 339 29. 249	29. 323 29. 359	63 58	68 64	57 65	68 65	77	64 71	69. 7 71. 0	78 80	62 52		. 564 . 422	. 373 . 537	. 48 . 44
Means	29. 438	29. 420	29. 380	29. 413				69. 4	76. 4	70. 4	72.1			. 575	. 573	. 567	. 57
Aug. 1	29. 137	29. 135	29. 171	29. 148	67	67	59	70	81	67	72.7	8-5	65	. 622	. 474	. 393	. 49
2	29. 274	29. 335	29. 388	29. 332	54	60	55	60	72	62	64. 7	76	56			. 340	. 34
3 4	29. 472 29. 346	29. 417 29. 435	29. 318 29. 497	29. 402 29. 426	60 56	63 60	61 53	69 59	74	66 58	69. 7 63. 3	77 73	51 56	. 398	. 429 . 385	. 430	
5	29, 556	29. 537	29. 598	29. 564	55	58	57	60	74	61	65. 0	75	50	. 367	. 270	. 412	. 3
6	29. 581	29. 550	29. 500	29. 544	52	62	58	58	76	63	65. 7	76	49		. 369		
7 8	29. 528 29. 364	29, 492 29, 209	29. 449 28. 986	29. 490 29. 186	59 60	61 64	60	63 63	69 68	64 69	65. 3 66. 7	74 69	59 60			. 465 . 635	
9	29. 328	29. 384	29. 351	29. 354	56	59	60	59	69	64	64. 0	74	56	. 409	. 367	. 465	. 4
10 11	29. 492 29. 541	29, 484 29, 552	29. 469 29. 496	29. 482 29. 530	58 64	63 65	62 64	63 67	69 69	67 66	66. 3 67. 3	72 71	54 60	. 416		. 489 . 569	
12	29.400	29, 330	29. 343	29. 358	64	67	68	65	72	71	69. 3	74	63	. 563	. 595	. 644	
13	29, 229	29, 252	29. 341	29. 274	69	68	66	70	76	68	71.3	81	67	. 695	. 577	612	
14 15	29. 356 29. 556	29, 356 29, 654	29. 363 29. 670	29. 358 29. 627	61 59	69 55	64 47	63 62	78 61	67 50	69. 3 51. 0	80 70	62 49		. 588 . 354	. 556 283	
16	29. 727	29. 704	29. 644	29. 692	48	54	56	52	65	60	59.0	66	42	. 282	. 272	. 396	
17 18	29, 655 29, 477	29. 587 29. 404	29. 506 29. 303	29. 583 29. 395	50 60	60	55 63	52 64	74 75	62 68	62. 7 69. 0	75 76	44 51	. 334	. 331 . 449	. 340	
19	29. 296	29. 262	29. 303	29, 290	58	64 60	64	62	69	56	62.3	72	55	429	398	. 509 . 391	
20	29. 401	29.312	29. 303	29. 339	51	58	55	52	72	58	60. 7	74	45	. 361	. 296	'. 393	. ::
21 22	29. 311 29. 305	29. 277 29. 334	29. 195 29. 338	29. 261 29. 326	53 48	56 52	55 47	55 52	67 62	60 50	60. 7 54. 7	71 72	49 50		. 303 . 256		. 3H
23	29.314	29, 286	29. 305	20.302	45	49	47	48	57	49	51.3	69	45	. 260	. 242	. 297	31
24	29. 402	29. 395	29. 381	29, 393	48	52	48	50	61	54	55. 0	62	47		. 20	. 256	27
25 26	29. 491 29. 376	29. 454 29. 339	29. 391 29. 296	29. 445 29. 337	45 52	54 60	48 50	48 59	63 70	54 62	55. 0 63, 7	65 72	41 49		. 298 . 385	. 256	24
27	29, 292	29. 288	29, 272	29. 284	55	61	60	60	72	65	65. 7	76	55	. 367	. 390	. 451 .	36
28	29.308	29, 293	29. 306	29. 302	58	66	60	60	74	63	65. 7	79	49	. 456	. 532	. 478 .	48
29 30	29. 405 29. 343	29. 396 29. 310	29. 361 29. 250	29, 387 29, 301	55 59	60 64	59 62	58 61	69 69	64 65	63. 7 65. 0	69 67	56 60	473	. 398 . 529	. 433 . . 516 .	50
31	29. 298	29, 218	29. 213	29. 243	63	66	64	65	72	68	68.3	75	62			. 513.	
Means	29, 405	29. 390	29. 365	29, 386		!		59. 6		!				. 416	—i		_

the northern and northwestern lakes at Detroit, Michigan.

	VAI	POR.				w	IND.					clo (0	ount udin — cl	ess. ear	and deci-	nelted
Sa	Hum turatio	ildity. on == 1,0	000.		Direction om when		m	city, iles, our.		t velocity, per bour.	Resultant direction.	(10 :	sky. = sk ly o cast.	y en- ver-	of evapo inches	nt of rain or r, in U. S. decimals.
7 а. т.	2 p. m.	9 p. m.	Мевп.	7 p.	2 P. II.	9 p. m.	7 a. m.	2 р. ш.	9 p. m.	Resultant in miles, p	Resultan	7 a. m.	2 р. т.	9 p. m.	Amount U. S. mals.	Amount snow, and de
. 831 . 695 . 695 . 695 . 731 . 699 . 822 . 823 . 799 . 823 . 798 . 731 . 693 . 768 . 731 . 693 . 768 . 738 . 893 . 789 . 893 . 795 . 893 . 894 . 894 . 895	.545 .531 .472 .603 .535 .636 .535 .582 .537 .535 .448 .472 .477 .744 .477 .774 .477 .774 .648 .627 .563 .512 .535 .535 .545 .545 .545 .545 .545 .545	. 440 . 663 . 723 . 658 . 724 . 716 . 899 . 786 . 704 . 741 . 738 . 812 . 741 . 741 . 741 . 740 . 793 . 758 . 740 . 793 . 740	.605 643 630 697 795 731 691 795 731 691 7660 639 841 807 744 748 7682 706 668 732 779 763 668 732 779 763 765 765 765 765 765 765 765 765 765 765	SE SW SW SW SW SW SW W W W SW.	SW S.W SW SW SW SW E S.SW SW W.SW W.SW SW W.SW SW W.SW SW N.NW E. NE E. NE SW SW N.NW SW SW N.NW SW SW SW SW SW SW SW SW	SE S. S. S. S. S. S. S. S. S. S. S. S.	1. 5.7 7.5.0 4. 5.8 7.1. 8.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9.0 9	7.005000345502285500755255450003553	10.53 4.33 7.05 1.88 6.85 1.55 2.05 2.88 3.03 3.03 2.78 2.88 2.15 2.88 2.15 2.88 2.15 2.88 2.15 2.88 2.15 2.15 2.15 2.15 2.15 2.15 2.15 2.15	6.70 6.00 4.27 6.33 6.33 3.00 2.33 3.23 4.00 3.00 3.00 3.00 3.00 3.00 3.00 3.0	S. 28 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. N. 76 E. N. 77 W. N. 77 W. N. 78 E. N.	8 8 9 10 6 4 9 10 8 0 0 0 6 0 0 3 0 0 10 7 8 8 4 4 4 3 3 8 6 6 6	9 8 10 7 8 2 8 9 3 2 2 3 0 7 7 5 4 6 9 9 10 8 8 8 4 4 4 5 5 5 2 E 7 6 7 7	10 8 6 4 10 9 4 3 5 5 3 7 7 4 5 5 7 7 8 8 6 6 10 9 6 6 6 7 7 4 4 8 8 7 8		.04 .66 .53 .92 .34 .10
. 794	. 587	. 725	. 702				<u></u>			1. 1	S. 46 W.	5. 1	6. 1	6. 6		2.94
. 848 . 653 . 554 . 819 . 708 . 641 . 819 . 776 . 831 . 841 . 948 . 886 . 727 . 780	448 457 511 525 322 412 793 796 758 644 395 659 461 395 564 459 461 502 497 797 634 459 461 502 477 477 477 477 477 477 477 477	. 595 . 612 . 735 . 698 . 769 . 723 . 720 . 740 . 891 . 850 . 891 . 895 . 612 . 816 . 613 . 872 . 816 . 613 . 614	630 681 681 680 721 840 702 843 843 843 843 843 849 780 673 798 670 670 670 670 670 670 670 670 670 670	SW NW NW NW NW SW SE SE SW N W.NW W.NW W.NW SW SW SW SW SW SW SW SW SW SW SW SW SW SW SW SW SW	W	S. SW. NW N SE W.NW. N S. SW N. NW.	11. 5	2.5 8.3 4.0 8.7 6.0 4.5 10.0	2.8 12.0 2.0 3.8 1.0 2.0 2.5 0.5 1.5 1.5	6.00 7.00 7.00 7.00 7.00 7.00 7.00 7.00	N. 41 W. N. 78 E W. S. 32 W. N. 83 W. N. 83 W. N. 65 E. S. 42 W. S. 75 W. North . S. 39 W. N. 45 W. N. 45 W. N. 45 W. N. 45 W. N. 56 W. N. 56 W. N. 56 W. N. 57 W. N. 58 W. N. 59 W. N. 59 W. N. 50 W. N.	10 7 3 8 2 2 3 8 10 10 10 9 10 0 0 0 5 8 4 4 6 3 3 3 9 9 2 6 6 6 4 9 10 10	4 6 10 6 5 6 9 10 6 9 10 10 9 4 4 9 9 3 3 3 9 9 5 4 4 9 9 7 7 5 8 8 7 8 5 4 10 10	0 4 10 4 5 7 10 10 10 10 10 10 5 7 7 8 3 10 10 10 10 10 10 10 10 10 10 10 10 10		. 660 . 550 . 07 . 22 . 24

	Barome	ter reduc	ed to tem	perature		TE	MPE	RATU	re—	-FAH	RENHE	IT.			VAP	OR.	
Date.	of 32°	Fabr., ex	pressed in ad decima	n United	p	t bull oint pora	of	P	bul eratu	re c	tem-	m.	'n.	Elast inc ma	bes :		
	7 & III.	2 p. m.	9 p. m.	Mean.	7 a. m.	2 p. II.	9 p. m.	7 a. m.	2 р. т.	9 р. ш.	Mean.	Maximum	Minimur	7 R. TD.	2 p. m.	9 p. m.	1
1866. ept. 1	29. 170 29. 169	29. 119 29. 260	29. 136 29. 366	29. 142 29. 265	67 68	0 70 62	66 61	69 71	83 79	o 09 66	° 73.7 72.0	° 82 79	66 65	. 635 . 644	. 558 . 329		
3 4 5 6	29. 450 29. 425 29. 365 29. 511	29. 427 29. 243 29. 399 29. 461	29. 446 29. 207 29. 498 29. 440	29. 441 29. 292 29. 421 29. 471	60 59 57 51	62 65 57 62	56 66 54 56	63 61 61 55	71 70 69 76	68 68 68 68	64. 7 66. 3 63. 3 64. 7	72 73 70 76	58 52 58 47	. 478 . 473 . 412 . 321	. 436 . 550	. 33 . 61: . 33	
7 8 9 10	29. 366 29. 382 29. 699 29. 602	29. 264 29. 418 29. 662 29. 484	29, 220 29, 519 29, 615 29, 331	29. 283 29. 440 29. 659 29. 472	55 53 47 54	55 56 59 61	55 52 52 59	57 56 49 58	59 66 65 68	57 55 55 61	57. 7 59. 0 56. 3 62. 3	66 70 68 71	55 54 45 46	. 407 . 363 . 297		. 407 . 349 . 349	9
11 12 13 14	29. 127 29. 090 29. 376 29. 343	29. 013 29. 062 29. 408 29. 328	29, 001 29, 180 29, 389 29, 538	29. 047 29. 111 29. 391 29. 403	63 59 49 50	68 59 55 52	58 51 50 41	65 62 53 52	70 72 62 63	63 57 54 46	66. 0 63. 7 56. 3 53. 7	71 71 65 65	60 56 49 45	. 549 . 460 . 295	. 65e . 340 . 340 . 243	. 410 . 290 . 300	6. 5.
15 16 17	29. 756 29. 539 29. 445	29. 729 29. 446 29. 443	29. 655 29. 272 29. 405	29. 713 29. 419 29. 431 29. 340	38 52 50	45 55 53	45 65 49	40 54 52	53 57 57	49 66 51	47. 3 59. 0 53. 3	55 66 69	34 48 43	. 203 . 362 . 334	. 194 . 407 . 350	. 24 . 60 . 32	1
18 19 20 21	29. 379 29. 343 29. 359 29. 442	29. 337 29. 415 29. 204 29. 500	29, 305 29, 437 29, 182 29, 522	29. 398 29. 248 29. 488	45 46 47 41	46 49 45 46	47 45 47 44	48 48 49 44	49 54 48 50	49 48 48 46	48. 7 50. 0 48. 3 46. 7	54 54 50 52	45 46 45 44	. 284 . 297 . 218	. 271 . 292 . 360 . 258	. 310 . 26	0
22 23 24 25	29. 614 29. 542 29. 416 29. 469	29. 598 29. 507 29. 401 29. 422	29. 555 29. 463 29. 419 29. 498	29. 589 29. 504 29. 412 29. 463	37 45 54 54	46 47 58 51	45 52 59 45	38 48 58 55	53 61 61 54	48 54 61 46	46.3 54.3 60.0 51.7	55 61 59 61	34 45 50 44	. 260 . 365 . 404	. 219 . 125 . 443 . 335	. 36 . 47 . 28	2
26 27 28 29	29. 626 29. 636 29. 695 29. 665	29. 596 29. 619 29. 628 29. 625	29. 545 29. 597 29. 609 29. 600	29. 589 29. 617 29. 644 29. 630	43 47 45 49	52 60 61 58	48 52 51 52	44 50 46 50	69 71 67	51 54 53 53	52. 3 57. 7 56. 7 56. 7	65 71 73 68	40 44 42 43	. 286	. 256 . 398 . 403 . 363	. 36	2
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3 4 5	29. 534 29. 664 29. 890	29. 487 29. 700 29. 853	29. 492 29. 778 29. 850	29. 504 29. 714 29. 864	41 43 43	48 47 44	42 38 41	44 45 46	54 53 54	44 40 45	47.3 46.0 48.3	60 53 56	40 39 34	. 218 . 251 . 238	. 256 . 244 . 157	. 24 . 20 . 20	1 3 5
6 7 8 9	29. 815 29. 704 29. 544 29. 366	29, 744 29, 641 29, 463 29, 328	29. 681 29. 564 29. 368 29. 244	29. 747 29. 636 29. 458 29. 313	36 50 55 50	55 63 61 58	57 58 54	37 56 56 56	63 74 74 65	52 60 61 56	50. 7 63. 3 63. 7 59. 0	66 75 74 64	55	. 282 . 420 . 282	. 363 . 3 89	443	6 3 1
10 11 12 13	29. 284 29. 465 29. 451 29. 539	29. 365 29. 463 29. 434 29. 569	29. 412 29. 416 29. 411 29. 597	29. 354 29. 448 29. 432 29. 568	55 54 53 48	57 50 55 54	55 54 52 51	56 55 55 49	58 62 58	57 57 55 55	58. 0 56. 7 57. 3 54. 0	60 59 62 60	53 59 53 45	. 322	. 255 . 340 . 363	.37	9
14 15 16 17	29, 716 29, 900 29, 979 29, 640	29, 730 29, 875 29, 807 29, 532	29. 789 29. 872 29. 699 29. 505	29. 745 29. 882 29. 828 29. 559	45 44 41 40	53 52 57 57	47 45 49 50	47 45 42 42	61 62 70 71	49 48 52 53	52.3 51.7 54.7 55.3	60 62 71 72	46 41 40 40	. 273 . 275 . 244 . 221	. 256 . 293 . 280	. 36 . 30 . 32	8
18 19 20 21	29, 525 29, 414 29, 541 29, 304	29. 434 29. 434 29. 485 29. 174	29. 374 29. 464 29. 402 29. 014	29. 444 29. 437 29. 476 29. 164	51 56 58	58 57 61 59	53 56 59 57	57 58 64	72 65 70 66	63 60 66 65	59. 7 60. 7 64. 7 65. 0	73 67 71 69	40 55 56 62		. 416 . 407	. 394 . 407 . 354	F
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26 27 28 29	29. 304 29. 557 29. 472 29. 299	29, 284 29, 608 29, 280 29, 254	29, 397 29, 607 29, 238 29, 221	29, 328 29, 591 29, 330 29, 258	38 34 43 39	40 40 49 40	36 37 31 39	40 36 46 42	42 44 57 44	39 38 38 42	40. 3 39. 3 47. 0 42. 7	46 44 59 56	38 34 38 40	170	. 221 . 195 . 242 . 195 . 151	907	7
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the northern and northwestern lakes at Detroit, Michigan.

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the northern and northwestern lakes at Detroit, Michigan.

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	7 a. II.	2 p. m.	9 p. m.	Mean.	7 a. m.	2 p. m.	9 р. ш.	7 a. m.	2 p. m.	9 p. m.	Mean.	Meximum	Minimum	7 A. III.	D. III.	9 p. m.	Monn.
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the northern and northwestern lakes at Monroe City, Michigan.

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7 . B. B.	2 p. m.	9 p. m.	Меап.	7 g	2 p. m.	9 p. m.	7 & D.	2 p. m.	9 p. m.	Resultar in miles	Resultar	7 a. m.	2 p. m.	9 p. m.	Amount U. S. mals.	Amount mow, and d
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	7 в. ш.	2 p. m.	9 p. ii	Mean.	7 a. m.	2 р. ш.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Mean.	Maximum	Minimur	7 a. m.	2 p. m.	9 p. m.
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the northern and northwestern lakes at Monroe City, Michigan.

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7 a. m.	. Q Q.	9 P. II.	Mean.	7 p. m.	2 p. m.	9 p. m.	7 a. m.	2 p. H.	9 р. ш.	Resultant volin in miles, per	Resultan	7 8. 13.	8 p. m.	9 p. m.	Amount U. 8.	Amount mow,
. 854 . 860 . 680 . 776 . 776 . 733 . 814 . 822 . 846 . 831 . 829 . 841 . 831 . 829 . 841 . 831 . 828 . 840 . 830 . 733 . 812 . 840 . 836 . 767 . 777 . 519 . 819	. 497 756 . 594 . 653 . 653 . 653 . 653 . 653 . 653 . 683 . 683 . 684 . 684 . 684 . 684 . 685 . 685 . 685 . 685 . 685 . 685 . 685	1. 000 806 831 899 882 883 863 869 860 860 869 869 802 858 869 802 858 869 878 869 878 869 878 878 883 878 878 878 878 878 878 878	. 784 837 678 768 7769 676 879 7754 791 796 7751 7744 768 783 774 811 861 867 783 779 849 783 779 849 773 773 772	S. S. S. S. S. S. S. S. S. S. S. S. S. S	8 W E 8W NE SW NE NE NE NE NE NE NE SW SW SW	S. W. NW SE NW NE NE NE NE NE NE NE NE NE NE NE NE NE	12 12 2 2 25 24 4 4 12 4 25 12 25 25 25 25 25 25 25 25 25 25 25 25 25	2 4 2 12 12 12 25 12 12 12 12 12 12 12 12 12 12 12 12 12	22 4 4 2 2 4 4 2 2 4 4 4 2 2 4 4 4 2 2 4 4 4 2 2 4 4 4 2 2 4 4 4 2 2 4 4 4 2 2 4 4 4 2 2 4 4 2 2 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 2 4 4 4 4 2 4 4 4 4 2 4	5.3 6.23 7.10.7 6.73 6.3 6.3 6.3 7.16 6.0 12.2 12.3 6.3 16.3 16.3 16.3 16.3 16.3 16.3 16.	West N. 87 W. North South N. 8 E. N. 45 E North N. 45 E N. 43 W. N. 79 E.	26 55 31 10 10 22 51 10 10 10 10 10 10 10 10 10 10 10 10 10	5 3 2 10 9 6 10 9 7 7 7 2 4 4 2 5 5 5 10 10 10 10 15 2 0 10 10 11 3 5 5 7	7 2 1 1 2 0 0 3 10 11 2 10 5 2 0 0 10 10 10 10 2 0 0 0 10 10 10 2 0 0 0 5 5 4. 4		.0\$.19 .45 .66
. 636 . 900 . 890 . 802 . 772 . 918 . 738	. 663 . 540 . 464 . 659 . 618 . 697 . 776 . 633 . 690 . 350 . 776 . 541 . 641	. 914 841 853 843 750 819 831 761 874 867 874 867 878 878 879 879 879 891 891 903 816 898 903 816 898 903 816 898 903 816 898 903 817 898 903 818 818 818 818 818 818 818 818 818 81	.785 .710 .775 .675 .705 .707 .774 .803 .781 .803 .747 .792 .708 .747 .708 .735 .735 .733 .733 .733 .733 .733 .733	NE. NE. W. NW. NE. SE. SE. NW. NW. NE. SE. SE. NW. NW. NW. NW. NW. NW. NW. NW. NW. NW	W. SE. SE. SE. SE. SE. SE. SE. SE. SE. SE	ENES NW NW NW NW NW WW	82442244223123482342343356335821432 2525	4 35 12 4 25 5 4 12 25 2 2 4 4 4 4 4 5 5 4 5 5 2 2 2 12 12 12 12 12 12 12 12 12 12 12	25 45 13 25 25 25	9. 7 19. 0 3. 0 12. 7 11. 7 26. 7	North S. 81 E North S. 73 E S. 73 E S. 73 E S. 73 E S. 74 W N. 75 E S. 145 W N. 45 E S. 45 W S. 45 W S. 45 W N. 45 W N. 45 W N. 45 W N. 19 E S. 40 W N. 19 E S. 40 W N. 19 E S. 40 W N. 19 E S. 40 W N. 19 E S. 40 W N. 19 E S. 10 W N. 19 E S. 10 W N. 19 E S. 10 W N. 19 E S. 10 W N. 19 E S. 10 W N. 19 E S. 10 W N. 19 E S. 10 W N. 19 E S. 10 W N. 19 E S. 10 W N. 10 W	3 1 3 4 2 0 7 0 7 0 10 10 9 9 0 0 0 7 8 1 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	7 4 0 4 3 0 0 5 4 7 5 5 10 0 0 1 2 0 0 9 3 10 0 10 7 6	0 10 0 13 0 0 10 8 10 3 3 7 5 10 10 10 10 10 10 10 10 10 10 10 10 10		

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26 27 28 29 30	29. 478 29. 364 29. 099 29. 150 29. 268	29. 412 29. 263 29. 044 29. 152 29. 228	29. 372 29. 207 29. 143 29. 157 29. 508	29. 421 29. 278 29. 095 29. 153 29. 335	28 32 47 33 27	40 41 50 35 26	42 47 38 32 28	29 35 50 35 29	46 52 55 40 31	45 49 40 33 31	40. 0 45. 3 48. 3 36. 7 30. 3	47 52 56 40 34	26 33 40 33 27	. 142 . 283 . 162 . 124	. 169 . 113 . 295 . 139 . 083	. 297 . 203 . 142 . 119	. 10 . 10
Means	29.416	29, 365	29. 398	29. 393		<u> </u>	<u></u>	35. 1	48. 3	39. 2	40. 9			. 170	. 181	. 197	
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the northern and northwestern lakes at Monroe City, Michigan.

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7 P. B.	9 7 8	Mean.	7 a. m.	2 p. m.	9 p. m.	7 P. H.	2 p. m.	9 p. ti	Resultant vel	Resultan	7 a. m.	2 p. m.	9 p. m.	Amount U. S. mala	Amount snow, and de
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the northern and northwestern lakes at Cleveland, Ohio.

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11 19	29. 314 29. 494	29. 314 29. 271	29. 326 29. 301	29. 318 29. 355	55 55	58 59	55 52	57 57	62 62	58 59 55	59. 3 58. 0	63 63	54 52	. 407 . 407	. 429 . 460	. 349	
13	29. 402	29. 461	29, 494	29. 452	53	55	54	54	58	57	56.3	58	52	. 389	. 393	. 378	.367
14 15	29. 589 29. 770	29. 594 29. 768	29. 670 29. 774	29. 618 29. 771	50 41	54 58	49 50	55 43	59 62	51 52	55. 0 52. 3	69	51 42	. 349 . 231		. 321	
16	29. 737	29. 722	29, 627	29. 695	45	62	52	47	65	54	55.3	66	45	. 273	. 516	. 362	24
17 18	29. 550 29. 480	29. 499 29. 426	29. 461 29. 398	29. 503 29. 435	43 45	63 61	52 55	45	67 69	55 59	55. 7 58. 3	67 71	45 45	. 251 . 273	. 592 . 430	. 349 . 390	.361
19	29. 397	29. 414	29. 423	29.411	51	62	62	54	71	65	63, 3	71	52	. 335	. 436	. 516	. 429
20 21	29. 509 29. 306	29, 458 29, 191	29, 413 29, 192	29. 460 29. 230	59 59	65 64	61 60	62	75 73	65 60	67.3 65.0	75	60 60	. 460 . 460	. 483 . 476	. 463 . 518	
22	29. 111	29. 219	29. 260	29. 197	54	52	49	57	62	52	57. 0	68	52	. 378	. 256	. 308	, 314
23 24	29. 238 29. 358	29. 207 29. 322	29, 291 29, 464	29. 245 29. 381	41 35	45 40	39 38	45 39	54 43	41 40	46.7	50 51	34	. 905 . 152	. 208 . 181	. 203 . 203	. 199 168
25 26	29. 546	29. 479	29. 423	29. 483	37 36	43 44	40	40	46	42 43	42.7 42.7	47 47	34	. 181 . 173	. 93%	. 22 1	21 3
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26 29	29. 408 29. 159	29, 275	29. 240 29. 144	29. 308 29. 155	43 50	60 47	56 44	46 52	63 50	60 46	56. 3 49. 3	64	42	. 23è . 334	. 478	396	. 371
30	29. 164	29. 161 29. 154	29. 238	29. 185	41	40	35	43	43	37	41.3	62 55	37	. 931	. 908	. 178	, guri
31	29, 434	29. 474	29. 504	29. 471	36	38	35	39	41	36	39.3	48	35	. 173	. 190	. 165	. 176
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the northern and northwestern lakes at Cleveland, Ohio.

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84	Hum turatio	idity. n == 1,0	100.		Direction om when		170	ocity iles, our.		tant velocity, leg, per bour.	Resultant direction.	(10: tire	sky.) = sky ly or cast.)	er-	of evaporation, in inches and deci-	t of rain or in U. S. ecimals.
7 a. m.	i d	9 p. m.	Mean.	7 a. m.	2 p. B.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Resultant in miles,	Rosultan	7 a. m.	2 p. m.	9 p. m.	Amount U. 8. mals.	Amount mow,
. 854 . 858 . 831 . 502 . 802 . 823 . 824 . 805 . 824 . 805 . 805 . 805 . 805 . 806	. 681 . 776 . 848 . 661 . 754 . 807 . 793 . 796 . 801 . 670 . 613 . 592 . 804 . 802 . 804 . 780 . 813 . 780 . 831 . 780 . 843 . 780 . 843 . 780	. 819. . 751 . 790 . 810 . 894 . 884 . 825 . 836 . 836 . 836 . 816 . 819 . 812 . 825 . 833 . 781 . 809 . 890 . 897 . 856 . 816 . 817 . 818 . 819	. 782 . 795 . 823 . 659 . 828 . 797 . 847 . 833 . 832 . 832 . 828 . 828 . 827 . 747 . 800 . 788 . 827 . 783 . 812 . 826 . 761 . 763 . 830 . 761 . 840 . 763 . 840 . 763 . 840 . 763 . 840 . 763 . 840 . 763 . 840 . 763	SW.SE.SW.NW.S.NW.SS.NW.SS.NW.NE.NS.SS.NW.S	S W.NW N. W. by N N. NW W. NW W. W. W. W. W. W. W. W. W. W. W. W. W. W	S	194 2 2 2 4 4 2 2 2 4 2 4 2 3 2 2 4 4 4 2 2 2 2	12 4 2 12 2 12 2 12 12 12 12 12 12 12 12 12	4 2 2 2 2 2 4 4 2 5 3 5 2 2 2 2 2 4 4 2 2 3 5 2 2 2 2	9.33 5.57 11.57 15.07 11.77 11.77 11.77 12.33 11.77 11.77 13.33 14.33 11.10 11.17 11.17	S. 62 W S. 45 E S. 45 E N. 73 W E N. 45 E N. 63 W West West West West West N. 34 W N. 34 W N. 34 W N. 34 E N. 65 E S. 82 W N. 145 E S. 82 W N. 145 E S. 82 W N. 145 E S. 82 W N. 145 E S. 82 W N. 145 E S. 83 E S. 84 E N. 145 E S. 85 E S. 85 E S. 85 E S. 85 E S. 85 E S. 85 E S. 85 E S. 85 E S. 85 E S. 86	4 5 10 0 0 10 3 10 10 10 10 10 10 10 10 10 10 10 10 10	56 10 53 00 10 36 810 55 10 10 10 10 10 10 10 10 10 10 10 10 10	10 0 0 0 0 10 10 10 10 10 10 10 10 10 10		.02 .01 .03 .07 .1, 51 .33 .36 .11 .33 .10 .33
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Means	29. 378	29. 348	29. 366	29. 364				39. 5	46. 9	41.8	42.7			. 194	. 231	. 216	214
Dec. 1 2 3 4 4 5 6 7 8 9 100 11 122 13 14 15 16 17 18 19 20 21 22 24 25 26 27 28 29 30 31	29. 525 29. 526 29. 367 29. 367 29. 369 29. 536 29. 537 29. 464 29. 195 29. 261 29. 261 29. 723 29. 261 29. 763 29. 763 29. 763 29. 364 29. 763 29. 243 29. 243 29. 243 29. 243 29. 243 29. 243 29. 243 29. 413 29. 567 29. 429 29. 42	29, 538 29, 469 29, 179 39, 111 29, 548 29, 362 28, 762 29, 146 29, 362 29, 527 29, 740 29, 474 29, 474 29, 474 29, 590 29, 182 28, 592 29, 590 29, 182 29, 500 29, 182 29, 500 29, 182 29, 500 29, 182 29, 500 29, 182 29, 500 29, 182 29, 500 29, 182 29, 500 29, 182 29, 500 20, 500 20, 50	\$9, 519 \$9, 464 \$9, 181 \$9, 310 \$9, 574 \$9, 411 \$9, 210 \$29, 562 \$9, 356 \$29, 562 \$9, 463 \$9,	29. 527 29. 494 29. 942 29. 135 29. 553 29. 553 29. 416 29. 391 29. 510 29. 510 29. 510 29. 491 29. 528 29. 52	28 23 30 38 39 42 52 23 11 12 15 19 4 27 27 31 28 13 29 29 10 11 13 5 6	97 36 41 44 42 47 45 46 16 11 30 20 19 31 30 24 28 21 20 19 31 20 19 31 20 19 31 20 19 31 20 19 31 20 19 31 20 19 31 31 31 31 31 31 31 31 31 31 31 31 31	98 31 40 38 38 38 43 46 33 17 12 18 15 19 30 24 27 30 4 23 33 35 28 15 28 15 29 19 20 19 19 19 19 19 19 19 19 19 19 19 19 19	29 24 33 40 34 41 45 55 55 12 13 11 16 20 59 28 33 30 14 3 31 20 20 11 12 20 11 12 12 13 14 14 14 14 14 14 14 14 14 14 14 14 14	30 42 46 48 44 45 52 50 18 23 14 22 21 20 33 34 21 21 21 21 21 21 21 21 21 21 21 21 21	30 33 34 40 41 46 49 36 31 31 32 31 32 33 33 33 33 33 33 33 33 33 33 33 33	99. 7 33. 0 40. 3 42. 7 40. 0 40. 0 40. 0 94. 3 11. 7 13. 0 11. 7 13. 0 31. 3 15. 7 30. 3 15. 7 30. 3 15. 7 15. 0	34 24 74 49 44 75 37 5 39 5 5 39 5 5 39 5 5 39 5 5 39 5 5 39 5 5 39 5 5 5 5	29 29 33 39 34 31 45 38 19 12 13 16 5 22 26 23 29 5 12 11 10 11 10 11 11 10 11 11 11 11 11 11	1492 1192 1293 1293 1293 100 061 074 194 041 1194 1195 1195 1195 1195 1195 1195 11	. 177 . 192 . 236 . 246 . 257 . 234 . 106 . 060 . 060 . 061 . 139 . 060 . 143 . 139 . 060	151 921 192 277 145 077 085 077 092 144 100 130 152 194 176 176 074 074 074 074	189 244 1 255 1 25

the northern and northwestern lakes at Cleveland, Ohio.

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. 775	. 694	. 800	. 757					<u></u>	<u></u>	3.4	8. 62 W.	7.7	7.3	6.8		2. 70
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. 3	29. 411 29. 098	29. 284 29. 085	29, 233 29, 138	29. 309 29. 107	64	70 68	63 65	70 64	80 69	69	73. 0 67. 0	82 70	63	. 516 . 529	. 59 6 . 671		
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10	29, 620	29. 554	29. 518	29, 564	55	67	62	59	80	67	68.7	80	50	. 380	. 487	. 429	. 45
11 12	29. 591 29. 523	29. 545 29. 442	29. 463 29. 368	29. 533 29. 444	67 65	70	67 67	70 69	76 77	69 71	71. 7 72. 3	78	58 66	. 622 . 564	,	. 635 . 608	
13 14	29. 283 29. 325	29. 312 29. 324	29, 245 29, 320	29. 280 29. 323	71 70	73 76	70 72	73	77 80	72 74	74. 0 75. 3	78 81	69 70	. 731 . 706		. 706 . 757	
15	29. 320	29. 382	29. 360	29. 354	72	74	71	74	81	74	76.3	82	72	. 757	. 745	. 718	.74
16 17	29, 438 29, 438	29. 437 29. 352	29. 385 29. 311	29. 420 29. 367	72 73	76	73 68	75 75	83	76 70	78.0 75.3	83 82	72 68	. 744 . 784		. 771 . 658	
18 19	29. 265 29. 420	29. 193 29. 430	29. 252 29. 426	29. 237 29. 425	70 57	72 65	61 62	72 59	73 74	62 64	69.0 65.7	75	62	. 706	. 771	. 593	66
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23	29. 182 29. 364	29. 228	29. 256	29, 222	60	66	65	62	75	68	68.3	74	64	. 491	. 519	. 577	. 52
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26 27	29. 421 . 29. 368	29. 384 29. 324	29. 375 29. 263	29. 393 29. 318	64 69	74	69 70	66 71	80	72 73	72.7 74.7	81 80	66 66	. 569 . 682	. 758 717	. 668	
28	29. 271	29. 187	29. 168	29, 209	67	74	68	70	83	71	74.7	84	63	. 622	. 718	. 644	. 66
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31	29. 334	29. 168	29. 256	29. 253	57	60	60	61	75	68	68.0	79	53	. 412		.411	
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4 5	29. 161 29. 367	29. 234 . 29. 346	29. 294 29. 349	29. 230 29. 354	58 57	59 58	59 59	59 60	61 68	61 63	60. 3 63. 7	68 69	58 54	. 469 . 426	. 473 350	. 473 . 447	
6	29. 399	29. 383	29. 379	29. 387	59	61	55	63	73	63	66. 3	75	54	. 447	. 376	. 397	.38
7 8	29. 427 29. 372	29. 375 29. 193	29. 361 29. 093	29. 388 29. 219	56 57	65 63	62 58	60 58	72 73	66 62	66, 0 64, 3	74	54 53		. 442		. 44
9 10	29. 059 29. 495	29. 188 29. 395	29. 234 29. 389	29. 160 29. 426	55 55	65 63	56 59	56 56 57	71 72	61 63	62. 7 64. 0	71	56 56	. 420 - 407	. 537 . 455	. 383 . 447	. 447
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12 13	29. 489 29. 193	29. 145	29. 110	29. 388 29. 149	66	73	70	63 69	76 76	71	68. 3 72. 0	78 77	56 65	. 599	. 771	. 720	. 697
14 15	29. 204 29 414	29. 220 29. 471	29, 233 29, 525	29, 219 29, 470	64 61	69 64	67 54	65 65	74 70	69 57	69. 3 64. 0	76 71	63 57	. 583 483	. 641 . 516	. 635 . 379	458
16	29. 635	29. 606	29, 480	29, 574	49	56	48	53	64	55	57. 3	65	51	. 295	. 343	. 243	24
17 18	29. 570 29. 432	29. 521 29. 363	29. 431 29. 244	29. 507 29. 346	50 57	60 67	59 63	55 69	70 73	66 66	63. 7 67. 7	70 75	46 61	. 3 99.	. 385 . 581	. 509	. 496
19 20	29. 161 29. 267	29. 138 29. 276	99, 138 29, 229	29, 146 29, 255	61 53	58 59	58 53	-63 56	60 68	6 0 58	61. 0 60. 7	69 70	59 53	. 510.	. 456 . 360	. 456	. 474
21	29. 192	29, 163	29. 131	29. 162	55	58	57	58	66	63	62.8	70	55	. 393	376	326	.385
22 23	29. 119 29. 149	29, 178 29, 206	29, 210 29, 136	29. 169 29. 164	53 50	54 56	53 50	56 59	64 64	55 53	58. 3 56. 3	65 65	55 51	. 334	985 343	321	. 333
24 25	29. 241 29. 373	29. 254 29. 356	29, 275 29, 310	29. 257 29. 346	48	55 55	49 50	51 54	61	53 55	55. 0 57. 7 69. 0	62 65	49 48	. 296	354. 314.	295	315
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26 27 28 29	29. 264 29. 274	29. 265 29. 235	29, 249 29, 204	29, 259 29, 238	56 56	62 67	61 63	61 61	72 72	63 65	65. 3 66. 0	73	62 57	. 383	429. 595.	549	. 509
29 30	29. 267 29. 302	29. 271 29. 423	29, 279 29, 224	29. 266	50 54 58 56 59 57	61 63	60 61 65	54 56 61 61 60 58 63	70 69	******	64.3 64.0	68 73 72 71 72 76	61 54	. 487	416. 495.	478	. 460
31	29. 299	29, 261	29, 246	29, 316 29, 269	61	63	65	63	70	88	67.0	76	63	510	482	577.	523
Means	29. 303	29, 295	29. 267	29. 968	-			50 3		80.6	63.7			. 122	467	440	40

the northern and northwestern lakes at Buffalo, New York.

	VAI	POR.				w	DID.					clo	ount udin	0 8 4,	d deci-	melted
Se	Hum	idity. n == 1,0)00,		Direction om when		m	ocity iles, our.		itant velocity, illes, per hour.	Resultast direction.	(10:	sky. = sk ly o cast.	aer- À en-	of evaporatio	nt of rain or w, in U. S. decimals.
4 4	29 9.09	9 P. B	Mean.	7 P. B.	20 H	9 9 E	7 8. 18.	2 p. H.	9 p. m.	Resultan in miles,	Resultas	7 . 10.	2 p. II.	9 P. II.	Amount U. S. mala.	Amount snow,
. 670 - 7104 - 986 - 983 - 854 - 851 - 761 - 761 - 902 - 903 - 903 - 903 - 858 - 904 - 891	. 562 644 585 947 734 686 779 689 686 777 689 817 704 711 826 586 774 704 711 826 685 768 778 738 738 738 738 738 738 738 738 73	. 735 . 700 . 843 . 948 . 854 . 804 . 896 . 896 . 898	6693 6633 8933 8939 7622 819 7555 654 664 873 873 876 876 876 876 877 877 878 878 878 879 775 797 898 818 818 878 878 878 878 878 878 878 87	S. SW. S. SW. S. SW. SW. SW. SW. SW. SW.	8W 8W W.8W. W.	N. NE. SE W. SW. SW. SW. W. SW. W. SW. W. SW. W. SW. W. SW. W. SW. W. SW. W. SW. S	22 4 4 4 4 4 12 25 4 4 4 4 2 2 2 4 2 4 2 2 2 2 2 2 2 2	19 4 4 4 35 4 4 25 25 12 12 25 12 12 12 12 12 12 12 14 12 14 4 9 11 12 14 4 19 11 19 14 4 19 11	2 12 12 25 4 12 25 4 12 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	4.03 3.07 3.07 3.03 13.03 14.37 3.60 13.30 14.33 13.03 13.03 13.03 14.33 15.03 16.33 17.03 18.77	S. 68 W. S. 81 W. S. 81 W. S. 82 W. S. 83 W. S. 84 W. S. 68 W. S.	1 10 10 10 10 10 10 10 6 6 8 10 10 10 10 10 10 10 10 10 10 10 8 3 3 2 6 6 10 6 6	100 99 18 100 82 22 00 4 10 10 10 10 29 3 7 8 11 10 10 10 10 10 10 10 10 10 10 10 10	8 6 6 4 10 3 0 6 10 0 0 10 10 10 10 10 10 10 10 10 10 10		
. 850 - 868 - 665 - 703 - 939 - 776 - 765 - 875 - 874 - 763 - 776 - 846 - 763 - 718 - 861 - 718 - 809 - 861 - 738 - 861 - 738 - 878	. 692 852 740 629 962 962 511 545 708 552 708 644 704 704 705 716 860 764 705 716 860 764 705 716 860 860 860 860 860 860 860 860 860 86	.813 -854 .854 .718 .890 .892 .776 .568 .776 .727 .838 .949 .966 .812 .561 .636 .743 .800 .690 .798 .798 .798 .798	. 785 . 865 . 714 . 741 . 741 . 901 . 703 . 603 . 740 . 751 . 785 . 744 . 731 . 753 . 868 . 766 . 614 . 786 . 688 . 719 . 745 . 747 . 747 . 747 . 748	SW NW NW NW N N N N	8.8W. W.NW. W.NW. W.NW. W.NW. W.NW. W.NW. W.NW. W.SW. NE W.SW. W.SW. W.SW. W.SW. W.SW. W.SW. W.SW. W.SW. W.SW.	W.SW. W.SW. N.N. N.N. N.N. N.N. N.N. N.N	4 4 4 4 4 4 2 2 2 2 2 2 2 2 2 2 2 2 2 2	35 35 35 12 4 15 12 2 4 4 4 4 12 12 12 12 14 14 12 12 12 12 14 15 12 12 14 15 16 16 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	19 12 4 9 2 4 4 9 2 12 12 12 12 12 12 12 12 12 12 12 12 1	5.5 16.00 5.33 12.077 7.30 2.30 7.00 7.37 7.33 7.00 7.37 7.33 7.00 7.37 7.33 7.00 7.33 7.00 7.33 7.00 7.33 7.00 7.33 7.00 7.33 7.00 7.00	N. 76 W. N. 29 W. N. 50 W. N. 50 W. N. 50 W. N. 50 W. S. 73 E. S. 63 W. N. 63 W. N. 63 W. N. 64 W. N. 64 W. N. 65 W. N.	7.2 10 9 1 10 10 10 2 4 10 10 10 8 11 10 2 10 10 10 10 10 10 10 10 10 10 10 10 10	5.5 10 10 10 10 10 10 10 10 10 10	10 10 10 10 10 10 10 10 10 10 10 10 10 1		1. 88
. 713 . 940 . 937 . 886	. 756 . 570 . 700 . 656	. 890 . 831 . 783 . 843	. 787 . 780 . 967 . 796	SE N. NW. E.by N. NE	8. 8W. NW 8W NE	SE NE W.SW. NE	2 4 2 2	4 4 4	4 9 4 9	2.3	8. 29 E . N. 21 W. 8. 49 W.	10 10 10	10 9 10 6	1 0 10 10		4. 80

	Barome	ter reduc	ed to tem	perature		TE	MPE	RATU	RE—	-PAH	rephe	17.			VAP	OR.	
Date.	of 32°	Fahr., en inches an	pressed i	n United	P	t bul oint pora	of	1	y bu berati open	are o	r tem- of the	á		ine	rtielt ches :		
	7 t. III.	9. F.	9 p. m.	Keen.	7 B. II.	i d	9 p. m.	7 s. m.	2 p. m.	9 p. m.	Ken.	Maximum	Minimum	7 p. m.	2 p. m.	9 p. sj.	5
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4 5	29. 374 29. 151	29. 214 29. 245	29. 178 29. 314	29. 247	68	75 66	64 55	61 70	80 72	61	69. 7 67. 7	81 72	61 61	. 473 . 658	. 800 . 559	. 354	
6	29. 437	29. 428	29. 414	29. 426	57	61	58	60	71	61	64. 0	71	55	. 426	. 403	. 443	į. 4
7 8	29. 312 29. 143	29. 111 29. 294	29. 038 29. 367	29. 154 28. 268	55 57	58 60	59 54	58 58	59 65	60 56	59. 0 59. 7	63 65	54 57	. 39:1 . 452	. 469 . 451	. 467 . 391	
9	29. 497	29. 548	29. 522	29. 522	54	62	56	56	67	58	60. 3	68	52	. 391	. 489	. 422	ŗ,
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13 14	29, 240 29, 325	29. 286 29. 163	29. 307 29. 363	29. 278 29. 284	52 32	55 63	55 47	59 34	64 66	60 50	61.0 50.0	66 67	57 50	. 296 . 160	. 314 . 536	. 367 . 283	
15	29. 562	29. 650	29, 659	29. 624	40	48	43	43	55	44	47.3	56	41	. 208	. 243	. 264	ı.
16 1 7	29. 563 29. 279	29. 424 29. 137	29. 327 28. 330	29, 438 29, 249	47 62	69 59	54 52	49 63	65 60	56 55	56. 7 59. 3	66 66	45 55	. 297 . 542	. 516 487		
18	29. 301	29, 288	29. 243	29. 277	50	51	49	51	522	50	51.0	58	51	. 348	. 361	. 335	i.
19 20	29. 236 29. 343	29, 295 29, 206	29, 371 29, 041	29. 301 29. 197	47	50 52	49 54	48 49	53 53	51 45	50. 7 49. 0	54 56	50 49	. 310 . 2 97	. 3≥1 . 375	321 404	
21	29. 164	29. 280	29. 358	29. 267	49	51	46	50	55	48	51.0	68	49	. 335	. 321	. 284	ί.
22 23	29. 524 29. 630	29. 575 29. 577	29. 559 29. 521	29. 553 29. 576	41 38	48 51	42 45	42 40	53 60	44	46. 3 49. 0	57 61	39 38	. 244 . 203	. 269 . 255	. 241 273	
24	29. 534	29. 453	29. 406	29. 464	44	63	59	46	69	64	59. 7	70	44	. 262	. 223 . 495	433	
25 26	29. 424	29. 274	29, 298 29, 492	29. 332	61	60	51	65	61	52	59. 3	68	53	483	. 505	. 361	
20 27	29. 491 29. 518	29. 495 29. 545	29. 192	29. 493 29. 543	46 50	49 51	43 55	47 52	53 57	43 56	47. 7 55. 0	54 64	45 43	. 297 . 334	. 295 . 295	. 278 490	
28	29.668	29. 623	29. 559	29. 617	50	52	61	59	69	63	61.3	71	45	. 334	. 163	. 510	
29 30	29. 621 29. 546	29. 650 29. 598	29. 514 29. 492	29. 595 29. 545	48 46	67 61	56 56	51 49	74 67	61 58	62. 0 58. 0	73 69	47 46	. 296 . 271	. 568 . 457	. 443 . 422	
feans	29. 385	29. 350	29. 340	29. 358				54. 4	64. 1	57. 1	58. 5			. 382	. 457	. 411	١.
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4	29. 404 29. 562	29. 411 29. 643	29. 437 29. 702	29. 417 29. 636	43 41	45 43	39 35	46 43	51 50	41 39	46.0 44.0	64 51	43	. 230 . 231	. 23 0 . 186		
5	29. 911	29.820	29. 787	29.839	31	46	37	32	55	41	42.7	55	31	. 162	. 199	. 168	٠.
6	29. 824 29. 610	29. 724 29. 546	29. 618 29. 494	29. 722 29. 550	38 57	51 62	45 59	42 59	61 64	48 61	53. 7 61. 3	61 66	36 47	l. 177 l. 439	. 242 . 589	. 960 473	
8	29.47 0	29. 441	29, 375	29. 429	58	62	56	59	66	57	60.7	67	59	. 469	. 502	436	έ,
9 10	29. 407 29. 362	28.318 29.358	29.317 29.382	29. 347 29. 367	54 56	67 60	58 54	56 58	75 67	60 58	63. 7 61. 0	77 67	53 59	. 391 . 422	. 554 . 495		
11	29. 433	29. 394	29, 350	29. 392	52	59	53	54	65	55	58.0	65	54	362	. 490	376	Ĺ
12 13	29. 391 29. 456	29. 332 29. 457	29. 343 29. 464	29. 355 29. 459	46	51 55	48 44	47	62	50 46	53. 0 51. 7	83 60	48	. 997	229 367 .	. 309 L 983	
14	29. 622	29. 645	29. 671	29. 646	42	49	43	43	57	47	49.0	57	43	. 254	. 912	. 225	ŀ
15 16	29. 792 29. 760	29. 774 29. 689	29. 758 29. 682	29.775 29.710	41 39	53 56	47 56	43	61	52 58	52.0 53.7	63 65	42 42	. 23 1 . 22 5	. 997 . 356	. 257 . 422	
17	29. 523	29, 429	29, 412	29. 455	48	58	-56	50	63	58	57, 0	63	51	. 379	. 416		L
18 19	29, 495 29, 422	29. 448 29. 424	29, 428 29, 432	29. 457 29. 426	52 53	65 60	49 57	53 58	71 65	52	58. 7 61. 7	71 71	51 50	. 375 . 336	. 537 . 451	. 309	
20	29. 530	29. 428	29. 415	29. 458	54	61	54	55	67	57	59.7	69	56	. 404	. 457	370	ί.
21 22	29. 379 28. 958	29. 291 29. 141	29, 150 29, 169	29. 273 29. 0 8 9	56 59	59 51	58 48	63 64	70 59	67 56	66. 7 59. 7	72 79	58 57	. 356 . 433	. 917	. 363 . 230	
23	29. 169	29. 137	29. 175	29. 160	44	43	38	48	49	42	46.3	56	42	. 236	190	. 177	, :
94 25	29, 226 29, 545	29. 297	29. 405 29. 535	29. 309	32 33	38	35 31	34	41	38	37.7	44	34	. 155	. 190		
26	29. 423	29. 540 29. 352	29. 214	29. 540 29. 330	322	36 41	44	34 35	40 43	34 46	36.0 41.3	43 46	36 37	. 175 . 155	. 160 . 251	. 139 . 241	Ŀ
27	29. 430	29. 494	29. 560	29. 195	38	42	38	40	46	40	42.0	48	42	. 903	915	963 907	
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30 31	29. 087 29. 308	29. 064	29, 100	29. 084 29. 394	39 32	45 36	38	41 33	50 40	40 35	43. 7 36. 0	52 42	43	212	. 300 . 934 . 160	903	
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eans	29. 463	29. 431	29, 416	29, 437				47.7	58.8	50. 6	52.4			. 990	. 398	. 309	. :

the northern and northwestern lakes at Buffalo, New York.

	VAP	OR.				WI	fD.					clo	ount adine == cle	.	oration in and deci-	melted bes and
Sal	Hum turatio	idity. n == 1,0	100.		Direction.		in n	elocit niles, hour	per	it velocity, per hour,	Rosultant direction.	(10= tire	ky) = sky ly ov mut.)	en. Ter-	of evap	t of rain or in U.S. incl als.
7 6. 10.	9 p. 18	9 p. H.	Mean.	7 a. m.	2 P. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Resultant vel	Rornitan	7 a. m.	2 p. m.	9 p. m.	Amount U. S. male.	Amount mow, decima
- 846 - 898 - 943 - 898 - 898 - 892 - 816 - 937 - 873 - 840 - 593 - 750 - 853 - 944 - 593 - 753 - 853 - 925 - 853 - 925 - 853 - 925 - 853 - 925 - 853 - 925 - 853 - 925 - 853 - 925 - 853 - 925 - 853	. 794 856 856 762 718 731 740 896 838 561 561 836 940 953 972 798 972 798 973 974 973 974 974 975 974 975 974 975 975 975 975 975 975 975 975 975 975	. 610 . 891 . 898 . 898 . 659 . 895 . 940 . 872 . 476 . 880 . 918 . 872 . 805 . 837 . 859 . 937 . 859 . 937 . 859 . 937 . 859 . 937 . 859 . 937 . 859 . 937 . 859 . 850	793 882 877 786 726 898 847 829 796 839 743 854 896 961 906 720 757 885 818 869 961 964 869 964 869 964 869 964 869 964 869 964 869 964 869 964 869 964 869 964 869 869 869 869 869 869 869 869 869 869	NE. W S by W E NE S W N N N N W N N W W S W W N W S E W N N E W W W E L by S N E W N E L W N W W E L W N W E L W N W N W E L W N W N W E L W N W N W E L W N W N W E L W N W N W E L W N W N W E L W N W N W E L W N W N W E L W N W N W E L W N W N W E L W N W N W N W E L W N W N W N W E L W N W N W N W N W N W N W N W N W N W	N.NW. W.SW. S.SW. E.SE. SW. W.SW. W.SW. W.SW. W.SW. W.SW. NE. NE. NE. NE. NE. NE. NE. NE. NE. NE	SE.W. NE.W. NEW. SW. NE.	24 22 4 2 2 2 2 4 4 2 2 2 4 4 4 2 2 2 2	4 25 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	4 2 2 4 2 2 2 2 12 2 4 2 2 2 2 1 1 2 2 5 2 2 2 2 2 2 2 2 2 2 2	1.37 9.73 9.73 9.77 1.00 30.17 1.00 2.77 2.33 4.00 2.77 2.33 4.00 2.77 2.73 3.33 4.00 2.77 2.73 3.73 3.73 4.73 4.73 4.73 4.73 4.73 4	N. 23 W. N. 66 E. S. 49 W. S. 19 W. N. 19 W. N. 63 W. N. 63 W. N. 64 W. N. 65 W. N. 65 W. N. 45 E. N. 45 E. N. 45 E. N. 10 E. N. 10 E. N. 10 E. N. 11 E. S. 45 W. N. 12 E. N. 13 E. N. 14 E. N.	10 10 10 1 10 10 10 10 10 10 10 10 10 10	10 10 0 3 1 10 9 9 4 10 10 10 10 10 10 10 10 10 10 10 10 10	10 0 10 0 0 0 0 0 0 10 10 10 10 10 10 10		. 30 1. 30 . 20 . 08 . 76 . 02 . 10 . 18 . 32 . 52 . 66 . 76
. 859	. 739	. 849	. 815							2.3	8. 69 W.	6.6	8. 0	5. 5		6.94
. 927 . 765 . 765 . 765 . 833 . 896 . 661 . 878 . 972 . 876 . 877 . 923 . 916 . 831 . 916 . 831 . 916 . 931 . 916 . 931 . 916 . 931 . 916 . 931	. 666 658 590 590 445 452 669 669 669 706 689 731 690 723 731 690 731 690 731 690 731 690 731 731 731 731 731 731 731 731 731 731	. 8692 9433 8244 6536 6536 6536 6536 6536 6536 6536 65	. 826 . 789 . 787 . 661 . 665 . 630 . 683 . 687 . 797 . 761 . 802 . 802 . 802 . 802 . 803 . 818 . 811 . 812 . 476 . 812 . 682 . 683 . 687 . 750	NE	N.NW.W.SW.SE.NE.NE.W.SW.SW.SW.SW.SW.SW.SW.SW.SW.SW.SW.SW.S	NE	24 12 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	4 19 12 12 2 2 12 2 12 2 12 4 14 4 4 4 4 19 60 45 4 19 2 19 19 19 19 19 19 19 19 19 19 19 19 19	9 2 2 2 2 4 4 2 2 2 2 4 4 4 2 2 2 4 4 4 2 2 2 4 4 4 2 2 2 4 4 4 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 4 4 5 4 4 1 2 2 2 2 4 5 4 4 1 2 2 2 2 4 5 4 4 1 2 2 2 2 4 5 4 4 1 2 2 2 2 4 5 4 4 1 2 2 2 2 4 5 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	8.0 2.0	N. 61 E. 8. 65 W. 8. 65 E. 8. 65 W. 8. 65 W. 8. 65 W. 9. 65 W. 9. 65 W. 9. 65 W. 9. 66 W. 9. 66 W. 9. 66 W. 9. 66 W. 9. 66 W. 9. 66 W. 9. 67 W. 8. 68 W. 9.	10 0 10 10 10 10 10 10 11 10 11 10 10 10	10 24 4 4 20 10 11 9 9 10 7 11 0 0 2 9 9 10 10 10 10 10 10 10 10 10 10 10 10 10	0 10 0 0 0 0 10 10 10 10 10 10 10 10 10		

	Barome	ter reduc	ed to tem	perature		71	MPE	RATI	RE-	-FAH	rephi	IT.			VAF	OR.	
Date.	of 32°	Fahr., ex	pressed i and decim	n United	l p	t bul oint port	of	p	pen i	ire d	r tem-	ä	ď	Elas in m	ticity ches als.	, in I	Ľ. R. deci-
	7 P. B.	9. G	e F	Mean.	7 a. ii.	2 p. m.	9 p. ii.	7 a. m.	2 p. m.	9 p. m.	Mean.	Maximum	Minimum	7 a. m.	S P. II.	9 p. m.	Mean.
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the northern and northwestern lakes at Buffalo, New York.

	AVI	POR.				w	IND.					clo (0	ount udine	ess. Sar	atton, in	melted
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7.	8 p. m.	9 p. m.	Mean.	7 a. W.	2 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Resultan in miles,	Resultan	7 a. m.	2 p. m.	9 p. m.	Amount U. 8. mal	Amount smow, and des
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Fig. Fig.	OR.	VAP			EIT.	IRENH	-FAE	JRE-	RAT	EMPI	T		nereture	ed to tem	ter raduc	Ramme	
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the northern and northwestern lakes at Fort Niagara, New York.

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80	Hum turatio	idity. n = 1,0	X0.	Fr	Direction om when	i. 108.	n	ocity iles, our.		t velocity, per hour.	Resultant direction.	(10:	sky.) y en- ver-	of evaporation, inches and de	t of rain or r in U. S. inchesals.
7 a. m.	2 p. m.	9 p. m.	Mean.	7 a. m.	2 p. m.	9 p. ii.	7 a. m.	2 p. m.	9 p. m.	Resultant in miles, 1	Resultan	7 a. m.	2 р. ш.	9 р. ш.	Amount U. S. male,	
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REPORT OF THE SECRETARY OF WAR.

	Baroma	ter reduc	ad to tam	namatmaa		TE	MPEF	LATU	r e —	FAHI	RENHE	17.			VAPO	R.
Date.	of 32°	Fahr., es	xpressed	in U. S.	P	t buli oint o porat	of	P		ire o	tem-	si	ei ei		hes i	r, in U.S and dec
	7 P. II	2 p. m.	9 p. m.	Mean.	7 a. m.	2 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Mean.	Maximum	Minimum	7 a. m.	2 p. m.	9 р. п.
1866,	on			m ===	0	0	° ~	0	0	0	0	<u>.</u>	0			
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3 4	29. 769 29. 867	29. 784 29. 766	29. 796 29. 624	29. 783 29. 752	64 59	68 66	65 60	65 61	71 68	67 63	67. 7 64. 0	72 69	64 59		. 644 . 612	
5	29. 660 29. 853	29. 631 29. 847	29. 792 29. 837	29. 694 29. 846	66 55	67 70	62 65	68 57	70 73	65 67	67. 7 65. 7	70 73	65 55	. 612	. 692 . 693	. 516 . 5
7	29. 726	29. 628	29. 534	29, 629	57	58	56	59	60	58	59. 0	68	57	. 439	. 456	. 422 4
8 9	29. 617 29. 940	29. 676 30. 032	29. 792 30. 025	29. 695 29. 999	54 54	58 64	56 61	57 56	61	58 64	58. 7 62. 3	62 69	54 54	. 378 . 391	443 556	. 422 . 4 . 497 . 4
10	30. 030	29. 987	29. 867	29. 961	55	62	57	55	65	60	60.0	66 65	54	. 433	. 516	. 436.4
11 12	29. 739 29. 382	29. 536 29. 434	29. 426 29. 760	29. 567 29. 525	60 60	63 67	59 61	62 62	65 69	61 64	62, 7 65, 0	70	56 60	. 491	. 635	. 473 . 5 . 497 . 5
13 14	29. 647 29. 702	29. 726 29. 659	29. 760 29. 744	29. 711 29. 702	55 52	64 65	56 52	56 55	66	58 54	56. 7 58. 7	67 66	55 52		. 569	362.4
15	29. 737	30, 082	30, 130	30.050	46	53	47	48	56	50	51.3	57	45	. 284	. 363	.20.1
16 17	30, 105 29, 752	29. 947 29. 757	29. 823 29. 805	29. 958 29. 771	45 62	55 58	50 54	48 65	57 60	53 56	52.7 60.3	60 65	45 54			34.4
18 19	29. 770 29. 724	29. 734 29. 729	29, 667 29, 875	29. 724 29. 776	49 50	50 52	47	51 52	53 54	50 51	51.3 52.3	56 84	47	. 321	. 321	213.3 216.3
20	29.883	29.716	29. 552	29.717	48	50	46	51	53	48	50.7	53	45	296	. 321	. 24.3
21 22	29. 564 29. 900	29, 724 30, 092	29. 887 29. 993	29. 725 29. 995	47 42	48 50	46	49 45	50 52	48 50	49. 0 49. 0	51 53	45 42		. 309	. 2ml 2 2. 4 E .
23	30. 076	30, 101	30.066	30.081	40	53	47	42	55	- 50	49. 0 56. 3	56	39	. 221	. 376	2.3.2
24 25	30. 021 29. 955	29, 960 29, 719	30, 016 29, 702	30. 032 29. 792	45 61	65 51	52 47	47 63	68 55	54 50	56.0	69 60	44	. 273 . 510	∤. 3±? l	. 362.4
26 27	29. 959 29. 861	30, 000 29, 953	29. 878 30. 035	29. 945 29. 949	50 45	60	45 51	53 48	63	48 54	49.3 55.0	55 64	43 45	. 321		. 260 . 7 . 335 . 3
28	30.084	30.090	30. 044	30.073	54	62	57	56	64	60	60. 0	65	54	. 391	. 529	435.4
29 30	30. 050 29. 968	30. 062 29. 940	29. 955 29. 934	30. 022 29. 947	49 48	56 58	51 57	51 51	58 60	54 59	54. 3 56. 7	62 63	49 47	. 321 . 296		.335.I
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3	29.856	29, 890	29. 925	29.890	45	48	45	48	50	48	48.7	65	45	. 260	. 309	260 .7 21t .2
4 5	29. 852 30. 342	30. 060 30. 334	30, 176 30, 292	30. 029 30. 323	43 37	44 52	41	46 39	47 54	44	45. 7 46. 0	51 55	41	. 23 8	. 362	. 23.3
6 7	30. 201 30. 034	30, 219 29, 968	30, 108 29, 934	30, 176 29, 985	35 55	54 64	50 58	37 57	57 66	52 60	48. 7 61. 0	57 67	34 49	. 178 . 407		. 334 . ± . 456 . €
8	29. 858	29. 860	29.770	29. 829	56	65	54	59	68	61	62.7	69	56	. 409	. 577	325. 0
10	29. 757 29. 737	29. 732 29. 764	29. 665 29. 865	29. 718 29. 789	55 54	60 59	54 52	58 57	62 61	58 55	59. 3 57. 7	65 62	54 53	. 393	. 473	349.
41 12	29. 947 29. 787	29. 997 29. 732	29, 849 29, 854	29. 931 29. 791	51 50	56 52	52 47	54 52	58 55	55 50	55. 7 52. 3	61 56	50 47	. 335 . 334	. 422 . 349	
13	29. 946	29. 953	30, 031	29. 979	50	51	49	52	54	5L	52. 3	55	49	. 334	. 335	321.7
14 15	30. 157 30. 268	30. 125 30. 281	30, 154 30, 212	30, 145 30, 254	48 51	52 55	49 53	50 54	54 58	51 55	51. 7 55. 7	54 59	48 50	. 309	. 362 . 393	376.3
16	30. 279	30, 232	30. 355	30. 289	48	60	65	51	63	67	60.3	68	48	. 296	. 478	
17 18	29. 920 29. 864	29. 934 29. 825	29. 879 29. 881	29, 911 29, 857	47 50	62 63	58 56	50 52	65 65	60 58	58. 3 58. 3	66 67	47 49	. 283 . 334	. 516 . 549	422.0
19 ·20	29. 882 29. 969	29. 855 30. 023	29. 846 29. 864	29. 861 29. 952	56 54	67	62 58	57 56	70 64	64 60	63. 7 60. 0	70 66	53 54	. 43 6	. 6 22 . 4 97	529.3 456.4
21	29. 782	29, 626	29. 607	29. 672	60	64	65	62	71	67	66.7	71	56	. 491	. 503	591,53 349,44
22 23	29. 375 29. 707	29. 414 29. 654	29. 657 29. 658	29. 482 29. 673	62	59 52	52 42	65 44	61 55	55 44	60. 3 47. 0	70 56	52 42	. 516 . 241	. 349	. 262. *
24 25	29. 691 29. 988	29. 855 30. 102	29. 957 30. 050	29. 834 30. 047	35 36	40 41	38 37	38	43 44	45	40. 3 40. 7	50 43	35 36	. 165 . 186	206 218	9.3. 19
26	29. 864	29, 825	29. 881	29.857	50	63	56	52	65	58	58.3	67	49		200	181 15 422 45 224 2
27 28	29. 865 30. 070	30, 100 29, 987	30. 050 29. 870	30, 005 29, 976	38 37	47	42 42	40 39	50 50	45 44	45. 0 47. 3	50 50	38 37	. 903 . 194	309	241.3
. 29	29. 673	29. 576	29.628	29. 626 29. 584	49	52	48	51	54	50	51.7	55	47	321	362	3(9), 33 9(9), 35
30 31	29. 516 29. 666	29. 583 29. 771	29. 654 29. 994	29. 564	40 36	44 37	40 35	43 38	46 40	43 38	44. 0 38. 7	52 45	40 35	186	181	165.17
Means	29, 890	29. 902	29. 909	29. 900		_	_	40.0	57. 6	-	53. 4	_	_	910	413	347, 35

the northern and northwestern lakes at Fort Niagara, New York.

VAPO	DR.				w	IND.					clo	ount adine — cle	86.	and deci-	r melted inches
Humic Saturation	dity. 1 = 1,0	00.		Direction om when		m	ocity iles, our.	, in per	t velocity, per hour.	Resultant direction.	(10 = tire	sky.) = sky ly ov cast.)	er-	of evap	nt of rain or w, in U. S. decimals.
7 p. m. 2 p. m.	9 . 11 .	Mean.	7 a. m.	2 p. m.	9 p. m.	7 a. m.	2 р. п.	9 p. m.	Resultant velc in miles, per h	Resultan	7 a. m.	2 p. m.	9 p. m.	Amount U. S. mals.	Amount snow,
. 831 . 846 . 899 . 911 . 944 . 850 . 822 . 895 . 895 . 895 . 895 . 896 . 812 . 825 . 877 . 876 . 896 . 893 . 895 . 893 . 895 . 893 . 895 . 893 . 895 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 896 . 798 . 897 . 798 . 898 . 772 . 898 . 772 . 898 . 772 . 898 . 772 . 898 . 773 . 898 . 777 . 831 . 872 . 888 . 776 . 899 . 876 . 777 . 831 . 872 . 888 . 899 . 876 . 779 . 880 . 899 . 876 . 876 . 879 . 870 . 880	. 888 . 898 . 893 . 831 . 836 . 833 . 876 . 833 . 832 . 833 . 832 . 833 . 832 . 833 . 736 . 867 . 736 . 873 . 736 . 850 . 736 . 853 . 736 . 853 . 736 . 853 . 736 . 737 . 736 . 737 . 736	. 855 903 896 869 860 874 838 849 891 805 815 815 816 839 813 853 813 853 814 855 818 852 863 863 863 863 863 864 865 863 864 864 865 865 866 867 867 867 867 867 867 867 867 867	W 8 8 W 8 W W W W W W W W W W M .	W 8 W W W W W W W W W W W W W W M	8 8 NW NE W W W NE SW W W W W NE SW W W NW NW NW NW NW NW NW NW NW NW NW N	2 12 12 2 4 4 4 5 2 2 12 5 2 5 2 5 2 5 3 5 5 4 4 2 12 12 2 5 5 4 2 2 12 12 12 12 12 12 12 12 12 12 12 12	12 35 25 35 45 4 25 35 35 45 4 25 35 35 35 4 2 4 4 2 2 4 4 4 2 2	25 45 45 25 35 2 12 4 4 4 4 4 4 4 5 60 25 60 2 12 45 35 12 2 2 5 60 2 2 5 60 2 5 60 2 60 2 60 2 6	9. 7 7 10. 7	8. 45 W. 8. 77 W. West N. 50 W. 8. 32 W. West S. 65 W. N. 16 E. N. 15 E. 8. 68 W. 8. 52 W. N. 43 E. 8. 4 W. N. 46 W. S. 60 W. S. 12 W. N. 53 W.	8 9 7 6 8 6 8 10 6 5 9 7 5 5 6 7 8 10 10 10 10 7 5 5 8 10 7 8 5 5	10 6 8 7 7 4 10 9 2 6 10 7 10 7 10 7 10 7 10 7 10 7 10 7 10	8899897107777106786610898109667868		. 58 . 20 . 06 . 84
. 855 . 853	. 840	. 849							10. 0	N. 87 W.	7. 6	7. 0	7. 8		4. 56
. 961 . 831 .816 . 854 .777 . 856 .767 . 779 . 816 . 867 . 817 . 812 . 874 . 818 . 884 . 818 . 884 . 818 . 884 . 818 . 885 . 861 . 805 . 861 . 805 . 861 . 805 . 861 . 805 . 861 . 805 . 861 . 805 . 861 . 805 . 861 . 805 . 861 . 800 . 806 . 836 . 872 . 233 . 836 . 882 . 872 . 836 . 872 . 836 . 872 . 836 . 872 . 836 . 886 . 8	. 890 . 836 . 836 . 770 . 756 . 762 . 861 . 880 . 803 . 805 . 859 . 859 . 893 . 805 . 893 . 805 . 893 . 805 . 893 . 805 . 893 . 805	. 857 . 835 . 801 . 765 . 815 . 892 . 756 . 633 . 829 . 817 . 841 . 828 . 838 . 834 . 876 . 891 . 841 . 876 . 891 . 861 . 763 . 841 . 763 . 841 . 876 . 892 . 893 . 893 . 894 . 894 . 896 . 896 . 897 . 898	W	NW	NW S.W S.W NE S.W NE S.W NE S.W NE S.W NE S.W NE S.W NE S.W NE S.W NE S.W NE S.W W W S.W S.W W W W S.W W W W S.W W W W	2 25 60 35 4 2 12 4 12 35 25 1 12 12 25 75 1 12 25 35 4 60	12 45 12 45 12 45 4 45 45 45 45 45 45 45 45 45 45 45 4	4 60 4 4 25 2 12 4 4 5 5 25 4 4 4 4 5 5 12 4 5 5 7 3 5 60 4 4 4 5 60	6.0 43.3 24.0 20.0 22.7 23.7 17.3 17.3 17.3 17.3 17.3 17.3 17.3 1	South N. 45 W. N. 55 W. South N. 9 W. N. 55 W. South N. 80 W. N. 85 W. N. 86 E. N. 45 E. N. 45 E. N. 45 E. North N. 25 W. West West South	8 7 10	3565644378645464847486787476075	8976767809886775576867889669810B10		

	Barome	ter reduc	ed to tem	nerature		TE	MPR	RATU	RI-	-Fah	RENHE	IT.			VAP	OR
Date.	of 32°	Fabr., ex inches a	pressed i	n United	P	t buli oint c pora	of	P		re c	tem- of the	ä	i.		bes a	, in U. S and dec
	7 a. m.	2 p. m.	9 p. m.	Mean.	7 a. B.	2 p. m.	9 p. m.	7 a. m.	2 р. ш.	9 p. m.	Mean.	Maximum	Minimum	7 a. m.	2 p. m.	9 p. m.
1866. Nov. 1 2 3 3 4 5 5 6 6 7 7 8 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 26 28 29 30 Means.	29, 922 29, 774 30, 080 30, 295 30, 375 30, 404 30, 170 29, 890 30, 165 29, 890 30, 165 29, 910 29, 577 29, 711 29, 611 29, 611 29, 818 29, 871 30, 022 29, 527 29, 713 29, 611 29, 611 29, 611 29, 818 29, 871 30, 022 30, 018 29, 735 29, 735 29, 735 29, 748	29. 758 29. 889 29. 889 30. 341 30. 351 30. 361 30. 362 29. 884 29. 859 29. 955 30. 229 29. 952 29. 962 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 767 29. 603 29. 825 29. 623 29. 623 29. 822	29. 566 29. 993 30. 219 30. 402 30. 493 30. 393 29. 978 29. 921 30. 103 30. 126 29. 953 30. 126 29. 953 30. 126 29. 953 30. 126 29. 953 30. 126 29. 953 30. 29. 756 29. 830 29. 710 29. 905 29	29. 749 29. 889 20. 140 30. 146 30. 346 30. 369 30. 076 29. 884 29. 983 30. 173 29. 952 29. 164 29. 666 29. 783 29. 983 29. 645 29. 887 29. 645 29. 645 29. 645 29. 648	0 34 41 32 36 31 26 35 41 37 46 35 34 32 42 36 37 41 40 33 36 22 25 30 39 50 50 50 34 53 34	0 45 39 37 32 42 50 55 46 48 42 43 50 34 37 38 43 40 34 35 52 28 34 43 50 52 41 35	9 397 366 35 38 477 478 422 409 388 433 337 376 316 224 425 277 388 437 377 389 437 332	o 36 444 38 33 38 43 34 40 48 37 36 38 40 44 33 32 82 27 27 27 27 27 27 27 27 27 27 27 27 27	0 47 42 411 39 34 45 52 58 50 44 5 53 46 6 52 55 44 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	0 42 40 38 35 33 36 0 49 50 0 44 42 40 45 40 35 39 42 40 45 50 40 34 38 33 8 26 30 30 40 45 50 40 34 39 3	41.77 42.00 37.737.3 33.337.746.3 35.077 46.03 50.7746.03 40.03 40.03 40.07 40.03 40.07 40	0 47 44 42 40 35 53 557 52 51 45 53 557 52 41 46 44 40 33 53 66 66 67 67 67 67 67 67 67 67 67 67 67	。 33.55 33.35 43.55 43.	218 155 186 151 117 165 231 251 178 170 241 186 181 186 181 112 218 211 218 211 218 211 218 211 218 211 218 211 218 211 218 211 218 211 218 211 218 211 218 211 218 211 218 211 218 218	199 212 194 215 228 334 393 376 284 3.99 241 190 251 238 221 170 073 170 278 218 218 194 218 170 218 170 218 170 218 218 218	309 31 241 32 221 32 203 32 203 32 203 32 162 15 194 15 181 33 186 33 181 4
Dec. 1 2 3 3 4 4 5 6 6 9 9 10 11 12 13 13 14 16 17 18 19 21 22 23 24 26 26 27 28 29 30	30, 017 30, 084 29, 918 29, 453 29, 909 30, 032 29, 761 29, 203 29, 556 29, 703 29, 556 30, 041 30, 077 30, 244 29, 640 29, 374 30, 359 29, 247 29, 247 29, 247 29, 247 29, 247 29, 257 29, 730 29, 803 30, 013 30, 30, 30, 30, 30, 30, 30, 30, 30, 30,	30. 109 29. 807 30. 014 29. 382 29. 57 29. 792 29. 118 29. 652 29. 610 29. 987 30. 129 30. 129 30. 129 30. 129 30. 185 29. 813 30. 327 30. 185 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 29. 612 20. 612	30, 047 29, 974 29, 751 29, 647 30, 017 29, 158 29, 342 29, 716 29, 716 29, 969 29, 822 30, 201 30, 089 29, 822 30, 201 30, 679 29, 874 29, 474 29, 474 29, 474 29, 393 29, 474 29, 393 29, 474 29, 393 29, 474 29, 393 29, 474 29, 393 39, 485 30, 486 30, 48	30, 058 29, 985 29, 894 29, 978 29, 912 29, 570 29, 209 29, 560 29, 734 30, 163 30, 163 30, 163 30, 29, 529 29, 853 30, 199 62, 245 29, 256 20, 256 20	27 23 30 40 32 35 40 43 25 14 16 13 11 17 20 14 32 5 -0 27 37 34 25 16 21 11 11 11 11 11 11 11 11 11 11 11 11	30 35 45 40 41 40 50 30 80 52 54 11 17 20 22 28 82 40 32 24 27 23 20 17 17 17	277 299 396 388 377 422 233 17 15 15 15 15 15 15 15 15 15 15 15 15 15	29 25 33 42 34 43 46 28 16 15 13 15 19 23 37 28 18 27 15 18 14	32 37 48 44 44 42 51 32 23 27 21 24 42 22 25 35 35 35 35 35 37 30 22 31 32 32 33 35 36 36 36 36 37 37 37 37 37 37 37 37 37 37 37 37 37	30 32 40 38 40 40 45 45 20 20 17 16 11 17 18 24 30 5 16 27 17 18 29 17 17 17 17 17 17 17 17 17 17 17 17 17	30. 0 31. 3 40. 3 40. 3 40. 0 39. 3 40. 0 7 47. 3 7 17. 7 17. 7 17. 7 17. 7 17. 7 39. 3 40. 0 24. 0 24. 0 24. 0 17. 3 17	355 3748 445 443 443 534 443 535 544 19 20 22 22 25 31 35 30 16 35 32 30 25 30 25 30 25 30 25 30 30 30 30 30 30 30 30 30 30 30 30 30	277 233 300 336 332 335 400 42 233 114 115 115 15 15 15 15 15 15 15 15 15 15 1	. 132 . 221 . 155 . 208 . 238 . 238 . 049 . 059 . 059 . 059 . 059 . 059 . 071 . 074 . 048 . 155 . 039 	. 178 260 . 208 . 218 . 221 . 334 . 074 . 112 . 080 . 085 . 084 . 041 . 142 . 221 . 142 . 095 . 095 . 074 . 075 . 076 . 077 . 077 . 077 . 077 . 077	255 2 186 30 181 181 181 182 25 25 26 26 26 26 26 26 26 26 26 26 26 26 26

the northern and northwestern lakes at Fort Niagara, New York.

	VA	POR.				W	IND.	,				clo	ount udin	ess. es.r	ation in d deci-	melted inches
8a	Hum turatio	idity. n = 1,0	000.		Direction om when		m	ocity iles, our.	, in per	t velocity,	Resultant direction.	(10: tire	sky. = sk; e!y o cast.	y en- ver-	of evaporatinches and	nt of rain or v. in U. S. decimals.
7 & m.	2 p. m.	9 p. H	Mean.	7 F ii	2 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Resultant vel	Resultan	7 B. III.	2 p. m.	9 p. m.	Amount U. S. mals.	Amount fnow,
. 802 . 756 . 722 . 811 . 800 . 768 . 833 . 850 . 807 . 802 . 836 . 833 . 732 . 733 . 732 . 733 . 732 . 733 . 732 . 734 . 732 . 734 . 732 . 734	. 847 .744 .824 .816 .816 .816 .850 .856 .836 .836 .836 .836 .836 .840 .767 .882 .892 .767 .882 .892 .767 .893 .895 .895 .895 .895 .895 .895 .895 .895	744 732 811 1.000 820 820 853 786 856 820 840 820 797 732 811 800 768 758 758 820 840 840 840 856 857 857 858 858 859 859 859 859 859 859 859 859	. 798 . 794 . 809 . 876 . 797 . 783 . 813 . 813 . 821 . 821 . 821 . 813 . 808 . 808 . 808 . 808 . 808 . 808 . 777 . 797 . 691 . 413 . 746 . 755 . 741 . 785	SW SW NE NE NE SW SW S	8 W W W W S S W S N W S N W S N W S N W S N W S N W S S N W S S S W S S W S S W S W S W S S W S S W S S W S S W S S W S S W S S W S S W S S S W S S S W S S S S W S	S W W W W W W W W S W S W S W N W N W N W N W S S W S W S W S W S W S W S W S W S W S W S W S W S W S W	2 60 4 60 35 12 25 4 12 25 560 35 12 4 4 12 25 25 25 25	45 45 45 42 45 45 45 45 45 45 45 45 45 45 45 45 45	75 45 25 25 60 12 25 25 45 60 25 25 45 45 45 45 45 45 45	40. 3 3 410. 7 33. 0 12. 7 33. 0 12. 7 36. 0 16. 0 26. 7 7 526. 0 16. 0 26. 7 31. 7 32. 32. 32. 32. 32. 32. 32. 32. 32. 32.	West 8, 72 W 8, 48 W 80 uth N. 82 W 80 th 8, 63 W 45 E 8, 8 W 45 E 8, 12 W 8, 45 W 80 uth 8, 32 E 8, 31 W 8, 45 W 9, 45 W	556665577884106676655710066765557100	74 47 77 55 45 65 66 86 77 100 107 66 100 77 66 100 107 67 77 77 77 77 77 77 77 77 77 77 77 77	97 98 76 66 10 10 87 10 11 10 11 10 11 10 11 10 10 10 10 10		
. 778 . 775 . 746 . 703 . 829 . 792 . 750 . 655 . 659 . 682 . 648 . 623 . 648 . 623 . 648 . 623 . 648 . 623 . 648 . 623 . 648 . 623 . 648 . 623 . 648 . 623 . 648 . 623 . 648 . 623 . 648 . 623 . 648 . 623 . 648 . 692 . 598 . 598 . 598 . 598 . 692 . 538 . 641 . 669 . 641 . 669	. 794 . 807 . 777 . 750 . 756 . 859 . 861 . 712 . 598 . 685 . 685 . 685 . 685 . 685 . 688	. 673 . 694 . 910 . 811 . 910 . 820 . 732 . 762 . 634 . 556 . 671 . 525 . 506 . 671 . 525 . 610 . 782 . 596 . 671 . 661 . 671 . 671 . 671 . 675 . 675 . 675 . 675 . 676 . 676 . 671	. 748 748 749 797 797 760 791 796 694 64 64 655 695 582 695 582 695 695 695 695 695 695 695 695 695 695	NW	NW SW SW SW SW SW SW SW SW SW SW SW SW SW SW SW SW SW SW SW W S	NW	35 4 35 25 25 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	25 25 45 35 4 25 35 12 990 12 25 45 12 25 45 12 25 45 12 25 45 12	35 12 35 45 2 12 35 35 35 35 35 35 35 45 4 25 35 35 35 45 4 25 35 45 4 12	31. 77 13. 73 38. 3 10. 77 38. 3 10. 77 33. 3 26. 00 31. 77 26. 00 31. 77 26. 00 31. 77 26. 00 31. 77 20. 77 13. 00 10. 77 10. 00 1	N. 45 W. S. 45 W. South S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. S. 45 W. West South	8 6 6 5 10 6 7 7 10 7 7 6 6 6 10 10 6 6 6 6 7 7 7 10 7 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	7 4 4 10 7 6 6 10 10 6 7 10 10 6 7 10 10 8 7 5 7 6 6 7 7 10 7 10 8 7 7 10 10 10 10 10 10 10 10 10 10 10 10 10	8 6 6 10 8 9 10 7 10 8 9 9 9 8 10 8 6 6 10 10 10 10 8 8 8 6 8 8 8 8 8 8 8 8 8		

	Baromet	er reduce	d to tem	nerature		TE	MPE	RATU	RE-	-FAH	RENHE	37.			VAP	OR.	
Date.	of 32°	Fahr., ex inches ar	pressed i	United	P	bull oint o	of	p		ire o	tem-	e.	ď	Elas inc ma	bes :		
	7 a. m.	2 p. m.	9 p. m.	Mean.	7 a. m.	2 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Mean.	Maximum	Minimum	7 a. m.	2 p. m.	9 p. m.	Menn.
1866.					•	_	•	-	-	•	•	•	•				,
July 1	29. 991	29, 955	29. 914	29. 953	61	69	62	69	78	68	71. 7	80	41		. 568		
2 3	29. 982 29. 758	29. 954 29. 676	29, 833 29, 575	29. 923 29. 670	60 66	75 73	68 70	65 71	83 87	72 76	80. 0 78. 0	83	52) 51		. 760 . 623		
4	29. 513 29. 728	29. 442	29. 533	29. 496	64	69	66	69	77	69	71.7	78	5 1		. 601 . 928		
5 6	29. 725	29. 656 29. 658	29. 623 29. 687	29. 669 29. 671	68 72	80 85	69 74	75 78	87 90	73 79	78.3 82.3	88 91	55 62		. 920 . 135		
7	29. 707	29. 725	29. 617	29. 683	71	71	74	79	90	79	82.7	91	64	. 651	. 502	. 77.	2.4
8 9	29. 642 29. 895	29. 574 29. 921	29. 681 29. 944	29. 632 29. 920	74 55	75 62	65 63	81 64	83 72	70 68	78.0 68.0	84 73	61 50		. 760 . 423		
10	30, 009	29. 937	29, 911	29. 952	59	66	61	66	77	69	67.3	79	43	. 407	. 492	. 43	υ.
11 12	29. 902 29. 813	30. 067 29. 743	29, 818 29, 780	29. 929 29. 779	60 69	69 78	71 74	65 76	77 90	74 76	72. 0 81. 3	81 91	49 53		. 601 . 677		
13	29. 707	29. 595	29, 569	29. 624	73	78	79	79	94	82	85.0	95	65	. 730	. 741	94	9.
14 15	29. 743 29. 737	29. 707 29. 685	29. 700 29. 674	29. 717 29. 699	67 77	75 81	69 77	73 83	82 94	74 84	76, 3 87, 0	94 96	61 62		. 773 . 890		
16	29.813	29. 734	29, 707	29.748	80	87	72	87	97	79	87. 7	98	119	. 928	. 146	. 69	Û,
17 18	29. 869 29. 592	29. 660 29. 605	29, 652 29, 641	29. 727 29. 613	76 72	82 68	71 63	85 77	96 72	77 66	86. 0 71. 7	97 79	68 54		. 909 . 631		
19	29. 786	29. 805	29. 807	29. 799	58	66	63	64	74	67	68. 3	76	49	. 403	. 53	. 52	2.
20	29.870	29. 847	29. 721	29.813	56	70	65 67	62 65	78	70	70.0 71.7	79	47		62		
21 22	29. 684 29. 511	29. 647 29. 540	29, 565 29, 465	29. 632 29. 505	60 64	67 72	67	66	78 77	72 72	71, 7	78 76	53 56		. 514 . 718		
23	29. 546	29. 592	29, 623	29, 587	63	69	65	68	76	68	70, 7	77	54	. 509	. 614	5. 57	٠.
24 25	29. 731 29. 715	29. 684 29. 687	29, 658 29, 703	29. 691 29. 702	61 66	70 72	69 68	66 72	79 82	75 73	73. 0 75. 7	82 84	55 55		. 619 . 650		
26	29. 834	29. 786	29.810	29.810	63	76	73	68	85	76	76. 3	86	53	. 509	. 775	. 77	1.
27 28	29, 789 29, 656	29. 686 29. 549	29, 583 29, 538	29, 686 29, 581	65 62	76 71	68 69	70 68	86 81	73 75	76.3 74.7	86 82	54 56		. 76. . 624		
29	29. 513	29, 559	29, 483	29. 518	68	65	65	72	81	69	74.0	80	55	. 631	. 400	. 56	4.
30 31	29. 541 29. 676	29. 560 29. 702	29. 606 29. 621	29. 576 29. 666	63 64	64 66	65 65	70 68	77 80	68 70	71. 7 72. 7	78 80	55 53		. 556 . 566		
deans	29. 743	29. 708	29. 676	29. 709	<u> </u>	<u> </u>		71. 6	82.6	72.7	75. 6	<u> </u>		. 566	. 617	. 63	5.I
Lugust 1	29. 511	29. 420	29. 428	29. 453	65	70	71	70	74	75	73.0	76	55	. 550	. 679	70	4.
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the northern and northwestern lakes at Charlotte, New York.

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the northern and northwestern lakes at Charlotte, New York.

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the northern and northwestern lakes at Charlotte, New York.

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Hun Saturatio	nidity. on == 1,0	000,		Direction om when		120	locity iles, our.	per	t velocity, per hour.	Resultant direction.	(10 =	sky.) = sky ely ov cast.)	7 em- 7 er-	of evap	of radio In U.S.
7 p. ii.	9 p. ii.	Mean.	7 P. IJ.	8 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Resultant in miles, p	Resultan	7 a. m.	2 p. m.	9 p. m.	Amount U. S. mals.	Amount snow, decime
. 619 . 624 . 698 . 669 . 698 . 661 . 628 . 628 . 694 . 619 . 746 . 399 . 794 . 546 . 750	. 505 . 732 . 719 . 802 . 775 . 792 . 8.33 . 667 . 727 . 750 . 834 . 811 . 691 . 812 . 719 . 829 . 792 . 683 . 893 . 893 . 893 . 893 . 893 . 893 . 893 . 893 . 893 . 893 . 893 . 893 . 893 . 797 . 797	. 583 . 700 . 693 . 696 . 696 . 696 . 724 . 750 . 732 . 745 . 765 . 769 . 765 . 769 . 763 . 764 . 764 . 825 . 764 . 825 . 728 . 825 . 728 . 825 . 728 . 825 . 728 . 724 . 724 . 725 . 726 . 727 . 726 . 727 . 727 . 727 . 728	S	S	8	35 2 2 12 135 4 4 4 12 12 12 25 45 12 12 12 12 12 12 12	35 25 12 4 25 12 25 12 12 12 12 12 12 12 45	2 12 45 45 2 4 12 25 60 12 45 12	35. 0 7 8. 7 5. 3 16. 3 20. 7 0 10. 3 3 19. 0 0 111. 0 0 27. 3 19. 0 113. 0	S. 89 W. N. 33 W. N. 12 W. West. S. 62 W. S. 45 W. West. S. 72 W. S. 45 E. S. 45 W. N. 45 E. S. 45 W. N. 45 E. S. 45 W. S. 45 W. South. South. South. South. South. South. South. South. South. South. South. South. South. South. South. South. South. South. S. 45 W.	10 10 10 10 0 0 3 3 3 0 10 2 8 8 10 10 10 10 10 10 10 10 10 10 10 10 10	10 10 10 11 13 0 10 5 0 10 10 10 10 10 10 10 10 10 10 10 10 1	10 10 10 10 0 0 0 0 11 10 10 10 10 10 10		21 .04 .04 .09 .00 .07 .18 .13 .03 .03 .03 .03 .03 .04 .04 .04 .05 .05 .05 .05 .05 .05 .05 .05 .05 .05
.747 .664	. 773	. 729				<u></u>			4.9	8. 86 W.	7. 9	7. 7	8. 1		3. 35
. 890 . 675 . 782 . 463 . 607 . 840 . 895 . 646 . 719 . 669 . 762 . 613 . 733 . 559 . 692 . 712 . 834 . 529 . 671 . 733 . 682 . 712 . 834 . 929 . 671 . 733 . 692 . 721 . 692 . 721 . 692 . 721 . 692 . 731 . 693 . 807 . 754 . 797 . 893 . 807 . 754 . 797 . 893 . 807 . 768 . 846 . 840 . 797 . 768 . 876 . 834 . 845 . 840 . 730	. 685 . 788 . 490 . 762 . 806 . 816 . 816 . 816 . 816 . 819 . 702 . 702 . 702 . 702 . 703 . 791 . 730 . 791 . 730 . 788 . 800 . 802 . 802 . 802 . 802 . 803 . 804 . 806 . 806 . 806 . 807 . 808 . 787 . 808 . 788 . 788 . 788 . 788 . 788 . 788 . 788 . 788 . 788 . 781 . 840 . 840 . 840 . 840 . 840 . 840 . 840 . 840 . 840 . 840 . 840 . 840 . 840 . 840 . 840	.750 .678 .552 .736 .781 .735 .730 .616 .688 .701 .765 .797 .714 .803 .830 .846 .830 .846 .830 .757 .788 .788 .788 .788 .788 .788 .788	NWS.WS.S.W	NW	NW	45 4 45 12 12 25 12 45 25 45 60 45 12	35 4 12 12 4 12 4 12 12 12 12 12 12 12 12 12 12 12 12 12	12 12 25 4 12 45 35 35 35 25 25 25 25 45 60 45	28. 3 9. 3 20. 7 19. 7 14. 3 46. 7 42. 0 23. 0	S. 30 W. S. 19 E. S. 12 W. West. S. 77 E. West. S. 53 W. South S. 31 W. S. 45 W. West. N. 44 W. West. N. 20 W. N. 61 W. S. 45 W.	8 10 5 10 4 10 0 10 10 10 10 10 10 10 10 10 10 10 1	8 7 2 10 4 10 2 2 4 3 1 1 10 10 10 10 10 10 10 10 10 10 10 10	10 0 10 10 10 10 10 10 10 10 10 10 10 10		255 . 332 . 199 . 069 . 1,000 . 166 . 100 . 200 . 101 . 414 . 448

	Barome	ter reduc	ed to tem	nerature		TE	MPE	RATU	RE—	-FAH	RENHE	IT.			VAP	OR.	
Date.	of 32°	Fahr., ex	pressed in ad decima	n United	P	t bull oint pora	of	p		ire c	tem-	i	٠		ticity hos i		
	7 a. m.	2 p. ii.	9 p. m.	Mesn.	7 a. m.	2 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Mean.	Maximum	Minimum	7 a. m.	2 p. m.	9 p. m.	Monn.
1866. July 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 24 25 26 27 28 29 30 31	29, 977 29, 984 29, 678 29, 650 29, 650 29, 650 29, 664 29, 580 29, 680 29, 728 29, 728 29, 734 29, 734 29, 734 29, 734 29, 732 29, 732 29, 733 29, 671 29, 732 29, 733 29, 733 29, 733 29, 734 29, 732 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733 29, 733	29, 999 29, 903 29, 667 29, 657 29, 654 29, 663 29, 870 29, 673 29, 674 29, 769 29, 769 29, 769 29, 769 29, 769 29, 769 29, 670	29, 839 29, 798° 29, 587 29, 638 29, 660 29, 680 29, 680 29, 681 29, 901 29, 763 29, 567 29, 567 29, 564 29, 578 29, 5	29, 972 29, 895 29, 711 29, 642 29, 642 29, 642 29, 589 29, 589 29, 581 29, 614 29, 718 29, 614 29, 718 29, 614 29, 718 29, 614 29, 718 29, 685 29, 797 29, 661 29, 502 29, 685 29, 502 29, 681 29, 503 29, 681 29, 503 29, 681 29, 503 29, 681 29, 503 29, 681 29, 503 29, 681 29, 503 29, 681 29, 503 29, 681 29, 503 29, 681 29, 503 29, 681 29, 503 29, 681 29, 503 29, 681	0 54 60 66 67 71 70 72 60 58 62 66 69 68 67 73 74 69 65 55 67 62 65 63 67 60 63 67 65 67 67 67 67 67 67 67 67 67 67 67 67 67	61 70 73 67 72 71 73 72 71 63 69 68 68 68 68 68 68 68 68 68 68 68 68 68	0 64 68 71 65 68 69 70 67 67 77 72 74 74 61 69 67 71 66 68 70 70 70 71 66 68 68 69 71 69 71 69 71 71 71 71 71 71 71 71 71 71 71 71 71	0 59 66 67 73 73 76 76 66 63 66 67 77 77 78 77 69 67 69 67 69 67 69 69 69 69 69 69 69 69 69 69 69 69 69	0 71 78 83 72 78 80 81 75 77 77 78 80 82 78 85 67 77 77 77 77 77 77 77 77 77 77 77 77	o 70 74 79 69 73 75 76 73 77 72 74 80 80 81 81 65 70 75 74 77 72 73 76 77 77 77 77 77 77 77 77 77 77 77 77	75. 3 78. 7 75. 0 74. 3 74. 3 72. 7	0 75 78 84 78 78 81 82 77 74 78 80 81 88 85 88 87 73 75 79 81 76 82 83 85 88 81 86 81 81 81 81 81 81 81 81 81 81 81 81 81	0 491 67 655 62 68 69 65 55 59 66 66 68 69 73 74 60 55 54 62 66 68 69 69 69 69 69 69 69 69 69 69 69 69 69	. 351 438 545 561 573 652 572 641 438 631 757 641 380 502 572 641 380 644 457 564 466 466 467 467 469 469 469 469 469 469 469 469 469 469	. 403 625 677 595 704 469 637 550 637 552 422 422 422 564 564 663 635 637 554 554 554	516 604 618 626 652 652 652 622 745 622 745 566 622 626 627 631 631 632 632 632 632 632 633 633 634 634 634 634 634 634 634 634	492 656 669 669 669 669 669 669 669 669 669
Means Aug. 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 30 31	28. 716 28. 528 28. 528 28. 528 28. 625 28. 625 28. 621 28. 621 28. 622 28. 621 28. 622	29. 689 29. 412 29. 425 29. 442 29. 678 29. 663 29. 66	28. 655 28. 458 28. 458 28. 558 28. 657 28. 558 28. 558 28. 557 28. 557 28. 558 28. 557 28. 557 28. 558 28. 557 28. 558 28.	29. 686 29. 435 29. 434 29. 654 29. 654 29. 655 29. 655 29. 655 29. 655 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 656 29. 657 29. 657	64 69 62 57 66 60 58 59 57 58 66 61 47 55 54 55 66 61 64 55 57 57 57 57 57 57 57 57 57 57 57 57	707 676 680 615 655 657 64 71 638 656 655 665 657 688 655 665 665 665 665 665 665 665 665	676 64 57 59 66 55 64 60 60 60 60 60 60 60 60 60 60 60 60 60	7173 68 65 66 66 66 66 66 65 51 59 66 65 55 58 66 66 67 68 66 66 66 66 66 66 66 66 66 66 66 66	78. 6 78. 74. 75. 64. 67. 72. 75. 72. 81. 70. 72. 62. 64. 70. 66. 65. 65. 69. 77. 75. 77. 77. 77. 77. 77. 77. 77. 77	72770166708703557758870038886155598888888888888888888888888888888	74.3 73.7 71.0 65.3 68.7 69.0 66.0 64.0 68.0 68.7 68.7 68.7 68.7 68.7 68.7 68.7 68.7	78 74 75 75 70 72 75 81 72 66 65 66 65 69 73 78 75 79 77 79 77 79 77 79 79 79 79 79 79	65662576595555461825546965835545555585983555	. 503 . 655 . 509 . 386 . 438 . 438 . 438 . 447 . 412 . 386 . 483 . 380 . 483 . 380 . 543 . 380 . 543 . 380 . 543 . 385 . 385 . 385 . 485	. 489 . 624 . 482 . 631 . 369 . 314 . 416 . 550 . 470 . 489 . 438 . 389 . 420 . 389	5955 5506 516 412 407 5566 516 516 516 516 516 516 516 516 51	57/ 59/ 515/ 515/ 525/ 500/ 500/ 545/ 545/ 545/ 545/ 545/ 54

the northern and northwestern lakes at Sackett's Harbor, New York.

	VA.	POR.				w	IND.					clo	noun oudin	ess.	tion, in	melted
Se		idity. on == 1,0	000.		Direction om when		10	locit; niles, nour.	y, in per	ltant velocity, les, per hour.	Rornitant direction.	(10	sky. = sk ely c cast.	y en. Y en.	of eve	int of rain or w, in U. S. decimals.
7 P. B.	S P. II.	9 p. B.	Me.m.	7 p. m.	2 p. m.	9 P. II	7 a.m.	2 p. 75.	9 p. m.	Resultan in miles,	Remitan	7 p. m.	2 p. m.	9 p. m.	Amount U. 8. mals.	Amount snow, and de
. 703 . 685 . 672 . 716 . 731 . 731 . 731 . 736 . 786 . 754 . 764 . 764 . 761 . 718 . 718 . 740 . 796	. 532 . 653 . 650 . 758 . 734 . 618 . 618 . 648 . 574 . 632 . 632 . 644 . 622 . 644 . 622 . 644 . 626 . 648 . 639 . 639 . 653	704 720 637 796 796 796 797 797 761 663 774 653 653 677 704 704 704 777 731 712 689 661 712 689 661 663 712 689 661 663 663 712 663 712 663 712 663 712 663 714 715 715 715 715 715 715 715 715 715 715	. 646 . 686 . 643 . 757 . 719 . 694 . 679 . 677 . 716 . 679 . 677 . 714 . 722 . 683 . 690 . 736 . 693 . 736 . 693 . 749 . 693 . 749 . 693	8	W	8	12 12 12 12 12 12 12 12 12 12 12 12 12 1	4 4 4 12 35 4 4 12 12 12 4 4 4 4 12 12 12 12 12 12 12 12 12 12 12 12 12	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	2.333.07 6.37 6.37 6.07 6.07 6.07 6.07 6.07 6.07 6.07 6.0	8. 28 W. 8. 17 W. 8. 8 W. 8. 32 W. 8. 45 W. 8. 27 W. N. 43 E. West. 8. 40 W. 8. 35 W. 8. 56 W. 8. 43 W. 8. 30 W. 8. 30 W. 9. 30 W. 9. 30 W. 9. 30 W. 9. 30 W. 9. 30 W. 9. 30 W. 9. 30 W. 9. 35 W. 9. 30 W. 9. 30 W. 9. 35 W. 9. 30 W. 9. 30 W. 9. 35 W. 9. 30 W. 9. 30 W. 9. 35 W. 9. 30 W.	0 6 5 5 5 10 0 10 6 0 0 3 4 0 8 7 5 7 7 10 1 7 7 0 2 9 0 9 0	0 99 7 10 8 3 7 10 5 0 4 2 2 9 6 2 2 3 5 10 5 8 5 5 8 4 6 4	2 10 6 6 10 0 3 5 0 0 0 3 0 10 9 4 9 8 9 4 6 6 3 9 0 0 1 1 2 1 9		. 18 . 24
. 744	. 629	. 706	. 694							2. 1	S. 40 W.	5.0	5. 5	4.7		1. 50
. 663 . 807 . 743 . 836 . 670 . 685 . 772 . 765 . 685 . 783 . 783 . 783 . 783 . 783 . 783 . 783 . 783 . 783 . 783 . 783 . 783 . 783 . 786	. 653 . 677 . 599 . 690 . 639 . 639 . 639 . 624 . 639 . 639 . 639 . 644 . 655 . 631	758 751 704 636 636 799 636 731 731 731 740 724 685 723 685 731 647 731 647 731 647 731 647 731 731 731 731 731 731 731 731 731 73	. 691 . 745 . 692 . 745 . 695 . 745 . 716 . 705 . 804 . 708 . 696 . 696 . 697 . 779 . 779 . 779 . 779 . 778 . 708	SS.WS.WS.WS.WS.WS.WS.WS.WS.WS.WS.WS.WS.WS.WS.WS.WS.WS.S.S.WS.S.S.WS.S.S.WS.S.S.WS.S.S.WS.S.S.WS.S.S.W	8 W	SSWSWSWSWSWSWSW.	12 12 25 4 12 25 12 25 0 12 25 25 12 25 25 12 25 12 25 25 25 12 25 12 2 25 25 25 25 25 25 25 25 25 25 25 25	25 35 45 35 12 25 12 25 12 12 12 12 12 12 12 12 12 12 12 12 14 15 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	2 55 4 9 0 2 2 2 2 12 4 4 0 25 12 12 13 13 13 13 13 13 13 13 13 13 13 13 13	0.7 2.0 5.7 90.7 1.3 6.7 7.7 5.0 27.7 20.7 213.0 11.3 3.3 5.7	S. 23 W. S. 62 W. S. 62 W. West N. 48 E. N. 45 W. N. 76 W. S. 45 W. N. 76 W. S. 45 W. S. 34 E. S. 45 W. S. 34 E. S. 45 W. S. 34 E. S. 35 W.	10 6 3 10 8 9 9 0 0 10 7 0 3 10 10 10 10 10 10 10 10 10 5 8 9 2 10 5 6 6 5	10 3 2 10 10 10 10 7 8	10 5 10 8 10 8 0 10 5 0 0 10 10 10 10 10 10 10 10 10 10 10 10		1. 72 . 74 1. 28 . 16 . 22 . 12 . 16 . 14 . 10 . 10 . 10 . 10 . 10

Reductions of the monthly meteorological register of the survey of

	Baromet	ter reduc	ed to temp	perature		TE	MPE	RATU	RE-	-FAH	RENHI	IT.			VAPO	R.	_
Date.			pressed in d decima		P	t bul oint o pora	ρÍ	p		ire d	tem-	į	ei		ticity bes a		
	7 9 E	2 p. m.	9 p. m.	Mean.	7 a. m.	2 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Mean.	Maximum	Minimum	7 a. m.	2 p. m.	9 p. m.	Monb.
1866.					·	•	•	•	•	•	•	•	۰			_	<u> </u>
Sept. 1 2	29, 666 29, 503	29, 564 29, 434	29. 465 29. 575	29. 565 29. 504	59 69	76 75	68 70	64 73	85 82	72 75	73. 7 76. 7	85 83	59 69		. 775 . 773		
3	29. 766	29, 775	29. 736	29.759	58	64	63	62	71	68	67.0	75	58	. 429	. 503	. 509	. 48
4 5	29. 753 29. 538	29, 538	29. 588 29. 633	29. 670 29. 570	61 66	69	66 59	65 71	74	71 66	68. 0 70. 3	80 74	61 60	. 483	. 641	. 572 . 407	. 59 . 54
6 7	29. 756 29. 786	29. 758 29. 656	29. 748	29. 754	57	65	59	65	73	64	66.3	73	56	. 399	. 510	. 433	3.44
8	29.416	29. 536	29. 426 29. 662	29. 623 29. 538	54 58	65 58	59 56	59 61	70 62	63 61	64. 0 61. 3	70 63	53 56		. 550 . 429		
9 10	29. 797 29. 935	29. 839	29. 842 29. 774	29. 869 29. 849	57 44	62	52 55	60 48	69	55	57. 5	63	55	. 426	!	. 349	. 39
11	29.705	29. 639	29. 281	29. 488	52	62	65	56	66	59 68	58. 7 66. 7	69 68	41		. 462 . 502		
12 13	29. 258 29. 472	29. 311 29. 601	29. 308 29. 656	29. 292 29. 576	62 58	65 59	64 57	67 63	70 65	68 63	68. 3 63. 7	70 66	63 58	. 489	. 550	. 543	3.52
14	29.680	29. 549	29. 550	29, 593	51	60	53	55	65	57	59.0	65	50		. 420 . 4 51		
15 16	29. 872 30. 364	29, 966 29, 912	30. 012 29. 767	29. 950 29. 914	45	47 52	49 53	49 45	54 56	55 59	52. 7 53. 3	56 59	44 39		. 231 . 335		
17	29, 580	29. 634	29, 672	29. 629	57	61	57	61	66	61	62.7	66	55		. 470		
18 19	29. 677 29. 630	29. 670 29. 627	29. 645 29. 758	29. 664 29. 672	50 50	53 50	52 48	53 51	58 54	56 52	55. 7 53. 3	60 55	49 48		. 336 . 308		
20	29.778	29.620	29. 523	29. 640	46	54	53	49	57	56	54.0	58	44	. 271	. 378	. 363	3.33
21 22	29. 436 29. 849	29. 587 29. 914	29. 690 29. 954	29. 571 29. 906	64 43	52 49	47	67 47	55 55	52 49	58.0 50.3	67 56	47		. 349 . 269		
23	30, 067 29, 987	30, 019	29. 997	30, 028	38	50	49	41	57	53	50. 3	57	37	. 190	. 268	. 295	5.25
24 25	29. 987 29. 845	29. 917 29. 671	29. 861 29. 596	29. 922 29. 704	47 56	60	53 59	51 60	67 70	59 62	59. 0 64. 0	67 70	42 55		. 425 . 516		
26	29. 824 29. 859	29. 844	29. 825	29. 831	49	50	47	53	54	50	52, 3	61	46	. 295	. 308	. 283	3. 29
27 28	30. 044	29. 877 30. 022	29. 895 29. 973	29.877 30.013	45 49	59 61	54 53	49 53	65 66	58 57	57. 3 58. 7	67	41		. 420 . 470		
29 30	30, 021 29, 885	29. 979 29. 847	29. 925 24. 875	29. 974 29. 869	46 46	60	55 58	49 50	66 68	59 63	58. 0 60. 3	67 68	45 46	. 363	. 438 . 443	. 380). 39
Means	29. 748	29. 719	27. 704	29. 727						60. 3				 	. 447	_	_
					_	_						_			_	_	
Oct. 1	29. 935	29. 905	29. 819	29. 886	47	60	56	50	67	60	59.0	67	46	. 283	. 425	. 396	3. 36
3	29. 726 29. 745	29. 598 29. 773	29. 548 29. 783	29. 624 29. 767	58 43	55 41	56 41	63	74	61 45	66. 0 46. 3	76 60	55 41	416	. 161 . 179	. 383	3.32
4	29. 956	30. 015	30.118	30, 030	35	41	38	38	46	42	42.0	46	33	. 165	. 192	. 177	r. 17
5 6	30. 296 30. 200	30. 099	30. 225 30. 010	30. 260 30. 103	29	53	39 46	31	60	42 49	36. 5 49. 0	49 60	26 32	. 137 . 144	. 310	. 199	. 16
7	29. 967	29.860	29.813	29. 880	48	62	58	51	68	62	60.3	68	44	. 29 6	. 476	. 429	. 40
8 9	29. 804 29. 835	29. 776 29. 802	29.779 29.804	29. 786 29. 814	57 54	64 58	58 56	61 57	69 64	61 60	63. 7 60. 3	69 65	57 52	. 412	. 529 . 403	. 443 . 396	. 39
10 11	29. 871 29. 858	29, 820	29.881	29. 876 29. 823	51		55	55		59	57.0	62	50	. 321		. 380	. 35
12	29.805	29. 775	29. 792 29. 727	29. 769	47 43	56 53	52 54	50 46	62 59	56 58	56, 0 54, 3	63	46 42	. 283 . 238	. 369 . 323	. 365	. 30
13 14	29. 795 29. 942	29. 787 29. 967	29. 815 29. 977	29, 799 29, 962	46	51 50	47 49	49 48	57 55	53	53.0	57	45	. 271	. 295	. 244	. 27
15	30.082	30. 074	30. 072	30, 076	45 45	54	51	49	62	55 55	52. 7 55. 3	55 62	42	. 260 . 247	. 295 . 312		. 29
16 17	30. 100 29. 807	30. 015 29. 75 7	29. 851 29. 747	29. 989 29. 770	40 50	56 55	54 51	44 55	64 61	59 55	55. 7 57. 0	64 62	39	. 195	. 343	. 351	. 29
18	29. 844	29.860	29.818	29. 841	48	58	54	53	65	58	58.7	65	50 46	. 295 . 269	. 354 . 349 .	365	. 34
19 2 0	29. 810 29. 925	29. 791 29. 865	29. 771 29. 823	29. 791 29. 871	56 56	64 60	60 56	61	73 66	66 60	66. 7 62. 0	73 66	56 56	. 383 . 396	476		
21	29.772	29.675	29. 568	29. 678	58	67	61	63	77	70	70. 0	77	55	. 416	. 527 .	416	. 45
22 23	29. 401 29. 540	29. 418 29. 512	29. 534 29. 513	29. 451 29. 522	62 47	57	51 45	67 52	64 56	58 50	63. 0 52. 7	70 58	52 45	. 489 . 257	373. 262.	982	. 38
24	29. 563	29. 593	29.711	29. 622	40	42	38	44	47	42	44. 3	50	37	. 195	. 202 .	177	. 19
25 26	29. 899 29. 914	29. 956 29. 767	29. 959 29. 690	29. 938 29. 790	32 32	35	31 38	36 36	40 43	34 42	36. 7 40. 3	41	30 98	. 199 . 199	139.	139	. 15
27	29. 674	29.826	29. 974	29, 825	35	42	36	39	45	40	41.3	47	35	1, 159	. 228 .	160	. 313
28 29	30. 079 29. 725	29. 938 29. 591	29. 864 29. 473	29, 960 29, 596	35 47	40 51	39 55	38 50	44 54	43 57	41.7 53.7	44 57	34 38	. 165 . 283	195. 335.	407	. 362
30 31	29. 313 29. 561	29. 308 29. 658	29. 363 29. 792	39. 328 29. 670	51 40	47 40	41 36	55 44	50 45	48 41	51. 0 43. 3	58 47	44 36	. 321	283. 182.	236	. 240
Means	29. 830	29. 786	29. 796	29. 609				49. 7	58. 0	52. 9	53. 2			. 279	318.	298	297

No observations taken in November, the observer being sick.

the northern and northwestern lakes at Sackett's Harbor, New York.

	VAP	OR.				w	IND.					elo	oun udin	e ss.	tion, in d deci-	melted inches
Sat	Hum	idity. n == 1,0	100.		Direction om when		n	ocity iles, our.	, in per	t velocity,	Resultant direction.	(10:	sky. = sky ly o cast.) y en- yer-	of evaporation	ot of rain or r, in U. S. decimals.
7 a. m.	2 p. n.		Mean.	7 a. m.	2 p. m.	9 p. m.	7 P. III.	2 p. H.	9 p. m.	Resultant in miles, p	Resultan	7 a. m.	2 p. H.	9 р. ш.	Amount U. 8. mals.	Amount snow,
. 727 . 807 . 772 . 783 . 754 . 718 . 703 . 825 . 704 . 747 . 743 . 743 . 743 . 743 . 781 . 698 . 738 . 781 . 698 . 738 . 781 . 698 . 738 . 710 . 762 . 763 . 710 . 763 . 710 . 766 . 706 . 716 . 716 . 716 . 716 . 716 . 716	644 663 764 629 751 772 653 786 7731 752 747 7735 698 612 620 577 747 747 738 648 649 649 649 649 649 649 649 649 649 649	804 768 743 754 757 766 636 727 771 805 761 843 793 647 762 640 710 747 727 808 647 747 727 808 640 710 733 647 732 752 752 752 752 752 752 752 752 752 75	725 761 762 769 718 691 743 770 814 792 761 718 742 827 719 758 827 748 734 801 801 769 676 683 670 762 752 752 752 752 752 753	NE S S S S S S S S S S S S S S S S S S S	S	S	4 25 2 4 12 12 4 0 0 4 45 35 4 4 5 12 12 12 12 12 12 12 12 12 12 12 12 12	2 12 2 12 12 12 12 25 4 12 25 25 25 2 12 2 12	4 4 2 2 25 2 25 12 0 2 2 25 35 25 25 4 12 2 4 4 12 2 4 25 4 0 0 0 0 2 2	1.3 12.7 0.7 1.7 9.0 12.7 16.3 1.3 1.3 35.0 10.0 17.0 21.3 18.0 17.0 20.7 12.3 1.3 1.7 20.7 12.3 1.7	South	6 9 9 7 3 10 9 9 7 10 10 10 10 10 10 10 10 10 10 10 10 10	5 6 10 10 2 10 10 10 10 10 10 10 10 10 10 10 10 10	6 10 8 8 8 J 0 10 6 0 0 10 10 10 10 10 10 10 10 6 6 0 0 0 10 10 10 10 10 10 10 10 10 10 10		
. 754	. 702	. 743	. 735				ا —	<u></u>	<u> </u>	2.7	8. 61 W.	6.6	7.0	5.1		5.34
. 786 . 723 . 698 . 719 . 789 . 628 . 749 . 743 . 786 . 787 . 781 . 710 . 677 . 681 . 661 . 667 . 610 . 636 . 719 . 746 . 747	642 216 554 617 599 695 747 675 681 561 562 631 562 631 562 637 631 562 637 637 637 637 637 637 637 637 637 637	765 713 684 661 744 772 925 765 761 747 756 606 620 743 743 756 635 765 765 765 765 766 646 661 712 876 874 776 876 876 876 876 876 876 876 876 876	. 731 . 551 . 665 . 666 . 766 . 752 . 780 . 751 . 752 . 733 . 723 . 693 . 615 . 635 . 645 . 621 . 654 . 626 . 636 . 636 . 636 . 636 . 636 . 636 . 636 . 636 . 636 . 636 . 637 . 638 . 638 . 639	NE. S. S. S. NE. NE. NE. NE. NE. NE. NE. NE. NE. NE	W. S. S. S. S. S. S. S. S. S. S. S. S. S.	Calm. NE NE Calm. NE Calm. NE Calm. NE Calm. NE	2 12 35 2 2 2 12 2 2 2 4 2 2 2 4 4 4 4 5 5 4 4 3 5 5 4 12	2 12 25 25 4 4 4 12 12 25 12 2 4 4 13 15 5 25 4 35 5 25 25	0 2 4 4 2 0 2 0 0 12 12 4 2 2 25 45 25 35 4 4 4 35	0.77	N. 45 E. N. 48 E. S. 45 W. S. 44 W. S. 34 W. N. 45 E. N. 45 West N. 13 E. N. 45 E. S. 12 W. N. 12 W. N. 12 E. N. 12 E. N. 23 W.	35 10 80 00 84 45 55 77 60 00 99 40 10 10 10 10 10 10 10 10 10 10 10 10 10	0 0 6 10 10 10 10 10 10 8	0 10 0 0 3 0 0 0 0 3 9 4 4 0 0 0 0 8 0 0 5 5 5 3 0 6 9 2 10 10 10 10 10 10 10 10		

Reductions of the monthly meteorological register of the survey of

	Baroma	ter reduc	ed to tem	nose tu se		TI	MPE	RAT	re-	-FAH	Renhe	IT.			VAP	OR.	
Date.	of 32°	Fahr., ex inches a	pressed in	United	P	bul oint pora	oź	P		ire (r tem- of the	si	.	in		y, in (
	7 p. m.	2 p. ii.	9 p. m.	Mean.	7 p. m.	2 p. II.	9 p. B.	7 a. m.	2 p. m.	9 p. m.	Mean.	Maximum	Minimum	7 8. 10.	2 p. m.	9 p. m.	Meun.
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3	29.941	29.859	29. 730	29.843	28	41	38	31	47	44	40.7	47	26			. 151	
4	29. 574	29, 228	29.386	29. 396	44	49	43	48	54	47	49.7	54	39			. 225	
5	29. 839	29, 913	30.006	29, 919	40	42	36	44	47	39	43.3	47	35	. 195		. 173	
6	30.049	29. 939	29. 730	29, 906	36	38	38	40	41	41	40.7	41	34	. 160		. 190	
7	29. 744	29.756	29. 699	29. 733	42	44	40	45	48	43	45.3	48	36-	. 228		. 20	
8	29. 399	29. 122	29. 218	29, 246	45	50	44	49	54	49	50.7	54	35	. 247		. 2:3	
.9	29. 434	29. 447	29. 370	29. 417	35	33	30	38	36	34	36.0	44	29	. 165		. 121	
10	29. 451	29. 529	29. 581	29. 520	24	22	24	27	25	26	26.0	34	18	. 095		106	
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12 13	29, 697 29, 844	29. 816 29. 829 29. 830 30. 123 30. 154 30. 108 30. 196 30. 122 30. 178 29. 693 29. 453 29. 677 29. 442 29. 625 29. 460 29. 964 29. 841 29. 913			21	22 23	15	25 24	27	18	23.0	27	13	. 067		. 095 . 095	
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15	30. 217			ő	8	4	2	ii	7	06.7		- 8	. 021		. 018		
16	29. 886			6	14	16	8	18	19	15.0		- ö	. 034		. 056		
17	29.313			18	19	20	22	23	23	22.7	23	13	. 053		. 074		
18	29, 935	3 29, 816 29, 829 29, 830 3 30, 123 30, 154 30, 108 7 30, 196 30, 122 30, 178 6 29, 693 29, 453 29, 675 8 29, 442 29, 625 29, 460 9 964 29, 841 29, 913			18	23	25	22	28	30	26.7	30	15			075	
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20	30. 270	30. 350	30, 446	30, 355	- 5	- 3	-16	- 3	- i	-13	05.7	12	-19		. 015		Ü
21	30, 433	30, 128	30, 125	30, 229	-22	١ĭ	- ž	-20	4	- 1	05.7	7	-30			. 02	
22	29, 973	29, 778	29, 643	29, 798	22	32	33	27	37	39	34.3	40	- 5	. 061		. 110	
23	29. 378	29. 300	29, 005	29, 228	37	39	42	43	44	46	47.3	46	35			. 215	
24	29. 147	29. 205	29. 262	29, 205	38	38	34	42	42	38	40.7	46	34			. 144	
25	29, 410	29.480	29, 578	29, 489	32	32	28	35	34	30	33.0	36	25			. 130	
26	29. 683	29.668	29, 496	29. 612	27	28	29	30	31	31	30.7	31	23			. 137 .	
27	29, 263	29, 128	29. 136	29, 176	30	31	22	32	34	25	30.3	34	21			. 084 .	
28	29. 152	29. 212	29. 317	29. 227	13	15	12	17	17	15	16.3	23	11			. 041 .	
29	29, 626	29.711	29, 682	29.673	6	10	14	8	13	18	13.0		- 0	. 034		. 037 .	
30	29, 923	30, 000	30, 036	29, 986	10	13	15	14	17	18	16.3	18	7	. 023		. 052 .	
31	30, 001	29, 910	29.877	29. 929	12	19	21	14	23	24	20.3	24	9	. 053	. 058	. 07 9 .	06
	00.000	00.000	00, 600	00.800			-		00.0	<u></u>	07.2	—-;					16
(eans	29. 733	29.702	29. 689	29. 708		• • • •		22. 3	29. 6	20.9	27.3			. 099	. 115	105.	10

the northern and northwestern lakes at Sackett's Harbor, New York.

	VAP	OR.				w	IND.					clo	nount udine == cle	88.	oration, in and deci-	melted
Sa	Humi turation		00.		Direction om when		m	ocity iles, er ho	per	Resultant velocity, in miles, per hour.	Resultant direction.	(10= tire	sky.) = sky ely ov cast.)	en-	of evap	of rain or ', in U. S. decimals.
7 a. m.	2 p. m.	9 р. ш.	Мевп.	7 a. m.	2 p. m.	9 p. m.	7 a. m.	2 p. m.	9 p. m.	Resultar in miles,	Resultan	7 в. ш.	2 p. m.	9 p. m.	Amount U. S. mals.	Amount . snow, and de
.655 .569 .704 .677 .762 .762 .645 .543 .549 .480 .533 .449 .480 .615 .511 .688 .679 .679 .679 .530	768 698 554 674 738 705 622 622 622 529 418 371 467 755 712 623 655 712 433 350 651 712 433 353 661 433 353 712 433 651 712 651 438 651 712 712 712 712 712 712 712 712 712 71	556 610 522 698 736 638 617 754 520 644 520 541 520 463 463 463 463 463 463 463 463 463 463	660 696 587 692 692 695 680 683 578 589 424 428 438 449 449 479 479 777 716 709 454 499 4565	W	W. S. S. S. S. S. S. S. S. S. S. S. S. S.	Wswsssswswssswssswssswssswswsssswsssswsssswssssw	12 2 2 12 12 12 12 12 4 4 45 25 4 4 4 0 2 25 4 4 25 4 4 5 12 2 2	12 4 4 4 5 4 5 5 4 5 5 1 2 2 2 12 4 4 0 12 12 0 2 5 12 4 4 4 5 5 4 5 1 2 2 5 4 5 5 1 2 2 5 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	4 4 12 12 2 35 35 25 4 4 25 0 2 35 35 2 4 25 5 2 2 35 35 2 4	38. 3 28. 3 12. 7 1. 2 1. 7 2. 0 16. 3 1. 0 6. 0 15. 7 28. 3 18. 0 12. 3 8. 7 10. 7 8. 7	S. 7 W. South South S. 57 W S. 83 E. S. 63 W S. 17 W West South N. 84 W Worth N. 84 E. N. 39 E. S. 39 W N. 45 E. N. 45 E. N. 45 E. N. 45 E. N. 45 E. N. 45 E. N. 45 E. N. 45 E. N. 45 W N. 45 W N. 45 W N. 43 W N. 43 W N. 43 W N. 45 W N. 43 W N. 45	6 10 1 10 9 10 10 9 9 10 9 9 10 10 10 10 10 10 10 10 10 10 10 10 10	3 10 0 10 8 10 10 10 10 10 10 10 10 10 10 10 10 10	8 1 4 10 0 10 0 10 6 0 10 4 3 10 10 10 10 10 10 10 10 10 10 10 10 10		. 05 . 16 . 15 . 66 . 99 . 00 . 33 . 22 . 19 . 10 . 00 . 00
. 635	. 467	. 610	. 571	8	E	Calm	2	2	<u> </u>	1.0	8. 45 E . 8. 58 W.	8	8.3	6. 4		2.

SUPERIOR CITY, WISCONSIN.-ALTITUDE 640 FEET.

	TABLE	t B.—Ka	zimam, 1	alaimam,	and mes	u barom	etrical p	ressare fo	B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1866 inclusive, reduced to 33º Fahr. + 38 inches.	onth froz	n the year	u 1859 to	1866 Inc	lustve, re	duced to	32° Fahi	+ \$8 tr	choe.
Year.	,	annaary.	•	F 4	February.			March.			April.			May.			June.	
	Nex.	렱	Kean.	Ker.	Min.	Mean.	Max.	Min.	Mean.	Max	Min.	Mean.	Ker	Kin.	Mean.	Mer	Min	Mean.
1889 1861 1861 1862 1864 1865	1, 994 1, 884 1, 834 1, 734 1, 689 9, 371	0.547 0.973 0.973 0.582 0.634 0.952 1.013	1.873 1.336 1.418 1.866 1.879 1.350 1.479	1. 708 1. 668 1. 717 1. 916 1. 802 1. 869 1. 845	0.640	1, 289 1, 192 1, 351 1, 423 1, 205 1, 386 1, 386	1, 736 1, 940 1, 630 1, 781 1, 780 1, 709 2, 018	0. 367 0. 723 0. 723 0. 709 0. 709 0. 706	1, 941 1, 263 1, 263 1, 335 1, 225 1, 280 1, 464	1, 793 1, 798 1, 769 1, 769 1, 705 1, 750	0.400 0.657 0.831 0.618 1.114 0.245 0.614	1, 222 1, 182 1, 376 1, 351 1, 362 1, 369 1, 269	1,486 1,762 1,686 1,503 1,503 1,503 1,503	0. 555 0. 954 0. 954 0. 956 0. 728 0. 721 0. 731	1, 261 1, 231 1, 331 1, 331 1, 244 1, 261 1, 261	1, 597 1, 535 1, 535 1, 673 1, 600 1, 637 1, 633	0.710 0.903 0.910 0.968 0.964 0.841	1. 244 1. 336 1. 336 1. 331 1. 230 1. 73
		July.			August.		ž	September.			October.		Z	November.		A	December.	
	Ker	ģ	Mean.	Ker.	ğ	Mean.	žė ,	ģ	Mean.	Ķ	Kb.	Mean.	Ķ	ġ	Keen.	Ker	Kir.	Moan.
1890 1861 1862 1863 1865 1865 1866	1.680 1.578 1.578 1.765 1.765 1.775 1.775	0.0892 0.0867 1.190 0.928 0.928 0.914	1111111 8332246 8432244	1 1 500 1 1 500 1 1 500 1 1 500 1 1 500 1 1 500 1 1 500	0.0991 0.056 0.056 0.055 0.057 0.057 0.057	11.363	1,742 1,816 1,826 1,734 1,711 1,711 1,711	0.000 0.000	4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1, 1958 1, 858 1, 9, 040 1, 9, 036 1, 1, 973 1, 745 1, 745	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1 1 256 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1, 852 1, 684 1, 730 1, 730 1, 739 1, 739 1, 739	0000000 8888 8888 777 772 8888 8888 8888	1111111 1930 1930 1930 1930 1930 1930 19	41:19:008 9:00:00:00:00:00:00:00:00:00:00:00:00:00	222222 8888 80887 808 808 808 808 808 808 808	1.386 1.386 1.388 1.281 1.281

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			1. 327) 1. 544 1. 724 1. 355 1. 355 1. 315 1. 487
		Moen.	1. 276 1. 276 1. 324 1. 328 1. 308 1. 318
Pond.	Year.	Min.	0.585) 0.671 0.671 0.541 0.541
., for sea	•	Max.	1. 852) 1. 944 1. 963 1. 963 1. 973 1. 856 1. 856 2. 371
TABLE B.—Maximum, minimum, and mean barometrical pressure reduced to 35º Fabr., for seasons.		Mean.	1,336 1,336 1,337 1,337 1,367
educed to	Winter.	Min.	0.547 0.513 0.748 0.547 0.541 0.541
ressure r		Max.	2 003 1, 884 1, 983 1, 983 1, 894 2, 371
netrical p		Meen.	1.335 1.339 1.339 1.339 1.361 1.361 1.363
en beron	Automa.	Min.	0.000 0.000 0.000 0.000 0.000
n, and m	7	Max.	11.852 11.852 11.853 11.873 11.759 11.880 800 11.890
minimus		Mean.	1, 253 1, 290 1, 290 1, 305 1, 305 1, 312 1, 253
eximum,	Summer.	Min.	0.855 0.855 0.855 0.855 0.855 0.878
LE B.—M	_	Max.	1.680 1.600 1.671 1.671 2.657 1.765 1.765 1.765
TAB		Mean.	1, 206 1, 236 1, 330 1, 377 1, 365
	Spring.	Min.	0.400 0.250 0.723 0.723 0.700 0.945
		Max.	1.733 1.940 1.738 1.738 1.730 1.878 1.878
	Year.		1859 1860 1961 1963 1963 1964 1965

SUPERIOR CITY, WISCONSIN.

			F	TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1966 inclusive.	-Maximu	n, minim	um, and	mean ten	nperatur	o for each	h month	from the	year 185	9 to 1866	inclusive	٠,		
Year.	7	January.		F 4	February.			March.			April.			May.			Julie.	
	Max.	jg Kj	Mean.	Max	Ma.	Mon.	Max	Min.	Mosn.	Ker	Kli	Moan.	Kax	녗	Megn	ž	Min.	Mean.
1839 1860 1861 1863 1864 1865 1866 45 45 45 45 45 45 45 45 45 45 45 45 45	588332 3	8441588	12.9 12.9 12.9 12.9 10.0 10.0 10.0 10.0	\$\$\$ 4 \$\$\$	5 555555	25.5.5.6.6.6.6.6.6.6.6.6.6.6.6.6.6.6.6.6	8248828	######################################	2888825 8004800	835588	5485845	7. % % % % % % % % % % % % % % % % % % %	588888	2882288	47.9 51.9 47.8 47.8 3	288288	888888	222223

SUPERIOR CITY, WISCONSIN.

			F	ABLE B.	-Mardmi	ım, minis	TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1866 inclusive.	l mesn te	mperatu	e for eac	h month	from the	year 185	9 to 1866	inclusive	٠			'
Year.		July.			August.		ž	September.			October.		z	November.		Ω	December.		
	Max.	폋	Mean.	Max.	Min.	Mean.	Max	Min.	Мевп.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	15.
880 880 880 880 880 880 880 880 880 880	8621183	1838363	කුලු කුළු කුළ ල සමන ට මෙන බ	288888	54848488	5.05 05.05 5.05 05.05 7.7.05 1.00 05.05	128888285	2888828	8883888888 88886488	200 2 20 20 20 20 20 20 20 20 20 20 20 2	22 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1444444 146466444 466166444	÷82222882	0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	୧୦-୩୯୮୭୩ ଅଷ୍ଟ୍ରିଅଷ୍ଟ୍ରମ	8844448	E848888	21.1.1.2.2.1.1.1.2.2.1.1.1.1.1.1.1.1.1.	DIOMI OF III

SUPERIOR CITY, WISCONSIN.

RET.	ARY	OF	WAR.
	Renge		82 22 22 22 22 22 22 22 22 22 22 22 22 2
٠		Mean.	488848 488848
inclusive	Year.	Min.	######################################
9 to 1866		Max.	882888
from 185		Mean.	10,7 12,1 12,0 17,8 17,8
nd years	Winter.	Klp.	577777
eallons a		Max.	\$ 4 1 4 2 4 4
TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1866 inclusive.		Mean.	114444444 224444
tempera	Autumo.	Min.	2777820 20
and mean		Max.	28888888
olmum, (Mean.	65.2.2.6.2.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0
mum, m	Summer.	Kin.	4 2 2 3 3 2 8
B.—Max		Max.	&&&&&&&&
TABLE		Mean.	833888
	Spring.	4	הַּהְּרְהְרִּ
		Max.	E # 8 8 8 8 8
	Yes.		1869. 1960. 1961. 1963. 1963. 1866.

A			TAB	LE B.—	Amount c	frain an	d melted	snow for	months,	seasons,	and year	, in Unk	led States	TABLE B.—Amount of rain and melted snow for months, seasons, and years, in United States inches and decimals.	lecimals.		
į	Jan	Feb.	Mar.	April.	May.	June.	July.	Aug.	Sept.	Oct.	Nov.	Dec.	Spring.	Summer.	Autumn.	Winter.	Year.
1859 1980 11861 11861 1186 1186 1186 1186 1186	:-:-:000 8588888	1499999 828888	11110414 8825498	14415949 18884585	74-04-14-14-14-14-14-14-14-14-14-14-14-14-14	1.0.90.1.4.9 4.5.50.00.00.00.00.00.00.00.00.00.00.00.0	0441444 8248384	0444444 83522222	44444444444444444444444444444444444444		60900100 8080100 80801000	666644468	7.044499 825827989	8007.50 588858	44.17.44.4 44.26.22.28.3	এ এ এ এ এ এ এ ৫ ৯ ৯ ৯ ৯ ৫ ৫ ৯ ৯	2828388 2828388

ONTONAGON, MICHIGAN.-ALTHUDE 610 PEET.

	TABL	R B.—M	aximum,	LE B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1865 inclusive, reduced to 39º Fabr. + 29 inches	ı, and me	en beron	netrical p	ressure fo	or each m	onth from	n the year	r 1859 to	1865 incl	usive, re	fuced to	32º Fahr.	+ 28 in	spee.	
Year.	·	January.		P4	February.			March.			April.			May.			Jane.		
	Max.	Min.	Mean.	Max.	Mtb.	Mean.	Max.	Min.	Mean.	Max	Min.	Mean.	Max.	Mîb.	Mean.	Max.	Mh.	Mean.	•
1899 1.893 1.893 1.893 1.893 1.893 1.893 1.893 1.893 1.893 1.893 1.775 1.893 1.775 1.893 1.775 1.893 1.775 1.893 1.775 1.893 1.775 1.893 1.775 1.775 1.893 1.775 1	1,882 1,884 1,787 1,775 1,775	0, 579 0, 715 1, 030 0, 742 0, 583	1, 303 1, 318 1, 378 1, 213 1, 245 1, 245	1, 761 1, 730 1, 699 1, 946 1, 719	0, 444 0, 527 0, 747 0, 567 0, 652 0, 817	1, 299 1, 224 1, 352 1, 438 1, 161 1, 375	1, 757 1, 929 1, 678 1, 678 1, 668	0, 591 0, 502 0, 602 0, 824 0, 711 0, 639	1. 326 1. 325 1. 271 1. 363 1. 192	1. 859 1. 655 1. 773 1. 775 1. 690 1. 690	0. 492 0. 739 0. 997 1. 078 0. 311	1.235	1, 514 1, 597 1, 722 1, 593 1, 506	0.627 0.214 1.035 0.516 0.728 0.688	1. 263 1. 263 1. 233 1. 233 1. 229	1. 680 1. 602 1. 627 1. 732 1. 732 1. 534	0. 412 0. 949 0. 841 0. 841	1, 288 1, 288 1, 337 1, 309 1, 318	

ONTONAGON, MICHIGAN.—ALTITUDE 610 FEET.

	TABL	E B.—M	skimum,	B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1965 inclusive, reduced to 32º Fahr. + 28 inches.	, and me	an baron	netrical pr	ressure fo	or each m	onth fron	n the yes	r 1859 to	1865 incl	usive, re	duced to	32° Fahr	. + 28 is	ches.	,
Year.		July.		•	Angust		82	September.			October.		Ž	November.		A	December.		
	Max	Ķ	Mean.	Max.	Kib.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Moan.	
1839 1 689 1 689 1 689 1 689 1 689 1 575 1 575 1 548 1 575 1 548 1 574 1 545 1	11111	0.648 1.013 1.024 1.022	1. 257 1. 273 1. 237 1. 316 1. 355	1. 568 1. 635 1. 605 1. 568 1. 568	1. 630 0. 927 1. 058 0. 892 0. 987 0. 987	1.308 1.308 1.307 1.315	1.846 1.846 1.786 1.786 1.573	0. 812 0. 747 0. 667 0. 852 0. 977	1.363	1. 927 1. 927 1. 963 1. 963 1. 937	0.845	1.367 1.365 1.891 1.276 1.346	1.99 1.753 1.753 1.871 1.871	0.988 0.988 0.673 0.673	1. 216 1. 207 1. 289 1. 282 1. 282	1. 708 1. 923 1. 962 2. 017 1. 857 1. 730	0, 856 0, 781 0, 745 0, 605 0, 605	1, 339	BIOMI OF

ONTONAGON, MICHIGAN. + 28 INCHES.

			TABLE	t B.—Ma.	zimum, 1	minimom	TABLE B.—Maximum, minimum, and mean barometrical pressure reduced to 35º Fabr., for seasons.	un berom	etrical pi	restare re	duced to	32° Fahi	., for sea	Sons.		
Tour	_	Spring.		<i>-</i> 22	Sammer.			Autamn.			Winter.			Your		Renee
	Max.	Min	Mean.	Ker	Kin.	Mean.	Max.	Mh.	Mean.	Ker	Min.	Moen.	Max.	Mîn.	Moan.	
1859- 1860- 1861- 1863- 1864- 1864- 1864-	1, 856 1, 929 1, 738 1, 878 1, 680	0.498 1.286 0.214 1.774 0.600 1.333 0.600 1.346 0.711 1.266	1,286 1,274 1,373 1,346 1,256	1. 0669 1. 752 1. 752 1. 758	0,00,0 113 247 247 247 277 277	1, 279 1, 308 1, 344 1, 313	1. 663 0, 413 1, 379 1, 640 0, 385 1, 315 1, 676 1, 578 1, 578 1, 640 0, 385 1, 315 1, 678 1, 578 1,	0,085 0,385 0,708 0,448 0,314	1. 376 1, 315 1. 249 1. 331 1. 326 1. 246	1. 893 1. 923 1. 868 1. 857 1. 784	0. 444 0. 587 0. 745 0. 567 0. 568	1.314 1.308 1.367 1.367 1.869 1.850	1.991 1.923 1.929 1.929 1.946 1.790	0.085 0.385 0.914 0.443 0.314	1, 308 1, 382 1, 323 1, 338 1, 258	11111111111111111111111111111111111111

·			F	ABLE B.	-Maximu	m, minin	pus 'ann	mean te	TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1865 inclusive.	re for eac	h month	from the	year 180	19 to 1865	inclusiv	å		
Year.		January.		ř.	February.		. •	March.			April.			May.			June.	ı
	Kex.	jg K	Mean.	Max.	Kin	Moan.	Max	Kla	Mean.	Max	Min	Mean.	Max.	Ę	Mean.	Max	Kib.	Mean.
1859 1860 1861 1862 1863 1864	388 32 8	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	15.8 16.8 19.8 14.8 14.8	828243	1 1 1 1 1 1 1 1 1 1	14.2 18.1 18.1 17.6 19.9	244488	6 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	688888	443884	21 11 11 12 8 12 7	37.8 24.7 35.8 35.8 35.8 35.8	8 4 8 8 8	200000	53.0 46.0 51.6 49.5 48.8	88888	******	60 50 50 50 50 50 50 50 50 50 50 50 50 50
		July.			August		æ	September.			October.		Z	November.		А	December.	
	Mer	Kin.	Kogn.	žį,	Ą	Moan.	Max.	į,	Mean,	Max.	Ę,	Meen.	Max.	Min	Mean.	Max.	Min.	Mean.
1859 1860 1861 1861 1863 1864	**************************************	\$222	62.7 64.7 61.4 67.0	88888	38181	88.6 61.7 61.9 61.9 5.5	88882 8	88888	57.00 57.00 57.00 57.00 57.00 57.00	882238	25 25 26 26 26 26 26 26 26 26 26 26 26 26 26	114184 21550	824488	ๆ 7 🛪 ๆ ๆ	99.89	884844	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	19.29.29.19.19.29.29.29.29.29.29.29.29.29.29.29.29.29

ONTONAGON, MICHIGAN. + 28 INCHES.

			TABLE I	3.—Maxlı	num, mh	olmum, a	nd mesn	temperat	ure for se	sasons an	d years !	rom 1859	TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1865 inclusive.	nclusive.		
Year.		Spring.			Summer.		,	Autumn.		-	Winter.			Year.		90 80
,	Ker	Min.	Mean.	Max.	ij	Mean.	Max.	Min	Mean.	Ker	Kln.	Mean.	Max.	Ę,	Mean.	
1859. 1860. 1861. 1862. 1865. 1864.	82828	0 15 13	45.88 88.88 88.88 88.88	88.24.88	2 2 8 8 8	88 42 61.8 85 95 33 62.0 88.5 95 30 60.4 88 95 30 60.4 89 96 50 60.4 89	8.888822 2	21-2-1 2-1	4.24 4.24 4.04 4.04 8.84 8.84	332848	4547 7 7	14.2 18.1 15.2 21.6 19.4	88.24.88	98 - 31 +0.3 98 - 31 +0.3 98 - 31 +0.3 98 - 38 6 98 - 40.6		EPORT OF T

ONTONAGON, MICHIGAN.-ALTITUDE 610 PRET.

į			T	BLE B.—	Amount	of rain a	nd melter	d snow fo	or months	, невиопя	, and yea	ers, in Ur	ited State	TABLE B.—Amount of rain and melted snow for months, seasons, and years, in United States inches and decimals.	d decimals.		
i	Jan.	¥eb.	March.	March. April.	May.	June.	July.	Aug.	Sept.	Ost.	Nov.	Dec	Spring.	Summer.	Summer. Autumn.	Winter.	Your.
1859 1860 1860 1860 1860 1860 1860 1860 1860	역 4 역 1 년 리 4 영 2 월 8	1111419 528888	20 9 9 9 9 18 27 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	0.46 0.094 0.09 0.17	44411944 878282	1.1.1.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2	1.90.944 89.09.88	991.441 883.6208	91.44441 85586528	149991 582282	11-140-04 528-64-53 16-64-53	4.19409 4.88699 4.886999	588498 588498	2.36 2.36 1.75 3.26 1.39 1.84 3.76 4.88 5.33 2.99 1.16 3.18 2.54 2.31 2.86 8.69 9.10 8.03 2.35 4.01 2.77 3.29 0.07 3.29 3.06 8.17 3.89 3.06 3.36 8.17 3.89 3.06 3.06 3.06 3.06 3.06 3.06 3.06 3.06	수요록시작요 유원요광광환	자주 역 역보다 도염성등육공	200233 20088

MARQUETTE, MICHIGAN.—ALTITUDE 634 PERT.

	TABL	B.—Ma.	rimam, 1	alpinum,	and me	in barom	etrical pa	ressure fo	or each m	onth fro	n the yes	ır 1859 to	1866, inc	thaive, n	educed to	32º Fah	B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1866, includive, reduced to 35º Fahr. + 38 inches	ohes.
Your.	7	anuary.		Ñ.	February.			March.			April.			May.			June.	•
	Max.	Kin.	Mean.	Max.	Klp	Moan.	Max.	Min.	Mean.	Max.	Ę	Mean.	Max.	XIII.	Mean.	Max.	d M	Mean.
885 886 888 888 886 886 886 886 886	1.782 1.962 1.912 1.706 1.696 2.612 2.612	0.552 0.875 0.698 0.548 0.737 0.730	1.216 1.288 1.307 1.243 1.165 1.810 1.334	1. 648 1. 648 1. 678 1. 921 1. 780 1. 780	0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000	1, 262 1, 195 1, 195 1, 424 1, 330 1, 330	1, 715 1, 957 1, 857 1, 807 1, 560 1, 560 1, 601 1, 657	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1, 224 1, 315 1, 315 1, 347 1, 153 1, 171 1, 171	1.848 1.754 1.689 1.757 1.628 1.674 1.546	0. 507 0. 713 0. 658 0. 721 0. 382 0. 382 0. 382	1,280 1,347 1,347 1,366 1,289 1,289 1,280	1,580 1,750 1,534 1,534 1,530 1,530	0.721 0.9284 0.955 0.955 0.0655 0.0655	1.216 1.246 1.296 1.292 1.171 1.193	1.5591 1.558 1.620 1.734 1.571 1.674	0.787 0.745 0.863 0.619 0.795 0.748	1, 256 1, 256 1, 256 1, 256 1, 194
		July.			August		Š	September	•		October.		Z	November.		H	December.	
	Max	Kip.	Moan.	Max.	· dj	Mesn.	Max.	Min.	Moan.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.
1850 1860 1861 1863 1864 1866	1, 676 1, 623 1, 623 1, 487 1, 547 1, 595 1, 475	0.784 0.700 0.967 0.960 0.968 0.968 0.968	1,350 1,250 1,250 1,306 1,306 1,315 1,315	1, 608 1, 603 1, 663 1, 573 1, 574 1, 480 1, 592 1, 656	1. 055 0, 956 1. 029 0. 904 0, 756 0, 964 0, 935	1, 383 1, 284 1, 259 1, 276 1, 270 1, 211 1, 339 1, 376	1, 726 1, 842 1, 861 1, 737 1, 744 1, 493 1, 642 1, 594	0.766 0.703 0.772 0.941 0.884 0.739 0.739	1, 335 1, 347 1, 309 1, 314 1, 340 1, 196 1, 291 1, 246	1.668 1.833 1.685 1.850 1.850 1.475 1.475	0.587 0.696 0.641 0.672 0.500 0.675 0.621	1. 332 1. 357 1. 253 1. 255 1. 329 1. 167 1. 342 1. 362	1, 659 1, 659 1, 655 2, 157 1, 777 1, 656 1, 972 2, 002	0.480 0.355 0.405 0.842 0.708 0.582 0.433 0.433	1, 314 1, 192 1, 193 1, 380 1, 380 1, 138 1, 138 1, 250 1, 263	1. 687 1. 953 1. 881 1. 975 1. 782 1. 740 1. 660 1. 680	0. 825 0. 753 0. 663 0. 820 0. 387 0. 391 0. 692	1.354 1.354 1.354 1.354 1.134 1.236

MARQUETTE, MICHIGAN. + 28 INCHES.

			TABLE	B.—Ma	cimam, n	olnimum	TABLE B.—Maximum, minimum, and mean berometrical pressure reduced to 33º Fabr., for seasons.	n berom	etrical pr	ossure re	duced to	39º Fah	., for sea	Sons.		
Year.	-	Spring.		30	Sammer.		•	Autama,			Winter.			Year.		
	Ker	녗	Kean.	Max	Ą	Moan.	Mex.	Kla	Moan.	Max.	Min.	Mean.	Max.	Kin	Mean.	
1859 1860 1861 1863 1863 1864	1 1 857 1 857 1 857	2000000 200000000000000000000000000000	25 25 25 25 25 25 25 25 25 25 25 25 25 2	1. 565 1. 565 1. 565 1. 565	0.745 0.745 0.748 0.748 0.748	1.286 1.286 1.286 1.286 1.286 1.296 1.200	11.42.14.14.15.15.15.15.15.15.15.15.15.15.15.15.15.	2222222 \$8.50 \$2.5	11111111 200212 20021 200212 20021 200212 20021 200212 20021 200212 20021 200212 20021	1.953 1.953 1.913 1.975 1.780 1.780	0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000	1.270 1.277 1.314 1.314 1.191 1.212 1.212	1,953 1,957 1,957 1,921 1,740 1,939 4,21	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.288 1.288 1.313 1.179 1.179 1.258 1.258	1. 598 1. 673 1. 623 1. 428 1. 650 1. 867

MARQUETTE, MICHIGAN.—ALTITUDE 634 FEFT.

			H	TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1866, inclusive.	-Maximu	m, minin	um, and	mean te	mperatur	e for eacl	h month	from the	year 185	9 to 1866,	inclusive			į
Tour.	7	January.		F4	February.			March.			April			May.			June.	
	Max	Ę	Mean.	Max.	ij	Moan.	Max.	Min.	Moan.	Max.	Min.	Mean.	Max.	Mh.	Mean.	Max.	Min	Mean.
1859 51 1860 51 1861 53 1863 77 1863 45 1864 47 1866 45	2823448	1 1 1 1 1 1 1	11.00 11.00	5884 4488	ដូនដូន	17.28 19.08 19.08 19.08 19.08 19.08	8727883	2507777	ង	*38288	37 c 07 c 00 c 00 c 00 c 00 c 00 c 00 c	F 8 8 14 F 8 8 8 0 0 2 4 2 8	88888 2 8	893836	85.00 85.00 85.17.7 1.17.00 1.10.00 1.	88889 20 23 23	******	60.00 60.00 60.00 61.7 61.7 61.7

MARQUETTE, MICHIGAN.—ALTITUDE 634 FEFT.

•			£	ABLE B.	Maximu	m, while	um, and	mean to	TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1966, inclusive.	e for eac	h month	from the	year 185	to 1866	Inclusiv			
		July.		·	Angust.		3 2	September.			October.		×	November.		A	December.	
	Max.	Min.	Meen.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min	Mean.	Max.	Min.	Mean.	Max.	Kin.	Meen.
1859 1960 1961 1962 1963 1964 1965	8888898 8888898	4 #88#484	88889984 08888918	£2828282	388885 <u>4</u> 3	8 - 8 - 8 - 8 - 8 - 8 - 8 - 8 - 8 - 8 -	22688888	8882283 2	រដ្ឋនិង្គង់នុង 	88555828	88855888	14441444	2848 P82	2000 000	ងដូងង្គដូងង្គ 800-17-498	882222	1	4.444444444 6660000000000000000000000000

MARQUETTE, MICHIGAN. .

		e e	41.0 41.0 42.1 42.1 42.1 43.3 43.3 118.5 40.8
ø		Moen	1131444
, inclusiv	Year.	Min.	789975
to 1866		Max.	នឧដ្ឋឧដ្ឋ
bom 185		Mosn.	20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00
d years	Winter.	Min.	รีซีซีซีซี รี รี ซี
an success		Max.	2822744
are for se		Mean.	44444434 20484812
emperat	Autumn.	Mb.	
d mean	,	Max.	8288838
TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1986, inclusive.		Moan.	
um, min	Summer.	Min.	*****
—Maxin		Max.	88 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
CABLE B.		Moan.	14828884 8200088
E'	Spring.	Kip.	1757
		Ker	88888 28
	Your,		1859. 1860. 1861. 1863. 1864. 1864. 1866.

Marquette, Michigan.—Altitude 634 feet.

									-								
Year.	Jan.	Feb.	Mar.	April	May.	June.	July.	Aug.	Sept.	Oet.	Nov.	Dec.	Spring.	Summer.	Autumn.	Winter.	Year.
6381							3.26	3,45	25	1. 42	ا ا	2.24	\Box :		96.8		
1860	1.41	8	8	2	98	2.89	0.83	2.13	1.65	25	2,35	6	6.80	5. 20.	5.90	6.55	25, 73
1961	4.16	4.57	1, 15	e4 53	3 25	7. 11	2. 47	33.	۶. ج	2, 10	3.35	96 0	괁	11, 10	10,87	11.67	86.98
1862	8 8	96.0	1.38	ස ස	24	7.5	2, 75	ස ස	4. Si	3.07	2, 10	25.03	5	7.92	9, 49	3,95	36.35
1963	1.96	3.65	28.	1	1.21	2, 60	20.	٠. ج	69 68	4.38	S S	3,06	14)	14. 18	9,09	5.64	63.08
1864	£.	1.87	1.14		55	0,31	1.95	1.73	8	2 17	4.24	86 61	8	රි ස්	8, 70	6.73	22.
1865	1.00	5.6	3 38	3,93	9 83	86 ෆ්	3 36	1.11	1.01	1.84	0.59	1.88	17	8.35	3, 44	5, 97	88.98
998	23	1.21	1.97	1,75	Q 78	4.36	නු ප්	28	2 15	86	200	1.7	33	10,90	8,74	4.63	29.49

THUNDER BAY ISLAND, MICHIGAN.—ALTITUDE 614 FEFT.

	TABLE	8 B.—M4	vzimum,	ILE B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1839 to 1965 inclusive, reduced to 33º Fahr. + 28 inches.	, and me	an baron	otrical p	ressure f	ог евсh п	nonth fro	n the yea	r 1859 to	, 1965 incl	usive, re	duced to	32º Fah	. + 288 In	ches.
Year,		January.		Pi	February.			March.			April.			May.			June.	
	Max.	Min.	Moan.	Max.	Min.	Mean.	Max.	Min.	Moen.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Mln.	Mean.
1850. 1860. 1861. 1862. 1863.	1. 864 1. 874 1. 979 1. 885 1. 893 1. 826	90000000000000000000000000000000000000	111111111111111111111111111111111111111	1. 751 1. 750 1. 746 1. 992 1. 623 1. 681	0,578 0,573 0,573 0,800 0,841	1. 337 1. 327 1. 372 1. 372 1. 196 1. 156	1. 683 1. 683 1. 683 1. 684 1. 765 1. 777	0.050 0.050 0.050 0.050 0.050 0.050 0.050	1. 130 1. 262 1. 248 1. 291 1. 375 1. 252 1. 252	1. 776 1. 964 1. 730 1. 707 1. 763	0.558 0.481 0.756 0.534 0.981	1. 285 1. 309 1. 287 1. 435 1. 363 1. 366	1.948 2.17.11 2.005.11 5.000 5.47.11	0.867 0.876 0.530 0.928 0.761 0.761	1. 238 1. 238 1. 238 1. 238 1. 288 1. 288	1. 694 1. 643 1. 735 1. 735 1. 799 1. 608	0, 991 0, 732 0, 675 0, 677 0, 746 0, 956	1, 367 1, 239 1, 319 1, 315 1, 430 1, 568

	TABLI	H	aximum,	B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1865 inclusive, reduced to 35° Fahr. + 28 inches.	ı, and me	an baron	netrical p	ressure f	or each m	onth fro	n the yes	ır 1859 to	1865 inc	lusive, re	duced to	32º Fahr	r. + 28 tr	1ches.
Your.		July.			August.		ž	September.)	October.		Z	November.		Q	December.	
	Ker	į	Moan.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.
1850 1960 1961 1965 1965	1441144 866 866 866 866 866 866 866 866 866 8	0000110 863883 86386 8638 86386 86386 86386 86386 86386 86386 86386 86386 86386 86386 8636	4444444 \$123388	444444 8688 8689 8689 8689 8689 8689 868	1. 161 0. 947 0. 947 0. 9683 0. 9683	1. 351 1. 353 1. 355 1. 355 1. 355 1. 355	1. 839 1. 834 1. 796 1. 840 1. 684	0, 753 0, 939 1, 053 0, 872 0, 939 1, 171	1, 384 1, 414 1, 393 1, 392 1, 358 1, 447	1. 688 1. 776 1. 747 1. 853 1. 950 1. 666 1. 815	0.979 0.722 0.668 0.771 0.873 0.873	1.336 1.336 1.335 1.334 1.396 1.396	1.967 1.771 2.171 2.129 1.866 1.780	0, 557 0, 506 0, 744 0, 813 0, 772 0, 421	1.423 1.218 1.261 1.376 1.300 1.290	9 069 1. 956 1. 917 2. 067 1. 977 1. 941	0.969 0.630 0.941 0.705	1, 406 1, 400 1, 450 1, 243 1, 243 (1, 383)

THUNDER BAY ISLAND, MICHIGAN. + 28 INCHES.

	,		TABLE	TABLE B.—Maximum, minimum, and mean barometrical pressure reduced to 32º Fabr., for sessons.	ximum,	mpalala	, and me	an baron	etrical p	ressure r	duced to	32° Fah	., for sea	sons.		
Tear,		Spring.		22	Summer.		,	Autumn.			Winter.			Year.		
	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Kange.
1869 1960 1960 1963 1964 1964	1. 879 1. 984 1. 987 1. 916 1. 916 1. 763 1. 880	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1. 292 1. 286 1. 309 1. 354 1. 370 1. 295 1. 336	1.824 1.683 1.686 1.735 1.735 1.739	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.375 1.334 1.387 1.389 1.407	1. 967 1. 834 1. 903 1. 950 1. 750 2. 070	0.557 0.556 0.668 0.771 0.421 0.658	1.385	2 069 1.979 1.977 2 067 1.977 1.941	0.573 0.573 0.573 0.573 0.540 0.640	1.333 1.333 1.333 1.346 1.346 1.349	2 069 11.974 2 120 11.992 11.941	0.050 0.481 0.530 0.534 0.421	1.356 1.321 1.323 1.323 1.375 1.375 1.312 (1.389)	2, 019 1, 503 1, 449 1, 429 1, 428 1, 520

THUNDER BAY ISLAND, MICHIGAN.

			£4	TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1865 inclusive.	-Maxim	in, minh	num, and	mean t	emperatu	re for eac	h month	from the	year 185	9 to 1865	inclusive			
Year.		January.		Pa	February.			March.			April.			May.			June.	
	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min	Mean.	Max.	Min.	Mean.	Max.	Min.	Moan.	Max.	Min.	Mean.
1839 1860 1862 1863 1863 1864	848984 <u>8</u>	4014074	8858285 8466748	252225	1	8899848 8000000	######	11.000 11	8.5.3.8.8.8 6.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0	\$\$\$ 25 \$ 3	8585285	4444444 444444 444444	7825828	288588	4444 4444 6444 644 644 644 644 644 644	\$555 8 85	8214488	85.85.868 8-20.858
-		July.			Angust.		8	September.			October.		×	November.		A	December.	
-	Ķ	Ŋ.	Mean.	Kar	별	Kegn.	ż	ള	Keen.	Kar.	Kip	Moan.	Ker	Klp.	Koen.	Max.	Min.	Mean.
1859 1980 1981 1981 1985 1985	5528688	15461 24	8088268 520859	\$62828 8	######################################	RESERT - 804000	222333	######################################	RR RR R R R R R R R R R R R R R R R R	22833 2	888888	47.47.44 10.88.148	282222	8:88 a 28	28224284 marecoa	882444	พระพลา	19. 19. 19. 19. 19. 19. 19. 19. 19. 19.

_		ŗ.	TABLE B.	.—Maxim	alm, mur	imum, az	TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1839 to 1865 inclusive.	emperati	are for se	asons an	d years f	rom 1859	to 1865 i	nclusive.	٠.	
!		Spring.		42	Summer.			Autumn.			Wlater.			Year.		
l	ji K	녗	Mosn.	Max.	Min.	Mean.	Max.	Kin.	Mean.	Mar.	Min.	Mean.	Max.	Mia.	Moan.	Kange.
1860 1860 1861 1862 1863 1865	3825838	22100011	8444444 8010404	\$28828 8	8444488	25.25.85.88.88 02.40.82.11	8227773	8138 ₀ 28	444444 	44244	1	8888888 848848	\$£8828	1	1144444 6444 644	2858628 2

THUNDER BAY ISLAND, MICHIGAN.

			TAR	ile B.—.	Amount o	f rain an	d melted	mow for	months,	sessons,	and year	s, in Uni	ted States	TABLE B.—Amount of rain and melted mow for months, seasons, and years, in United States inches and decimals.	decimals.		
į	Jan.	Peb.	March.	YPATY .	May.	June.	July.	Aug.	Sept.	Oet O	Nov.	Dec.	Spring.	Sammer.	Summer. Autums.	Winter	Year.
1866 1860 1866 1865 1865	444444 8888814	884-199-	4:4:444 888882 4	44444444444444444444444444444444444444	11:44414 8825582	4:40444 \$\$\$\$\$\$	4444411 8258888	0404464 68844 1488 1780	4444-44 8448-88	4444444 2488827	4:4:440	88891 8889 889 894 794 71	ಆನ್ ಪ್ರವಸ್ತ್ ಸ್ಥ ಭೆಪ್ಪಾ ಹಾಗ್ಯ ಸ್ಥಿ ಪ್ರಾಥ ಭೆಪ್ಪಾ ಹಾಗ್ಯ ಸ್ಥಿ ಪ್ರಾಥ	6.10 8.71 8.71 9.06 4.85 8.37	10.80 7.82 14.82 8.51 8.51 8.51	9.00 7.00 7.00 7.00 9.00	37. 14 36. 38 31. 38 31. 38 31. 38

TAWAS CITY, MICHIGAN.—ALTITUDE 587 FEET.

	TABLI	E B.—M	E.B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1839 to 1866 inclusive, reduced to 32º Fahr. + 28 inches.	minimum	, and me	an baron	etrical p											i
Year.	-3	January.		P4	February.			March.			April.			May.			June.	
	Max.	Ka.	Moan.	Ker	၌	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.
1659	1. 1. 1889.1	0.00 7853	1.438	1.704	0.967	1.343	1.839	0.059	1.149	1.732	0, 556	1.321 1.318	1.849	0.991	1.387	1.900	1.022 0.627	1. 397
1861 1868 1863		9.9.9 7.5.8 9.5.8 0.5.8	111 35 35 136 136	11 d	0.573 0.713 0.715	1.364 1.364 1.466	11.1. 28.3.	0.0 0.635 0.00 0.00	1.255	1.83	0.00 8.45 8.45 8.45 8.45 8.45 8.45 8.45 8.45	1.985 1.44 1.455		0.0.0	 883 883	 8.5.8	0.00 0.03 0.03 0.03 0.03 0.03 0.03 0.03	1.338
1865. 1866.		0.950 823 823	1.357	1.834	0. 564 0. 919 0. 919	1. 219 1. 458 1. 417	11.802 1.735 1.735	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	1.256 1.295 1.423	1.751 1.896 1.741	0.959 0.567 0.845	1.372	1. 515 1. 746 1. 602	0. 930 0. 852 0. 801	1.1838	1. 599 1. 599 1. 660	0.867 0.962 0.813	1.343
		July.			August.		ž	September.	ı.		October.		Z	November.			December.	
	Ķ	녍	Mean.	Max.	je je je je je je je je je je je je je j	Koan.	Max.	Min.	Moan.	Max.	Min.	Mean.	Max.	Min.	Mesn.	Max.	Min.	Mean.
B600 B601 B861 B872 B873 B873 B874 B874	11.976	0.9907 0.946 1.111 0.996 0.996	1 336 1 336 1 336 1 386 1 386 1 386	1.1667 1.1667 1.1667 1.1687 1.1687	1. 161 1. 020 1. 102 0. 995 0. 995 0. 965 0. 965	11.1.1.383 1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.	11.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1	0. 935 0. 935 0. 946 0. 948 0. 948 0. 949 0. 949 0. 935	1. 388 1. 388 1. 388 1. 388 1. 388 1. 380	1. 707 1. 735 1. 832 1. 873 1. 986 1. 684 1. 1. 907	1.018 0.717.0 0.0920 0.922 0.922 0.738 0.648	1.339 1.353 1.353 1.353 1.297 1.401 1.401	20 130 1.74 1.74 1.796 1.796 1.573	0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000	1. 239 1. 239 1. 239 1. 239 1. 262 1. 262	2004 1.914 2.109 2.039 1.934 1.934	0.0000000000000000000000000000000000000	

			TABLE	B.—Max	dmam, n	ninimum	TABLE B.—Maximum, minimum, and mean barometrical pressure reduced to 32º Fabr., for seasons.	ın barom	etrical pr	essure re	duced to	32° Fah	r., for sea	Sons.		
Year.		Spring.		- e2	Summer.		,	Autumn.			Winter.			Year.		Range
	Max	Min	Mean.	Max.	Min.	Mean.	Max	Min.	Mean.	Max.	Min.	Меап.	Max.	Min.	Mean.	
200 15 20	1 1 862 1 1 863 1 1 863 1 1 866 1 7 856 1 7 896	00000000000000000000000000000000000000	1, 285 1, 350 1, 350 1, 365 1,	1.900 1.676 1.615 1.632 1.632 1.737 1.737	0.0907 0.087 0.938 0.986 0.963	1,390 1,347 1,373 1,380 1,380 1,380	28 138 1. 848 1. 856 1. 856 1. 396 1. 738 1. 907	0.000 0.050 0.053 0.053 0.084 0.044 0.044 0.073	1. 385 1. 387 1. 400 1. 415 1. 318 1. 318 1. 350	94 94 94 94 94 94 94 94 94 94 94 94 94 9	0.595 0.573 0.670 0.635 0.854 0.879	1,356 1,358 1,422 1,414 1,325 1,357 1,357	441.44444 601.36444 601.366 601.466 601.166	00000000000000000000000000000000000000	1.368 1.342 1.341 1.376 1.399 1.314 1.384 1.384	9 073 1.465 1.475 1.389 1.588 2.139

TAWAS CITY, MICHIGAN.

	A.16 I	Ų.	W 22.50
		Mean.	88.288.888 - 20.25.25
	June.	M.C.	8818388
و		Max.	£ £ £ 5 5 5 5 5 5
faclusive		Mean.	82538884 60108841
19 to 1866	May.	Min.	**********
year 18:		Max.	335558
from the		Mean.	,
TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1866 inclusive.	April.	Mfp.	%1289°888
ire for ea		Max.	888888
emperatu		Mean.	22.28.28.25.22 1.00.0000000000000000000000000000000
d mean t	March	Min.	125-6-251
mum, so		Max.	887±385.3
um, mini		Mean.	***************************************
—Maxim	February.	Min	0 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
CABLE B.	1	Max.	46224826
		Moan.	2.00.00.00.00.00.00.00.00.00.00.00.00.00
	January.	Ŕ	11111
		Max.	44824884
	Year.		1889 1890 1890 1892 1892 1894 1896

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TAWAS CITY, MICHIGAN.

•		 gi	24 2 3 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3
	ı:	Mean.	
	December.	Min.	2422232
•	ı	Max.	4488444
inclusive		Mosn.	20 4 8 2 2 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
9 to 1866	November.	K lp.	888884458
year 185	Z	Max.	282828
TRELE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1966 inclusive.		Mean.	\$\$\$\$\$\$\\\4\$\$ 0\\\\\\\\\\\\\\\\\\\\\\\\\\
th month	October.	Min.	******
re for eac		Max.	# 84 4 8 84
mperatu		Mean.	88.4.888.94
I mean to	September.	Min.	******
nam, so	22	Max.	Et 8 5t58
am, mini		Mess.	83 658 88.9
-Maxim	August.	ij	3855433 2
ABLE B.		Ker	2000 2000 2000 2000 2000 2000 2000 200
H		Mean.	8889999 4080499
	July.	Min	수점 축 4성축관축
		Max.	26.28.22.22
	Year.		1850 1960 1981 1982 1985 1985 1986 1986 1986 1986 1986 1986

TAWAS CITY, MICHIGAN.

•		-	ABLE B.	TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1866 inclusive.	ım, mink	mam, en	d mean t	emperatu	are for se	us suoss	d years f	rom 1859	to 1866 in	nclusive.		
Y our.		Spring.			Summer.			Autumn.			Winter.			Year.		
	Max.	Min.	Mean.	Max.	M.jp.	Mean.	Max.	Жh	Mean.	Max.	ğ	Mean.	Kax	ri Kin	Mean.	
1659	æ	ı	6	82	8	55.23	£	\$	46.7				82	1	#	86
1861	28	ន្ទ	4 8 0 -	22	84	8 8 8	FE	11	4 7.0	8 5	28	21.9 2	5 8	22	<u>4</u> 4 - 0	89
	E 8	-	30.	8	8	3	2	12:	47.9	8	1	26	8	1	g:	a 8
1864		(N C)	6 6	88	±8	86	23	25	4	2	֓֞֟֞֟֟֟ ֓֟֞֞֞֞֟֞֟֟	2.8	88	֓֟֞֟֝֟֟֟֟֟֟֟֟֟֟֟֟֟֟֟֟֟֟֟֟֟֟֟	14	S O
1666		۽ آ	41.6	83	8	2.7	8	8	8	5	6	2.6	33	₽? 1	¥:	5
	:	-	37.9		8	8 9	:	a	9. 9.	Ç	1	F.	:	Î	į	

A			TAI	BLE B.—4	Amount c	of rain an	d melted	mow for	r months,	sessons,	and year	s, in Uni	ted States	IABLE B.—Amount of rain and melted mow for months, season, and years, in United States inches and decimals.	decimals.			
	Jen	Feb.	Mar.	April.	May.	June.	July.	Aug.	Sept	Oct.	Nov.	Dec.	Spring.	Summer.	Autamn.	Winter.	Year.	
1859 1960 1982 1985 1964 1964 1966	11114161 28882483	40141146 882233488	88282528	9.50 8.80 8.50 8.50 8.50 8.50 8.50 8.50 8	111114144 855486 8	25.11.11.11.12.25.25.25.25.25.25.25.25.25.25.25.25.25	11194110 882528258	2583884 3583884	1.0440:194 1988 888 1988 888 1988 888	85 448848	401114401 888282	100001101	444444444 48844488	4444444444 444444444444444444444444444	44494494 12228222	44444444 44888888	85.78.98.78 86.78.98.78 86.88.78.88	2221 0242 03

MILWAUKEE, WISCONSIN.—ALTITUDE 591 FEFT.

B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1866 inclusive, reduced to 33º Fahr. + 28 inches.	June.	Min. Mean.
32º Fahr.		Max
educed to		Mean.
nclusive, 1	May.	Mib.
to 1866 lt		Ker
year 1859	۰.	Mean.
from the	April	r. Mia.
ch month		p. Max.
tre for eac	4	n. Mean.
al pressu	Marob.	x XGn.
rometric		p. Max.
mean ba	ery.	. Moen.
um, and	Pebruary.	년 -
n, minim		. Max.
Meximus	ė	Mosn.
99	January.	. MGFr.
TABL		Max
	Year.	

MILWAUKEE, WISCONSIN.-ALTITUDE 591 FRET.

	TABL	B.—M.	eximum,	LE B.—Maximum, minimum, and meen barometrical pressure for each month from the year 1859 to 1866 inclusive, reduced to 32º Fahr. + 28 inches.	ı, sınd me	en beron	netrical p	ressure f	or each m	onth from	n the yea	ur 1859 to	1866 incl	tusive, re	duced to	32º Fahr	+ 28 to	ches.
Your.		July.		Ì	August		æč	September.			October.		Z	November.		a	December.	
	Max.	Kin	Meta.	Kar	Ä	Mean.	Max	Ę	Mean.	Max	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.
1859 1.653 1960 1.653 1961 1.653 1963 1.673 1964 1.696 1966 1.696	1.683 1.683 1.673 1.649 1.649	0,902 1,027 1,047 1,181 1,139 1,015 1,015	1.340 1.340 1.340 1.365 1.369	1. 552 1. 604 1. 604 1. 601 1. 661 1. 661 1. 661	1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.	245.38 245.38 245.38 245.38 245.38 245.38	1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,	0.0 971 1.1.04 1.075 1.075 1.075 1.095 1.095 1.095 1.095 1.095	1. 286 1. 442 1. 408 1. 351 1. 364 1. 368	11111111111111111111111111111111111111	2020000 2020000 2020000 20200000000000	11111111111111111111111111111111111111	1.1.89 1.1.81 1.1.87 1.1.757 1.1.557 1.9.50	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.1.1.1.286 2.4.580 1.1.254 1.1.291 1.1.291 1.200 1.300	1.1.958 9.058 1.958 1.970 1.910	0.00 818 0.00 818 0.00 818 0.00 818 0.00 818 0.00 818	1, 323 1, 457 1, 486 1, 408 1, 284 1, 357 1, 357

MILWAUKEE, WISCONSIN. + 28 INCHES.

			TABE	E B.—Ma	xfmum,	minimum	TABLE B.—Maximum, minimum, and mean barometrical pressure reduced to 330 Fahr., for seasons.	n baron	etrical p	fessitre re	duced to	320 Fah	r., for se é	Mond.		
Year,		Spring.			Sammer.		_	Autumn.			Winter.			Year.		Res
	Max	၌	Kean.	Max.	Min	Moen.	Max.	Min.	Mean.	Max.	Ę,	Mes.	Max.	Klp	Mean.	
1800 1860 1861 1863 1864 1865	1. 889 1. 743 1. 743 1. 864 1. 874	0.00.511 0.00.655 0.00.677	1. 305 1. 325 1. 325 1. 373 1. 313	1. 683 1. 638 1. 708 1. 708 1. 696	0.171.0.000 0.000.000 0.000.000 0.000.000	25.41.1.1.1.1.2.2.2.2.2.2.2.2.2.2.2.2.2.2.	1. 7888 1. 815 1. 815 1. 757 1. 1967 1. 1757	0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.00000 0.00000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.00000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.00000 0.000	200 000 000 000 000 000 000 000 000 000	4-1-9-9-1-9 00-1-9-9-1-9 00-1-9-9-1-9-9-9-9-9-9-9-9-9-9-9-9-9-9-9	0.00000 0.00000 0.00000000000000000000	11.365 11.446 11.345 11.345 11.345	9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	0.000 0.000 0.0047 0.00738 0.00738	25. 25. 27. 27. 20. 27. 27. 27.	1, 499 1, 386 1, 129 1, 129 1, 129 1, 129 1, 129

			H	TABLE B.—Maximum, minimum, and mean temperature for each mouth from the year 1859 to 1866 inclusive.	Maxim	ım, minfi	num, enc	I mean te	mperatu	re for eac	h month	from the	year 185	9 to 1866	Inclusive	ر ا		
Year.		January.		Ħ	February.			March.			April.			May.			June.	
	Max.	Min.	Mean.	Max.	Min.	Мевп.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min	Mean.
1859 1861 1861 1863 1863 1864 1864	*445344	72500	24.24.08.08.01.00.00.00.00.00.00.00.00.00.00.00.00.	224428	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	27.2 28.6 26.8 27.5 27.5 27.5 27.5 27.5 27.5 27.5 27.5	888488	2×4±1×	048 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	22522	ន្តន្តន្តន្	24444 800 800 100 100 100	88822	88888	20.00.00.00 00.00.00.00 00.00.00	82888	858284	252535 2000 2000 2000 2000 2000
1866.	4	July.	19.5		August.	19.1	8 6	September.	li		94 October.	52		November.	Si .	8 0	42 December.	2
	Max	Ė	Mean.	Max.	형	Mean.	Mex	Min	Mesn.	Max	M(la.	Mean.	Max.	d R	Mean.	Max	Min.	Mean.
1859. 1860. 1861. 1862. 1863. 1865.	2888222	6444848	88 80 12 12 12 12 12 12 12 12 12 12 12 12 12	888583388	84481444	68.1 70.6 69.1 69.1 67.1	288338B	\$582558	88.88.88.88 88.88.88 89.74 80.74 80.74 80.74 80.74	\$168 653\$	28888288	20.00 20.00	88888628	95 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	6%%%%%% 6%%%%%% 6%%%%%%	±48828±±	1	0.4.5.9.9.9.8.8 0.4.1.9.9.9.8.8 0.0.1.0.0.0.0.0

MILWAUKEE, WISCONSIN.

ŀ		RI	EPORT	OF	TH
	Denge		98	825	13
		Mean.	444 804	\$ 4.4 000	4
clusive.	Year.	Min.	779	# R C	Î
to 1866 ir		Max.	828	5888	66
TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1866 inclusive.		Moan.	8 2 8 5 2 4	8.28	8
d years fi	Winter.	Min.	25	200	22
Asons an		Max.	3,23	328	38
are for se		Mean.	4.65.05 4.62.05	46.7.2 7.2.2	. e
temperat	Autumn.	Min.	3 8	4.08	ਜ ਜ
nd mean		Max.	## ## ## ## ## ## ## ## ## ## ## ## ##	65 a5 a	ř
ılmam, s		Mean.	67. 1 67. 1 67. 4	16.88.88 1.23.28	8.76
nam, mp	Summer.	Min.	138	48 8	# 학
3.—Maxh		Max.	852	553	35
TABLE I		Mean.	के <u>छ</u> ८ र र	2.5; d	Q
	Spring.	Min.	9,24	֓֞֞֞֓֓֓֞֟֓֓֓֟֓֓֓֓֟֟֓֓֓֟֟֓֓֓֓֓֓֓֓֟֓֓֓֟֟֓֓֓֟֓֓֓֟֓֓֓֟֓֓֟֓֓֟֓֓֟֓	
		Max.	888	22.29	388
	Year,		1859 1890 1881 1881		1866.



MILWAUKEE, WISCONSIN.—ALTITUDE 591 FEFT.

	Year.	2009 8 9 9 8 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	Winter.	9994554 5845762
decimals.	Autumn.	のためのたためた 80884875518
TABLE B.—Amount of rain and melted snow for months, seasons, and years, in United States inches and decimals	Summer. Autumn.	කුණුදැදෑ 888825
ited States	Spring.	4944444 8285288
rs, in Un	Dec.	0.44-1.04 21-34-2826
and year	Nov.	84 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
sensons,	1 00	19149149 80 4820 25
r months	Sept.	2585282£
snow fo	Aug.	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
nd melted	July.	
of rain a	June.	4-1-4-0-0-4- 20-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2-2
Amount	May.	학소전전역기억 알訳그역 도 교
BLE B.—	March. April.	0441414 82.22.22
TA	March.	다.디디디디디 없양 3 휴 및 플링
	Feb.	-140494- 5288882
	Jan	크여덕덕역으역 참왕수중등활왕
A A	į	

	TABL	E B.—Ma	rdmam,	mplma	, and me	an barom	etrical p	rossure fe	LE B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1863, inclusive, reduced to 32º Fahr. + 28 inches.	onth from	n the yea	r 1859 to	1863, fac	lugave, re	duced to	32º Fah	+ 28 1	ches.
Year.	. •	January.		<u> </u>	February.			March.			April			May.			June.	
	Max.	Min.	Moan.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Mh.	Mesn.	Max	ŊĮ.	Mean.	Max.	Min.	Mean.
1859 1960 1962 1963	1. 971 1. 845 1. 698 1. 772	0, 852 0, 630 0, 957 0, 832	1. 409 1. 357 1. 422 1. 327	1.805 1.718 1.750 1.880	0. 582 0. 352 0. 625 0. 890	1. 349 1. 263 1. 354 1. 467	1.705 1.827 1.591 1.786	0. 738 0. 499 0. 696 0. 954	1. 339 1. 362 1. 300 1. 370	1. 867 1. 675 1. 633 1. 738	0, 568 0, 796 0, 787 0, 752	1. 323 1. 259 1. 385 1. 377	1. 683 1. 717 1. 703 1. 584	0.567 0.579 0.831	1. 269 1. 309 1. 333 1. 330	1.588	0. 695 1. 096 0. 992 0. 910	1, 326
		July.			Angust.		ž	September.			October.		Ž	November.		A	December.	
	Max	MGD.	Mean.	Max.	M'in.	Meen.	Max	Min.	Mean.	Max	Min.	Mosn.	Max.	Min.	Mean.	Max.	Min.	Meen.
1859 1960 1861 1962 1963	1.660 1.673 1.588	0.892 0.972 1.075	1.315 1.367 1.369	1, 609 1, 641 1, 689	1. 099	1.353	1.818 1.807 1.864 1.734	0.950 0.960 1.102 1.094	1.416 1.441 1.399 1.428	1. 702 1. 754 1. 754 1. 842 1. 842	0, 910 0, 907 0, 665 0, 970	1,445	1.974 1.607 1.735 1.973	0. 698 0. 588 0. 649 0. 649	1.257	2, 127 1, 879 2, 059	0.959 0.723 0.967 0.838	11.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1
					6.1	GRAND HAVEN, MICHIGAN.	AVEN,	MICHIG	+	28 INCHES.					•			
					TABL	E B.—Ma	ximam,	wholmum	TABLE B.—Maximum, minimum, and mean barometrical pressure reduced to 33º Fabr. for seasons.	an baron	netrical p	ressure re	duced to	32° Feb	. for sea	. 900		
Year.		1		Spring.			Summer.			Autumn.			Winter.			Year.		
		7	Max.	MGn.	Meen.	Max.	ĘĘ.	Mean.	Max.	KG	Moan.	Kax.	ģ	Mean.	Max.	널	Mean.	Kange.
1859 1860 1961 1963 1963			1. 867 1. 827 1. 703 1. 786	0. 567 0. 499 0. 696 0. 752	1.310 1.310 1.306	1. 660 1. 673 1. 694	0.972	1.331	1. 974 1. 807 1. 864 1. 973	0.088 0.588 0.655	1. 436 1. 370 1. 329 1. 391	2, 248 2, 127 2, 059 2, 059	0.000 3358 0.000 3358 0.000 0.000	1.402	2, 127 1, 879 2, 059	0. 367 0. 352 0. 625	1.333	1. 560 1. 527 1. 434

GRAND HAVEN, MICHIGAN.

			Ę	TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1863, inclusive.	-Maximu	m, minin	nam, and	mean te	mperatur	e for eacl	h month	from the	year 1856	to 1863,	inclusiv	.		
Year.		January.		[24	February.			March.			April.			May.			June.	
	Max.	Min.	Mean.	Max.	Mh.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.
1859. 1860. 1861. 1863.	5%18	04%	25.29 20.29 20.29 20.29 20.29	8858	1365	8.83.83 7.01.6 7.01.0	25. 25.	21 1 1 10	31.3 31.1 31.9 31.6	75 24 25 26 26 26 26 26 26 26 26 26 26 26 26 26	288°	45. 46.1 43.6 46.1	8883	3288	60.1 51.5 54.6	2888	3448	66.0 6.0 6.0 6.0 6.0 6.0 6.0 6.0 6.0 6.0
		July.		,	August.		đ.	September.	•		October.		Z	November.		Q	December.	
	Max.	Min.	Mean.	Max.	Mîn.	Mean.	Max.	Min	Mean.	Max.	Mh.	Mean.	Max.	Min.	Mean.	Max.	Klp.	Mesn.
1859 1980 1881 1883 1983	2828	8 4 6 8	70.7 67.7 72.3 69.8	28 6 68 26 6 68	& & & & & & & & & &	67. 7 70. 9 78. 9	8443	4818	59.2 51.9 61.4	5852	1888	47. 1 49. 1 51. 7 51. 4	8888	96 61 11	40.2 36.6 37.7 37.5	8248	2000	8888 1111
						GB	AND H.	AVEN. 1	GRAND HAVEN, MICHIGAN,	AN.								

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			LABLE B.	-Maxim	um, mia	mam, sc	id mean t	emperati	ire for se	Meons and	l years f	rom 1859	TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1839 to 1863, inclusive.	inclusive.		
Year,		Spring.			Summer.			Autumn.			Winter.			Year.		
	Max.	Min. Mean.	Mean.	Max.	Min.	Mean.	Max.	Min.	Мева. Мах.	Max.	Kla	Mean.	Max.	Min.	Mean.	Kange.
1859 1860 1870 1872 1873	ESSI	10 1 1 8	1. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4.	86.98 10.88 44.04		38.85 1.000	68.1 79 9 47.9 68.8 77 119 60.3 60.3 60.3	800	48. 47.9 80.03 7.03	8228	, caso	9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	85.6	11	47.3	107 107 94

Year.	Jan.	Feb.	TABI March.	TABLE B.—Amount of rain and melted snow for months, th. April May. June. July. Aug. Sept.	mount of	f rain and June.	d melted a	mow for		oct.	and years	, in Unit	seasons, and years, in United States inches and decimals. Oct. Nov. Dec. Spring. Summer. Autumn	Inches and Summer.	decimals.	s. D. Winter.		Your.
1859 1860 1861 1863	1.66	1.06	0.06 9.06 9.08 1.12	0.96 0.96 0.96 0.94	2,444 5,624,8	941.94.0, 20.00,00,00,00,00,00,00,00,00,00,00,00,00	9. 34 1. 36 0. 90 1. 36	88 88 88 88	44.40 24.40 88.88	8825 2288	9.1.93 9.1.93 9.0.93 9.0.93	8.00.1.0 8.00.00 8.00.00	5484 2848 2848	10.18 7.84 7.77	بـ ور ور <u>م</u>	8885	8 4 8 8	25. 35 21. 33
					ū	DETROIT,	, MICHI	GAN.—	MICHIGAN.—ALTITUDE 587	E 587 FR	FRET.							
	TABL	E B.—M	aximum,	minimum	ı, snd me	an baron	netrical 1	ressure f	or each n	nonth fre	m the ye	ar 1859 t	B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1866 inclusive, reduced to 32º Pahr. + 38 inches.	lusive, red	luced to	12º Fahr.	uj 86 +	ches.
Your.		January.			February			March.			April.			May.			June.	
	Max.	Min.	Меап.	Max.	Win.	Mean.	Max.	Min.	Mean.	Max.	·up	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.
1859. 1860. 1861.	1.964		1.324	1.693 1.832 1.814		1.353	1.641		1. 254	11.938	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.334	1.752 1.686 1.685	0.00 99.00 10.00 1	1.273	1.612	0.608	1.296
1967 1967 1965 1866	1.948 1.948 530 530	0.100 0.000 000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.000 0.	11111 32388	1.1.1.94 808 93 808 83	10001 88898 88898	11111 25832	25.1.1.1. 25.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2	9889	11111	11111 1021111 10222	8.86.96 8.96.96 8.96 8	1.313	11111 888 888 888 888 888 888 888 888 8	0.1005 6004 6004 6004 6004 6004 6004 6004 6	2888	11111 222 222 223 223 223 223 223 223 22	0.00.0 80.00.0 80.00.0 80.00.0	11111
		July.			August		ož.	September			October.		Z	November.		- A	December.	
	Max.	Min.	Mean.	Max.	Klin	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.
1859 1860 1862 1863 1863 1864 1865	1. 641 1. 579 1. 666 1. 666 1. 612 1. 682	0.963 0.953 1.046 1.132 1.129 1.1058	1, 396 1, 331 1, 333 1, 409 1, 409	1, 704 1, 620 1, 678 1, 786 1, 680 1, 680 1, 727	1, 035 1, 181 1, 155 1, 056 0, 946 0, 986 0, 886	1,345 1,398 1,396 1,387 1,325 1,445 1,386	1.800 1.810 1.707 1.869 1.869 1.660	1, 106 1, 028 0, 983 1, 034 1, 046 1, 001	11.11.11.11.12.12.12.12.12.12.12.12.12.1	1,698 1,778 1,739 1,609 1,609	0.730 0.735 0.881 0.906 0.941 1.014	1, 394 1, 361 1, 401 1, 430 1, 305 1, 400 1, 492	1. 630 1. 684 2. 029 1. 786 1. 982 2. 099	0.637 0.771 0.879 0.881 0.708 0.670	1. 272 1. 256 1. 386 1. 376 1. 342 1. 461	1. 975 1. 968 1. 968 1. 951 2. 038 1. 958	0.876 0.760 1.001 0.852 0.777 0.767 0.636	1.145

GRAND HAVEN, MICHIGAN.

Max. Min. Mean. Max. Min. Mean.	'			Ħ	ABLE B.	TABLE B.—Maximum, minimum, and mean temperature for each mosth from the year 1859 to 1963, inclusive.	m, minia	oum, and	mean ter	mperatur	e for each	b month	from the	year 1856	to 1863,	inclusiv	ė		
Min. Mean. Max. Min. Mean. Max.		•	Spunry.			*ebruary.			March.			April.			May.			June.	
10 10 10 10 10 10 10 10	1	Max.	Min	Mean.	Max.	Ą	Mean.	Max.	Mis.	Mean.	Max.	Ę,	Mean.	Ker	Ę	Mean.	Max.	Ŋ.	Mean.
Min. Mean. Max. Min. Mean. Min. Mean.	1 :	÷∺±3	0420	25.25 27.28 21.28 21.08	222	135	95.58 97.58 95.16	8254	21 1 1 10	37.3 31.3 31.9	3873	2880	45.2 46.1 46.1 46.1	8834	\$288	60 57.5 58.5 1	2888	8448	8828 0048
Min. Mean. Max. Min. Mean. Max. Min. Mean. Max. Min. Mean. Max. Min. Mean. Max. Min. Mean. Min. Min. Min. Mean. Min. Mean. Min	11		July.			Angust.		, so	eptember			October.		Z	ovember			ecember	
Spring S	1 "	Max.	Ę	Mean.	Max.	j K	Mean.	Max.	Min.	Moan.	Max.	Ę	Mean.	Ker	Ę	Mean.	Max	Klb.	Mesn.
TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1863, inclusive. Ranger, Min. Moan. Max. Min. Mean. Min. Min. Min. Min. Min. Min. Min. Mi	1 :	2828	8248	70.7 67.7 69.8	25 25 25	****	7.05 8.97 8.92	8432	4848	88.25.88 8.0.4.0	5855	1888	47.1 40.1 51.7 51.4	8888	80.31	40.8 37.7 37.5	8648	P 90 9	ន្តម្ភង្គ
Spring. Summer. Autumn. Max. Min. Mean. Min. Min. Min. Mean. Min. Mean. Min. Mean. Min. Mean.	1						8	AND H	AVEN, 1	KICHIG!	AN.								
Spring. Summer, Autumn. Max. Min. Mean. Max. Min. Mean. Max. Min. Mean. Max. Min. Mean. Max. Min. Mean. Max. Min. Mean. Min.						TABLE B.	.—Maxin	ajan , mnc	Imum, su	d mean t	emperate	are for se	44004 an	d years fi	om 1859	to 1863, 1	inclusive.		
Max. Min. Moan. Max. Min. Moan. Max. Min. Moan. Max. Min. Min. Max. Min. Max. Min. Max. Min. Max. Min. Max. Min.					Spring.			Jummer.			Autump.			Wlater.			Year.		
7.7 7.7 88. 1.1 8. 4.7 9. 1. 8. 4.7 9. 1. 8. 4.7 9. 1. 8. 4.7 9. 1. 8. 4.7 9. 1. 8. 4.7 9. 1. 8. 4.7 9. 1. 8. 4.7				Max.	Min.	Moan.	Max.	Kla	Mean.	Max.	Min.	Mean.	Mex.	Min.	Mean.	Max.	Mia.	Mean.	
				FFFI	G-+3	7-3-4-6 7-3-4-6 70-3-4-6	2 08	\$ 40	20.05 1.00 E	8458	8022	8,448 0.0.00 0.0.00	92249	0000	8 6 6 6 6 6 6 6 6 6 6 6	8 36	0.52	47.74 47.04	101

DETROIT, MICHIGAN.

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	to 1866 i		Max.	\$ \$ \$ 9 9 9 9 9 9
	from 1859		Mean.	**************************************
	od years	Winter.	M.	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1866 inclusive.		Max	828888
	ture for		Mosn.	25.00 25.00
	tempera	Autumn.	ų Į	22 22 24 24 25 25 25 25 25 25 25 25 25 25 25 25 25
DETROIT, MICHIGAN.	nd mean		Max.	28.8888
ott, Mic	ofmam, 6	_	Mean.	69.7.7 69.0.4.9 71.1.1 66.9
DETRO	ion (man	Summer.	Mia.	29 33 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	B.—Max		Max.	& & & & & & & & & & & & & & & & & & &
	TABLE	b i	Mean.	% # 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
		Spring.	Min.	ಸ್ಟ್ ಹಳಳು ಇಳ
			Max.	2232223
		Year.		1859 1960 1965 1965 1965 1965 1966

DETROIT, MICHIGAN.

			TAI	BLE B.—.	Amount (of rain an	d melted	EDOW for	r months,	seasons,	and year	s, in Uol	ted States	TABLE B.—Amount of rain and melted mow for months, seasons, and years, in United States inches and decimals.	decimals.		
1 007	Jap	Feb.	March.	April.	May.	June.	July.	Aug.	Sept.	Oct	Nov.	Dec.	Spring.	Summer.	Autumn.	Winter.	Year.
1829 1860 1862 1862 1863 1865 1866	44288948		8004411944 8004411944		9-1-10104-10 28882E23	4444444 4448348	2288823 2488823		91-91-944 5845888	-:444-144 818:118		24858848 84858848	44554545 5828885	9. 42 9. 83 9. 83 10. 92 6. 81 11. 14	6.51 12.19 7.150 7.14 7.13 6.09	작작작작·숙역 일왕52월 38 1	888.889.9 864.885

MONBOE CITY, MICHIGAN.—ALTITUDE 567 FEFT.

	TABLI	B.—M	aximum,	B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1966 inclusive, reduced to 32º Fahr. + 28 inches	ı, and me	an baron	setrical p	ressure f	or each m	onth fror	n the ye	и 1859 и	1866 Inc	ludve, re	duced to	32° Fahr	. + 88 1	aches.
Year.	7	Annary.		24	February.			March.			April			May.			June.	
	Max	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max	Min.	Mean.	Max.	Min.	Monn.	Max.	Min.	Mean.
1859 2 059 1860 2 059 1861 1861 1990 1863 1965 1966 1864 1866 1961	9.050 1.990 1.990 1.998 1.908 1.901	0.000 6.000	1. 454 1. 463 1. 404 1. 518	11.8.1.9.0.1.1.8.0.1.1.9.0.1.1.1.9.0.1.1.1.1.1.1.1.1.1.1	20000000000000000000000000000000000000	11.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1	1.1.98.0 1.7.50 1.7.50 1.7.50 1.7.50 1.7.50	0.00 922 0.00 922 0.00 922 0.00 923	1.359 1.359 1.359 1.359 1.359 1.453	25.7.1. 25.7.1. 25.7.1. 25.8. 26.8.	0.000000 98.8000000 0.000000000000000000	11.363	1, 713 1, 738 1, 738 1, 693 1, 574 1, 687	0.987 0.987 0.988 0.988 0.989 0.919 0.919	1.335 1.335 1.365 1.361 1.361 1.361	1.658 1.574 1.747 1.687 1.715 1.510 1.656	0, 704 1, 160 0, 940 0, 940 1, 071 1, 071	1. 338 1. 373 1. 373 1. 373 1. 375 1. 375

MONROE CITY, MICHIGAN.—ALTITUDE 567 FEET.

	TABLI		aximum,	B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1866 inclusive, reduced to 32º Fahr. + 28 inches.	, and me	an barom	etrical p	ressure fo	or each m	onth from	n the yea	r 1859 to	1866 lnc	lundve, re	duced to	32º Fahi	. + 28 In	chen.	
Your.		July.			August.		ž	September.			October.		Z	November.		A	December.		
	Max	Min	Mean.	Max.	뷝	Mean.	Max	Mg.	Mean.	Max	ij	Meen.	Max.	Kip	Mean.	Max	Min.	Mean.	Re
1859 1860 1861		1, 136 0, 922 1, 009	1.451 1.356 1.371	1.634	1. 251 1. 069 1. 151	1.397 1.385 1.436	1.855 1.834 1.864	1.027 1.131 1.055	1.494	1. 7.70 1. 7.33 1. 821	0.950 0.813 0.791	1.471	9 004 1.661 1.753	0.0.0 0.72 836 836	1.324	94 94 95 80 25 80 80 25 80 80 80 25 80 80 80 25 80 80 80 80 80 80 80 80 80 80 80 80 80 80 8	0. 905 0. 830 1. 052	1. 473 1. 491 1. 562	TOKI
1962 1963 1965		1.087 0.11.030 0.11.030 0.11.030	7828	11111 2883 2883 2883	1.1.064	11.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1	1. 767 1. 688 1. 688	28.8.2.3 88.8.2.3	36.55.55 56.55.55 56.55	1.821 1.928 1.729 1.729	0.970	1111. 84. 84. 88.	96.39	. 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.	4.14.1. 0.00.00.00.00.00.00.00.00.00.00.00.00.	00000 885 885 885 885 885 885 885 885 88	1111. 84848 84848	OF 1
	_	970.7	1. 0/4	3	3	5	7. 00 T	1.012	1. CO	20.7	3	Ş	3	2002		<u>.</u>	90	1.080	ш

MONROE CITY, MICHIGAN. + 28 INCHES.

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0.648 0.628 0.628

MONROE CITY, MICHIGAN.

	-		F.	TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1866 inclusive.	-Maxim	ım, mini	num, and	mean te	mperatu	re for esc	h month	from the	year 185	9 to 1866	inclusive			
Year.	•	January.		Ďi .	February.		•	March.			April.			May.			June.	
	Max.	Kin	Mean.	Max.	Min.	Mean.	Max.	Min	Mean.	Max.	Min.	Меап.	Max.	Min.	Мевп.	Max.	Min.	Mean.
1889 1861 1861 1862 1964 1966	4448648	2	25 25 25 25 25 25 25 25 25 25 25 25 25 2	82842826	1	20 20 20 20 20 20 20 20 20 20 20 20 20 2	8228848	538841.1	8888889 4448188	22.22.25.25.25.25.25.25.25.25.25.25.25.2	888888	444448 0001112	828838	81883833 812	52.8 57.8 61.9 61.7 57.1	8889583	8248882	88882548 6886086
		July.			Angust.		Š	September.			October.		Ä	November.		Q	December.	
	Max	Min.	Mean.	Max.	M. Dr.	Mean.	Max.	' Min.	Mean.	Max.	Min.	Mean.	Max.	M(ii.	Mean.	Max.	၌	Mean.
88.90 98.00 90 90 90 90 90 90 90 90 90 90 90 90 9	ទនននងខ្លួនន	2242443	なれなななれな - 11100208	2422222	\$22224 452	4:44:49 00000000000000000000000000000000	22333333	5854223 8	26.25.26.25.25.20.24.10.20.24.10.20.20.20.20.20.20.20.20.20.20.20.20.20	46686628	******		35188248	<u> </u>	628411886 826623369	86888288	1 1 1 1 1 2 2 2 2 2	889.489.89 20000111

MONROE CITY, MICHIGAN.

		A	PORT OF TE
Y	Range		104 108 101 107 105 105 105
		Mean.	\$\$\$\$\$\$\$\$ *****************************
sclusive.	Year.	Min.	1
to 1866 tı		Max.	ន្ធន្ធន្ធន្នន <u>័</u> ន្ធន្ន
rom 1859		Mean.	2322232 1040744
d years f	Winter.	Min.	1
авопа вп		Max.	2888622
TAELE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1866 inclusive.		Mesn.	400 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
temperat	Autumn.	Mln.	智はは影響のも智
nd mean	,	Max.	28338288
ılmum, e		Mean.	257728 84444 84444 84444
num, mta	Summer.	Min.	2448848
.—Maxi		Max.	**************************************
TABLE P		Mean.	44444444 62100000
-	Spring.	Min.	ဗိုလ္ကလ္ နည္လ်င္
		Max	8688828
	Your.		1859 1860 1961 1963 1964 1965

MONROE CITY, MICHIGAN.

Y OI	· W	AB.
	Year.	24.23 25.23
	Winter.	444.444. 25.23.25.
decimals.	Autumn.	e 4014. 4901. 888. 888.
TABLE B.—Amount of rain and melted snow for months, sessons, and years, in United States inches and decimals.	Summer.	9 : 1 4 9 5 1 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9
ted States	Spring.	. 역 즉 육 는 육 는 용 경 표 명 된 경 원
n, ia Un	Dec.	#1149941 \$553857 8
and year	Nov.	9539895 5833888 5833888
, sessons,	Oct	44949494 8328948
r months	Sept	9444444 54464531
i snow fo	·8nV	4-1845844 18328333
nd melted	July.	48500888 48500888
of rain a	June.	2441144 8282252
Amount c	May.	4444494 8828888
ue B.—,	April	25882528
TAB	March.	04444944 8585688
	Feb.	1,001,11,12
	Jan.	20169101 88883488
Vaer		1859 1860 1961 1962 1963 1963 1965 1966 1966 1966 1966

CLEVELAND, OHIO.—ALTITUDE 645 FRET.

	TABLE	B.—Max	mum, m	fotoum,	and mean	л рагоше	trical pre	ssure for	B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1866 inclusive, reduced to 33º Fahr., + 28 inches.	nth from	the year	r 1859 to	1866 inclt	ısive, red	faced to 3	% Fahr.	, + 28 tn	ches.
Year.	ה	anuary.		E 4	February.			March.			April.			May.			June.	
	Max	Min	Mean.	Max	Min.	Mean.	Max.	Min	Mean.	Max.	Min.	Mean.	Max.	Mi	Mean.	Max.	Min.	Mean.
1859 1861 1861 1863 1863 1864 1864	1. 892 1. 893 1. 893 1. 894 1. 894 2. 509	0.950 0.731 0.866 0.797 0.838 0.849	1.371 1.376 1.376 1.33 1.341 1.388 1.461	1, 768 1, 761 1, 794 1, 936 1, 665 1, 665 1, 969	0. 700 0. 781 0. 685 0. 695 0. 845 0. 937	1. 279 1. 279 1. 228 1. 228 1. 274 1. 374	1, 628 1, 718 1, 506 1, 726 1, 647 1, 675 1, 753	0.817 0.861 0.864 0.535 0.535 0.534	1,286 1,336 1,303 1,187 1,268 1,268 1,371	1, 894 1, 670 1, 692 1, 593 1, 796 1, 630	0.0547 0.0529 0.0908 0.0908 0.0908	1.288 1.228 1.378 1.318 1.382 1.382 1.382	1.614 1.654 1.610 1.641 1.523 1.820 1.485	0.927 0.640 0.973 0.782 0.929 0.539	1,248 1,245 1,303 1,276 1,192 1,192 1,183	1, 685 1, 571 1, 496 1, 574 1, 574 1, 532 1, 532	1, 030 0, 628 0, 963 0, 963 0, 854 0, 751	1.34 1.284 1.289 1.387 1.388 1.388
		July.			August		ž	September.			October.		Ä	November.		А	December.	
	Max.	Min.	Mean.	Mex	Kün	Meen.	Max.	Kla.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.
8559. 1860. 1861. 1862. 1864. 1865.	1, 288 1, 288 1, 588 1, 663 1, 567 1, 587 1, 581 1, 581	1. 008 0. 995 0. 997 1. 070 1. 075 1. 018	1, 284 1, 284 1, 286 1, 304 1, 315 1, 315	1. 439 1. 554 1. 556 1. 646 1. 559 1. 550 1. 550	0.925 0.985 0.985 1.070 1.070 0.975 0.938	1.303 1.303 1.357 1.360 1.360 1.360	1, 776 1, 754 1, 757 1, 608 1, 797 1, 584 1, 584 1, 618	0.917 1.060 0.903 0.907 1.058 0.850 1.057 1.067	1, 359 1, 404 1, 346 1, 383 1, 416 1, 285 1, 404 1, 343	1. 659 1. 617 1. 718 1. 662 1. 512 1. 712 1. 778	0.963 0.918 0.913 0.913 0.916 0.896 1.111	1, 374 1, 348 1, 329 1, 367 1, 362 1, 316 1, 434	1. 675 1. 567 1. 681 1. 949 1. 782 1. 795 1. 795	0.890 0.513 0.713 0.896 0.875 0.886 0.896	1, 286 1, 205 1, 205 1, 320 1, 380 1, 380 1, 380 1, 380	1, 838 1, 925 1, 925 1, 925 1, 910 1, 907 1, 863	0.0 838 0.0 908 0.0 908 0.0 908 0.0 908 0.0 919 0.0 919	1. 336 1. 385 1. 385 1. 385 1. 381

			TABL	I B.—Ma	zimum, 1	TABLE B.—Maximum, minimum, and mean barometrical pressure reduced to 33º Fahr. for seasons.	, and me	ın barom	etrical pa	ogsure r	od been to	32º Fah	r. for sea	Sops.		
Your.		Spring.		82	Summer.		7	Autump.			Winter.			Year.		Range
	Max.	Min.	Mosn.	Mex.	Min.	Mosn.	Max	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	
9980 9880 9880 9880 9880 9880 9880 9880	1. 694 1. 718 1. 747 1. 736 1. 647 1. 650 1. 733	0.547 0.640 0.650 0.630 0.535 0.535 0.535 0.535	1,268 1,268 1,279 1,299 1,209 1,285 1,287	1,776 1,586 1,556 1,703 1,699 1,645 1,607	1, 030 0, 628 0, 969 0, 907 0, 854 0, 867 0, 988 0, 751	1,347 1,289 1,303 1,308 1,321 1,321 1,341 1,341	1, 875 1, 764 1, 757 1, 943 1, 873 1, 873 8, 022	0.863 0.512 0.713 0.896 0.586 0.680 0.808	1.373 1.326 1.367 1.367 1.373 1.373 1.373 1.378	1.954 1.908 1.925 2.016 1.910 1.993 2.509	0.700 0.721 0.685 0.743 0.621 0.528 0.773	1.377 1.344 1.389 1.390 1.316 1.312 1.415	1.954 1.925 2.016 1.961 1.963 1.907 2.509	0.512 0.640 0.631 0.631 0.528 0.528 0.534	1, 317 1, 308 1, 331 1, 343 1, 267 1, 338 1, 338	1, 2885 1, 3885 1, 3885 1, 445 1, 443 1, 970

OHIO .
CLEVELAND,

CRET	ARY	OF	WAR.
TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1866 inclusive.	June.	Mean.	25.88.88 8.84.94.98 8.84.84.747
		Ķ	28831,488
		Max.	88888888
	May.	Meen.	2222222 2471241
		Min	888888
		Mex.	8888228
	April.	Mean.	\$\$\$\$4.25 8084404
		Mtn.	8881888
		Max.	£88586 2
	March.	Mean.	#
		Kh	88
		Max.	75 22 23 24 25 25 26
	February.	Moan.	3233537 91500000
		벍	22.25
		Max	&L538538
	January.	Moan.	87.5.88.9.8 8-1.00000
		룆	11 11
		Ä	8488848
	Year.		1849 1840 1841 1843 1843 1843 1844 1845 1846 1846 1846 1846 1846 1846 1846 1846

CLEVELAND, OHIO.

j		R	EPORT	OF	TH
		Meen.	# # # # # # # # # # # # # # # # # # #	8 8 8 8 - 1 -	28.5
	December.	j.	9020	00 GN	OR
	А	Max	828	222	24
TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1866 inclusive.		Mean.	4 8 4 8 8 9 9 9 8 7 8 8	\$ 4 4	12
99 to 1866	November.	थाप्र	ន្តភូមិ	ដដឱ	22
year 18	X	Max	ដនទង	223	8
from the		Monn.	**************************************	3 4 8 0 4 4 0 4 4	95 95
oh month	October.	चाम	8838	ន្តន	8
re for ea		Max.	88.25.28	¥68	8
mperatu	2	Mean.	සියියිදි 7 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	285 888	
I mean te	September.	Mîn.	\$\$ \$\$	848	â
man, sno	702	Max.	88333	888	88
um, mini		Mean.	1.895 kg 8.88 8	전 전 보 보 보 보 다 다 다	66.9
–Maxim	August	Min.	8488	484	\$
'able B.		Ker.	8888	828	88
L		Meen.	ඩු මු දි දි ග ≻ 4 4	5 <u>4</u> 8	76.3
	July.	Ę	8248		
		ri K	2888		
	Your.		1859 1860 1861 1861	1863. 1864. 1865.	1866

CLEVELAND, OHIO.

		F .	ABLE B	-Mexim	ım, mini	num, sod	TABLE B.—Maximum, minimum, and meen temperature for sessons and years from 1856 to 1866 inclusive.	mperatu	re for sea	sons and	l years ft	om 1859	to 1866 in	clustve.			ETA
Year.		Spring.		42	Summer.		4	Autumn.		•	Winter.			Year.		1	RY
	Max.	jg K	Keen.	Max.	Min.	Mean.	Max	NCIP.	Mean.	Mex.	Ę	Keep	Max.	Min.	Mean.		OF 1
1856 1861 1861 1863 1864 1864 1866	\$ 2 \$ 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	తోలల ^{చ్} చేతం	ខ្លឺកុំកុំកូន្ទុ សម្មាលមក្ខ១	255555	2688388	8.88.85.12.5 8.80.25.12.5 8.80.25.25	8888888	285522	223822288 20000440	828888	777777	2.00 00 00 00 00 00 00 00 00 00 00 00 00	8888322	24.04.64	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	101	WAR.

Vacy			TAB	ile B.—/	Amount o	f rain an	d melted	EDOW for	months,	Beasons,	and year	, in Uni	TABLE B.—Amount of rain and melted mow for months, seasons, and years, in United States inches and decimals.	nches sud	decimala		
	d d	ą d	Ke.	April.	May.	June.	July.	Ang.	Sept	j O	Nov.	de A	Spring.	Summer.	Summer. Autumn.	Winter.	Y 06.C.
1856 1860 1861 1861 1864 1864	4444444 2824828	4-144-1-14 8-1-158-8-8	1846968 2808275	4 4 4 1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	여덕역역적적 225 22 2 2 2 2	11449491 2188528	**********	444-44-4 82833248	*********	74444444 4882548	4444449 88888255	44-144444 25223888	00 00 00 00 00 00 00 00 00 00 00 00 00	01 04 11 12 12 13 13 14 15 15 15 15 15 15 15 15 15 15 15 15 15	9.00 10.00 1	අපුතුඩ්දෙලනු මුදුනිධිජූරිනී	%%1%%%% %%%%%%%%%%%%%%%%%%%%%%%%%%%%%%

BUFFALO, NEW YORK.—ALTITUDE 585 PRET.

	TABLI		extmum,	B.—Maximum, minimum, and mean berometrical pressure for each month from the year 1859 to 1866 inclusive, reduced to 33º Fabr. + 98 inches	, and mos	n berom	etrical pr	esture fo	r oach m	onth from	the year	1859 to	1866 incl	ustve, re	duced to	39º Fahi	186+:	poper.
Year.		January.		P4	February.			March.			April.	-		May.			June	
	Max.	A)	Moan.	Max.	Min.	Moan.	Max	Min.	Ken	Max.	Min.	Mean.	Max	AGP.	Mean.	Max	널	Koen.
1859 1.924 1860 1.924 1861 2.443 1862 2.43 1863 2.117 1865 1.867 1.856 1.856 1.856	1. 924 1. 924 1. 924 1. 92 1. 817 1. 856 2. 536	1, 001 0, 750 0, 775 0, 654 0, 606 0, 963 0, 963	1, 398 1, 405 1, 436 1, 361 1, 317 1, 385 1, 488	1. 879 1. 869 1. 778 2. 146 1. 869 1. 948 2. 053	0.596 0.685 0.678 0.827 0.797 0.867	1,417 1,389 1,384 1,580 1,580 1,406 1,406 1,406	1.715 1.659 1.553 1.871 1.695 1.764 1.764	0.923 0.849 0.664 0.870 0.870 0.570	1,306 1,374 1,274 1,271 1,230 1,891 1,360	2. 007 1. 828 1. 825 1. 767 1. 663 1. 826 1. 656	0.680 0.855 0.895 0.536 1.006 0.543	1.347 1.318 1.465 1.389 1.289 1.389 1.381	1. 647 1. 707 1. 650 1. 680 1. 680 1. 680 1. 557	0. 999 0. 390 1. 031 0. 797 0. 878 0. 877	1.346 1.286 1.356 1.345 1.201 1.201 1.201 1.201	1,747 1,577 1,838 1,536 1,646 1,565 1,565	0. 679 0. 960 0. 960 0. 757 0. 758 0. 709	1, 302 1, 337 1, 334 1, 334 1, 348 1, 368

BUFFALO, NEW YORK.-ALTITUDE 585 PEET.

	TABLI	M	B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1866 inclusive, reduced to 33º Fahr. + 28 inches.	minimam	ı, sınd me	en baron	netrical p	ressure fo	or each m	onth fro	n the yea	r 1859 to	1866 incl	andve, re	duced to	32° Fahr	+ 28 T	sper.	•
Year.		July.			August		2	September.			October.		Ž	November.		A	December.]	
	Max	M(la.	Moan.	Mer	Min.	Mean.	Max.	Mla.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Xex.	KGp	Moan.	
1859 1960 1961 1963 1964 1965 1966	1. 875 1. 806 1. 582 1. 731 1. 657 1. 668 1. 680	1, 047 1, 080 1, 084 0, 990 1, 183 1, 038 1, 038	1, 428 1, 338 1, 320 1,	1. 665 1. 618 1. 618 1. 741 1. 822 1. 567 1. 663	1, 092 1, 192 1, 192 1, 111 1, 015 1, 065	11.1406 11.1406 11.1406 11.288 11.388	1 886 1 1 866 1 1 840 1 1 851 1 1 863 1 1 863	0,982 1,059 0,822 0,873 1,010 1,168 0,912	1,428 1,390 1,368 1,468 1,437 1,437	1, 767 1, 699 1, 786 1, 735 1, 537 1, 537 1, 118 1, 118	1.000 0.1988 0.1988 0.1988 0.1988 0.1988 0.1988	1.419 1.414 1.378 1.383 1.436 1.228 1.323	1.956 1.728 1.735 1.822 1.822 1.845 9.007	0.0843 0.0878 0.0878 0.0866	1, 483 1, 289 1, 280 1, 401 1, 392 1, 382 1, 347	1, 965 1, 965 1, 965 1, 986 1, 761 1, 761	00000000000000000000000000000000000000	1. 453 1. 453 1. 453 1. 458 1. 458 1. 458 1. 458	BIOLLI OF 1.

BUFFALO, NEW YORK. + 28 INCHES.

TABLE B.—Maximum, minimum, and mean barometrical pressure reduced to 38º Fabr., for seasons.	Winter. Year,	Min. Mean, Max. Min. Mean,	0.566 1.4415 p. 007 0.033 1.387 1.334 0.073 1.387 1.334 0.073 1.386 1.387 1.334 0.083 1.387 1.334 0.083 1.387 1.334 0.083 1.387 1.383 1.441 0.083 1.383 1.441 0.083 1.383 1.441 0.083 1.383 1.441 0.083 1.383 1.441 0.083 1.38
ressure rec	Þ	Mar	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
netrical p		Mean.	11111 2882 242
san baron	Autuma.	MGr.	20.0.0.0 20.0.0.0 20.0.0.0.0 20.0.0.0.0.
o, and me		Mex	1. 956 1. 862 1. 940 1. 908
minima		Mean.	1. 329 1. 348 1. 352 1. 374
(extmum,	Summer.	ĘĘ.	2.1.2.0 9.00 9.00 0.00 0.00 0.00
LE B.—M		Max.	1.0823
TAB		Moan.	1.333
	Spring.	Mfp.	0890 7980 0880 0980 0980 0980 0980 0980
		Max.	9, 007 1, 859 1, 825 1, 871
	Year.		1860 1860 1861 1861 1863 1863

TABLE B.—Maximum, minimum, and mean temperature for each mouth from the year 1839 to 1866 inclusive.	February. March. April. May. June.	Max, Min. Mean, Max, Min. Mean, Max, Min. Mean, Max, Min. Mean, Max, Min. Mean,	57 23 28.6 67 17 35.9 68 25 42.4 84 37 59.7 83 51.7 81 43 65.0 <	August, Soptember. October. November. Desember.	Max. Min. Mean. Max. Min. Mean. Max. Min. Mean. Max. Min. Mean. Max. Min. Mean.	83 2 4 2 8 2 4 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6
Maximum, minimum, and		Mean.	612 6 6 7 8 8 4 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8		Mean.	25
TABLE B.—		Mean. Max.	2000 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	▼		\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$
	January.	Max. Min.	23 25 25 25 25 25 25 25 25 25 25 25 25 25	July.	Max. Min.	8828288 8348288
	Year.		1859 1860 1861 1861 1863 1865 1865			1859 1861 1863 1863 1864 1864

BUFFALO, NEW YORK.

,		•	BEIONI OF II
	Range.		នទំនងវិទីន
		Mean.	64444 14684 146864
clusive.	Your.	Kin.	,
to 1866 tn		Max.	828282
om 1859		Mean.	8888888 1158478
d years fr	Winter.	Min.	72007F
TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1866 inclusive.		Mex.	828282
are for se		Mean.	94.12.12.02.02.12.02.04.12.02.00.00.00.00.00.00.00.00.00.00.00.00
temperat	Autumn.	Min.	8082538
nd mean		Max.	88 48 8 4 8 4 8 4 8 4 8 4 8 4 8 4 8 4 8
ılmum, s		Mean.	\$25588 948988
nam, mh	Summer.	Mfn.	3288238
.—Maxi		Max.	\$\$\$ \$\$ \$\$
TABLE I		Mean.	\$\$\$\$\$\$\$ \$0+0 \$0+0 \$0+0 \$0
	Spring.	Min.	
		Max.	2688266
	Year.		1839. 1860. 1861. 1862. 1863. 1864.

BUFFALO, NEW YORK.

Year.			TAS	3LE B.—.	TABLE B.—Amount of rain and melted snow for months,	of redo ea	d melted	mow for	r months,	. seasons,	and year	r, in Un	Hed States	sessons, and years, in United States inches and decimals	decimals		
	Jan.	Feb.	March.	April.	May.	June.	July.	Aug.	Sept.	Oct.	Nov.	Dec	Spring.	Summer.	Autumn.	Winter.	Year.
1859 1960 1961 1961 1964 1964 1966 1966 1966	7-144441 8328878	0 4 4 4 1 4 1 4 1 4 1 4 1 4 1 4 1 4 1 4	-: 444 -:		ଷ୍ଟ୍ୟାମ୍ୟ ଖୁ ଏ ଅପ୍ୟୟ ଅଷ୍ଟେକ	4141144 8282428	04444111 88408088 888	444444444 25288288	14441944 12882833	-: 4444444 8888 88255	1.4.4.4.4.4.6.4.4.6.2.4.4.2.2.2.2.2.2.2.2	41141464 58855828	4.9.8.4.9.1.8 888 & 5.4.4		4.1122.5.01.44 88882.228	点 5 4 4 5 4 6 4 6 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	2282828 2282828

	TABLI	1 B.—M	rdmum,	minim	, and me	an baron	setrical p	ressure f	E BMaximum, minimum, and mean berometrical pressure for each month from the year 1859 to 1866 includive, reduced to 32º Fabr. +	onth fro	n the year	r 1859 to	1866 inc	tastve, re	d peonp	32º Fab	188+	28 inches.
Your.		January.			February.	_		March.			April.			May.			San S	
	Max.	녍	Meen.	Max.	Kla	Mean.	Max.	ri Kir	Mosn.	Ker	Kin.	Moen	Kax.	Kir.	Mean.	Max.	Kfb.	Mean.
1859 1860 1861 1863 1864 1865	9 9 466 9 445 9 9 168 9 9 250 9 9 57	1,193 1,143 0,958 1,110 1,110 1,195 1,256	1,763 1,855 1,740 1,707 1,707 1,881	4 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1.068 1.223 1.145 1.140 1.218 1.218	1,668 1,834 1,632 1,736 1,736 1,806	9 1 1 9 3 10 1 9 3 4 00 2 0 1 2 1 9 3 4 00 2 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1.810 0.905 1.891 0.963 0.963 1.198	1,744 1,625 1,759 1,759 1,671 1,659 1,761	al a	1, 257 1, 250 1, 250 1, 406 1, 406 1, 116 1, 116	1 679 1 834 1 727 1 721 1 744 1 744	1 980 1 980 1 980 1 980 1 990	0.836 1.839 1.274 1.196 1.043	1.688 1.688 1.689 1.609 1.692 1.577	1 90 1 90 4 8 90 4 90 90 90 90 90 90 90 90 90 90 90 90 90	1.353 1.309 1.104 1.254 1.400 1.145	1.654 1.654 1.756 1.756 1.654
		July.			Angust		đ	September			Detober.		ž	November.		A	December.	
	Max.	Min.	Mean.	Max	Min.	Mean.	Max.	Ę	Mean.	Max	Min	Mean.	Mer	ri K	Monn.	Max.	Min.	Mean.
1859. 1860. 1861. 1863. 1865.	41119199999999999999999999999999999999	1, 140 1, 140 1, 306 1, 306 1, 383 1,	1 653 1 653 1 650 1 657 1 745 1 715 1 715	1. 1. 893 1. 999 1. 999 1. 974 1. 975 1. 975	11111111 24444 2444 2444 2444 2444 2444	1,721 1,721 1,735 1,735 1,735 1,735 1,735	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	11.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	11.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1	1.77 1.77 1.73 1.819 1.90 1.90 1.90	4 5 5 5 6 5 6 5 6 5 6 5 6 5 6 5 6 5 6 5	0.963 1.151 1.806 1.306 0.876 1.800 1.635	11.1.1.604 17.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.	4 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	1.1.068 1.1.266 0.965 0.965 0.978	1.764 1.858 1.757 1.658 1.769 1.769

BUFFALO, NEW YORK.

			TABLE B	Maxic	one, mk	dimum, e	TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1866 inclusive.	temperat	are for a	SECORE SE	d years 1	rom 1859	to 1866 i	belusive.		
Year.		Spring.			Summer.			Autumn.			Winter.			Your.		Renge.
	Max.	Min.	Mean	Max.	Min.	Mean.	Max.	Min.	Meen.	Max	Min.	Mean.	Max.	Min.	Meen.	•
g							E	8	9							
	20.5	71	9	28	3	98	200	0	46	6	۳ ا	ន្តន	28	† {	47.1	8
1962	8.2	77		88	38	5 6	28	8 %		85	1	- e	8	7 - 1 1	47.4	<u> </u>
1863 1864	82	410	4 5 0 0	28	85	6.8 8.0	18	22	5, & 0, &	88	00	8 2.7 2.4	38	ه ه ا	46.5	4 8
	22	es re		8.8	\$ 8	69.00 69.00	88	23 23		28	<u> </u>	8 8 7 8	8.2	ក្ត 	47.4	នីន
	:						-							,		}

BUFFALO, NEW YORK.

Year.			TA	BLE B	Amount (of rain az	nd melted	l snow fo	r months	, seasons,	and yes	re, in Un	IABLE B.—Amount of rain and melted snow for months, sessons, and years, in United States inches and decimal	inches and	decimals		
	Jan.	Feb.	March.	April.	May.	June.	July.	Aug.	Bept.	Oct.	Nov.	Dec	Spring.	Summer.	Autumn.	Winter.	Year.
1859 1860 1961 1963 1963 1964 1964 1965 139	11.000001 12338 11338	0.9999.191 253294 2932 294	1.8.73.9.1. 28.89.1.4.1. 28.89.1.4.1.	1.03 1.74 1.74 1.90 9.90 9.26	역보다. 경험 20 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	91.191.19 9883.45 45.45	0444444 84080848	40000000000000000000000000000000000000	1.444.1944 2288888		こよるななないよ 乳の効性を下めぬ	41141444 58888828	4.000001.00 88885546	ლ ფ ფ ფ ფ გ ც. 20 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	4.1.1.0.1.0.1.0.1.0.1.0.1.0.1.0.1.0.1.0.	444544 8538238	5288888 5288888

			H	TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1866 inclusive.	Maxim	ım, minis	num, and	mean te	mperatur	e for eac	h month	from the	year 1854	9 to 1866	inclusive.			
Your.		July.			August.		82	September.	_		October.		×	November.		A	December.	
	жуж.	TI DE	Mean.	Mex.	Min.	Mean.	Max.	Min.	Moan.	Max.	Min.	Mean.	Max.	Min	Moen.	Max.	Ę	Mean.
1859 1860 1861 1863 1864 1865 1865	######################################	888 888	86.88.85.55 8.48.85.55 8.48.95.55 8.69.95 8.60.95 8.60.95 8.60.95 8.60.95 8.60.95 8.60.95 8.60.95 8.60.95 8.60.95 8.60.95 8.60.95 8.60	8888888	88888484	25.17.05.17.	258885 8 2	4844448	88.00 8.00 8.00 8.00 8.00 1.00 1.00 1.00	13288181	KKKARRK	26.00 20.00	82888238	82888282	98 98 98 98 98 98 98 98 98 98 98 98 98 9	84888422	86954588	නුනුනුපුනු සම්බන්ධ නුතුවනු සම්බන්ධ නුතුවනු

FORT NIAGARA, NEW YORK.

<u> </u>			CABLE B.	-Maxim	um, mu	dmum, s	TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1866 inclusive.	temperat	are for se	as suose	d years f	rom 1859	to 1886 is	aclusive.		
Year.		Spring.		æ	Summer.		7	Antumn.			Winter.			Year.		
<u> </u>	Max	Min.	Koan	Max.	Min.	Mean.	Max.	Min	Mogn.	Max.	Min.	Mean.	Max.	Mer	Mosn.	
1859 1860 1861 1863 1865 1865	\$£2825\$8	8 + 8 10 8 + 8	43.0 39.9 41.8 41.8 40.1	8888888	12 4 22 4 2 4	26.25 26.25 26.25 26.25 27.74	27 28 28 28 28 28 28 28 28 28 28 28 28 28	ZEEBEEE	25.02.04.02 0.10.04.02 0.10.04.03 0.10.04.03	8788842	11	9.2.2.2.2.2. 9.2.2.2.2.2. 9.2.2.2.2.3.	&&&&&&&&	1 1 9 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	\$\$\\\ \$\\\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\	& & & & & & & & & & & & & & & & & & &

FORT NIAGARA, NEW YORK.

A			TAE	3LE B.—	Amount c	of rain an	d melted	EDOW for	months,	Beagons,	and year	, in Uni	ed States !	IABLE B.—Amount of rain and melted mow for months, seasons, and years, in United States inches and decimals.	decimals.	,		_
į	Jan	Feb.	Mar.	April.	May.	June.	July.	Aug.	Sept	0ct	Nov.	Dec	Spring.	Summer.	Summer. Autumn.	Winter.	Year.	
1859 1860 1.681 1863 1863 1864 1864 1866 1.63 1.63 1.63	1.0000011 88218552	4 4 1 1 9 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	1444441 1618228	1.1.1.1.044 8.888888		1.4440014 2222888	144444444 82888433	94449444 888885558	역 내 속 내 내 박 속 영향 등 약 등 충 등 등 장	1-18844441 \$5.488888	844-14-104 1184-2588		44.444444 32.8445553	αςςςς;4° 6.48888889	& 5 1 1 5 4 5 4 5 4 5 6 5 6 5 6 5 6 5 6 6 6 6 6	9944444 68277984	88888299 3866824	

CHARLOTTE, NEW YORK.—ALTITUDE 273 FEFT.

			l				.						ē ·	B.—Marimum, minimum, and mean dryddeutent pressure for escu moniu ifom the yest 1839 to 1860 inclumys, feduced to 137 fabt. + 26 inches.
February.			•	March.		,	April			May.			June.	
Mean. Max. Min.		Moan.	Max	Kth.	Mean.	Max.	Kth.	Mosn.	Max.	Min.	Meen.	Max.	Min.	Meen.
1.777 8.155 0.686 1.777 8.155 0.686 1.779 8.155 0.685 1.779 8.183 0.890 1.748 9.289 1.187 1.682 9.296 1.157 1.857 2.496 1.250	<u> </u>	1.740 1.740 1.713 1.571 1.730	9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	1.133 1.133 1.225 1.225 1.325 1.132	111709 111709 111709 111009 111009 111009	004 and and and and and and and and and and	0.735 0.735 0.735 0.735 0.735 0.835 0.835	1.648 1.648 1.674 1.674 1.074	1. 958 1. 958 1. 958 1. 958 9. 103	1.238 1.230 1.073 1.073 1.178 0.962	1.637 1.611 1.639 1.675 1.361 1.361	1.857 1.854 1.821 1.821 1.854 1.854	0.970 1.273 1.273 1.373 1.373 1.373	11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

273 FEET.
-ALTITUDE
YORK.
E, NEW
CHARLOTTE

	TABLE	1 B.—Ma	ximum, 1	B.—Maximum, minimum, and mean berometrical pressure for each month from the year 1859 to 1886 inclusive, reduced to 33º Fahr. + 38 inches.	and mea	n barom	strical pr	essure fo	r each m	onth from	the year	r 1859 to	18 66 in eli	asive, re	duced to	32° Fabr	. + 28 Li	ches.	
Year.		July.		•	Angust.		2 2	September.			October.		Z	November,			December.		
	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Ma.	Moan.	Max.	Man	Moan.	
1859 1860 1861 1863 1864 1866	11.903 11.903 11.903 11.998 11.998 11.998	1, 286 1, 286 1, 367 1, 363 1, 473 1, 466 1, 366	1111688 1111688 111688 11689 11689	1. 896 1. 94 1. 94 1. 94 1. 94 1. 94 1. 33 1. 94 1. 33 1. 98 2. 34 1. 34	1.373 1.386 1.431 1.431 1.175 1.175	1, 674 1, 727 1, 727 1, 750 1, 750 1, 768 1, 768	4 4 4 4 4 4 4 4 6 6 6 6 6 6 6 6 6 6 6 6	1, 285 1, 286 1, 286 1, 286 1, 286 1, 286	1.727 1.770 1.774 1.766 1.8819 1.882 1.801	4444444444	1, 216 1, 216 1, 191 1, 354 1, 366 1, 366	1,1,248 1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.1.029 1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.	11111111111111111111111111111111111111	4444444 48828832	0.892 0.892 0.892 0.892 0.873	277.1. 287.1. 287.1. 128.1. 161.1. 161.1. 087.1.	DI CILI CI II

CHARLOTTE, NEW YORK. + 28 INCHES.

RET	AKY	OF	WAR.
	Renom		1. 403 1. 783 1. 608 1. 659 1. 659 1. 659
•		Mean.	1,1673 1,707 1,707 1,738 1,738 1,738
None.	Year.	Min.	0.930 0.945 0.795 0.795 0.867
ir. for se		Max.	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
o 32º Fal		Mean.	1.7.1.1.1.2.1.1.1.1.1.1.1.1.1.1.1.1.1.1.
reduced t	Winter.	ij	00 00 00 00 00 00 00 00 00 00 00 00 00
pressure		Max	4 4 4 4 4 4 4 4 조수 등 등 등 등 등 등 등 조수 등 등 등 등 등 등 등
metrical 1	٠	Mean.	1071 1071 1071 1071 1071 1071 1071 1071
esn baro	Autumn.	Min.	1, 029 1, 102 1, 102 1, 151 1, 151 1, 169 0, 704
n, and m		Max.	4 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
minimar		Mean.	1.05.0 1.
eximum,	Summer.	Min.	0.970 1.255 1.255 1.066 1.080
.z B.—M	TABLE B.—Maximum, minimum, and mean barometrical pressure reduced to 33° Fahr, for seasons. Summer. Autumn. Winter. Winter. Wax. Min. Mean. Max. Min. Mean. Max. Min. Mean. Max. Min.		4 - 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
TABI		Moan.	1, 656 1, 656 1, 1, 1, 688 1, 1, 1, 1, 688 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1
	Spring.	Min.	1.079 0.679 0.968 0.823 0.883
		Max.	9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9
	Year,		1869 1960 1967 1967 1967 1964 1966

CHARLOTTE, NEW YORK.

		.44.	BIORI OF IE
		Meen.	84888558 868844
	June.	Min.	288888
·		Max.	882888
5 inclusiv		Mean.	2000 2000 2000 2000 2000 2000 2000 200
59 to 186	May.	Min.	******
TABLE B.—Maximum, minimum, and mean temporature for each month from the year 1859 to 1866 inclusive.		Max.	666644
h from th		Mean.	44444444 8061400
ch mont	April.	Min.	8887848
ire for ea		Max.	5662866
emperato		Mean.	8488994
d mean t	March.	Min.	
mam, so		Max.	8 2 4 4888
um, mioi	÷	Mean.	2342342 220-241
—Maxim	February.	Min.	
ABLE B.		Max	323436
		Mean.	2889288 220020
	January	Kū.	1 1111
		Max.	2452248
	Year.	•	1839 1860 1861 1861 1862 1863 1863 1864 1866 1866 1866 1866 1866

CHARLOTTE, NEW YORK.

RET!	LRY	OF	WAR.
		Mean.	2222222 22222222 22222222
	December.	Klp	0 - 0 - 0 - 0 - 0
•	1	Kar	8523223
inclusive		Moan.	11.088 88.41.1 48.44.44.00.11
TABLE B.—Meximum, minimum, and mean temperature for each month from the year 1859 to 1966 inclusive.	November.	FCID.	8 6 6 8 1 8 9 4
s year 18	-	Kax	2424126
from the		Moan	2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.
ch month	October.	병	8821288
re for ea		Ker	\$2528528
emperatu		Moen.	888829988 88111988
d mean t	September.	ų Į	*********
mam, so		Kax	8222223
um, mini		Moan.	8.8.8.5.4.4.5.8 01.7.8.8.8
—Maxim	August.	M(in.	E2419888
CABLE B.		Ę	8888888
		Keen	8.8.8.5.1.4.8.4 8.8.8.5.1.4.8.4
	July.	별	2444484
		Max.	8882288
	Year.		1859 1860 1861 1865 1865 1865

YORK
NEW
CHARLOTTE

	!		TABLE I	.—Maxi	num' unt	nimum, 4	nd mean	tempera	ture for s	TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1966 inclusive.	id years	from 185) to 1868	inclusive	فد	
Year.		Spring.		uQ.	Summer.		7	Autama.		•	Winter.			Year.		8
	Max.	Min.	Mean.	Max.	Min.	Mean.	Max	Min.	Mean.	Max.	Ķ	Mean.	Max.	Min.	Mean.	
1859 1960 1960 1963 1964 1964 1966	22222		44444444 147282	888888	482288 8	86.00 L	85533597	20 00 00 00 00 00 00 00 00 00 00 00 00 0	25.00.00.00.00.00.00.00.00.00.00.00.00.00	8428888	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	882888		227.00002.	90 110 94 107 1111 1201

CHARLOTTE, NEW YORK.

	Year.	882282 3688588
١ ١		
	Winter.	4446464 4624888
lecimals.	Autumn.	සු පු පු පු පු පු පු කිසි කිසි පි පි පි පි පි කිසි කිසි පි පි පි පි පි
nches and	Summer.	කුලකුදු ඇතු කුල එම කිසියි කි
IABLE B.—Amount of rain and melted mow for months, seasons, and years, in United States inches and decimals.	Spring.	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
s, in Onl	Dec.	488228
and year	Nov.	44144444 24585518
Beasons,	Oct.	그목욕요느록속 요합있识器指한됐
r months,	Sept	828258328
mow for	Aug.	4-1441414 8288222
ıd melted	July.	44444444 86828488
of rain au	June.	4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.
Amount	May.	18111848 8484894
BLE B.—	April.	-: 44 C 44 44 44 52 88 88 88 88 88
TA	Mar.	
	Feb.	8248438 6116161
	Jan.	2111441 8352438
ă A	į	1859 1861 1861 175 1863 1171 1864 1864 1965 1966 1966 1966 1966

SACKETT'S HARBOR, NEW YORK.—ALTITUDE 266 PEFT.

	TABL	z B.—Ma	atman,	B.—Maximum, minimum, and mean barometrical pressure for each month from the year 1859 to 1866 inclusive, reduced to 33º Fabr. + 98 inches.	, and me	an barom	etrical pr	regions fo	r each m	onth fron	n the yea	ur 1859 to	1866 inc.	lusive, re	duced to	32º Fahr	. + 98 in	chos.
Year.	7	January.		H	February.			March.			April.			May.			June.	
	Max.	Mū.	Mean.	Max.	Min.	Mosn.	Max.	Min.	Mean.	Max.	Min.	Meen.	Max.	Min.	Mean.	Max	Mîn.	Mean.
1859. 1860. 1861. 1862. 1863. 1865.	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1, 293 1, 215 0, 867 1, 114 0, 836 1, 301 1, 208	1.690 1.681 1.793 1.759 1.660 1.676	20 20 20 20 20 20 20 20 20 20 20 20 20 2	1,034 0,996 1,090 1,090 1,067 1,107 1,107	1. 748 1. 652 1. 707 1. 873 1. 563 1. 775 1. 775	2 4 4 1 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	1. 148 1. 180 1. 051 1. 051 1. 161 0. 918 0. 859 1. 201	1.585 1.725 1.578 1.730 1.666 1.705	9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	1, 093 1, 148 1, 243 0, 823 1, 349 1, 162 0, 767	1.656 1.666 1.843 1.708 1.677 1.689	1.949 2.081 1.954 1.954 1.948 1.874 1.969	1, 078 0, 810 1, 956 1, 981 1, 280 0, 944	1, 664 1, 610 1, 676 1, 661 1, 554 1, 646 1, 548	1. 984 1. 902 1. 902 1. 911 1. 999 1. 918 1. 947	1,045 1,246 1,273 1,106 1,068 1,368 1,368	1, 579 1, 638 1, 638 1, 638 1, 719 1, 719
		July.			August		8	September.			October.		Z	November.		а	December.	
	Max.	Mîn.	Mean.	Max.	Min.	Mean.	Max.	Min	Moan.	Mar.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.
1859 1860 1861 1863 1864 1866	92 200 1 1 892 1 1 892 1 1 892 1 1 890 1 1 1 890 1 1 1 890 1 1 1 890 1 1 1 890 1 1 1 890 1 1 1 890 1 1 1 890 1 1 1 890 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1, 245 1, 306 1, 315 1, 305 1, 337 1, 337 1, 337	1 1 2 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	1.1.1.1.931 1.994 1.994 1.059	1, 379 1, 438 1, 438 1, 405 1, 523 1, 340 1, 375	1.636 1.653 1.737 1.706 1.708 1.708 1.708 1.708	44414444 6444444 6444444444444444444444	1. 147 1. 336 1. 336 1. 345 1. 364 1. 364	1,727 1,773 1,731 1,748 1,893 1,786 1,737	44444444444444444444444444444444444444	1, 200 1, 158 1, 158 1, 157 1, 426 1, 999 1, 308	1.690 1.784 1.784 1.574 1.574 1.675	4444444 8352325 888884 84844 848444	1, 049 0, 995 1, 135 1, 195 1, 105	1. 789 1. 598 1. 598 1. 598 1. 662 1. 653	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	1 134 0 901 1 239 1 1 156 1 1 130 1 1 130	1.1.1.2.2.1.1.1.2.2.2.1.1.1.2.2.2.1.1.2.2.2.2.1.1.2

			TABL	2 B.—M	szimum,	minimas	TABLE B.—Maximum, minimum, and mean barometrical pressure reduced to 32º Fabr., for seasons.	san baros	netrical 1	ressure 1	educed (0 32º Fa	dr., for s	sasons.		
Year.		Spring.		- C	Summer.		7	Autump.			Winter.			Year.		
•	Kar	Ą	Mean.	Max.	Min.	Mean.	Max.	Min.	Mean.	Max.	Min.	Meen.	Max.	Min.	Meen.	Kanga.
1859 1960 1965 1965 1965 1965 1966	25. 25. 25. 25. 25. 25. 25. 25. 25. 25.	1.078 0.810 0.823 0.918 0.839 0.767	1, 636 1, 666 1, 669 1, 699 1, 617 1, 641	1.1 et et 1 et et 1 994 et 1 999 et 1 9	1.045 1.246 1.273 1.106 1.088 1.340 1.191	1 618 1 655 1 655 1 655 1 668 1 719 1 719	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	1.042 0.995 0.995 0.995 1.151 1.904 0.951 0.999 1.258	1,734 1,711 1,679 1,739 1,739 1,739 1,768	9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	1.034 0.901 0.867 1.080 0.836 1.107 1.130	1,740 1,699 1,775 1,800 1,681 1,683 1,795	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0.810 0.810 0.867 0.883 0.836 0.836	1.675 1.683 1.710 1.737 1.639 1.703 1.703	1, 456 1, 689 1, 738 1, 738 1, 459 1, 459

BACKETT'S HARBOR, NEW YORK.

-			H	TABLE B.—Maximum, minimum, and mean temperature for each month from the year 1859 to 1866 inclusive.	-Maxim	m, minin	num, and	mean te	mperatu	re for eac	b month	from the	year 180	19 to 1868	Inclusiv	ė		
Year.		January.	_	F	February.			March.			April			May.			June.	
	Max.	Min.	Mean	Max.	MG.	Moan	Max	Kin.	Mean.	Max.	Min.	Mosn.	Kar	M.	Mean.	Max	Kin.	Mean.
1859 1861 1861 1862 1863 1865 1865 1865 1865 1865 1865 1865 1865	22,2388	1111111 2828838	22.00.00 20.	2222242	777777	885888 8718808	88888	35 7 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	88998899 4404436	252225E	1287848	4414417 0040268	2828282	88888	20 23 23 23 23 23 0 4 50 4 20 20	888338	8485682	2ු සි යු සි සු සු සට සහසා 20 20

SACKETT'S HARBOR, NEW YORK.

,		,	REPORT OF T
		Mean.	888888888 4888848
	December.	Min.	22,02,02,00
	А	Max.	24248882
inclusive		Mean.	90.8 40.3 40.3 1.6
9 to 1866	November.	Ė	8929222
year 185	Z	Max	23 38 8 27 6 27 6 27 6 27 6 27 6 27 6 27 6 2
TABLE B.—Maximum, minimum, and mean tempersture for each month from the year 1859 to 1866 inclusive.		Mean.	25.05.05.05.05.05.05.05.05.05.05.05.05.05
th month	October.	Min.	8888888
re for eac		Mex	5158858 51588 51588 5158 5158 5158 5158
emperatu		Mean.	88.00 66.00 67.00 7.00 88.00 7.00
d mean t	September.	Mîn.	18358282
man, and	82	Rax.	£ £ £22888
am, mbi		Mean.	68.9 67.6 71.17 77.0 73.0 86.3
Maxim	August,	Min.	22363223
ABLE B.		Max.	2823823
F		Mean.	886.84484 147.84484
	July.	jų Į	\$23882\$
		Max	81288838
	Year.		1826 1860 1861 1863 1863 1865

SACKETT'S HARBOR, NEW YORK.

ARY	OF	W.	AR.
	Range		108 109 109 118 118
		Mean.	4444444 4444444 8081448
TABLE B.—Maximum, minimum, and mean temperature for seasons and years from 1859 to 1866 inclusive.	Year.	Min.	व्यवस्थ
9 to 1866		Max.	282388
rom 185		Mean.	25 25 25 25 25 25 25 25 25 25 25 25 25 2
d years	Winter.	Ķ	3 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7
easons an		Max.	222233
ture for s		Mean.	20.03.03.03.03.03.03.03.03.03.03.03.03.03
tempers	Autum.	Min.	822222 5
nd mean		Max.	2222333
ofmum, e	_	Mean.	20.00 20.00
num, mh	Summer.	Min	1185353
8.—Maxi		Max.	288888
TABLE]		Mean.	88.41.46.4 6000000
	Spring.	Kb.	21 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
		Mex.	288832
	Your.		1859 1860 1861 1868 1868 1864 1864

Year. Jan. Feb. 1.88 1.88 1.88 1.88 1.88 1.88 1.88 1.	 TABLE B. 1.39 1.36 2.70 2.68 2.70 2.78 2.36 2.37 2.38 3.34 3.34 3.34	April. May. June. July. Aug. Sept. Oct. Nov. Dec. Spring. Summer. Autumn. 1.69 3.28 2.64 0.86 4.16 5.19 2.96 4.18 1.89 2.10 4.8 2.10 4.8 2.00 3.77 3.64 6.39 1.30 3.86 2.18 6.39 3.78 1.30 3.30 1.64 2.48 2.10 4.8 3.40 4.8 3.40 5.44 4.79 2.90 3.79 3.64 3.39 1.30 3.86 1.86 3.30 3.74 3.78 3.34 2.48 3.40 4.88 3.40 5.44 4.79 2.90 3.79 3.40 5.44 4.79 2.90 3.70 3.60 3.79 3.80 13.80 3.70 3.70 3.80 3.70 3.80 3.70 3.80 3.70 3.80 3.70 3.70 3.80 3.70 3.70 3.80 3.70 3.70 3.80 3.70 3.70 3.80 3.70 3.70 3.80 3.70 3.70 3.80 3.70 3.70 3.80 3.70 3.70 3.70 3.70 3.70 3.70 3.70 3.7	of ratio	July. 1.98 9.98 9.98 9.98 9.98 9.98 9.98 9.9	Aug. Aug. 1988 9.9.9.9.9.9.9.9.9.9.9.9.9.9.9.9.9.9.	Rept. 29.00	Oct 4 8 8 8 8 4 4 4 4 4 4 4 4 4 4 4 4 4 4	Nov. Nov. 16 25 25 25 25 25 25 25 25 25 25 25 25 25	7, in Univ. 18 19 19 19 19 19 19 19 19 19 19 19 19 19	Spring.	Summer. Summer. 9.80 12.46 2.80	Summer. Autumn. 8.80 13.86 13.80 6.89 13.80 6.89 6.89 13.80 6.89 6.89 79.80 13.80 6.89 6.89 6.89	Winter. 8.65 11.44 3.88	Y 98.76 99.76 99.76 99.76 99.76 99.76
· *- ^4 La	444		1144	1-9-i 1-85				5 8 8 2 8 8 2		4.0.0 8.88	다하다		9 5 5 2 5 5 3 5 5	(488

Table C.—Maximum, minimum, and mean of the barometrical pressure reduced to 32° Fahr. + 28 inches.

A	years.	,	anuary.		F4	February.			March.			April.			May.			June.) ;	
	No. of	Max.	Min.	Mean.	Max.	Min.	Monn.	Mex.	Kin	Moan.	Max.	Min.	Mean,	Max.	Min.	Mean.	Max.	Min.	Mean.	
Superior City Contonagon Marquette Milwantee Milwantee Grand Havon Thunder Bay Liland. Thunder Bay Liland. Monroc City Monroc	1464446814464	99999999999999999999999999999999999999	00000000000000000000000000000000000000	11.343 1.2885 1.2885 1.3886 1.440 1.7386 1.7386 1.7386 1.7386	11.1.1.1.999 9.998 9.998 9.998 9.998 9.55 9.55 9	0.000 0.000	11.386 11.386 11.386 11.386 11.386 11.386 12.386 17.738	2 2 311 2 2 311 2 2 311	0.000000000000000000000000000000000000	1, 303 1, 286 1, 286 1, 229 1, 279 1, 272 1, 272 1, 264 1, 564 1, 654	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	245 245 245 255 255 255 255 255 255 255	1.336 1.336 1.336 1.337 1.337 1.337 1.739 1.739	1.738 1.730	00000000000000000000000000000000000000	1.23 1.23 1.23 1.23 1.23 1.23 1.23 1.23	733 733 734 736 737 737 737 737 737 737 737 737 737	0.000000000000000000000000000000000000	11111111111111111111111111111111111111	

Table C.—Maximum, minimum, and mean of the barometrical pressure reduced to 32° Fahr. + 28 inches.

Mar. Min. Mean. Mar. Min. 1775 d. 6810 d. 717 d. 681 d. 778 d. 682 d. 778 d. 778 d. 683 d. 778 d. 778 d. 683 d. 778 d. 77	. Aud	years.		July.			August.		εč	September.			October.		Ž	November.		H	December.	
7 1.765 0.662 1.279 1.650 0.7 1.675 0.00 1.271 1.675 0.00 1.271 1.675 0.00 1.271 1.675 0.00 1.271 1.675 0.00 1.271 1.675 0.00 1.271 1.675 0.00 1.271 1.675 0.00 1.271 1.675 0.00 1.271 1.675 0.00 1.271 1.675 0.00 1.271 1.271 1.775 0.00 1.271 1.775 0.00 1.271 1.775 0.00 1.271 1.775 0.00 1.271 1.775 0.00 1.271 1.775 0.00 1.271 1.775 0.00 1.271 1.275 0.00 1.2		No. of	Max	Kin	Mean.	Max	KG	Mosn.	Max.	Klin	Mean.	Kat	ij	Moan.	Kar	Mila.	Moan.	Max	Mia.	Mosn.
7 2 2077 1 2018 1.6674 2.133 1.	Superfor City Marquette Marquette Marquette Grand Haven Thunder Bay Jaland, Tawas City Detroit Citereland Buffle Buffle Charkette Charlet Char	_		44444444444444444444444444444444444444			0.810 0.772 0.772 0.773 1.033 0.860 0.860 0.860 0.860 0.873 1.124 1.175	1 1 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	20 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.000000000000000000000000000000000000	11111111111111111111111111111111111111	90 90 90 90 90 90 90 90 90 90 90 90 90 9	00000000000000000000000000000000000000	28 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	99999999999999999999999999999999999999	0.000000000000000000000000000000000000	11111111111111111111111111111111111111	44:44444444444444444444444444444444444	0.000 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1.3863 1.443 1.443 1.745 1.765 1.765

TABLE C.—Maximum, minimum, and mean of the barometrical pressure reduced to 32° Fahr. for seasons and years, + 28 inches.

Spring.
Max. Min. Moan.
0
0.447
0.050
0,069 1.325
0.469
0.534
0, 380 1, 31H
0.670
- 0 7007

	Mean.	25.25.25.25.25.25.25.25.25.25.25.25.25.2		Mean.	161
June.	Klp.	*************	December.	Min.	
	Max.	888888588888	1	Max.	98888888888888888888888888888888888888
	Mean.	######################################	ن	Mean.	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
May.	KGn.	888888888888888	November.	Min.	1
	Max	22882222222		Max.	8889888 256666
	Mean.	######################################		Mean.	44444444444444444444444444444444444444
April.	Min.		October.	Min.	22222222222
	Max.	214124828284445		Max.	2885555588888
	Mean.	***************************************	ij	Mesa.	4449.000.000.000.000.000.000.000.000.000
March.	Min.		September	Kü	***************************************
	Max.	8288838555688		Max.	\$\$\\ \alpha\
	Mean.	1,5,5,2,2,2,2,2,2,2,2,2,2,2,2,2,2,2,2,2,		¥oen.	3.9.4.8 5.4.8.8.8.4.5.8.8.8.5 6.0.000000000000000000000000000000000
February.	Min.		August.	Min.	88844444848
	Max.	3423242241E88		Max.	2882889899
	Mean.	55 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		Mean.	228854455 528854455 528855
January.	Min.	24892,722,111,128	July.	Mia.	828281531483288
	Max.	2423342812822		Max.	2888888888888
Acets.	lo.oM	-0-1-4-8-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1	yours.	No.of	P0P14P8P7P7P
	SIRIIONE.	Buperior City Ontonagon Marquete Miwantee Grand Haven Thunder Bay Liliand Twas City Defroit Monroe City Oleveland. Buffelo. Fort Nigara. Charlotte Sackett's Harbor			Buperior City Ontonagon Marquette Milwantete Milwantete Grand Haven Grand Haven Thunder Bay Island Tawas City Mouro City Cloveland Buffalo Fort Niagura Charlotte Sackett's Harbor

TABLE D.—Maximum, minimum, and mean temperature for months, seasons, and years.

	remge.	111111111111111111111111111111111111111
	Mean.	88. 11. 25.54. 26.99. 26.90. 26.90. 26.90. 26.90. 26.90. 26.90. 26.90. 26.90. 26.90. 26.90. 26.90. 26.90. 26.90. 26.90. 26.90. 2
Year.	Min.	# F 1
	Max.	98 96 10.3 97 103 103 103 88 88
	Mean.	13. 28.28.28.29.00.00.00.00.00.00.00.00.00.00.00.00.00
Winter.	Min.	**************************************
	Max.	352222222222 262222222222222222222222222
	Mean.	62 44.8 45.0 45.0 47.8 85.2 85.2 85.2 85.2 85.2 85.2 85.2 85
Autumn.	Min.	1
	Max.	\$
	Mean.	8 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9
Summer.	Min.	888832658888
	Max.	28887188882 2388889 23888888888888888888888888
	Menn.	8
Spring.	NGB.	2000
	Max.	828282382382
years	No.of	
	Stations	Superior City Ontonagon Ontonagon Marquete Milwantee Grand Haven Grand Haven Tawas City Wonree City Cleveland Cleveland For Milkera Charlotte Sackett's Harbor

TABLE E.—Mean amount of rain and melted snow in United States inches and decimals for months, seasons, and years.

Stations.	No. of years.	Jan.	Feb.	March.	April.	May.	June.	July.	Ang.	Sept	Oet.	Nov.	Dec.	Spring.	Summer.	Autums.	Winter.	Y car.
Superior City	+	16 '0	1.59	1.44	20.03	9 %	5.34 5.34	3.01	3, 12	3.68	£ 33	1.31	0.71	. 36 35	88	7.31	86 %	25. 75
Marquette		28	2. 18 1. 78				2.2				8 9 8			80 80 80	86.93 98.93	8. 13 7. 94		
frand Haven	41-0	88:		04.0 5.83.5	858	# 5 i	~; eq.	81 SE :	583 666	8 2 8	Si Cal	여 3i	1.04. E.85	85	323	r. œ.	4.00 28	888 888
Detroit	0 00 1	28:	3								2 55 S			4 K	929	32.		
Monroe City		- a	- 6 5 5 5								5 8 3 6			. e.	⊋ 5; æ æ	. č.		
Baffalo		e e	£ 8								35			3 E	88	8.F		
Sackett's Harbor		2.3	- d								88			88	8 6	¥8		

	Mesn.	2252222223822222 2252242523822222		Mean.	20000000000000000000000000000000000000
Jane.	Ę.	*************	December.	K ja	1
	Ker	*#####################################	A	Max.	28828888888 9
	Mean.	######################################		Megn.	200 200 200 200 200 200 200 200 200 200
May.	Min.	222222222222222	November	TIN	1
	Max.	5788238228 57882584 5788258 57886 5788 5788		Max.	2393388855555 2
	Mean.	85.85.45.85.85.85.85.85.85.85.85.85.85.85.85.85		Mean.	4444445;35;36433 9224820000000000000000000000000000000000
April.	Min.	112220000000000000000000000000000000000	October.	Kin.	82488888888
	Max.	222282824225		Max.	288555588888888
	Mean.	**************************************		Мева.	22222222222222222222222222222222222222
March.	Min.	1 1 1 1 1 1 1 1 1 1	September.	Kin.	8888888888888
	Max.	8288843555888	2	Kex.	882882828888888888888888888888888888888
	Moan.	11111111111111111111111111111111111111		Mosm.	2248588885588885 8000000000000000000000000
February.	Min.		August.	Kin.	88844998484
	Max.	888831224488888		Max.	2882884888888888
	Мевр.	45 45 45 45 45 45 45 45 45 45 45 45 45 4		Mean.	84885465488855
January.	Min.	24 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	July.	Min.	84848453444884
	Max.	234233422262222		Max.	288895
years.	lo.oN	1011410111111	yours.	No.of	L011418117717
1	SHAROTH	Baperior City Outonagon Marqueite Miwankee Grand Haven Thunder Bay Island Tawan City Detroit Monroe City City veland Fort Niagara Charloite Garloite			Buperior City Ontonagen Marquette Milwankee Grand Haven Grand Haven Thunder Bay Island Tawas City Detroit Monroe City Cleveland Purfi Niagers Charlotte Sackett's Harbor

TABLE I.—Meteorological observations at Thunder Bay Island, in Lake Huron, Michigan, made under the direction of Lieut. Col. W. F. Raynolds, U. S. corps of engineers, brevet brigadier general, superintendent of the survey of the northern and northwestern lakes.

[Latitude of station, 45° 02' 17" N. Longitude W. of Greenwich, 83° 09' 26". Observer, J. J. MALDEN.

		redu	ced to Fahr.	Te:	mp.,	Va	por.	decimals.	ear sky.)			Wind			
Date,	Time of observation.	tal, U. S. inches and decimals.	ous, U. S. in- and decimals.	Dry bulb.	Wet bulb.	ches and decimals.	Humidity, Satura-	of rain and melte	Am't cloudiness. (10-sky er tirely overcast, 0-clear sky.	Observed direction; from whence,	Observed velocity; miles per hour.	Resolution and per l	reloci	ty, h	
	Time	Total,	Gaseous, 1 ches and d	Dry	Wet	Elasticity, ches and	Hum	Am't	Am't tirely	Obser	Obse	N.	8.	E	K
October, 1865	0. 30 0. 30 0. 30 0. 30 0. 30 0. 30 0. 30 0. 30 0. 30 0. 30 0. 30 0. 30 0. 30 0. 30 0. 30	29, 446 29, 270 29, 198 29, 262 29, 354 29, 275 29, 405 29, 367 29, 299 29, 299 29, 298 29, 259 29, 259 29, 365	29, 331 29, 175 29, 199 29, 199 29, 196 29, 196 29, 073 28, 934 29, 019 29, 205 29, 205 29, 219 29, 205 20, 205 205 205 205 205 205 205 205 205 205	543. 3 6 6 5 6 2 . 8 6 6 5 . 9 2 5 4 2 . 7 3 4 . 5 5 6 2 . 8 2 2 5 . 6 6 5 . 9 2 5 . 6 6 5 . 9 2 5 . 6 6 6 1 2 0 . 5 5 7 . 4 4 3 . 8 7 6 1 . 7 8 3 8 . 7	40. 7 51. 1 59. 5 61. 7 51. 2 40. 4 32. 3 20. 7 14. 6 18. 4 23. 5 31. 3 38. 5 50. 5 55. 7 41. 1 35. 3	226 348 471 505 348 226 163 100 067 082 112 200 338 401 460 232 180	. 744 . 724 . 726 . 729 . 761 . 813 . 775 . 775 . 762 . 802 . 804 . 773 . 721 . 648 . 680 . 756 . 756 . 756 . 756 . 756 . 776 . 819 . 770 . 780 . 826 . 774 . 741		7.4 7.3 8.6 6.1 7.1 4.0 6.1 5.6 6.7 9.1 4.5 6.7 9.1 4.5 6.7 9.1 4.7 5.7 9.7 9.7 9.7 9.7 9.7 9.7 9.7 9.7 9.7 9	S. 85 W. N. 60 W. N. 62 W	1.0 4.2 2.4 2.0 3.6 3.2 1.7 2.5 4.6 6.2 3.9	67. 4 34. 8 86. 2 43. 4 110. 5 82. 9 35. 2 73. 5 63. 0 76. 5 90. 2 25. 1 94. 3 46. 5 77. 9 74. 1 36. 3 41. 9	33. 8 11. 2 19. 1. 26. 9	38.0 76.6	Date of the same of the same
Mean of summer Mean of autumn Mean of winter	0. 30 0. 30 0. 30	29, 398 29, 373 29, 333	28, 992 29, 104 29, 239	58. 9 45. 9 21. 6	55, 4 43, 3 19, 7	. 405 . 268 . 094	. 786 . 787 . 707		4.6 6.7 7.8	N. 34 W. N. 1 W. N. 58 W.	1.8 1.2 3.1	290. 8 227. 2 291. 1			181
Means of all	=	29. 356	29. 124	40. 0	37. 4	. 231	759		6. 4	N. 28 W.	1.9	1267. 9		-	et.
December, 1863. January, 1864 February, 1864 March, 1864 A pril, 1864 May, 1864 June, 1864 June, 1864 June, 1864 October, 1864 October, 1864 October, 1864 December, 1864 December, 1865 June, 1865 April, 1865 June, 1865 June, 1865 June, 1865 June, 1865 September, 1865 September, 1865 October, 1865 September, 1865 October, 1865 October, 1865 October, 1865 October, 1865 October, 1865 December, 1865 December, 1865 December, 1865 October, 1865 October, 1865 October, 1865 October, 1865 October, 1865 October, 1865 October, 1865	12 12 12 12 12 12 12 12 12 12 12 12 12 1	29, 252 29, 357 29, 278 29, 419 29, 399 29, 343 29, 372	29, 147 29, 195 29, 052 29, 063 28, 918 28, 844 29, 032 29, 167 29, 121 29, 165 29, 300 29, 376 29, 376 29, 152 29, 230 29, 152 29, 230 29, 152 29, 230 29, 152 29, 230 29, 152 29, 230 29, 152 29, 230 29, 152 29, 230 29, 152 29, 232 29, 232 29, 233 29, 232 29, 233	244, 3 34, 7 455, 6 55, 1 665, 9 544, 2 42, 6 34, 8 92, 0 17, 3 32, 6 34, 4 442, 5 553, 6 61, 3 443, 5 59, 9 61, 3 43, 8	22. 3 32. 6 5. 5. 6 5. 6 1. 5 5. 6 1. 6 1. 6 1.	105 161 226 356 482 499 340 927 170 097 070 082 114 151 206 372 404 450 225 180	727 798 7790 814 764 791 806 654 654 674 743 745 823 756 773 818 818 818	0, 01 0, 21 0, 02 0, 07 0, 07 0, 08 0, 08 0, 08 0, 16 0, 11 0, 12 0, 11 0, 12 0, 11 0, 09 0, 09 0, 09 0, 09 0, 09 0, 09 0, 09 0, 09 0, 09 0, 09 0, 09 0, 10	6.53.5.5.3.1.8.6.9.3.2.3.4.5.6.3.5.6.3.5.6.4.5.6.3.5.6.3.5.6.4.5.6.3.5.6.5.6	8.5 W N.60 W.N.57 W.N.57 W.N.6 W.N.53 W.N.51 W.N.51 W.N.51 W.N.51 W.N.51 W.N.51 W.N.64 E.N.55 W.N.75 W.N.	24	62. 3 59. 0 71. 4 40. 2 130. 6 39. 6 22. 8 86. 4 44. 1 68. 2 78. 6 81. 8 33. 0 90. 0 19. 8 86. 0 175. 6 39. 8	99.34.0	1.3	元 日 三 日 三 日 三 日 三 日 三 日 三 日 三 日 三 日 三 日
Mean of summer Mean of autumn Mean of winter	12 12 12	29, 397 29, 373 29, 333	28, 988	59. 3 45. 9 21. 8	55. 8 . 43. 1 . 19. 8 .	410 265 093	784 (784 (702 (0. 85 0. 56 0. 55	4.5 6.6 7.9	N. 35 W. N. 29 W. N. 60 W.	1.7 1.0 3.0	233. 3 170. 0 279. 0		25	83. M. 70.

		reduc 32° F	ed to	Te:	mp., hr.	Vaj	or.	d snow, Imala.	. (10===ky en 0==clear sky.			Wind	١.		
Date.	of observation.	U. S. inches d decimals.	and decimale.	alb.	alb.	ity, U. S. in- ind decimals.	lly. Satura- n=1.000.	Am't of rain and melted snow U. S. inches and decimals.	Am't cloudiness. (10= tirely overcant, 0=cle	red direction; m whence.	red velocity;			ity, i	rection mile
•	Time	Total	Green Green	Dry bulb.						Observed from w	Observed ve miles per h	N.	8.	E.	w.
December, 1863 January, 1864 February, 1864 March, 1864 April, 1864 April, 1864 June, 1864 July, 1864 August, 1864 Cotober, 1864 December, 1864 January, 1865 January, 1865 February, 1865 March, 1865 July, 1865 July, 1865 July, 1865 July, 1865 July, 1865 May, 1865 July, 1865 September, 1865 September, 1865 November, 1865 November, 1865 Mean of summer Mean of summer Mean of summer Mean of sulter Mean of all	P.M. 11. 30 11.	29. 403 29. 298 29. 214 29. 214 29. 275 29. 358 29. 416 29. 401 29. 394 29. 394 29. 296 29. 296 29. 358 29. 35	\$39, 2844 29, 201 29, 1149 29, 1149 29, 195 29, 063 29, 919 29, 076 29, 163 29, 163 29, 163 29, 183 29, 215 29, 23, 23, 23, 23, 23, 23, 23, 23, 23, 23	27. 22. 3 24. 5 24. 5 34. 9 55. 3 56. 1 42. 9 22. 3 34. 8 42. 8 42. 8 42. 8 42. 8 42. 8 42. 8 42. 8 43. 6 44. 9 44	25. 2 19. 7 20. 4 22. 6 341. 0 51. 6 60. 2 650. 9 40. 6 32. 7 24. 4 39. 5 51. 3 39. 5 56. 2 40. 8 40.	. 119 . 097 . 100 . 106 . 163 . 323 . 482 . 498 . 171 . 098 . 171 . 063 . 115 . 206 . 378 . 400 . 216 . 340	. 750 . 732 . 708 . 726 . 798 . 772 . 741 . 812 . 757 . 719 . 656 . 668 . 673 . 737 . 736 . 814 . 753 . 739 . 739		87.32 67.7.6.34 6.34 6.34 6.34 6.34 6.34 6.34 6.33 6.33	8. 36 W. N. 66 W. N. 161 W. N. 12 W. N. 16 W. N. 16 W. N. 17 W. N. 18 W. N. 19 W. N. 10 W. N. 10 W. N. 10 W. N. 10 W. N. 10 W. N. 10 W. N. 10 W. N. 10 W. N.	1. 52 4. 21 4. 25 1. 90 4. 29 4. 29 4. 20 5. 3. 10 2. 90 1. 3. 10 2. 20 2. 3. 10 2. 3. 10 2. 3. 10 2. 3. 10 2. 3. 10 3.	51. 8 58. 0 77. 5 40. 6 123. 1 18. 6 78. 2 35. 5 9. 3 82. 9 28. 4 86. 3 93. 2 78. 9 36. 1	30. 7 10. 4 14. 8	2. 5 3. 6	13, 48, 18, 55, 82, 68, 121, 162, 75, 16, 45, 15,
fean of summer fean of autumn fean of winter Means of all	11.30 11.30 11.30 11.30	29. 398 29. 374 29. 334 29. 357	28. 987 29. 107 29. 239 29. 123	59. 5 46. 0 22. 0	55. 9 43. 2 20. 2 37. 8	. 410 . 266 . 095 . 233	. 773 . 780 . 705		4. 5 6. 7 7. 8 6. 3	N. 33 W. N. 61 W. N. 63 W. N. 43 W.	1.7 1.8 3.1	249. 7 165. 2			168. 295.
December, 1863 annary, 1864 rebruary, 1864 April, 1864 April, 1864 April, 1864 day, 1864 uly, 1864 day, 1864 leptember, 1864 December, 1864 December, 1864 April, 1865 April, 1865 April, 1865 May, 1865 Lup,	11 11 11 11 11 11 11 11 11 11 11 11 11	29, 415 29, 301 29, 217 29, 256 39, 360 29, 414 29, 402 29, 346 29, 374 29, 293 29, 365 29, 365 29, 365 29, 365 29, 410 29, 386 29, 410 29, 452 29, 411 29, 411 29, 426	29. 296 29. 205 29. 117 29. 150 29. 195 29. 053 29. 063 29. 071 29. 121 29. 161 29. 161 29. 294 29. 232 29. 152 29. 053 29. 053 29. 053 29. 053 29. 053 29. 185 29. 246	27, 9 21, 5 22, 5 22, 5 35, 1 44, 0 55, 6 66, 5 543, 2 23, 5 21, 6 24, 6 243, 1 35, 1 36, 6 43, 2 35, 1 36, 6 43, 1 36, 6 36,	25. 3 19, 7 22. 6 33. 1 41. 2 51. 7 51. 0 61. 7 51. 0 31. 5 31. 5 32. 6 32. 6 33. 6 33. 6 34. 6 35. 6 36. 6 37. 6 38. 6	. 119 . 096 . 100 . 106 . 165 . 227 . 351 . 484 . 495 . 341 . 230 . 613 . 154 . 154 . 362 . 154 . 362 . 154	745 729 706 721 800 769 746 802 781 726 655 665 700 734 728 810 734 744 728 744 744 744 744 744 744 744 744 744 74	0. 06 0. 03 0. 05 0. 05 0. 10 0. 10 0. 10 0. 07 0. 12 0. 07 0. 08 0. 09 0. 00 00 00 00 00 00 00 00 00 00 00 00 00	8.017.117.16.76.76.76.76.76.76.76.76.76.26.26.26.26.26.26.26.26.26.26.26.26.26	8. 14 E N. 64 W N. 60 W N. 60 W N. 65 W N. 26 W N. 45 W N. 63 E N. 42 W N. 63 E N. 42 W N. 14 E S. 44 E N. 14 E N. 14 E S. 54 W	1.34.54.11 2.54.11 2.54.13 4.13 4.13 6.3.44	59.9 58.2 77.2 46.1 126.3 59.3 13.1 41.4 59.8 70.6 92.1 93.1 193.1	3	0, 1 35, 3 80, 8 23, 2 3, 9	123, 102. 13, 29, 13, 49, 153, 127, 165, 71. 466, 20
fean of spring fean of summer fean of autumn fean of winter Means of all	_			1)		N. 2 W N. 25 W N. 41 W N. 59 W	1	448. 5 279. 152. 253.			

		reduc	sure sed to fahr.	Temp. Fahr.	Va	por.	d snow,	. (10=aky en., 0=clear aky.			Wind			
Date.	I'ime of observation.	al, U. S. inches and decimals.	and decimals.	oalb.	icity, U. S. in-	Humidity. Satura-	i a w	Am't cloudiness. (10-	Observed direction; from whence.	Observed velocity; miles per hour.	Resolt and per	reloc	ity, iz	
		To	Gaseous, ches and	Dry bulb.	Elasticity,	PP 1	<	Am't tirely	- 44 O	O P	N.	8.	E.	₩.
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day, 1864	8,30	29, 262	29, 036	45. 6	42.0	. 226	. 725		7. 2	N. 1 W.	4.0	194 6		2240	
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ctober, 1864	8, 30	29, 305	29,067	44. 8	42.0	. 237	. 792		7.8	N. 35 E. N. 43 W.	1.5	34. 1		-	
lovember, 1864	8.30	29, 293	29, 125	35, 9	33. 5	. 168	. 758		8, 1	S. 65 W. N. 70 W.	3.6		44. 1		l.
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		reduc	sure ed to Fahr.	Ter Fa	mp., hr.	Va	por.	d snow, mals.	earsky en-			Wind			
Date.	Time of observation.	U. S. inches d decimals.	and decimals.	alb.	oalb.	city, U. S. in-	Humidity. Satura-	Am't of rain and melted snow U. S. inches and decimals.	Am't cloudiness. (10=sky e tirely overcast, 0=clear sky	rerved direction; from whence.	Observed velocity; miles per hour.	and		ity, i	rection n miles
	Tine	Total, 1	e pe	Dry bulb.	Wet bulb.	Elasticity,	Hum	Am't	Am't	2	Obser mil	N.	8.	E.	w.
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		redu	sure ced to Fahr.	Tet Fa	np., hr.	Vaj	por.	ed snow, imals.	=sky en- lear sky.)			Wind.		
Date.	Time of observation.	U. S. inches d decimals.	and decimals.	alb.	balb.	leity, U. S. in- and decimals.	Humidity. Satura-	of rain and melte inches and deci	Am't cloudiness. (10 = tirely overcast, 0=cle	Observed direction; from whence.	wed velocity;	Resolu and v per h	elocity	
	1	Total, I	Gaicour, ches and	Dry bulb.	Wet bulb.	blasticity,	Ham	Am't	Am't trely		Observed v	N.	8.	E. W.
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March, 1864	6. 30	29. 254	29. 144	26. 9	24. 4	. 110	. 688		6.4	N. 18 E	24	69.9		<u> 2 5</u>
May, 1864	6. 30	29, 252	29. 180	48. 7	44. 2	238	685		7. 4	N. 20 E.	2.4 1.4	41.9	ji.	5.0
June, 1864	6.30	29, 412	29, 031	62. 4	56, 0 63 1	. 381 . 506	. 643 710	• • • •	4.9	N. 45 E	0.3	6.6	12.63	7.3 1.4
August, 1864	6. 30	29. 328	28. 805	71.7	64. 3	. 523	650		7. 1	N. 13 W.	2.2	65.6	!	15
September, 1864 October, 1864	6, 30 6, 30	29, 356 29, 305	29, 011 29, 066	57. 3 45. 6	52. 6 42. 6	. 239	. 713 . 759	••••	8.1	N. 41 E. N. 55 W.	2.4 0.9	54.9 16.8	•	7 8 21
November, 1864	6. 30	29. 290	29. 124	36. 1	33. 6	. 168	. 751		8. 4	8. 80 W.	1.8 3.7		9.4	3
January, 1865	6, 30	29. 255 29. 351	29, 274	24. 1 19. 7	17. 4	. 077	. 642		7. 9	N. 56 W.	3.2	54.5		22
February, 1865	6. 30	29, 462	29. 374	23. 6	21. 1	. 088	. 655	•••	8.6	N. 23 W.	1.0	26.9	-	1
April, 1865	6. 30	29. 377	29. 107	38. 9	35. 3	. 162	675		6.4	N. 79 W.	1.1	6.4		x
May, 1865	6.30	29. 342	29, 125	48. 2	43. 0	217	. 639		6.4	N. 51 E.	0.8 2.3	16.7	40.45	Q3
July, 1865	6. 30	29. 389	28, 976	64. 5	58. 4	. 413	675		6. 6	N. 8 W.	20	59.6	-]
August, 1865	6.30	29, 443	29, 006	65. 2	59. 6	. 438 . 491	. 688 766	••••	4.3 6.4	N. 11 E.	1.3	36. 2	50 9	7, 7i 1, 3i
October, 1865	6. 30	29. 400	29. 154	46. 9	43. 4	246	723		7. 6	N. 7 E	3.2	97.9]1	1.8
November, 1865	6. 30	29. 428	29. 245	40. 5	37. 1	. 183	. 707		7. 0	8. 79 W.	1.5		7.2	
Mean of spring	6. 30	29. 312	29. 142	38. 5	34. 9	. 170	695		7. 0	N. 1 B	1.7	306.7		3.5
Mean of summer	6.30	29, 363	28.943	65. 3	59. 3 45. 1	. 439	. 683		6. 1 7.3	N. 34 E.	0.8	117.0	7	9. 11
Mean of winter	6. 30	29. 339	29. 241	23. 6	21. 4	. 098	. 693		8. 2	N. 55 W.	1.9	199.7		
	—	29. 349 	29, 104	44. 0	40. 2	. 246	. 699	<u>:::</u>	7. 1	N. 17 W.	1.0	717.9		!!
December, 1963	6	29. 424	29. 312	27. 7	25. b	. 112	. 734 . 712	0. 14	8.4	8. 84 E.	1.7		5.0,5	24
Fahrnary, 1864	6	29. 308	29, 312 29, 210 29, 106	22.4	20. 5	. 098	. 712	0. 05 0. 19	7.5	8. 84 E. N. 63 W. N. 54 W.	2.6 3.2 1.4	36.0 55.6		7
March, 1864	6	29. 254	29. 143	27. 7	24. 9	. 110	695 675	0. 01	6.5		1 1. 4	36. 9	5.05	1.8
April, 1864	6	29. 356	29, 180	39. 2	36. 2	. 176 944	. 739	0. 24	7. 2	N. 38 E. N. 17 E. N. 45 E.	2.7 1.5	64.0		9.6 4.0
June, 1864	6	29. 411	29. 023	63. 6	56. 7	. 388	. 624		3. 8 7. 1	N. 45 E	0.6	12.8	i	2.1
July, 1864	6	29. 389 99. 396	26.888 98.803	70. 0	63. 7 64 R	. 501 523	. 69 6	0 07 0 17	7.1	8. 46 E. N. 14 W.	1.2 2.5	74 6	24. 62	28
September, 1864	ő	29. 358	29. 010	57. 9	53. 0	. 348	. 705	Ö. Ö1	7. 0	N. 35 E.	25	54. 4	3	7.8
Detober, 1864	6	29. 301 29. 291	29, 060 29, 123	46. 0 36. 3	42. 9 33. 7	. 241 . 168	. 755 . 747	0. 11 0. 04	7. 3 7. 0 8. 2 8. 4	N. 56 W. 8. 88 W.	110	18. 8	9.5	5
December, 1864	6	29. 250	29, 106 29, 143 29, 180 29, 006 29, 023 28, 888 28, 803 29, 010 29, 060 29, 123 29, 145 29, 267 29, 373	24. 3	22. 3	. 105	717	0. 23	9. 1	N. 65 W. N. 53 W. N. 11 W.	23 31	30.4	24. 62 3 2. 5	\$
February, 1865 February, 1865	6	29. 344 29. 461	29, 267 29, 373	20. 1 24. 0	17. 8 20. 4	. 077	. 641 . 646	0.07 0.02	7. 9 8. 7	N. 53 W. N. 11 W.	0.9	20.3 22.5		7
March, 1865	6	29. 283	29. 161	30. 2	27. 3	. 122	. 677	0. 17	8.0	N. 33 W.	4.3	110.6		7
April, 1600	6	29. 373 29. 338	29, 200 29, 119	39, 8 48, 9	.55. 9 43. 5	. 104 . 219	633	0. 04 0. 04	5.9	N. 44 W. N. 61 E.	0.9	18.4	9) 5
	6	29, 337	28. 964	60. 1	54. 9	. 373	. 721	0. 10	6.9	R 70 17	2.9		66. 8,5	4.4
June, 1865		zv. 369	28.971	68. 5	э у . 0 60, 3	. 416 . 446	. 678	0. 06 0. 02 0. 17	6.5 4.2 6.9	8. 39 E . N. 14 W . N. 28 E .	1.5 0.7	19. 9	10	. 6
June, 1865	6	29, 444			61 7	. 494	. 755	0. 17	6, 9	B. 1 E	1 24.0		60.4	i. 6
June, 1865	6	29, 444 29, 423	28. 929	66. 4	24. 1								-	
June, 1865	6 6	29, 444 29, 423 29, 396 29, 428	28, 929 29, 147 29, 245	66. 4 47. 6 40. 7	44. 0 37. 2	. 251 . 183	. 717 . 69 8	0.08	7. 7	N. 16 E. S. 86 W.	3.2 1.8	96, 1	3.8	6
June, 1865. July, 1865. August, 1865. September, 1865. October, 1865. December, 1865.	6 6	29. 344 29. 461 29. 283 29. 373 29. 338 29. 337 29. 369 29. 444 29. 423 29. 428	28. 929 29. 147 29. 245	66. 4 47. 6 40. 7	44. 0 37. 2	. 251 . 183		0. 08 0. 05	7. 7 7. 3	8. 86 W.	1.8	96.1	3.8	
June, 1965. July, 1965. July, 1965. August, 1865. September, 1865. October, 1865. December, 1865. December, 1965. Mean of spring. Mean of snumer	6 6 6	00 200	00 120	20 0	28 4	170	. 679 . 671	0. 08 0. 05 0. 52 0. 42	7. 7 7. 3 6. 9 6. 0	8. 86 W. N. 3 E	1.8		3.8	6
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December, 1863 anuary, 1864 february, 1864 Arch, 1864 April, 1864 August, 1864 Coulomber, 1864 Coulomber, 1864 Coulomber, 1864 Coulomber, 1864 Coulomber, 1864 Coulomber, 1864 Coulomber, 1864 Coulomber, 1865 Coulom	6 6 6 6 6	00 200	28, 929 29, 147 29, 245 29, 136 28, 941 29, 086 29, 235	20 0	28 4	170	. 679 . 671 . 729	0. 08 0. 05 0. 52 0. 42	7.7 7.3 6.9 6.0 7.6	N. 3 E N. 53 E	1. 6 0. 5 0. 7	96. 1 987. 4 61. 0 102. 0 198. 8	73	68 941

		reduc	sure sed to fabr.		np., hr.	Vaj	por.	elted snow, decimals.	s. (10=sky en- ,0=clearsky.)			Wind	•		
Date,	Time of observation.	U. S. inches d decimals.	as, U. S. in-	balb.	outb.	icity, U. S. in-	Humidity. Satura-		Am't cloudiness. (10:=	erved direction; from whence.	Observed velocity; miles per hour.	and		ity, i	rection u miles
	Time	Total,	Gaseous, 1	1	Wet bulb.	Elasticity,	Ham	Am't	Am't	ج	Obser	N.	8.	E .	w.
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		reduc	ssure ced to Fahr.	Ter	mp.,	Va	por.	elted snow, decimals.	sky en- ar sky.)			Wind			
Date.	Time of observation.	U. S. inches	and decimals.	ulb.	ulb.	Elasticity, U. S. in- ches and decimals.	Humidity, Satura- tion = 1.000.	Am't of rain and melted U. S. inches and decin	Am't cloudiness. (10-sky en- tirely overcast, 0-clear sky.)	Observed direction; from whence.	Observed velocity; miles per hour.	Resolution and per l	veloc	ity, la	rection mile
	Time	Total,	Gaseous, ches and d	Dry bulb.	Wet bulb.	Elasti ches:	Humi	Am'te	Am'te	Ормет	Obser	N.	s.	E.	w.
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Date.	Time of observation.	U. S. inches	us, U. S. in-	ulb.	oulb.	efty, U. S. in- and decimals.	Humidity. Satura-	8 0	Am't cloudiness. (10- tirely overcast, 0-cle	Observed direction; from whence.	ved velocity;	and		city, i	rection n miles
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		reduc	ed to Fahr.	Te:	mp.,	Va	por.	d snow	. (10=sky en 0=clear sky.)			Wind.			
Date.	Time of observation.	al U. S. inches and decimals.	ous, U. S. in- and decimals.	onlb.	bulb.	dasticity, U. S. in-	Humidity, Satura-	Am't of rain and melted snow U. S. inches and decimals.	Am'teloudiness. (10= tirely overcast. 0=clea	Observed direction; from whence.	Observed velocity; miles per bour.	Resolu and per l	veloci		
	-	Tot	Gaseous,	Dry bulb.	Wet	Elasticity, ches and	H	4		Obs		N,	8.	E.	W
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January, 1864	2. 30	29. 287	29, 183	25. 1	22. 7	. 104	. 680		7.5	N. 65 W.	2.2	36.5	10.8	68.2	77
February, 1864	2, 30	29, 199	29. 088	27. 1	24. 4	. 111	. 668		9.0	N. 66 W.	3. 1	39, 4			E
April 1864	9 30	29, 238	29, 120	30. 4	37 5	170	704		7. 0	N. 37 E .	3.3	81.4	***	6L0	
May 1864	2 30	29, 260	29, 009	51. 1	46. 1	951	667		7 3	N 17 E	1.9	84.5		82.1	y-
June, 1864	2.30	29, 429	29, 032	66. 4	58. 1	. 397	. 587		4.6	S. 70 E .	0. 8	Sat	8.9	23.7	
July, 1864	2.30	29. 405	28, 673	73. 2	65. 5	. 532	. 650		6, 8	S. 83 E .	2.2		8.3	67. 1	į.
August, 1864	2, 30	29, 328	28. 792	76. 3	66. 6	. 536	. 578		6.6	N. 11 W.	2.1	0.3. 3	S	Season.	
Detober 1864	9 30	99, 987	99 039	49 6	45.4	055	707		9.1	N. 16 E	3. 0	80,4	11.6	40.1	7
November, 1864	2, 30	29, 280	29, 111	38, 5	35. 2	169	687		8.7	S. 83 W.	3.3	14.	11 6	0.	'n
December, 1864	2, 30	29, 213	29, 108	25. 5	23, 2	. 105	. 685		9.5	N. 83 W.	2.6	10.1			18
January, 1865	2.30	29. 321	29. 239	22, 5	19.8	. 082	. 622		8.5	N. 65 W.	3. 6	48.	1		.[12
Wareh 1865	9 30	29, 440	29, 347	24. 9	25. 3	120	610		8.0	N. 9E.	2.7	37.4	110	E.	
April, 1865	2, 30	29, 361	29, 190	43. 2	38. 3	171	609		7. 0	8. 56 W.	1.5	26.	95 (16
May, 1865	2, 30	29, 339	29, 111	51. 2	45. 2	. 228	. 601		5. 3	N. 86 E .	1.8	3.7	25. 0	56.1	
June, 1865	2, 30	29. 354	28, 959	62. 7	56. 9	. 395	. 682		7. 0	S. 9E	2.7		RT. 1	N 12.1	-
Angust 1865	2.30	29, 390	28, 909	71.3	69.0	468	603	****	4.8	N. 31 W.	0.4			6	13
September, 1865	2, 30	29, 421	28. 894	70, 4	64. 2	527	702		6, 4	S. 5E.	3. 1	11.	94	8	
October, 1865	2, 30	29, 381	29.099	51.4	47. 1	. 282	. 692		7.3	N. 16 E .	3. 4	100.6	5	28.1	g,
November, 1865	2. 30	29. 404	29, 218	43, 4	38, 9	. 186	. 637		7. 1	S. 77 W.	1.7		10,		. 4
Mean of spring	2 30	29, 307	99, 195	41 8	37 5	181	640	****	7 1	NORE	1.6	259,		120	2 '-
Mean of summer	2, 30	29, 394	28, 935	69. 7	61.7	459	. 620		5.8	East	0. 4	1.1	8	78	ā.,
Mean of autumn Mean of winter	2. 30	29, 354	29. 058	52, 4	47. 6	. 296	. 680		7.5	N. 45 W.	0. 6	78.6	Seen		. 18
mean of winter	2, 30	29, 310	29, 213	20, 2	23. 7	. 104	. 660	••••	8.0	N. 59 W.	1.8		-		-
Means of all			-	-	-	=	=	=	=	N. 16 W.	0.7	501.3	3	.457	- 14
December, 1863	2	29, 427 29, 281 29, 189 29, 242 29, 367 29, 436 29, 436 29, 331 29, 331 29, 332 39, 283 29, 283 29, 283 29, 283 29, 283 29, 283 29, 283 29, 343 29, 343 29, 343 29, 343 29, 343 29, 343 29, 343 29, 343 29, 343 29, 343 29, 343 29, 342 29, 342 29, 342 29, 342 29, 342 29, 342 29, 342 29, 342 29, 342 29, 342 29, 342 29, 426 29, 427 29, 383 29, 406	29, 304	29, 2	26. 8	. 123	. 734	0, 06	9.1	S. 78 E .	2.3		15, 7	72	6
January, 1864	2 2	29, 281	29. 177	25. 5	22.8	. 104	. 659	0.09	7.4	N. 78 W.	3.3	20, 6	5	LANGE	,30
February, 1864	2	29, 169	29. 077	30.8	97.5	119	649	0.04	7.0	N. 65 W. N. 30 E	3.1	36.8			437
March, 1864 April, 1864	2	29. 367	29, 188	40. 8	37. 4	.179	704	0. 13	7. 5	N. 41 E .	4.3	98.4		84.1	0
May, 1864 June, 1864 July, 1864 August, 1864	2	29.263	29.013	51. 2	46, 1	. 250	. 660	0.09	7.2	N. 19 E	2.0	57.0	3	19,	6.,
June, 1864	2	29, 436	29, 042	66. 5	58. 4	. 394	. 580	0.10	4.6	S. 84 E	2.5	57.1	3.3	30.	1
August, 1864	2	29, 331	28, 792	76. 5	66. 7	539	579	0. 15	6.1	S. 81 E . N. 11 W.	2.6	27			
September, 1864 October, 1864	2	29, 351	28, 991	61.5	55, 2	. 360	. 646	0, 09	7.6	N. 22 E	2.8				
October, 1864	2	29, 288	29, 031	49.7	45. 5	. 257	. 708	0.15	7. 9	N. 62 E .	0. 6	(- X X		1 153	
November, 1864 December, 1864	2 2	29, 283	29, 116	38, 4	35. 0	. 167	683	0, 12	8.5	N. 86 W. S. 87 W	2.7				
January, 1865	2	29, 323	29, 241	20. 4	19.7	. 081	613	0.06	8.6	N. 65 W.	3.4	45 0	5, 6		н
February, 1865	2	29, 449	29. 349	27.7	24. 5	. 099	. 616	0, 02	8.2	N. 2E	1.8	50, 6		1.5	
March, 1865	2	29, 285	29, 144	34. 3	30. 9	. 141	. 658	0. 10	7. 6	N. 52 W.	2.6	51. 1		480	12
May 1865	2 2	29. 361	29, 189	43. 3	45 0	172	602	0.04	7. 7	S. 76 W. N. 85 E.	2.0		14. 2	51.3	
June, 1865	2	29, 358	28, 959	62. 7	57. 1	. 399	689	0, 09	6.9	S. 19 E	3. 2	Lubert	D41. U	1.00	
July, 1865	2	29, 399	28, 970	67. 9	60. 5	. 429	. 629		6. 1	N. 30 W	1, 1	28.0			33
December, 1864 Ianuary, 1865 Pebruary, 1865 March, 1865 April, 1865 May, 1865 June, 1865 July, 1865 August, 1865 September, 1865	2 2	29, 456	28. 981	71. 9	63. 0	475	. 613	0. 12	4.8	N. 37 E	0.5	124.0	S. Acces	-35.3	•
October, 1865	2	29, 327	29, 104	51. 4	47.0	979	684	0, 33	7.3	S. 1 W. N.11 E	3.2	97 0	84. 2	18.1	
September, 1865 October, 1865 November, 1865 December, 1865	2	29, 406	29, 219	43, 2	38, 8	. 187	. 642		7. 2	S. 77 W	1.8	34.4	12.1		ä
December, 1865												100000		4000	ш
Mean of spring	52	29, 310	29, 129	41. 9	37. 6	. 181	648	0. 59	7. 0	N. 18 E .	1.6	269. 9		92.6	1
Mean of summer Mean of autumn	2	90 356	28, 937	59, 6	47. 6	996	677	0. 74	5.8	N. 80 E N. 45 W.	0, 6	269, 9 11, 6 91, 6		114.4	9
Mean of autumn															
Mean of winter	2	29, 356 29, 314	29, 209	26. 2	23. 6	. 104	. 662	0.56	8.5	N. 66 W.	1.7	131.7			100

		redu	sure sed to Fahr.	Ter Fa	np., hr.	Vaj	or.	d mow.	aky en- ar aky.)			Wind	L .		
Date,	of observation.	U. S. inches d decimals.	and decimals.	oulb.	balb.	Elasticity, U. S. in-	Humidity. Satura-	of rain and melte 3. inches and dec	Am't cloudiness. (10-sky en- tirely overcast, 0-clear sky.)	ved direction;	ved velocity;	and		ity, i	rection n miles
	Time of	Total, I	Gascous, obes and	Dry bulb.	Wet bulb.	Elanti	Han	8:5	1 2	<u>.</u>	Observed miles pe	N.	s.	E.	w.
December, 1863. January, 1864. February, 1864. March, 1864. March, 1864. April, 1864. May, 1864. June, 1864. July, 1864. August, 1864. September, 1864. November, 1864. December, 1864. December, 1865. April, 1865. April, 1865. May, 1865. June, 1865. June, 1865. June, 1865. September, 1865. November, 1865. November, 1865. November, 1865. November, 1865. November, 1865. November, 1865. November, 1865. November, 1865. Mean of spring Mean of antumn Mean of winter. Means of all	P.M. 1. 30 1. 30 1. 30 1. 30 1. 30 1. 30 1. 30 1. 30 1. 30 1. 30 1. 30 1. 30	29. 428 29. 279 29. 188 29. 247 29. 369 29. 240 29. 410 29. 335 29. 353 29. 353 29. 353 29. 353 29. 353 29. 353 29. 353 29. 353 29. 362 29. 362	29. 304 29. 172 29. 076 29. 128 29. 015 29. 055 28. 880 29. 055 28. 986 29. 055 28. 986 29. 122 29. 111 29. 241 29. 353 29. 122 29. 111 29. 241 29. 149	29. 4 25. 4 27. 3 30. 7 51. 5 65. 5 76. 3 61. 5 76. 3 54. 4 27. 5 33. 3 43. 3	27. 0 23. 0 24. 7 27. 5 37. 4 46. 2 57. 6 65. 3 65. 3 45. 4 24. 4 24. 4 24. 4 24. 4 30. 6 38. 3	. 124 . 107 . 112 . 119 . 252 . 385 . 538 . 367 . 256 . 163 . 163 . 104 . 173	. 728 . 680 . 663 . 716 . 655 . 564 . 655 . 712 . 662 . 673 . 611 . 626 . 613		9.7.8.7.7.3.7.8.9.8.8.7.7.8.9.8.8.7.7.	8. 67 E. N. 75 W. N. 86 W. N. 39 E. N. 41 E. S. 86 E. S. 76 E. S. 76 E. N. 40 E. N. 40 E. N. 88 W. N. 88 W. N. 88 W. N. 4 W. N. 55 W.	2.4 3.1 2.3 2.0 2.1 2.0 2.1 2.0 2.1 2.0 3.3 4.3 3.1 2.1 2.1 2.1 3.1 2.1 2.1 3.1 2.1 3.1 2.1 3.1 3.1 3.1 3.1 3.1 3.1 3.1 3.1 3.1 3	66. 0 60. 5 63. 1 31. 2 2. 9 2. 6 47. 5	2.0	30. 0 28. 3 58. 9 53. 5 1. 0	94. 9 110. 5 95. 9 3. 6
May, 1865 June, 1865 July, 1865 August, 1865 August, 1865 October, 1865 November, 1865 November, 1865 Mean of spring Mean of summer Mean of autumn Mean of winter Mean of antumn	1. 30 1. 30 1. 30 1. 30 1. 30 1. 30 1. 30 1. 30 1. 30	29. 351 29. 366 29. 402 29. 460 29. 427 29. 385 29. 410 29. 313 29. 402 29. 359 29. 314 29. 347	29, 130 24, 968 26, 972 28, 985 28, 900 29, 105 20, 233 20, 132 24, 943 29, 062 29, 062 29, 068	50. 9 62. 4 67. 8 71. 2 70. 0 51. 1 43. 2 41. 7 69. 3 52. 2 26. 2	44. 9 56. 9 60. 4 63. 1 64. 1 46. 9 38. 8 37. 5 61. 7 47. 6 23. 6	. 221 . 398 . 430 . 476 . 527 . 280 . 186 	. 600 . 696 . 634 . 614 . 712 . 697 . 645 . 627 . 685 . 663		5.0 7.0 6.2 5.0 6.3 7.4 7.5 7.1 5.9 7.5 8.5	N. 66 E S. 21 E N. 40 W N. 41 E S. 2 E N. 16 E S. 61 W N. 17 E S. 79 E N. 45 W N. 69 W N. 23 W	1.7 3.3 0.9 0.3 3.3 3.0 2.1 1.7 0.5 0.6 1.9	90, 6 296, 6	19. 2	3. 7 20. 2 83. 7 92. 8	
December, 1863. January, 1864. February, 1864. March, 1864. April, 1864. May, 1864. July, 1864. July, 1864. September, 1864. October, 1864. November, 1864. December, 1865. April, 1865. April, 1865. April, 1865. July, 1865. July, 1865. September, 1865. September, 1865. December, 1865. December, 1865. May, 1865. July, 1865. September, 1865. December, 1865.		99. 433 99. 279 289. 273 289. 373 289. 373 289. 443 289. 335 289. 335 289. 329 289. 329 289. 355 289. 357 289.	29. 073 29. 135 29. 191 29. 020 29. 026 28. 881 28. 789 28. 992 28. 992 29. 112 29. 250 29. 153 29. 166 29. 153 29. 186 29. 153 29. 186 29. 153 29. 186 29. 153 29. 111 28. 964 28. 964 29. 111 29. 229	27, 0 30, 6 40, 6 51, 1 672, 5 76, 1 61, 1 49, 2 37, 8 225, 3 21, 8 227, 0 33, 6 45, 7 66, 2 67, 6 67, 1 69, 8 70, 7 42, 9	24, 5 27, 5 37, 3 46, 1 56, 5 56, 6 66, 8 66	.112 .118 .181 .252 .390 .362 .258 .165 .105 .105 .107 .107 .127 .397 .430 .478 .529 .277 .186	.669 .652 .712 .669 .589 .662 .596 .659 .720 .702 .676 .617 .637 .637 .637 .637 .637 .637 .637 .63	0. 29 0. 09 0. 42	7.11 7.7.14 6.09 7.7.8.16 8.8.18 8.7.48 9.8.40 9.8.40 7.7.6 7.7.6 7.7.6	N. 68 W E. W. W. W. W. S. 59 E E. S. 76 E W E. W. W. W. S. 68 F E E. W. W. W. S. 68 F E E. W. W. W. S. 68 F E E. W. W. W. S. 68 F E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. S. 68 E E. W. W. W. S. 68 E E. W. W. S. 68 E E. W. W. W. S. 68 E E. W. W. W. S. 68 E E. W. W. W. S. 68 E E. W. W. W. S. 68 E E. W. W. W. S. 68 E E. W. W. W. S. 68 E E. W. W. W. S. 68 E E. W. W. W. S. 68 E E. W. W. W. W. S. 68 E E. W. W. W. W. W. W. W. W. W. W. W. W. W.	1.44 3.58 4.10 2.22 1.64 3.58 1.93 2.26 1.95 2.16 1.95 2.16 1.95 2.16 1.95 2.16 1.95 2.16 1.95 2.16 1.95 2.16 1.95 2.16 1.95 2.16 1.95 2.16 1.95 2.16 2.16 2.16 2.16 2.16 2.16 2.16 2.16	90. 5 53. 7 29. 5 64. 3 56. 7 48. 5 10. 8 94. 6 23. 3 13. 7 15. 0	17. 2 4. 6 1. 1 57. 8	82, 3 29, 7 49, 1 68, 3 19, 8 11, 6 43, 5 13, 5	21. 2 10. 5 102. 9 123. 1 95. 0 53. 9 75. 0
Mean of summer	i	99. 317 29. 406 99. 363 99. 316 ——— 29. 350		_	_		. 634 . 6 91 . 6 67	0. 39 0. 89 0. 51 2. 21	5. 8 7. 5	N. 59 E. N. 40 W. N. 66 W.	0.8	252. 4 47. 5 109. 1 161. 2	_		91. 8 363. 0 278. 7

		reduc	Pressure reduced to 32° Fahr.		np., hr.	Vaj	por.	d snow	. (10=*ky en 0=clear sky.)	Wind.						
Date.	Time of observation.	Total U. S. inches and decimals.	and decimals.	ulb.	alb.	20	sidity. Satura-	Am't of rain and melted snow U. S. inches and decimals.	Am't cloudiness. (10= tirely overcast, 0=cle	erved direction; from whence,	served velocity;	Resolu and per l	tion oveloci			
	Time	Total	Gascons, ches and	Dry bulb	Wet bulb.	Elasticity, ches and	Humidity.	Am'te	Am't	Observed from w	Observed miles p	N.	8.	E.	R	
December, 1863. January, 1864. Kebruary, 1864. March, 1864. April, 1864. May, 1864. May, 1864. Jule, 1864. Jule, 1864. November, 1864. October, 1864. November, 1864. December, 1865. January, 1865. Kebruary, 1865. January, 1865. Jule, 1865. Jule, 1865. Jule, 1865. July, 1865. August, 1865. September, 1865. November, 1865. November, 1865. November, 1865. November, 1865. November, 1865. November, 1865. Mean of summer Mean of summer Mean of autunn	0, 30 0, 30 0, 30 0, 30 0, 30 0, 30 0, 30	29, 463 29, 440 29, 390 29, 418 29, 321 29, 408 29, 367	28, 988 28, 913 29, 116 29, 234 29, 138 29, 946 29, 071	67. 4 50. 3 42. 6 41. 4 68. 9 51. 5	63. 8 46. 2 38. 4 37. 4 61. 6 47. 1	. 527 . 274 . 185 . 183 . 463 . 296	. 624 . 726 . 709 . 653 . 666 . 642 . 699		7. 2. 9. 9. 7. 7. 7. 8. 8. 9. 9. 9. 9. 6. 6. 0. 1. 9. 8. 8. 8. 8. 8. 7. 6. 0. 0. 5. 7. 6. 7. 7. 7. 7. 7. 7. 7. 7. 7. 7. 7. 7. 7.	N. 11 E. N. 63 E. S. 75 E. N. 16 W. N. 29 E. N. 17 W. S. 89 W. N. 89 W. N. 59 W. N. 10 E. N. 82 W. N. 84 W. N. 14 W. N. 11 W. N. 82 W. N. 11 W. N. 12 E. N. 12 W. N. 14 E. N. 14 E. N. 14 E. N. 14 E. N. 14 E. N. 14 E.	3.9 2.5 3.9 1.7 1.9 2.9 2.9 1.9 3.5 3.7 1.9 2.9 2.9 1.9 1.9 1.9 1.9 1.9 1.9 1.9 1.9 1.9 1	45.5 55.9 80.3 23.1 23.1 26.4 50.6 55.6 13.6 16.6 13.1 16.6 19.1 19.1 19.1 19.1 19.1 19.1 19	18. 2 5 6 7 8 18. 6 8 18. 6 18	51. 5 87. 1 10. 1 44. 66. 66. 66. 1 28. 1 12. 1 12. 1 40. 1 12. 1 12. 1 12. 1 12. 1 12. 1 12. 1 12. 1 12. 1 12. 1 12. 1 12. 1 13. 1 14. 1	京都 日 日 日 日 日 日 日 日 日 日 日 日 日 日 日 日 日 日 日	
Mean of winter Means of all	-	-	29, 218 29, 093	-	_	-	_		7.3	N. 63 W.	0.8			****	Ē	
December, 1863. January, 1864 March, 1864 March, 1864 March, 1864 My, 1864 My, 1864 My, 1864 My, 1864 My, 1864 My, 1864 My, 1864 My, 1864 My, 1864 My, 1864 My, 1864 My, 1864 My, 1865	19 19 19 19 19 19 19 19 19 19 19 19 19 1	29, 291 29, 196 29, 266 29, 373 29, 279 29, 451 29, 428 29, 359 29, 304 29, 296 29, 304 29, 296 29, 304 29, 296 29, 309 29, 361 29, 361 29, 361 29, 468 29, 447 29, 448 29, 423 29, 423 29, 423	29, 329 29, 190 29, 190 29, 186 29, 186 29, 189 28, 986 29, 042 29, 121 29, 186 29, 182 28, 193 28, 999 29, 182	24. 7 26. 3 340. 4 50. 6 65. 2 72. 0 66. 6 20. 9 20. 9 20. 9 20. 9 20. 4 42. 5 61. 8 42. 5 61. 4 49. 7 42. 1	22, 3 23, 9 23, 9 237, 3 45, 9 45, 9 45, 9 45, 9 46, 5 45, 9 44, 8 48, 9	.101 .111 .122 .184 .254 .402 .531 .549 .373 .262 .163 .108 .077 .098 .299 .397 .434 .479 .526 .272 .184	682 675 674 734 687 636 674 622 690 748 713 7701 624 643 671 642 623 710 648 645 731 717 664	0, 12 0, 04 0, 04 0, 05 0, 06 0, 07 0, 07 0, 08 0, 09 0, 09 0, 13 0, 08 0, 09 0, 19 0, 09 0, 19 0, 00 0, 00 0, 00 0, 00 0, 00 0, 00 0, 00 00 00 00 00 00 00 00 00 00 00 00 00	7.57 7.6.22 7.6.22 8.29 9.11 7.21 7.21 7.21 7.21 7.21 7.21 7.21	N.86 W N.56 W N.57 E N.52 E N.51 E N.51 E N.51 E N.51 E N.51 W N.27 E N.51 W N.61 W N.61 W N.78 W N.89 W N.77 E N.58 W N.78 W N.78 E N.58 W N.78 E N.58 W N.78 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 W N.78 E N.58 E N.	1.8 3.0 1.8 4.7 3.8 9.9 9.3 1.3 9.9 9.0 1.0 9.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1	7. 1 54. 1 55. 2 87. 63. 1 63. 1 64. 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	2.9	11.7 25.1 25.1 25.1 25.1 25.1 25.1 25.1 25.1	MITTER TO THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN CO	
Mean of autumn Mean of winter Means of all	-	-	28, 946 29, 074 29, 226 29, 096	-	-	-	-	-	-	N. 49 W. N. 63 W. N. 32 W.	2.3	103.8	-100		201 18	

		Pressure reduced to 32º Fahr.			Temp., Fahr.		Vapor.		(=clear sky.)			Win	d.		
Date.	l'ime of observation.	U. S. inches decimals.	ous, U. S. in- and decimals.	alb.	baib.	otty, U. S. tn.	Humidity. Satura-	of rain and melte	Am't cloudiness. (10=	Observed direction; from whence,	Observed velocity; miles per hour.	Resol and per		ity, i	rection n miles
	Time	Total	Gaseous, T	Dry bulb.	Wet bulb.	Electicity, I ches and do	Humi	Am't	Am't	Oberr	Obser	N.	8.	E.	w.
December, 1863 January, 1864 February, 1864 March, 1864 April, 1864 May, 1864 June, 1864 July, 1864 July, 1864 July, 1864 July, 1864 July, 1864 October, 1864 December, 1864 December, 1864 December, 1865 March, 1865 March, 1865 May, 1865 July, 1865 July, 1865 August, 1865 October, 1865 November, 1865 December, 1865 More, 1865 May, 1865 May, 1865 May, 1865 May, 1865 May, 1865 May, 1865 More, 186	A. Nr. 11. 30	99, 456 99, 300 99, 300 99, 300 99, 373 99, 323 99, 361 99, 301 90, 301 90,	28. 340 29. 198 29. 198 29. 145 29. 026 29. 036 29. 036 29. 053 29. 15	28. 5 24. 1 25. 7 40. 2 49. 9 64. 6 64. 6 64. 6 64. 6 64. 6 64. 6 64. 6 64. 6 64. 6 64. 6 64. 6 64. 6 64. 6 66. 6 6 6 6	26. 22 21. 8 4 23. 4 4 23. 4 4 23. 4 4 23. 4 4 2 23. 4 4 2 23. 4 4 2 23. 6 6 6 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	. 116 . 102 . 109 . 121 . 185 . 254 . 403 . 555 . 372 . 257 . 163 . 105 . 179 . 227 . 397 . 433 . 471 . 520 . 966 . 182 	. 718 . 705 . 676 . 677 . 742 . 702 . 646 . 639 . 708 . 638 . 658 . 658 . 651 . 629 . 713 . 655 . 647 . 742 . 669 . 666 . 671 . 666 . 671 . 686		8.7.43.8 7.7.5.8 8.9.9.1 9.9.9.9.9.1 7.6.1 7.6.9 7.8.8 9.9.1 7.6.9 8.9.9 7.7.8 8.9.9 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.9.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8	N. 89 W. N. 57 W. N. 48 E. N. 57 E. N. 36 E. E. N. 36 E. E. N. 26 W. N. 19 E. N. 26 W. N. 19 E. N. 26 W. N. 10 E. N. 26 W. N. 10 E. N. 26 W. N. 10 E. N. 20 W. N. 10 W. N. 20 W. N. 10 W. N. 20 W. N. 10 W. N. 20 W. 20 W. N. 20 W.	3.3	42. 1 78. 4 37. 6 96. 0 66. 5 43. 4 11. 5 56. 2 23. 1 76. 3 18. 3	2. 0 10. 9 6. 3 3. 7 65. 1	3. 9 27. 8 52. 4 23. 3 9. 6 67. 5 21. 4 13. 3 28. 5 56. 0 38. 0	49. 2 19. 2 92. 5 105. 5 62. 8 98. 5
Means of all	11.30	29. 362	29. 100	16. 1	41. 9	. 261	. 686	=	7.3	N. 32 W.	0.9	594. 9	<u></u>	<u> </u>	367. 7
December, 1863 January, 1864 February, 1864 March, 1864 April, 1864 May, 1864 June, 1864 July, 1864 August, 1864 October, 1864 October, 1864 October, 1865 March, 1865 March, 1865 March, 1865 March, 1865 July, 1865 July, 1865 July, 1865 July, 1865 July, 1865 October, 1865 November, 1865 November, 1865 December, 1865 November, 1865 Moan of spring Mean of summer Mean of summer	11 11 11 11 11 11 11 11 11 11	29. 456 29. 305 29. 203 29. 299 29. 377 29. 452 29. 346 29. 346 29. 349 29. 34	29, 058 29, 058 28, 898 28, 787 28, 997 29, 058 29, 149 29, 137 29, 137 29, 167 29, 185 29, 133 28, 979 29, 002 28, 940 28, 147 29, 147 29, 147	49. 5 64. 2 74. 1 59. 6 47. 4 36. 5 24. 6 19. 9 24. 9 32. 1 61. 5 68. 6 68. 6 67. 7 48. 3	45.3 564.7 666.4 54.3 33.7 44.3 37.8 44.8 37.8 62.9 44.8 37.4	. 253 . 397 . 537 . 559 . 372 . 254 . 163 . 076 . 097 . 137 . 179 . 231 . 408 . 434 . 475 . 518 . 973 . 183	. 710 . 634 . 696 . 651 . 716 . 763 . 728 . 697 . 663 . 637 . 728 . 662 . 755 . 734	0. 05 0. 03 0. 19 0. 08 0. 17 0. 23 0. 07 0. 11 0. 12 0. 04	*7355888899775764678	N. 61 E. N. 9 E. N. 19 E. S. 50 E. N. 31 W N. 19 E. N. 32 W N. 87 W N. 85 W N. 5 E. N. 5 E. N. 64 W S. 87 W S. 87 W S. 11 E N. 32 W S. 11 E N. 12 W S. 11 E N. 12 W S. 11 E N. 12 W S. 11 E N. 12 W S. 11 E N. 12 W S. 11 E N. 12 W S. 11 E N. 12 W S. 11 E N. 12 W S. 11 E N. 12 W S. 11 E N. 12 W S.	2.00 3.33 2.11 4.02 1.66 2.77 3.77 4.08 1.91 2.27 0.77 2.58 2.1.92 2.70 2.70 2.70 2.70 2.70 2.70 2.70 2.7	1.9 52.4 45.6 58.4 68.4 54.3 78.7 75.6 45.6 96.0 2.2 19.7 19.4 18.5 18.5 195.4	32. 4 10. 3 5. 2 80. 9	47. 1 105. 0 10. 3 19. 2 38. 5 26. 6 4. 9	93. 1 82. 7 45. 3 98. 7 111. 4 114. 2 102. 5 53. 4 93. 5 71. 1 10. 2

		Pressure reduced to 32° Fahr.			mp.,	, mpor.		d snow	aky en	Wind.					
Date.	of observation.	U. S. inches	ns, U. S. in-	ulb.	ulb.	and decimals.	Humidity, Satura-	f rain and melted snov, inches and decimals,	Am'teloudiness. (10=skyen- tirely overcust,0=clearsky.)	Observed direction; from whence,	Observed velocity; miles per hour.			city, in	
	Time of	Total, U	Gaseous, ches and	Dry bulb.	Wet bulb.	-		_	_		-	N.	8.	E.	W.
December, 1863 January, 1864 February, 1884 March, 1864 April, 1864 May, 1864 June, 1864 June, 1864 July, 1864 October, 1864 October, 1864 October, 1864 Ovember, 1864 December, 1864 January, 1865 February, 1865 March, 1865 June, 1865 June, 1865 June, 1865 June, 1865 October, 1865 October, 1865 October, 1865 October, 1865 December, 1865 December, 1865 October, 1865 December, 1865 December, 1865 December, 1865 May 1865 May 1865 June, 1865 June, 1865 December, 1865 October, 1865 Mean of summer Mean of summer	A. M. 10, 30 10,	29, 464 29, 310 29, 297 29, 377 29, 377 29, 377 29, 342 29, 343 29, 314 29, 314 29, 364 29, 377 29, 366 29, 366 29, 366 29, 367 29, 477 29, 472 29, 452 29, 431	1 29, 341 29, 921 29, 195 29, 195 29, 195 29, 195 29, 027 5 29, 027 5 29, 027 5 29, 063 3 29, 144 1 29, 286 5 29, 136 5 29, 136 5 29, 18	0 27. 7 1 29. 7 1 29. 7 1 29. 9 5 39. 5 5 49. 3 3 7 7. 7 3 7 7. 7 3 7 7. 7 5 5 9. 0 9 1 19. 4 9 3 1. 7 9 3 1. 7 9 3 1. 7 9 3 1. 7 9 3 1. 7 9 3 1. 7 9 3 1. 7 9 3 1. 7 9 4 1. 7 9 6 1. 7 9 6 1. 7 9 6 1. 7 9 7 9 8 1. 7 9 8	0 25, 4 20, 7 22, 3 26, 1 45, 3 56, 6 64, 5 64, 5 64, 5 64, 5 65, 9 122, 0 17, 1 29, 0 123, 6 144, 7 156, 8	115	. 726 . 703 . 697 . 750 . 614 . 700 . 665 . 722 . 767 . 738 . 703 . 644 . 674 . 674 . 674 . 679 . 731 . 677 . 781 . 739 . 693		8.7.6.7.5.7.6.3.7.5.5.9.3.8.8.8.8.9.0.3.1.7.3.7.6.4.4.5.7.8.6.4.4.5.7.8.6.6.4.4.5.7.8.6.6.4.8.6.6.4.8.6.6.4.8.6.6.4.8.6.6.8.6.6.4.8.6.6.8.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.6.8.6.8.6.8.6.6.8.6.6.8.6.8.6.6.8.6.	0 8. 76 E. 8. 89 W. N. 59 W. N. 59 W. N. 44 E. N. 10 E. N. 10 E. N. 20 W. East . N. 28 W. N. 52 W. N. 52 W. N. 65 W. N. 65 W. N. 65 W. N. 79 E. S. 11 E. S.	1.93 3.31 2.55 4.16 0.87 2.11 3.99 3.91 1.88 2.63 3.31 2.33 1.28 2.33 2.33 2.33 2.33 2.33 2.33 2.33 2	73. 6 38. 7 52. 6 77. 5 51. 1 35. 3 12. 3 14. 1 90. 1	0.11 7.4 10.1 68.0	53.9 104.3 11.3 23.9 50.0 3.6 58.8 13.6 8.8	100 元 100 元
Mean of winter	10, 30	29. 34	29, 24	3 23, 8	21. 6	6.099	. 693		8.7	N. 67 W.	2.1	239, 9 107, 8 133, 8 148, 9		54.1	38
Means of all	10. 30	29, 36	29. 10	45, 1	41.5	2. 259	. 702		7.3	N. 38 W.	1.1	629, (20
December, 1863 January, 1864 February, 1864 March, 1864 April, 1864 May, 1864 June, 1864 July, 1864 July, 1864 July, 1864 Aegust, 1864 September, 1864 October, 1864 December, 1864 December, 1865 February, 1865 February, 1865 March, 1865 May, 1865 July, 1865 July, 1865 July, 1865 September, 1865 October, 1865 December, 1865	10 10 10 10 10 10 10 10 10 10 10 10 10 1	29, 31, 29, 29, 29, 28, 29, 28, 29, 36, 29, 36, 29, 36, 29, 36, 29, 36, 29, 36, 29, 36, 29, 36, 29, 36, 29, 36, 29, 36, 29, 36, 29, 41, 29, 46, 29, 40, 29, 40, 29, 42, 42, 42, 42, 42, 42, 42, 42, 42, 42	5 29, 214 8 29, 09 5 29, 15 1 29, 20 6 29, 03 6 29, 03 6 29, 03 6 29, 03 6 29, 06 7 28, 89 7 28, 79 7 28, 79 1 29, 14 1 29, 18 1 29,	2 23. 5 2 23. 5 3 2 24. 5 5 6 63. 5 6 63. 5 6 7 72. 5 8 58. 4 7 72. 5 8 13. 5 9 40. 4 8 66. 5 6 66. 5 6 66. 5 6 66. 5 6 66. 5 6 66. 5 6 66. 5 6 66. 5 6 66. 5 6 66. 5	20.	1,097 9,106 5,166 5,181 5,252 5,388 5,535 5,535 5,535 5,535 6,535 6,535 6,535 7,53	. 721 . 712 . 708 . 757 . 729 . 635 . 718 . 682 . 734 . 779 . 651 . 678 . 690 . 663 . 742 . 690 . 643 . 683 . 683 . 746 . 683 . 683 . 683 . 746 . 683 . 683 . 683 . 683 . 744 . 742 . 744	0. 08 0. 09 0. 15 0. 02 0. 02 0. 02 0. 02 0. 08 0. 07 0. 03 0. 04 0. 07 0. 03 0. 04 0. 07 0. 04 0. 02	7.78.82 7.29.27.99.45.00 6.00 8.31 9.9.29.99.28 7.44.88 6.6.56 6.57.88	S. 81 E. S. 84 W. N. 79 W. N. 79 W. N. 48 E. N. 10 E. N. 10 E. N. 10 E. N. 21 W. N. 85 E. N. 30 W. S. 86 W. N. 70 W. N. 49 W. N. 49 W. N. 77 W. N. 77 W. N. 77 W. N. 77 W. N. 14 W. N. 24 W. N. 68 W. N. 68 W. N. 90 E. N. 20 W. N. 90 E. N.	23 22 20 23 1.7 0.9 22 3.0 3.9 1.5	51. 4 68. 8 78. 5 31. 6 1. 6 73. 7 26. 9 59. 8 71. 6 62. 4 34. 0 14. 5 28. 1 93. 1 43. 0	10. 2 7. 8	56.1 90.5 14.5 16.6 72.5 16.6 10.0	S SUID S COS S S S S S S S S S S S S S S S S S
Mean of autumn Mean of winter Means of all	10	29. 349	29. 244	23, 2	21. 2	. 098	. 702	0, 76	8.8	N. 51 W. N. 68 W. N. 41 W.	1.6	147. 7 114. 4 629. 9			田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田田

Date. December, 1863			reduc	ed to Fahr.	Ter		Vaj	por.	d snow,	sky en-	Wind,						
B	Date.	of observation.	U. S. inches	us, U. S. in-	lb.	ulb.	ity, U. S. in.	ity. Satura-	f rain and melte inches and dec	oudiness. (10=	red direction; m whence.	91	per nour.				
Mean of summer 9, 30, 29, 417, 28, 957, 65, 8, 60, 3, 460, 716 5, 7, N, 26, W. 0, 9, 151, 6 81 Mean of autumn 9, 30, 29, 368, 29, 395, 84, 245, 0, 285, 765 7, 9, N, 65, W. 1, 4, 101, 2 241 Mean of winter 9, 30, 29, 367, 29, 111, 43, 9, 40, 4, 256, 724 7, 2, N, 42, W. 1, 2, 648, 5 303 Means of all 9, 30, 29, 367, 29, 111, 43, 9, 40, 4, 256, 724 7, 2, N, 42, W. 1, 2, 648, 5 573 December, 1863 9, 29, 471, 29, 359, 26, 624, 5, 112, 742, 0, 30, 8, 8, 8, 89, E. 3, 3 2, 0, 104, 6 5, 73 January, 1864 9, 29, 307, 29, 213, 30, 9, 19, 2, 004, 730, 0, 11, 7, 5, 8, 74, W. 3, 5 30, 5 104 February, 1864 9, 29, 272, 29, 160, 26, 324, 1, 111, 717, 0, 09, 8, 6, N, 88, W. 2, 7 2, 0 80 March, 1864 9, 29, 372, 29, 160, 26, 324, 1, 111, 717, 0, 08, 7, 0, N, 47, E. 2, 2, 46, 2, 49, 5 3, 24, 1, 111, 171, 171, 0, 08, 7, 0, N, 47, E. 2, 2, 46, 2, 49, 5 3, 24, 1, 111, 171, 171, 0, 08, 7, 0, N, 47, E. 2, 2, 46, 2, 49, 5 3, 2, 144, 2, 250, 752, 0, 07, 74, N, 5, E. 2, 6, 69, 8, 7, 5 3, 3, 2, 6, 69, 8, 7, 5 3, 3, 2, 4, 1, 111, 111, 111, 111, 111, 111,		_	_	_		_		ches a Humidi tior		Am'		-	N.	S.	E.	w.	
Means of all. 9, 30 29, 367 29, 111 43, 9 40, 4, 256, 724 7, 2 N, 42 W. 1, 2 648, 5 573 December, 1863. 9 29, 471 29, 359 26, 624, 5, 112, 742, 0, 30 8, 8 8, 8 9 3, 3 2, 0 104, 6 January, 1864. 9 29, 307 29, 213, 20, 9 19, 2, 094, 730, 0, 11 7, 5 8, 74 W. 3, 5 30, 5 104 February, 1864. 9 29, 20, 192, 096, 23, 021, 1, 105, 717, 0, 09 8, 6 N, 88 W. 27, 2, 0 8 8, 74 W. 3, 5 30, 5 104 9 29, 272, 29, 160, 26, 3, 24, 1, 112, 717, 0, 09 8, 6 N, 88 W. 27, 2, 0 8 8, 8, 89 40, 2 2, 0 8 60, 9 8, 7 20, 373, 29, 192, 213, 29,	December, 1863. January, 1864 February, 1864 March, 1864 May, 1864 May, 1864 May, 1864 June, 1864 June, 1864 June, 1864 October, 1864 October, 1864 December, 1864 December, 1865 March, 1865 March, 1865 March, 1865 May, 1865 May, 1865 May, 1865 Muser, 1865 May, 1865 May, 1865 November, 1865 December, 1865 December, 1865 December, 1865 December, 1865 Mean of summer Mean of autumn	A.M. 9, 30 9,	29. 467 29. 319 29. 201 29. 272 29. 377 29. 375 29. 365 29. 365 29. 365 29. 365 29. 366 29. 466 29. 466 29. 466 29. 427 29. 325 29. 427 29. 328 29. 427 29. 328 29. 427 29. 328	29. 355 29. 216 29. 216 29. 158 29. 197 29. 158 29. 197 28. 977 28. 977 29. 150 29. 15	0 26. 9 21. 5 23. 4 48. 2 23. 4 48. 2 24. 45. 2 3 48. 2 3 4 48. 2 3 4 48. 2 3 4 48. 7 1. 4 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	24. 7 19. 6 21. 5 24. 9 36. 2 44. 6 58. 6 64. 0 65. 2 16. 2 22. 8 22. 8 24. 0 16. 1 36. 2 44. 0 16. 1 36. 2 43. 3 6 43. 3 6 43. 6 44. 6 5 6 44. 6 5 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 45. 6 46. 6	. 113 . 096 . 105 . 252 . 383 . 363 . 364 . 242 . 160 . 103 . 073 . 073 . 073 . 134 . 174 . 434 . 462 . 249 . 178 . 249 . 178 . 178 . 249 . 178 . 249 . 178 . 249 . 178 . 249 . 178 . 249 . 178 . 249 . 178 . 249 . 178 . 249 . 178 . 249 . 178 . 249 . 178 . 249 . 178	. 736 . 729 . 713 . 714 . 765 . 743 . 657 . 729 . 703 . 750 . 708 . 654 . 689 . 702 . 703 . 672 . 757		5.7	N. 26 W. N. 65 W.	0.9	25. 7 45. 8 69. 3 83. 26. 7 6. 1 70. 3 21. 2 62. 0 4. 5 57. 6 68. 8 4. 4 0. 4 41. 6 36. 4 73. 2 18. 4 250. 2 151. 6	13. 5	50. 5 88. 6 0. 5 76. 1	3.1 2.1 47.2 41.6 1122.6 117.8 87.3 1.1 59.4 16.0 16.3 121.5	
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Mean of autumn 9 29, 379 29, 997 47, 7 44, 9, 282, 769 0, 79 7, 8 N, 60 W. 1, 4 131, 0 213 Mean of winter 9 29, 342 29, 246 22, 3 20, 4 096, 711 0, 90 8, 7 N, 71 W. 1, 9 104, 0 319 Means of all 9 29, 366 29, 112 43, 3 40, 1, 254, 733 2, 12 7, 3 N, 38 W. 1, 1 631, 8 547	Mean of spring. Mean of snumer. Mean of sutumn Mean of winter.	9 9 9	29, 328 29, 416	29, 147	38, 3 65, 1	35. 3 60. 0	. 181	723	0.41 0.02	7. 1	N. 17 E. N. 26 W.	1.4 0.9 1.4	152.8 131.0 104.0	::::		79. 8 213. 9 319. 1	

	-	redu	ced to Fahr.	Ter Fa	np.,	Vaj	por.	d snow	(10=sky en=clearsky.)			Win	d.		
Date.	Time of observation.	U. S. inches d decimals.	us, U. S. in-	oulb.	oulb.	city, U. S. in-	Humidity, Satura-	un't of rain and melted snov U. S. inches and decimals.	Am'telondiness. (10=sky er tirely overcast, 0=clear sky.	Observed direction; from whence.	Observed velocity; miles per hour.	and		of directly, is	
	Time	Total, I	Gaseous, ches and	Dry bulb.	Wet bulb	Elasticity, ches and d	-	45	Am't	Obser	Obser	N.	s.	E.	W.
December, 1863 January, 1864 February, 1864 March, 1864 April, 1864 May, 1864 July, 1864 July, 1864 July, 1864 October, 1864 November, 1864 December, 1864 December, 1865 December, 1865 April, 1865 April, 1865 May, 1865 July, 1865 April, 1865 September, 1865 October, 1865 October, 1865 November, 1865 May, 1865 December, 1865 November, 1865 November, 1865 November, 1865 November, 1865 November, 1865 May,	A M.	00 400	00 240	0	0	114	755		00	O N OS F	3.0	8.3		90 0	
January, 1864	8.30	29, 308	29, 214	20. 6	18.9	. 094	740		7.5	S. 82 W	2,8		12.6		87
February, 1864	8. 30	29, 197	29, 093	22.5	20. 7	. 104	. 715		8.6	N. 80 W.	2.5	12.9 47.2		55. 9	720
April 1864	8.30	29. 370	29, 101	38. 0	35. 5	178	780		7.9	N. 53 E.	3.4	62. 9		82.1	
May, 1864	8. 30	29, 286	29, 035	47. 1	43. 8	. 251	. 766		7.7	N. 1E.	3.4	104, 0		2.3	
June, 1864	8.30	29, 457	29, 069	61. 2	55. 7	.388	. 683		4.1	N. 23 W.	1.5	40.9			11
July, 1864	8. 30	29, 428	28, 908	67. 7	63. 0	. 520	760		5.0	N. 28 W.	2.7	18. 9			3
Sentember 1864	8 30	29. 343	99, 003	56. 9	59 0	360	771		7 7	N. 89 E	2.1	8.4		63.0	
October, 1864	8. 30	29. 314	29, 079	43. 5	41.3	235	.814		8.4	N. 37 W.	2.8	68. 4		63. 0	3
November, 1864	8.30	29, 308	29, 147	34. 5	32, 3	. 161	. 775		9.1	N. 88 W.	4.4	4.6		7.4	13
December, 1864	8. 30	29, 252	29, 148	23, 1	21, 3	. 104	620		9.3	N. 80 W.	3.7	28. 6			15
February 1865	8.30	29, 460	29, 379	21.7	19. 7	088	702		9.0	N. 5 E.	3.0	82.8	133	7.4	
March, 1865	8. 30	29, 309	29, 179	29. 2	27.1	. 130	. 736		8.0	N. 41 W.	1.8				
April, 1865	8.30	29, 362	29. 192	37.9	35. 0	. 170	. 728		8.0	N. 87 W.	2.3	9. 0			
May, 1865	8, 30	29, 364	99, 126	48. 2	55.0	238	770		5.0	N. 74 E.	1.7	14. 2	40 5	50, 0	133
uly, 1865	8, 30	29. 417	28, 991	63. 5	58. 4	. 426	723		6.4	N. 38 W.	1.4	34. 5			2
August, 1865	8, 30	29, 467	29, 010	64.8	59. 9	. 457	. 740		5.4	N. 17 W.	1.7	50, 6	1		1 3
September, 1865	8. 30	29, 458	28. 957	64.0	60. 9	. 501	. 826		5. 5	S. 6 W.	20		58. 9		1
November 1865	8.30	29, 406	29, 165	38 6	35. 7	177	753		8.0	N. BRW	3.4	94. 3		1.1	iii
December, 1865	C. 00			30, 0	30. 1				0.0			2			See.
Mean of spring	8, 30	29. 327	29, 147	37.7	34.8	. 180	. 740		7.3	N. 15 E.	1.6	274.7		83. 9	
Mean of summer	8. 30	29, 416	28, 961	64. 3	59, 4	455	737		5. 6	N. 38 W.	1.2	100.9			120
Mean of winter	8. 30	29. 339	29, 243	22 0	20. 1	. 096	717		8.6	N. 60 W.	2.0	174. 9			
Means of all	8. 30	29, 365	29. 112	42.7	39. 6	. 252	. 744		7.3	N. 38 W.	1.3	737. 5			586
		00 480													
December, 1863 January, 1864	8	29, 458	29, 345	26. 3	18 7	. 113	746	0.29	7.5	N. 86 E.	2.8	7.9		101. 8	85
February, 1864 March, 1864	8	29, 192	29. 085	22. 2	20, 5	. 107	719	0, 20	8.4	N. 84 W.	2.6	8.3			
March, 1864	8	29, 268	29.160	24.9	22, 9	.108	. 724	0.20	7.4	N. 50 E.	2.2	43. 7		51. 6 73. 9 0. 7	
April, 1864	8	29, 367	29, 191	37. 3	35. 1	. 176	. 789	0. 03	8.1	N. 50 E.	3.2	61. 7		73. 9	
May, 1864 May, 1864 June, 1864 July, 1864 August, 1864	8	29, 457	29, 071	60. 3	55. 2	386	704	0.01	4.3	N. 24 W.	21	57. 1		0.7	100
July, 1864	8	29.428	28, 914	66, 6	62.8	. 514	. 777	0.03	4.7	N. 20 W.	1.2	34. 9			2
August, 1864	8	29, 342	28, 800	68. 8	64. 0	. 542	. 758	0. 01	6.3	N. 37 W.	2.6	65. 3			4
October, 1864	8	29, 309	29, 077	42, 9	40.8	232	825	0.06	8.0	N. 44 W.	2.5	57. 5	-	66, 9	3
September, 1864 October, 1864 November, 1864 December, 1864	8	29, 305	29, 146	34.3	32. 1	. 159	.773	0.07	9. 0	8. 85 W	4.4		4.575 F		100
January 1864	8	29, 251	29, 148	23. 0	21. 2	. 103	. 720	0, 32	9, 4	N. 81 W.	5.3	24. 1	1000		16
January, 1865	8	29, 457	29, 370	21.1	19.9	087	708	0, 04	8.8	N. S F	3 1	50.8 87.0	.000	12.0	9
March, 1865	8	29, 307	29, 181	28, 2	26, 2	. 126	. 740	0.15	8.0	N. 28 W.	1.9				200
April, 1865	8	29, 361	29. 192	37.1	34. 4	. 168	. 739	0.07	8.0	N. 89 W.	2.5	1.0		27. 2	7
May, 1865	8	29. 301	29, 125	58 1	54.5	236	707		6.9	N. 53 E	1.1	20.3	10 0	27, 2	
July, 1865	8	29, 416	29, 001	62.5	57. 6	415	739		5.9	N. 40 W	1.9	97.9	10.0		9
June, 1865 July, 1865 August, 1865 September, 1865	8	29.465	29.013	63. 7	59.3	. 452	. 759	0.17	5. 4	N. 21 W.	1.4	40, 0		9.1	1
September, 1865	8	29, 457	28, 964	63, 1	60. 3	493	. 840	0.32	5.8	8. 9 W.	1.8		23. 5		1
November, 1865	8 8	29, 429	29, 246	38.9	35.5	176	749	0. 04	8.7	8. 85 W	2.8	85. 2	9.7	-	100
October, 1865			*****							N. 86 E. N. 88 W. N. 84 W. N. 50 E. N. 50 E. N. 50 E. N. 50 E. N. 24 W. N. 22 W. N. 63 E. N. 63 E. N. 85 W. N. 81 W. N. 88 E. N. 53 E. N. 53 E. N. 53 E. N. 54 W. N. 53 E. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 53 E. N. 54 W. N. 54 W. N. 55 E. N. 55 W. N. 55 E. N. 54 W. N. 55 E. N. 55 W. N. 55 E. N. 55 W. N. 55 E. N. 55 W. N. 55 E. N. 55 E. N. 55 E. N. 55 W. N. 55 E. N. 55 W. N. 55 E. N. 55 W. N. 55 E. N. 55 W. N. 55 E. N. 55 W. N. 55 E. N. 55 W. N. 55 E. N. 55 W. N. 55 E. N. 55 W. N. 55 E. N. 55 W. N.			20.4		200
Mean of spring	8	29, 325	29. 148	36.9	34. 2	. 176	. 745	0, 52	7.4	N. 11 E	1.5	275. 4	1600	50.4	Car.
Mean of summer Mean of autumn	8	29, 415	28. 965	63, 3	58. 9	. 449	754	0. 21	5. 6	N. 11 E N. 35 W. N. 65 W.	1.2	177. 2	1.10		128
	8	00 335	29, 240	91.7	19. 9	095	720	0. 30	8.6	N. 60 W.	2.0	102.7		****	233 306
Mean of winter															
Means of		_		_			_			N. 38 W.	-	_	-	10000	618

		redu	sed to	Ter	np., hr.	Vaj	or.	d snow, imals.	sky en-			Wind			
Date.	Time of observation.	U. S. inches d decimals.	us, U. S. in-	alb.	elb.	aty, U. S. in-	Humidity. Satura-	m't of rain and melted anow U. S. inches and decimals.	Am't cloudiness. (10=sky e thely overcast, 0=clear sky	ved direction; m whence.	Observed velocity; miles per hour.	Resolu and per	ition veloc hour.	ity, in	ection miles
	Time	Total, T	Gaseous, ches and	Dry balb.	Wet bulb.	Elasticity, ches and d	_	•	Am'te	Observed of from w	Obser	N.	8.	E.	w.
December, 1863. January, 1864 February, 1864 March, 1864 April, 1864 May, 1864 Jule, 1864 July, 1864 September, 1864 October, 1864 November, 1864 December, 1864 December, 1865 March, 1865 March, 1865 April, 1865 April, 1865 April, 1865 April, 1865 May, 1865 July, 1865 September, 1865 October, 1865 November, 1865 September, 1865 November, 1865 Decem	7. 30 7. 30	99, 449, 229, 229, 229, 239, 249, 249, 249, 249, 249, 249, 249, 24	\$9. 335, 22, 061 \$9. 150, 106 \$9. 150, 106 \$9. 107, 107, 107, 107, 107, 107, 107, 107,	26. 3 20. 0 22. 0 22. 0 26. 5 5 5. 6 6 6 7. 6 6 7. 6 1 2 2 8 8 1 2 9 1 8 1 8 1 8 1 8 1 8 1 8 1 8 1 8 1 8 1	94. 44 18. 5 20. 3 33. 1 42. 8 54. 3 66. 7 52. 1 40. 4 32. 0 14. 9 25. 4 55. 4 32. 0 14. 9 25. 4 35. 5 35. 5 36. 5 36. 5 36. 5 36. 5 36. 5 36. 5 36. 5 36. 5 36. 5	. 114 . 093 . 105 . 105 . 173 . 245 . 357 . 356 . 160 . 163 . 087 . 122 . 381 . 405 . 441 . 482 . 237 . 178 	. 741 . 745 . 724 . 732 . 802 . 788 . 709 . 794 . 773 . 837 . 779 . 779 . 711 . 738 . 748 . 720 . 804 . 744 . 755 . 766 . 766 . 775 . 776 . 776 . 777 . 777 . 779		8.7.8.7.8.2.3.611147726649958866.7788.9.8.9.8.7.5.6.5.5.5.7.8.7.5.7.8.9.8.8.7.5.6.5.5.5.7.8.	O M. 86 E. S. 87 W. N. 88 W. N. 88 W. N. 82 E. N. 6 W. N. 53 E. N. 6 W. N. 75 W. N. 75 W. N. 75 W. N. 75 W. N. 75 W. N. 75 E. W. N. 75 W.	3.53.07 3.77 3.13.13 3.33 2.11 2.57 3.07 2.1.55 1.1.56 1.2.52 2.53 3.8.1.1.55	2. 8 44. 22 56. 1 102. 8 64. 5 31. 3 63. 7 21. 4 68. 1 56. 4 83. 6 26. 2 31. 2 36. 6	10. 7	77. 4 77. 4 18. 0	92. 8 78. 0 10. 9 29. 3 13. 1 42. 6 60. 1 135. 9 164. 4 101. 1 11. 7 72. 8 1. 5 38. 3 22. 3 16. 9
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December, 1863. January, 1864 February, 1864 March, 1864 May, 1864 June, 1864 June, 1864 July, 1864 August, 1864 Cotober, 1864 November, 1864 November, 1865 Harch, 1865 March, 1865 April, 1865 July, 1865 July, 1865 July, 1865 September, 1865 October, 1865 May, 1865 Sure, 1865 Sure, 1865 Sure, 1865 September, 1865 May, 1865 September, 1865 Movember, 1865 November, 1865 Mean of summer Mean of summer Mean of autumn Means of all	777777777777777777777777777777777777777	29. 447 29. 183 29. 284 29. 383 29. 383 29. 353 29. 353 29. 354 29. 35	\$9, 334 \$9, 202 \$9, 078 \$9, 149 \$9, 192 \$9, 078 \$9, 078 \$9, 073 \$9, 140 \$9, 146 \$9, 134 \$9, 134 \$9, 134 \$9, 192 \$9, 192 \$9, 192 \$9, 166 \$9, 166 \$9, 148 \$9, 14	26. 3 19. 9 21. 4 23. 4 24. 9 25. 7 26. 5 26. 7 26. 7 26. 7 27. 3 26. 2 26. 7 27. 3	24. 3 20. 2 21. 5 3 42. 3 53. 5 61. 7 651. 6 62. 6 64. 6 2 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	. 113 . 092 . 104 . 103 . 171 . 242 . 374 . 500 . 516 . 102 . 230 . 160 . 120 . 120 . 227 . 377 . 396 . 434 . 471 . 170 . 178	. 746 . 731 . 723 . 733 . 810 . 743 . 784 . 784 . 789 . 780 . 733 . 711 . 739 . 755 . 762 . 762 . 768 . 768 . 768 . 769 . 769	0. 26 0. 07 0. 16 0. 16 0. 12 0. 02 0. 01 0. 03 0. 02 0. 04 0. 03 0. 04 0. 03 0. 04 0. 03 0. 04 04 04 04 04 04 04 04 04 04 04 04 04 0	87.8.119.667.8.9.465.05568.04988.7.5.665.5.7.8.1.3.0.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2	N. 80 E. N. 85 W. N. 85 W. N. 39 E. N. 65 E. N. 65 E. N. 49 W. N. 23 W. N. 35 W. N. 79 W. N. 79 W. N. 17 E. N. 18 W. N. 17 E. S. 18 W. N. 17 E. N. 18 W. N. 17 E. N. 18 W. N. 17 E. N. 18 W. N. 18 W. N. 18 W. N. 18 W. N. 18 W. N. 19 W. N. 10 W. N. 10 W. N. 10 W. N. 10 W. N. 10 W. N. 10 W. N. 10 W. N.	2.7 2.9 2.7 2.2 2.1 2.4 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1	64. 9 31. 3 60. 7 30. 5 56. 4 65. 6 64. 2 57. 0 32. 4 36. 1 3. 6	1. 7 39. 2	9 9	65. 1 140. 2 156. 8 106. 2 21. 7 81. 8 13. 2 33. 9 12. 3 19. 4
Mean of autumn	7	29, 373 29, 330 29, 356	29, 101 29, 235 29, 115	45. 6 21. 3 40. 8	43, 2 19, 5 38, 2	. 271 . 094 . 242	. 806 . 719 . 767	0. 81 0. 94 3. 02	7. 8 8. 3 7. 2	N. 67 W. N. 62 W. N. 42 W.	1.5	157. 4 117. 8 186. 7	—		

		reduc	ed to	Te:	mp.,		por.	imals.	9. (10=sky en- 0=clear sky.)			Wind	L		
Date,	Time of observation,	, U. S. inches d decimals.	and decimals.	oulb.	only.	eity, U. S. in-	Humidity, Satura-	tof rain and melted snow	Am't cloudiness. (10=tirely overcast, 0=cle	Observed direction; from whence,	Observed velocity; miles per hour,	Resolution and per l		ity, in	
	Time	Total, I	Gaseous,	Dry bulb.	Wet bulb.	Elusticity,	Humi	Am'te	Am'te	Obser	Obser	N.	S.	E.	W.
December, 1863 January, 1864 Pebruary, 1864 April, 1864 March, 1864 March, 1864 Mur, 1864 Mur, 1864 Mur, 1864 Mur, 1864 Mur, 1864 Mur, 1864 Mur, 1864 Petober, 1864 December, 1864 January, 1865 Pebruary, 1865 Murch, 1865 June, 1865 June, 1865 June, 1865 Mur, 1866 Mur	A.M.6. 366. 366. 366. 366. 366. 366. 366.	29. 443 29. 291 29. 178 29. 252 29. 359 29. 359 29. 452 29. 452 29. 358 29. 398 29. 299 29. 398 29. 398 29. 398 29. 366 29. 462 29. 462 29. 462 29. 462 29. 462 29. 462 29. 462 29. 463 29. 462 29. 463 29. 46	29. 3299 29. 2009 29. 2009 29. 148 29. 191 29. 041 29. 083 28. 839 28. 839 29. 137 29. 137 29. 137 29. 148 29. 100 29. 367 29. 195 29.	26. 21. 8 21. 8 23. 22. 34. 8 23. 24. 39. 56. 56. 3 34. 22. 44. 1 22. 44. 1 26. 0. 2 26. 0. 34. 2 26. 0. 34. 3 34. 37. 8 34. 4. 37. 8 35. 38. 38. 38. 38. 38. 38. 38. 38. 38. 38	24. 3 20. 1 21. 4 33. 0 20. 6 41. 5 52. 9 60. 2 20. 6 61. 7 540. 0 32. 0 20. 6 61. 7 55. 1 55. 1 55. 2 9 60. 2 55. 1 56. 7 56. 7 56. 5 67. 5 68. 7 56. 5 69. 2	.114 .091 .103 .103 .167 .237 .369 .485 .510 .348 .228 .161 .102 .070 .085 .118 .156 .223 .369 .422 .464 .424 .423 .178	.7755 .7266 .7144 .740 .8244 .800 .8122 .7611 .8344 .839 .778 .7311 .7632 .7811 .8022 .790 .762 .7744 .800 .762 .7744 .800 .8122 .790 .762		8.74.7.7.9.9.4.7.7.4.6.8.0.5.5.2.8.7.7.6.8.0.5.5.2.8.7.7.6.8.0.5.5.2.8.7.7.6.8.0.5.5.2.8.7.7.6.8.0.5.5.2.8.7.7.6.8.0.5.5.2.8.7.5.6.8.0.5.2.8.7.7.6.8.0.5.2.8.7.7.6.8.0.5.2.8.7.7.6.8.0.0.5.2.8.7.7.6.8.0.0.5.2.8.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0	N. 63 E. N. 74 W. N. 82 W. N. 82 W. N. 52 E. N. 52 E. N. 52 W. N. 17 W. N. 61 E. N. 53 W. N. 61 E. N. 78 W. N. 78 W. N. 18 W. N. 19 E. N. 19 E. N. 19 E. N. 19 E. N. 19 E. N. 19 W. N. 19 E. N. 19 E. N. 19 W. N. 19 E. N. 19 W. N. 19 E. N. 19 W. N. 19 E. N. 19 W. N. 19 E. N. 19 W. N. 19 E. N. 19 W. N. 19 E. N. 19 W. N. 19 E. N. 19 W. N. 19 W. N. 19 E. N. 19 W. N. 19 W. N. 19 E. N. 19 W. N. 10 W. N.	2.3.3.2.8.2.5.5.3.8.1.9.1.0.5.2.5.5.4.4.3.7.2.5.5.3.1.1.0.0.8.2.6.6.4.0.1.3.3.1.5.6.6.2.6.6.1.3.3.1.5.6.6.2.6.6.1.3.3.1.5.6.6.6.2.6.6.1.3.3.1.5.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.2.6.6.2.6.6.2.6.6.2.6.6.2.6.2.6.6.2.2.6.2.2.6.2.6.2.6.2.2.6.2.2.6.2.2.6.2.2.6.2	34. 6 36. 2 46. 3 34. 6 74. 8 72. 2 14. 0 30. 9 41. 4 19. 8	2. 0	4.9	25 25 25 25 25 25 25 25 25 25 25 25 25 2
Means of all	6, 30	29. 356	29. 118	40. 1	37. 7	. 239	777		7.3	N.42 W.	1.6	858. 6			790
December, 1863 annary, 1864 "ebruary, 1864 April, 1864 April, 1864 day, 1864 une, 1864 une, 1864 une, 1864 betober, 1864 betober, 1864 betober, 1864 betober, 1864 betober, 1865 betober, 1865 farch, 1865 une, 1865 une, 1865 une, 1865 une, 1865 betober, 1865 betober, 1865 covember, 1865 betober, 1865 covember, 1865 betober, 1865	666666666666666666666666666666666666666	29, 445 29, 283 29, 176 29, 245 29, 356 29, 278 29, 449 29, 333 29, 355 29, 391 29, 357 29, 452 29, 372 29, 372 29, 461 29, 461 29, 453 29, 461 29, 453 29, 461 29, 453 29, 461 29, 453 29, 461 29, 453 29, 461 29, 453 29, 461 29, 453 29, 462 29, 372 29, 372 29, 372 29, 372 29, 472 29, 473 29, 466 29, 372 29, 372 29, 372	29, 041 29, 093 28, 947 28, 823 28, 998 29, 073 29, 136 29, 148 29, 287 29, 287 29, 187 29, 193 29, 050 29, 050 28, 913 29, 241	343, 9 562, 2 664, 8 53, 4 41, 9 34, 1 16, 5 20, 0 25, 5 342, 6 56, 5 57, 6 60, 5 59, 1 60, 5 59, 1 60, 5 78, 2	32. 0 41. 5 559. 4 61. 4 551. 5 40. 0 32. 1 120. 5 14. 7 18. 2 23. 6 23. 6 23. 1 23. 5 55. 9 40. 6 60. 4 40. 6 60. 6	. 100 . 237 . 356 . 474 . 510 . 357 . 228 . 101 . 070 . 085 . 115 . 153 . 217 . 361 . 380 . 411 . 462 . 230 . 179	812 781 837 811 837 811 828 836 784 733 678 716 729 776 841 795 808 860 791 763	0, 21 0, 15 0, 12 0, 05 0, 05 0, 20 0, 22 0, 38 0, 09 0, 09 0, 03 0, 03 0, 02 0, 12 0, 03 0, 04 0, 05 0,	7.83 4.34 5.69 9.00 9.88 9.80 7.60 7.60 7.60 7.60 7.60 7.60 7.60 7.6	N. 58 E N. 75 W. N. 77 W. N. 62 E N. 51 E N. 51 E N. 52 E N. 52 W. N. 34 W. N. 52 W. N. 52 W. N. 53 W. N. 53 W. N. 53 W. N. 53 W. N. 53 W. N. 53 W. N. 53 W. N. 54 W. N. 53 W. N. 54 W. N. 53 W. N. 54 W. N. 53 W. N. 54 W. N. 54 W. N. 55 W. N. 56 W. N. 56 W. N. 57 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 58 W. N. 59 W. N. 59 W. N. 50 W.	1.3.6.5.3.3.8.3.7.3.9.7.7.5.5.6.6.9.3.1.0.9.9.5.1.2.2.2.4.5.4.2.2.2.0.1.2.1.2.3.1.1.6.6.6.6.6.6.6.6.6.6.6.6.6.6.6.6.6	16. 3 33. 4 62. 8 116. 2 58. 9 47. 2 57. 5 50. 7 55. 6 40. 5 9. 5 9. 5 9. 5 9. 5 9. 5 9. 5 9. 5 9	3. 2	63. 0 77. 4 68. 3	15 35 41 61 165 165 165

		reduc	ed to	Ter	mp., hr.	Vaj	or.	d snow	sky en			Wind			
Date.	Time of observation.	Total U. S. inches and decimals.	ns, U. S. in-	alb.	alb.	Elasticity, U. S. in-	Humidity. Satura- tion = 1.000.	s, inches and melted snow S, inches and decimals.	Am'teloudiness. (10=skyen- tirely overcast, 0=clear sky.)	erved direction; from whence.	ele po		rtion veloc hour.	ity, in	rection n miles
			Gascous, U	Dry bulb.	Wet bulb.		Hun	Am'	Am'	OP	Observed v miles per	N.	s.	E.	w.
December, 1863 January, 1864 February, 1864 March, 1864 March, 1864 May, 1864 June, 1864 June, 1864 July, 1864 August, 1864 October, 1864 November, 1864 December, 1864 January, 1865 March, 1865 July, 1865 July, 1865 July, 1865 July, 1865 July, 1865 July, 1865 July, 1865 July, 1865 July, 1865 May, 1865 July, 1865 October, 1865 October, 1865 November, 1865 December, 1865 December, 1865 Mean of summer Mean of summer Mean of winter Means of all	A.M. 5. 30	29, 441	29, 325	26. 2	24. 2	. 116	.747		8.2	s. 88 E .	1.1		1.1	36. 5	
January, 1864	5. 30	29, 287	29, 194	19. 8	18. 2	104	710		7.3	N. 66 W.	3.7	46. 3 27. 1 47. 4 61. 0 112. 7 69. 3			105.
March 1864	5, 30	29, 175	29, 071	22. 8	21.9	108	763		6.8	N. 47 E	2, 2	47. 4		50, 1	
April, 1864	5. 30	29, 354	29, 193	33. 9	32. 1	. 161	. 820		8.0	N. 50 E .	3. 2	61, 0		72, 2	
May, 1864	5.30	29, 274	29, 041	43. 1	40, 9	. 233	. 822		7, 6	N. 9 W.	3. 7	112.7			17.
June, 1864	5. 30	29, 445	29. 101	53. 8	50. 6	. 344	. 752		3.8	N. 30 W.	2.7	54.9		****	40. i
Anonet 1864	5, 30	29, 420	98, 898	64.5	61 0	503	811		6.0	N. 38 W	1.9	46.0			36.
September, 1864	5, 30	29, 355	29, 003	53. 2	51. 3	. 352	. 827		6.9	N. 49 E .	2.1	41.9		47.8	
October, 1864	5.30	29, 299	29, 071	41.8	39. 9	. 228	. 836		7.9	N. 46 W.	2.7	57. 6			59.
November, 1864	5, 30	29, 298	29. 137	34. 1	32. 0	. 161	. 783		8.9	8. 87 W.	4.8	69 5	7.7		143. 153.
January 1865	5, 30	29, 249	90, 989	16.3	14.5	069	676		8.7	N. 54 W	5.4	89. 8			123.
February, 1865	5, 30	29, 450	29, 365	19. 9	18, 1	. 085	719		8.2	N. 5 W.	2.7	75. 1			6,
March, 1865	5, 30	29, 303	29, 190	25, 2	23, 3	.113	. 724		7.9	N. 18 W.	2.5	73. 0			24.
April, 1865	5, 30	29. 346	29, 195	32. 9	30. 8	. 151	779		7.1	N. 78 W.	2.5	15. 6		11 1	72.5
June 1865	5, 30	29, 368	29, 138	53. 6	51.4	355	849		7.0	S. 15 W.	1. 2		35, 1	11. 1	9.
July, 1865	5. 30	29, 400	29, 028	56. 6	53. 5	. 371	. 803		5, 9	N. 45 W.	2.0	54.3			53.
August, 1865	5.30	29.461	29, 055	58. 6	55. 5	. 406	. 809		5, 2	N. 38 W.	1.1	27.6	45.	****	21. 30.
September, 1865	5, 30	29, 451	28, 994	43 0	38. 0	930	703		7.8	8. 34 W .	1.8	99.9	40. 0	14. 7	30.
November, 1865	5. 30	29, 421	29, 240	37. 8	35. 3	. 180	.769		7.6	West	3. 4	0. 2			99.
December, 1865															
Mean of spring	5, 30	29, 312	29. 194	33, 2	31. 2	. 163	. 783		7. 2	N. 3E	1.9	238. 1		19.3	189.
Mean of sutumn	5, 30	29, 404	99, 103	45 1	49.8	968	810		7.5	N. 87 W.	1.7				269.
Mean of winter	5. 30	29, 327	29, 232	21.0	19.3	. 095	. 725		8.3	N. 55 W.	2.9	300. 7			434.8
Means of all	5, 30	29, 353	29, 120	39, 3	37. 1	. 233	. 783		7. 2	N. 41 W.	2.1	995. 1			874.
Daniel 1000	5	00 400	00 202	00 1	04.0	110	720	0 51	0.0	N 70 F	1.9	11 0		58.6	
December, 1863	5	29, 284	29, 191	19, 8	18.3	. 093	740	0. 31	7.3	N. 79 E . N. 73 W. N. 59 W.	4. 2	90 0	SI .	4	100 (
February, 1864	5	29, 174	29,070	21.8	20, 2	. 104	. 734	0, 18	8. 2	N. 59 W.	3.0	44. 2			73.
January, 1864	5	29, 247	29, 141	22. 8	21.0	. 106	. 753	0.15	6.5	N. 37 E	2.4			46. 2	
April, 1864	5	29, 340	99, 180	49 3	40 1	996	890	0, 13	7.0	N. 9 W.	3.1	121. 1		11. 3	18.
May, 1864	5	29, 442	29, 102	53. 3	50, 1	. 340	. 785		3.8	N. 22 W.	3. 1	84. 2			35,
July, 1864	5	39, 419	28, 954	61.3	58.7	. 465	. 846	0.32	5, 3	N. 38 W.	2.0	47.9			36.
Ammus 1864	5	29. 329	28, 824	64. 3	50.0	. 505	. 819	0. 05	6.7	N. 41 W.	2.3	39 6		45.5	46.
September, 1864 October, 1864 November, 1864	5	99, 999	29, 012	41.7	39. 8	996	833	0. 09	7.7	N. 52 W.	3.8	53. 9		10.0	68.
November, 1864	5	29, 298	29, 138	34. 0	31. 9	. 160	. 781	0, 20	8.7	S. 87 W.	5. 1		6. 5		152.
December 1864	5	29, 251	29, 151	22, 1	20, 4	. 100	. 730	0. 25	8.4	N. 65 W.	5. 3	67. 4			147.
January, 1865	5	29, 358	29, 290	16, 1	14. 2	. 068	713	0. 01	8.8	N. DI W.	4.6 2.5	70.9			110.
March, 1865.	5	29, 305	29, 199	25. 1	23. 9	113	723	0, 19	8.2	N. 14 W.	2,3	68, 7			17.
February, 1865	5	29, 348	29, 199	32. 5	30. 5	. 149	. 780	0. 22	6.9	N. 82 W.	2.3	9. 2			67.
May, 1865	5	29.348	29. 140	40. 9	38. 5	. 208	. 794		5.6	N. 26 E.	1.2	34. 5	24	16. 7	
June, 1865	5	29. 366	29, 016	55. 0	50. 9	350	854	0.10	7.3	N. 45 W.	1.1	56 5	34.		57.
guly, 1802	5	29, 463	29, 055	58, 5	55. 5	408	. 815	0, 25	4.7	N. 37 E N. 51 E N. 9 W N. 38 W N. 41 W N. 49 E N. 52 W N. 52 W N. 51 W N. 65 W N. 51 W N. 44 W N. 65 W N. 65 W N. 45 E N. 65 W N. 48 E N. 49 E N. 49 E N. 49 E N. 49 W N. 49 E N. 65 W N. 40 E	1.0				19.
August, 1865	5	29, 450	28, 994	60. 5	58, 1	. 456	. 850	0.36	5.7	8. 33 W. N. 10 E 8. 88 W.	1. 2		36, 5		23,
August, 1865 September, 1865		90 400	29, 171	42. 9	40, 5	. 229	. 791	0.10	7.7	N. 10 E .	3, 1	94.5	4	16. 1	100.
August, 1865 September, 1865 October, 1855	5	20, 400		1.47. 6	35, 3	. 180					3, 5		4.0		100.
August, 1865 September, 1865 October, 1855 November, 1865	5														
August, 1865 September, 1865 October, 1855 November, 1865 December, 1865 Mean of spring	5					.160	. 789	0, 76	7. 2	N. 6 E.	1.9	352, 1		30, 9	
April, 1885 May, 1885 June, 1865 July, 1895 August, 1865 September, 1865 October, 1855 November, 1865 December, 1865 Mean of spring Mean of summer	5					. 160	. 782 . 821	0.76	7. 9 5. 5	N. 6 E. N. 40 W.	1.7	352. 1 232. 6		30. 9	200.
Mean of summer Mean of autumn	5					. 160	. 782 . 821 . 809	0, 76 0, 74 0, 85	7.9 5.5 7.3	N. 6 E. N. 40 W. N. 87 W.	1.7	352. 1 232. 6 141. 0		30, 9	200.
Mean of summer	5555	29, 310 29, 403 29, 370 29, 326	29, 150 28, 997 29, 104 29, 233	32. 9 57. 7 45. 0 20. 9	30. 9 54. 8 42. 7 19. 1	_	_	-		N. 6 E. N. 40 W. N. 87 W. N. 51 W.	1.7 1.8 2.8	352.1 232.6 141.0		30. 9	200. 282. 401.

		Pres reduce 32º I	ed to	Fa	np.,	Va	por.	ed snow	ar sky en			Wind			
Date.	Time of observation.	U. S. inches d decimals.	and decimals.	nalb.	oulb.	Elasticity, U. S. in-	Hunidity, Satura-	Am't of rain and melted snow U. S. inches and decimals.	Am't cloudiness. (10-sky entirely overcast, 0-clear sky.)	Observed direction; from whence,	Observed velocity; miles per hour.	Resolution and per l	reloc	of dir	
	Time	Total, I	Gaseous, ches and	Dry bulb.	Wet bulb.	Elast	Humi	Am't	Am't	Obser	Obser	N.	S.	E.	W.
December, 1863 January, 1864 February, 1864 March, 1864 April, 1864 May, 1864 June, 1864 June, 1864 Valy, 1864 August, 1864 September, 1864 Detober, 1864 Detober, 1864 December, 1864 January, 1865 February, 1865 March, 1865 June, 1865 June, 1865 June, 1865 June, 1865 Detober, 1865 September, 1865 December, 1865 December, 1865 December, 1865 December, 1865 December, 1865 December, 1865 December, 1865 December, 1865 Mean of syring Mean of summer Mean of summer Mean of winter Means of all.	A.M. 4. 30 5. 30 5	29, 435 29, 282 29, 175 29, 349 29, 349 29, 368 29, 418 29, 329 29, 360 29, 297 29, 251 29, 297 29, 251 29, 369 29, 369 29, 365 29, 365 29, 360 20, 360 20, 36	29, 322 29, 191 29, 071 29, 146 29, 148 29, 042 29, 09, 088 28, 958 28, 854 29, 071 29, 151 29, 292 29, 370 29, 194 29, 204 29, 204 29, 032 29, 032 29, 032 29, 032 29, 052 20, 052 20, 052 20, 052 20, 052 20, 052 20	25, 9 21, 9 22, 8 33, 7 42, 3 42, 3 41, 8 34, 0 22, 0 41, 8 34, 0 55, 6 55, 6 56, 6 60, 6	94, 0 18, 3 21, 0 32, 0 40, 2 50, 1 58, 5 61, 0 50, 5 331, 9 20, 3 14, 1 17, 4 23, 0 30, 2 38, 2 55, 5 58, 1	113 .091 .104 .103 .161 .296 .340 .505 .343 .296 .160 .067 .082 .112 .148 .206 .345 .345 .460	. 752 731 746 823 821 789 829 780 669 705 721 779 785 812 816 849		8.7.8.6.5.91 8.7.8.6.5.91 8.6.5.91 8.8.8.8.8.8.8.6.5.7.5.4.1.7.	0 M. 60 E. N. 60 W. N. 63 W. N. 59 E. N. 54 E. N. 74 W. N. 16 W. N. 55 E. N. 55 W. N. 64 W. N. 64 W. N. 64 W. N. 72 W. N. 72 W. N. 72 W. N. 72 W. N. 72 W. N. 72 W. N. 8. 8 W. N. 8. 8 W. N. 8. 8 W. N. 8. 8 W.	1.34.4.3.3.4.4.3.2.2.6.4.1.3.1.3.3.5.2.2.6.3.4.6.5.2.4.6.5.2.4.6.5.2.6.6.1.2.1.1.1.2.1.1.1.2.2.1.1.1.1.2.2.1.1.1.1.2.2.1.1.1.1.1.1.1.1.2.2.1	73. 0 43. 3 74. 9 45. 7 131. 9 89. 3 54. 0 64. 9 34. 6 68. 8 77. 9 18. 5 34. 1 57. 0 25. 9	9. 5	48, 3	15. 15. 15. 15. 15. 15. 15. 15. 15. 15.
October, 1865 November, 1865 December, 1865 Mean of spring Mean of summer Mean of autumn Mean of winter Means of all	4. 30 4. 30 4. 30 4. 30 4. 30 4. 30	29. 399 29. 421 29. 311 29. 401 29. 370 29. 326 29. 352	29, 172 29, 241 29, 152 28, 997 29, 105 29, 233 29, 121	42. 8 37. 9 32. 7 57. 5 45. 0 20. 8	30. 8 54. 7 42. 7 19. 0	. 1227 . 180 . 159 . 404 . 266 . 093	. 788 . 763 . 781 . 821 . 806 . 718		7.6 7.1 7.2 5.4 7.9 8.2	N. 11 E. N. 88 W. N. 3 E. N. 43 W. N. 87 W. N. 49 W.	3.6 3.8 2.1 1.9 1.8 3.2	3. 1 383. 0 257. 0 154. 3			2 000
December, 1863 January, 1864 February, 1864 March, 1864 March, 1864 May, 1864 June, 1864 June, 1864 June, 1864 June, 1864 June, 1864 June, 1864 June, 1864 June, 1864 June, 1865	444444444444444	29, 350 29, 367 29, 402 29, 463 29, 448 29, 399 29, 420	28, 959 28, 825 29, 017 29, 071 29, 138 29, 155 29, 296 29, 370 29, 196 29, 207 29, 026 29, 042 29, 057 28, 989 29, 171 29, 240	61, 1 64, 3 53, 1 41, 9 33, 9 22, 0 15, 7 19, 0 24, 7 32, 1 55, 4 55, 4 55, 5 60, 9 42, 9 38, 0	58, 4 60, 9 50, 6 39, 9 31, 8 20, 2 13, 8 17, 2 22, 8 30, 0 37, 8 55, 4 55, 5 58, 3 40, 4	. 458 . 503 . 344 . 226 . 159 . 099 . 066 . 081 . 111 . 146 . 203 . 341 . 360 . 405 . 428 . 228 . 180	.840 .813 .830 .828 .777 .725 .667 .700 .714 .776 .795 .852 .811 .848 .788 .760	0. 23 0. 05 0. 14 0. 20 0. 16 0. 19 0. 09 0. 22 0. 34 0. 11 0. 12 0. 10 0. 25 0. 20	5.0 6.4 6.5 7.8 8.1 8.8 8.0 6.7 5.3 7.2 5.6 7.2 7.1	N. 44 W, N. 26 W. N. 34 E. N. 49 W. S. 86 W. N. 59 W, N. 46 W. N. 24 W. N. 24 W. N. 12 E. S. 45 W. N. 44 W. N. 12 E. S. 45 W. N. 40 W. N. 12 E. S. 45 W. N. 40 W. N. 19 E. N. 40 W. N. 40 W. N. 19 E. N. 88 W.	1.0 1.6 2.9 1.3 1.2 3.7 3.7	79. 8 73. 2 42. 3 120. 3 71. 0 49. 8 63. 2 29. 0 65. 1 83. 2 96. 4 70. 1 82. 7 5. 7 29. 3 64. 4 29. 1	10. 6	6.0	15 15 15 16 16 16 16 16 16 16 16 16 16 16 16 16
Iean of spring	4 4	29. 327	28, 999 29, 104 29, 234	57. 5 45. 1 20. 7	54. 6 42. 7 19. 0	. 401 . 266 . 092	. 820 . 805 . 715	0, 75 0, 90 0, 96	5. 2 7. 1 8. 0	N. 3 W. N. 47 W. N. 58 W. N. 55 W.	1.9 1.9 2.8				2000年

		reduc	sure sed to Fahr.	Tel Fs	mp., Ar.	Vaj	por.	d move, male.	a. (10=sky en- 0=clear sky.)			Wind	L		
Date.	Time of observation.	Total U. S. inches and decimals.	ns, U. S. in.	ęj.	lb.	Elasticity, U. S. in-	lity. Satura- n == 1.600.	frain and melte inches and deci	Am't cloudinem. (10-	red direction; m whence.	Observed velocity; miles per hour.	and		ity, i	rection n miles
			Gascous, 1 ches and d	Dry bulb.	Wet bulb.	Elastic ches :	Humid	Am'to	Am't c	Observed Trom		N.	8.	E.	w.
December, 1863	A.M. 3. 30 3. 30 3. 30 3. 30 3. 30	29. 437 29. 278 29. 178 29. 252 29. 346	29, 326 29, 185 29, 075 29, 150 29, 188	26. 0 20. 1 22. 0 22. 6 33. 6	93. 9 18. 2 20. 3 90. 9 31. 8	. 111 . 093 . 103 . 102 . 158	. 731 . 752 . 720 . 741 . 810		8.1 7.6 7.4 5.7 7.6	0 N. 48 E N. 59 W. N. 54 W. N. 23 E N. 51 E	1.4 5.9 3.1 3.5 2.0 4.0	29. 4 98. 1 59. 7 98. 8 37. 7		33. 7 49. 2 47. 8	127. 8 73. 7
June, 1864 July, 1864 August, 1864 September, 1864 October, 1864 November, 1864	3, 30 3, 30 3, 30 3, 30 3, 30 3, 30	29, 496 29, 416 29, 327 29, 363 29, 295 29, 297	29, 061 28, 957 28, 825 29, 021 29, 069 29, 139	53. 4 61. 3 64. 3 53. 1 42. 0	50. 4 58. 5 60. 9 50. 5 40. 0	. 345 . 460 . 502 . 342 . 226 . 158	. 792 . 837 . 810 . 824 . 826 . 771		3.6 4.8 6.4 6.5 7.1	N. 38 W. N. 38 W. N. 20 W. N. 58 E. N. 46 W. S. 84 W.	2. 4 2. 4 2. 3 2. 1 3. 3	69. 5 59. 2 68. 7 32. 2 69. 3	16.0	52.9	53. 2 46. 9 25. 0 72. 9 148. 7 138. 9
December, 1863 January, 1864 Kerbruary, 1864 March, 1864 May, 1864 June, 1864 June, 1864 July, 1864 Acquest, 1864 Reptember, 1864 October, 1864 November, 1864 January, 1865 February, 1865 April, 1865 April, 1865 June, 1865 June, 1865 June, 1865 June, 1865 September, 1865 October, 1865 September, 1865 October, 1865 November, 1865 December, 1865	3. 30 3. 30 3. 30 3. 30 3. 30 3. 30	29, 364 29, 454 29, 306 29, 357 29, 350 29, 367	29, 298 29, 374 29, 198 29, 212 29, 150 29, 030	15. 4 18. 8 24. 5 32. 0 40. 0	13. 6 17. 0 22. 6 29. 9 37. 6	. 065 . 060 . 110 . 146 . 200	. 661 . 697 . 720 . 774 . 790 . 850		8.5 8.0 6.7 5.9	N. 44 W. N. 10 W. N. 23 W. N. 88 W. N. 15 E 8. 25 W.	5.3 4.4 2.5 2.3 1.9 1.1	98. 6 68. 8 92. 6 2. 4 31. 4	23. 5	8.8	138.9 96.5 12.3 40.0 58.0 11.0 63.8 28.9 15.0
A ugust. 1965. September, 1965. October, 1965. November, 1965. December, 1965. Mean of spring.	3. 30 3. 30 3. 30 3. 30 3. 30	29. 465 29. 447 29. 399 29. 421	29, 060 28, 966 29, 171 29, 240	58. 6 61. 0 42. 9 38. 1	55. 5 58. 3 40. 4 35. 5	. 405 . 459 . 228 . 180	. 808 . 847 . 769 . 761	••••	3.6 5.3 7.2 7.1	N. 45 W. 8. 29 W. N. 16 E. N. 88 W.	3.0 1.3 1.1 4.4 3.9 2.1		98.9	37. 9	28. 9 15. 0 92. 4 16. 8 928. 8
Mean of sutumn Mean of winter Means of all	3. 30 3. 30 3. 30	29, 370 29, 326 29, 353	29, 104 29, 236 29, 236 29, 124	45. 2 20. 7 38. 9	36, 6	. 265 . 092 . 229	. 803 . 714		6.9	N. 39 W. N. 43 W. N. 32 W.	2.1 3.4 2.3	290, 4 436, 5 1392, 5			238. 9 415. 5 899. 3
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January, 1864 February, 1864 March, 1864 April, 1864 May, 1864 June, 1864	3 3 3 3 3	29, 270 29, 180 29, 254 29, 346 29, 267 29, 495	29, 182 29, 078 29, 152 29, 190 29, 040 29, 079	22, 1 22, 5 33, 4 42, 6	90. 4 90. 8 31. 7 40. 3	. 102 . 102 . 102 . 156 . 297 . 346	. 749 . 714 . 747 . 802 . 811	0. 25 0. 18 0. 07 0. 01 0. 17	7.3 5.5 7.6 7.4 3.8	N. 51 W. N. 63 W. N. 15 E. N. 61 E. N. 7 W. N. 30 W.	5.2 4.7 3.9 2.0 4.0	62. 0 95. 2 28. 4 124. 7 89. 4		96. 4 51. 0	124. 7 124. 1 124. 1 16. 5 49. 3
July, 1864 August, 1864 September, 1864 October, 1864 November, 1864	3 3 3 3	29, 411 29, 396 29, 365 29, 295 29, 297	28, 947 28, 824 29, 022 29, 068 29, 137	61. 5 64. 5 53. 3 42. 1 34. 0	58. 7 61. 0 50. 6 40. 0 31. 9	. 464 . 502 . 342 . 327 . 160	. 832 . 806 . 820 . 823 . 777	0. 07 0. 20 0. 28 0. 28	4.5 6.2 6.1 7.7 8.0	N. 15 E N. 61 E N. 7 W. N. 30 W. N. 40 W. N. 25 W. N. 57 E N. 44 W. S. 81 W. N. 57 W.	3.3 2.5 2.6 3.9	58. 2 58. 7 43. 0 73. 8	22. 4	66, 1	124. 7 124. 1 16. 5 49. 3 49. 4 26. 4 71. 9 148. 3 145. 4 94. 5
fanuary, 1865	3 3 3 3	29, 366 29, 455 29, 310 29, 360 29, 349	29, 301 29, 373 29, 200 29, 213 29, 149	15. 3 18. 7 24. 5 31. 9	13. 4 16. 8 22. 6 29. 9	. 065 . 080 . 110 . 145	. 659 . 699 . 706 . 772	0. 09 0. 12 0. 12 0. 30 0. 13	8.5 7.9 7.8 6.6	N. 10 W. N. 22 W.	2.3	95. 7 64. 8 97. 2 6. 8 33. 1	23.0		94. 5 11. 7 38. 7 50. 4
December, 1863 January, 1864. February, 1864 March, 1864 April, 1864 May, 1864 June, 1864 June, 1864 July, 1864 August, 1864 Detember, 1864 December, 1864 December, 1865 Jebruary, 1865 Jebruary, 1865 July, 1865 July, 1865 July, 1865 July, 1865 July, 1865 July, 1865 Jevember, 1865 December,	3 3 3 3	29, 405 29, 466 29, 447 29, 399 29, 490	29. 055 29. 055 29. 065 28. 985 29. 179	51. 7 54. 9 58. 6 61. 1 42. 9	49. 6 51. 8 55. 4 58. 5 40. 4 35. 6	. 353 . 352 . 404 . 462 . 227 . 180	. 846 . 802 . 804 . 844 . 787	0. 22 0. 15 0. 25 0. 04 0. 10 0. 09	6.6 5.5 5.7 3.6 5.7 6.9	N. 9 W. 8. 11 W. N. 43 W. N. 49 W. 8. 59 W. N. 13 E. N. 87 W.	0.8 2.9 1.3 0.9 5.0 3.0	95. 9 150. 9	16.8		29.9
December, 1965	3 3 3 3	29. 314 29. 400 29. 370 29. 330	29, 156 28, 996 29, 106 29, 236	39. 5 57. 4 45. 2 20. 7	30. 5 54. 5 42. 8 18. 8	. 157 . 400 . 266	. 771 . 814 . 801 . 713	0. 81 0. 86 0. 64 1. 67	6.7 5.0 6.8 7.9	N. 38 W. N. 43 W.	3.6	385. 4 274. 0 233. 2 439. 8			29. 4 290. 5 227. 4 498. 9
Means of all	3	29. 353	29. 12:	36. 9	36. 6	. 228	. 775	3. 38	6.6	N. 36 W.	2.2	1332. 4			976. 2

		Pres reduce 32° I	ed to	Ter Fa	np., hr.	Vaj	por.	elted snow decimals.	sky er			Wind			
Date.	Time of observation.	U. S. inches i decimals.	us, U. S. in- ind decimals.	ulb.	ulb.	city, U. S. in-	Hundity, Satura-	Am't of rain and melted a	Am't cloudiness. (10=sky entirely overcast, 0=clear sky.)	Observed direction; from whence.	Observed velocity; miles per hour.	Resolt and per l		ity, in	
	Time	Total, I	Gaseous, ches and	Dry bulb.	Wet bulb.	Elasticity,	Humi	Am'te	Am't	Obser	Obser	N.	S.	E.	w.
December 1963	A.M. 4 30	90 495	99 399	95.0	24.0	113	. 752		8.6	o N. 60 E .	1.3	24. 2		42.1	
December, 1863 January, 1864	4. 30	29, 282	29, 191	19. 9	18. 3	. 091	. 731		7.4	N. 60 W.	4.6				124.
February, 1864	4. 30	29, 175	29, 071	21.8	20. 2	. 104	. 723		8. 1	N. 63 W.	3.3	43. 3	****	41 7	86,
March, 1864	4. 30	29, 249	29, 146	22, 8	20. 0	161	- 740		6.5		2.8	45.7	****	63.0	
May 1864	4, 30	29, 343	29, 042	42 3	40. 9	226	821		8.1	N. 7 W.	4.1	131. 9			15
Inne, 1864	4.30	29, 428	29, 088	53, 1	50. 1	. 340	. 789		3.9	N. 16 W.	3.1	89. 3			25
July, 1864	4, 30	29, 418	28, 958	61, 1	58,	. 460	. 843		5. 2	N. 41 W. N. 34 W.	2.3	54. 0			35
August, 1864	4. 30	29, 329	28, 824	64. 3	61.0	242	817		6.6	N. 55 E .	2.5	34 6		48.3	4
etober 1864	4, 30	29, 300	29, 071	41 6	30,	226	890		7.9	N. 59 W.	3, 0	56. 7			
November, 1864	4, 30	29, 297	29, 137	34. 0	31. 9	. 160	. 780		8.7	S. 86 W.	5. 2		9.5		15
December, 1864	4, 30	29, 251	29, 151	22, 0	20. 3	. 100	. 726		8.3	N. 64 W.	5.3	73, 9			14
anuary, 1865	4, 30	29, 359	29, 292	15. 9	14.	067	705	****	8.7	N. 48 W. N. 9 W.	4.6	69.9			100
March 1865	4, 30	29, 306	29, 194	94 0	23. (112	721		8.1	N. 17 W.	2.6				
pril, 1865	4.30	29, 352	29, 204	32. 3	30. 5	. 148	. 779		6,9	N. 72 W.	2, 0	18.5			5
May, 1865	4.30	29, 349	29, 143	40. 5	38.	206	. 795		5. 5	N. 21 E	1.2	34. 1	24 3	14.1	-
une, 1865	4.30	29, 365	29, 020	55.6	59.6	368	819		5.4	S. 8 W N. 47 W.	2.7	57. 0	04. 1		6
ugust. 1865	4. 30	29, 464	29, 057	58. 5	55.	407	. 816		4.1	N. 38 W.	1.1	25. 9			1
September, 1865	4, 30	29. 448	28, 991	60. 6	58. 1	457	. 849		5. 7	S. 33 W.	1. 2		29.8	21. 9	15
October, 1865	4. 30	29, 399	29, 172	42. 8	40.	190	. 788		7. 6	N. 11 E.: N. 88 W.	3.6	109. 2		21.9	Til
December 1865	4. 30	29, 421	29. 241	34. 5	33. 9	100	. 100		7. 1	A. OC W.	0,0	0. 1			
Mean of spring	4, 30	29. 311	29, 159	32. 7	30. 8	. 159	. 781		7. 2		2.1	383. 0		20. 5	
Mean of summer	4.30	29, 401	28, 997	57.	54.	404	. 821		5. 4	N. 43 W. N. 87 W.	1.9	257. 0		****	236
Mean of autumn	4, 30	29, 370	29, 10,	20. 6	19.	. 200	718		7.9	N. 49 W.					63
December, 1863 January, 1864 Pebruary, 1864 April, 1864 April, 1864 March, 1864 Muy, 1864 Muy, 1864 Muy, 1864 Muy, 1864 August, 1864 Detober, 1864 Detober, 1864 Detober, 1864 Detober, 1865 Pebruary, 1865 February, 1865 March, 1865 June, 1865 June, 1865 June, 1865 June, 1865 Detober, 1865 September, 1865 Detober, 1865 Detober, 1865 December, 1865 December, 1865 December, 1865 December, 1865 Mean of summer Mean of summer Mean of winter	4, 30	29, 352	29. 121	39. (36. 8	. 230	. 781		7.0	N. 39 W.	-	1170. 9	-	_	949
	A.M.														
December, 1863	4	29, 438 29, 278 29, 178	29, 325	26. 0	24. (. 113	. 743	0, 29	8.2		1.0	14.5		26.7	-
January, 1864 February, 1864	4	29, 278	29. 186	19. 9	18.3	092	741	0, 13	7.6	N. 54 W. S. 59 W.	4.3	79.8	46 0		100
March 1864	4	99, 178	29. 147	99. 7	20.5	100	740	0. 19	6.2	N. 28 E	27	73. 2	200	40.1	
March, 1864 April, 1864	4	29, 249 29, 347 29, 269 29, 430	29, 186	33. 7	31.	. 160	. 819	0. 24	7.8	N. 52 E . N. 9 W.	2.3	42.3		54.1	
May, 1864 June, 1864 July, 1864	4	29, 269	29. 043	42. 4	40, 5	2.226	. 817	0. 12	7.7	N. 9 W.	4.0	120, 3			E
June, 1864	4	29. 430 29. 417	98 050	61 1	58	458	940	0.99	5,0	N. 36 W. N. 44 W.		49.8			4
August, 1864	4	29, 328	28, 825	64. 3	60. 9	. 503	. 813	0, 05	6, 4	NT OR THE	2.3	63. 2			
September, 1864	4	29, 361	29, 017	53, 1	50, 6	344	. 830	0. 14	6, 5	N. 34 E	1.6	29.0		26. 7	
October, 1864	4	29, 297	29, 071	41.9	39.5	226	. 826	0. 20	7.8	N. 49 W. S. 86 W.	3.3	65. 1	10 6		15
Tours 1004	4	99, 297	29, 155	99. (20.	000	795	0. 19	8.1	N. 59 W.	5. 2				-
November, 1864		MINTE MARK	00.000	15.5	13.8	. 066	. 667	0. 09	8.8	N. 46 W.	4.5	96, 4		***	10
November, 1864 December, 1864 January, 1865	4	29, 362	29, 290	Tres. 1		01. 001	. 700	0.08	8.1	N. 8 W.	2.5	70.1			3
November, 1864 December, 1864 January, 1865 February, 1865	4	29, 362 29, 451	29. 370	19. (17.5	1001			0.0	N. 24 W.	A 100 M	Differ A			6
December, 1864 January, 1865 February, 1865 March, 1865	4 4 4	29, 362 29, 451 29, 307 29, 354	29, 290 29, 370 29, 196	19. 0	17.5	111	714	0. 34	6.7	N. 85 W		5.7		1000	
December, 1864 January, 1865 February, 1865 March, 1865	4 4 4 4	29, 328 29, 361 29, 297 29, 254 29, 362 29, 451 29, 307 29, 354 29, 350	29, 370 29, 196 29, 207 29, 147	19. 0 24. 7 32. 1 40. 1	17, 9 22, 8 30, 0	. 111	. 714	0, 34	6, 7	N. 85 W. N. 12 E	2,2	5.7 29.3		6.0	40.00
December, 1864 January, 1865 February, 1865 March, 1865	4 4 4 4 4	29, 362 29, 451 29, 307 29, 354 29, 350 29, 367	29, 236 29, 370 29, 196 29, 207 29, 147 29, 026	19. 0 24. 7 32. 1 40. 1 52. 3	17. 9 22. 8 30. 0 37. 8 50. 9	. 111 0 . 146 8 . 203 2 . 341	. 714 . 776 . 795 . 859	0, 34 0, 11 0, 12	6, 7 5, 3 7, 2	N. 85 W. N. 12 E. S. 45 W.	2,2	5.7 29.3	34. 3	6.0	3
December, 1864 January, 1865 February, 1865 March, 1865	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	29. 367	29. 026	52.3	50.5	341	859	0. 12	7.9	S. 45 W. N. 44 W.	2, 2 1, 0 -1, 6 2, 9	04.4		1000	360
October, 1864 November, 1864 December, 1864 January, 1865 February, 1865 March, 1865 May, 1865 June, 1865 June, 1865 July, 1865 August, 1865 August, 1865 September, 1865	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	29. 367	29. 026	52.3	50.5	341	859	0. 12	7.9	S. 45 W. N. 44 W. N. 40 W	2.2 1.0 1.6 2.9 1.3	29, 1		1000	3682
December, 1864 January, 1865 February, 1865 March, 1865	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	29. 367	29. 026	52.3	50.5	341	859	0. 12	7.9	S. 45 W. N. 44 W N. 40 W S. 31 W N. 19 E	2.2 1.0 1.6 2.9 1.3	29, 1	30, 6	1000	2000
December, 1864 January, 1865 February, 1865 March, 1865	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	29, 362 29, 451 29, 307 29, 354 29, 350 29, 367 29, 402 29, 463 29, 448 29, 399 29, 420	29. 026	52.3	50.5	341	859	0. 12	7.9	S. 45 W. N. 44 W N. 40 W S. 31 W N. 19 E	2.2 1.0 1.6 2.9 1.3 1.2 3.7	29, 1		38. 1	2000
December, 1864 Ianuary, 1865. February, 1865. March, 1865 April, 1865 May, 1865. June, 1865 July, 1865 August, 1865 September, 1865 October, 1865 November, 1865 December, 1865	4 4 4 4 4	29, 367 29, 402 29, 463 29, 448 29, 399 29, 420	29. 026 29. 049 29. 057 28. 989 29. 171 29. 240	52, 3 55, 4 58, 5 60, 9 42, 9 38, 0	50. 55. 55. 55. 58. 340. 435. 4	2 . 341 4 . 360 5 . 406 8 . 459 4 . 228 4 . 180	. 859 . 811 . 813 . 848 . 788 . 760	0, 19 0, 10 0, 25 0, 20 0, 18 0, 0, 09	7. 2 5. 6 5. 6 7. 2 7. 1	S. 45 W. N. 44 W. N. 40 W. S. 31 W. N. 19 E. N. 88 W.	2.2 1.0 1.6 2.9 1.3 1.2 3.7	29, 1 107, 9 4, 2	30, 6	1000	10
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		reduc	ed to	Te	mp., thr.	Va	por.	d snow	ar sky.			Wind			
Date.	l'ime of observation.	Total U. S. inches and decimals.	Gaseous, U. S. In-	alb.	alb.	and decimals.	Humidity. Satura-	Am't of rain and melted sno U. S. inches and decimals.	Am't cloudiness. (10-sky e tirely overcast, 0-clear sk)	Observed direction; from whence.	ved velocity;	Resolu and per		ity, i	rection n miles
	Time	Total	Gaseor ches	Dry bulb.	Wet bulb.	Elasticity,	Humid	Am'to	Am't e	Observ	Observed miles per	N.	s.	E,	w.
December, 1863 January, 1864 February, 1864 March, 1864 April, 1864 April, 1864 July, 1864 July, 1864 July, 1864 October, 1864 October, 1864 October, 1864 December, 1864 December, 1865 February, 1865 February, 1865 March, 1865 July, 1865 July, 1865 July, 1865 July, 1865 September, 1865 October, 1865 October, 1865 December, 1865 December, 1865 December, 1865 December, 1865 December, 1865 Mean of spring, Mean of summer Mean of winter Means of all.	A.M., 33 30 3. 30	29, 437 29, 432 29, 29, 178 29, 29, 36 29, 267 29, 267 29, 267 29, 267 29, 363 20, 29, 256 29, 37 29, 363 29, 29, 465 29, 37 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 29, 31 30 30 30 30 30 30 30 30 30 30	29, 326 29, 185 29, 107 39, 150 39, 150 39, 168 20, 042 29, 081 29, 089 29, 129 29, 139 29, 219 29, 20, 20, 20, 20, 20, 20, 20, 20, 20, 20	26. 0 20. 1 22. 0 23. 6 42. 4 53. 4 42. 9 21. 9 21. 9 21. 9 21. 9 32. 0 40. 0 55. 0 40. 0 55. 0 40. 0 40. 0 57. 4 45. 2 45. 3 45. 3 45. 3 45. 4 45. 4 45. 4 45. 4 45. 4 45. 4 46. 4	23, 9 18, 2 20, 3 31, 8 40, 2 50, 4 56, 9 50, 5 40, 0 13, 6 40, 0 22, 6 49, 9 52, 0 49, 9 55, 5 55, 5 54, 3 56, 5 40, 0 22, 6 40, 0	.1111 .093 .103 .103 .102 .158 .225 .345 .226 .502 .342 .226 .065 .060 .110 .337 .459 .228 .405 .459 .228 .405 .405 .405 .405 .405 .405 .405 .405	. 731 . 752 . 720 . 741 . 810 . 812 . 792 . 837 . 771 . 723 . 661 . 720 . 774 . 790 . 806 . 808 . 847 . 761 . 789 . 761 . 774 . 817 . 803 . 714 . 817 . 777		8.16.7.47.7.5.6.6.46.5.17.8.8.0.5.6.9.9.6.7.9.6.7.9.6.7.9.6.7	O N. 48 E. N. 52 W. N. 54 W. N. 52 W. N. 53 E. N. 54 W. N. 23 E. N. 51 E. N. 9 W. N. 20 W. N. 38 W. N. 20 W. N. 44 W. N. 44 W. N. 44 W. N. 45 W. N. 45 W. N. 45 W. N. 45 W. N. 45 W. N. 45 W. N. 45 W. N. 45 W. N. 48 W. N. 39 W. N. 43 W. N. 32 W. N. 32 W. N. 32 W. N. 33 W. N. 43 W. N. 32 W. N. 32 W. N. 32 W. N. 33 W. N. 32 W. N. 33 W. N. 43 W. N. 32 W. N. 33 W. N. 32 W. N. 33 W. N. 34 W. N. 33 W. N. 34 W. N. 35 W. N.	1.44 5.23 1.13 2.00 4.09 2.24 2.33 5.03 5.33 1.11 1.11 4.42 2.13 3.22 2.13 3.33 4.09 2.24 2.33 3.33 3.33 4.09 2.24 2.33 3.33 3.33 4.09 2.09 2.09 2.09 2.09 2.09 2.09 2.09 2	98. 1 52. 7 98. 8 37. 7 121. 0 69. 5 59. 2 69. 3 32. 2 69. 3 98. 6 92. 6 92. 4 31. 4 31. 4 31. 4 31. 4 31. 4 31. 4 31. 4 31. 4 31. 6	23, 5	42. 2 47. 8 52. 9	19. 6 53. 9 46. 9 25. 0 72. 9 148. 7 138. 9 96. 5 12. 3
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		Pres reduc 32º F	ed to	Ter Fa	np., Ar.	Var	or,	d mow	arky on-			Wind	•		
Data.	Time of observation.	U. S. inches d decimals.	us, U. S. in and decimals.	alb.	oalb.	Elasticity, U. S. la-	Humidity. Satura- tion = 1.000.	Am't of rain and melted mov U. S. inches and decimals.	Am't cloudiness. (10=sky en- tirely overcast,0=elear sky.)	Observed direction; from whence.	Observed velocity, miles per hour.	Resolution in the second in th	ntion veloc bour.	ity, b	ectica a mile
•	Time	Total, I	Gaseous, ches and	Dry bulb.	Wet bulb.	Elasti	Hum	Am't	Am't c	Ober	Obser	N.	8.	B.	₩.
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March, 1864	2.30	29. 258	29. 155	22. 7	21.0	102	740		5. 7	N. 12 E.	30	83. 7	1	19. 6	
April, 1864	2.30	29. 347	29. 191	33. 6	31.7	. 156	. 805	- -	7.5 7.6	N. 62 E.	1.8	25.9	J	47. 7	1:-
may, 1504	2.30	29, 201	29.079	53. 5	50. 5	348	792	••••	3.6	N. 38 W.	2.9	69.2			밀
July, 1864	2.30	29. 411	28. 944	61.8	58. 9	. 466	. 835		4.4	N. 43 W.	2.5	56.8			12
August, 1864	2, 30	29, 326	28. 827	64. 7	61. 1	. 499	. 797	[- -	6.0	N. 6 W. N. 38 W. N. 43 W. N. 28 W. N. 59 E.	5.4 2.7 3.0 1.8 4.0 2.9 2.5 2.6 4.9 5.7	16. 3 100. 0 49. 1 83. 7 25. 9 124. 2 69. 2 56. 8 67. 0 39. 0 82. 5]	ء-تفأ	×
October, 1864	2.30	29, 293	29, 066	42 1	30. 8 40. 0	227	826	•	7.5	N. 44 W	3.6	82.5		00.8	7
November, 1864	2, 30	29, 296	29. 136	34. 0	31.8	. 160	. 779		7.5 7.8	N. 44 W. 8. 82 W N. 57 W.	4. 9		20. 3	ļ	
December, 1864	2,30	29. 259	29. 159	22. 0	20. 2	. 100	. 722		8.1 8.5	N. 57 W.	5.7	95.5	· · · ·	ļ	147
February, 1865	2.30	29, 456	29, 377	18. 8	16.9	079	. 692		8.0	N 10 W	9.9	60.5]		10.
March, 1865	2. 30	29. 311	29, 201	24. 6	22. 6	. 100	. 707		8.0 7.8	N. 16 W. N. 44 W. N. 5 W.	121	91.4	20. 3 21. 1 15. 1		35.
April, 1865	2.30	29. 362	25. 217	32. 1	29. 9	. 144	. 767		6.6	N. 44 W.	1.0	22.4	<u> </u>	ļ- -	2L
иву, 1600 Гипе 1865	2.30	29. 350	29, 131	51.9	40.7	334	842		61	N. 5 W.	1.5	90.0	21.1		14
July, 1865	2, 30	29, 406	29.050	55. 2	51. 9	. 356	. 793		5.2 6.1 5.7	N. 39 W.	2.8	67. 8	3		.! SL
August, 1865	2.30	29. 464	29. 062	58. 7	55. 4	. 402	. 799	J	3.4 4.9	N. 45 W.	1.2	26.3	۱::·;	- -	25
September, 1865	2.30	29, 116	20, 967	43. (28. 5 40. 4	227	780		7.0	N. 13 R.	5.3	160.5	12. 1	37. 6	22
November, 1865	2.30	29. 420	29. 241	38. 9	35. 6	. 179	. 749		6.8		3.0	2.			86.
December, 1865		00 216	00 150	30.4	-	1:::	-	ļ	6.7	37a-48	2.9	200	<u>.</u>		ļ
Mean of summer	2.30	29. 310	28, 999	57. 6	54. 6	. 100	. 706		4.9	North	2.2	353.4		26	192
Mean of autumn	2.30	29. 370	29, 103	45, 3	42. 8	. 266	. 799		6.6	N. 52 W.	2.3	299. 5 353. 4 948. 6 409. 0	3		331
Mean of winter	2. 30	29. 330	29. 236	20. 8	18. 9	. 092	. 712	<u> </u>	7.9	N. 48 W.	3.5	409.0	P		173
	-	29. 354	29, 125	39. 1	36, 7	. 229	. 772		6. 6	N. 36 W.	2.4	1410. 7	<u> </u>		1007
December, 1863 January, 1864 February, 1864 April, 1864 April, 1864 April, 1864 May, 1864 June, 1864 July, 1864 July, 1864 July, 1864 October, 1864 November, 1894 December, 1864 December, 1865 March, 1865 March, 1865 July, 1865 Jul	2	29, 441 29, 275 29, 187 29, 262	29. 32 8	26. 2	24. 1	. 113	. 733	0. 27	7. 7	8. 80 W. N. 51 W. N. 62 W.	1.3		7.1		40
January, 1864	2 2	29, 275	29, 181	20. 5	18.6	094	. 736	0. 15 0. 39	7.6	N. 51 W.	5.9	99.5	g		194. 71.
r coruary, 1004	2	29, 262	29, 159	22 0	21.	102	739	0. 18	7. 7 7. 6 7. 2 5. 6 7. 5 7. 5	N. 1 E	5.9 2.8 1.8	88.	5	1.6] 71
April, 1864	2 2	29, 262 29, 346 29, 269 29, 424 29, 406 29, 327 29, 364 29, 295 29, 260 29, 368 29, 457 29, 312 29, 369	29, 191	33.	31. 8	. 157	. 805	0. 20	7. 5	N. 1 E. N. 59 E. N. 5 W. N. 24 W.	1.8	98. 97. 139. 96. 50. 65. 99.		47. 0	
May, 1864	2 2	29. 269	29. 04:	42.6	40.	. 227	. 803	0.06	7.5	N. 5 W.	9.3	139. 6	ğ		1 11
July, 1864	2 2	29, 409	28, 940	62.0	50. (59. 1	. 347	831	U. 20	4.4	N. 40 W	3.9	50.9		l:::-	39
August, 1864	2	29. 327	28. 827	64. 9	61. 1	. 500	. 792		5, 6	N. 30 W	2.4	65.4			3
September, 1834	2 2	29. 364	29. 017	53. 6	50. 8	. 346	. 813	0.11	5.8 7.5 7.7 8.0	N. 40 W N. 30 W N. 64 E N. 40 W	24	29.	§	59 . 5	
November, 1864	2	29, 295	29. 132	34	32.0	163	774	0. 23	7.7	8. 22 W	5.0	30.0	in in		14
December, 1864	2	29. 260	29. 160	22. (20, 9	. 100	. 725	0. 20	8.0	8. 92 W N. 63 W N. 43 W N. 10 W	5.5	75.	\$] <u>.</u>	144
January, 1965	2	29. 368	29. 303	15.	13.0	065	. 656	0.08	4.8	N. 43 W	4.1	92.4		- -	87
March. 1865	2	29, 312	29, 202	24.	22	1. 110	709	0. 15	7.7	N. 63 W N. 43 W N. 10 W N. 13 W N. 37 W N. 12 W 8. 36 W N. 35 W	2.3	75. 92. 4 60. 6 101. 7	7		10
April, 1865	2 2	29, 360 29, 350 29, 367 29, 467	29, 215	32.	30.	. 145	. 764	0. 35 0. 10 0. 19 0. 27	6.3	N. 37 W	0.8	18			l ii
May, 1865	2 2	29, 350	29. 153	40.	37.	198	. 772	0. 10	5.2	N. 12 W	0.6	51.9	16.6	J	1 44
July, 1865	2	29, 407	29, 059	55.	52	354	789	0. 27	5.8	N. 35 W	2.6	71.	10.0	1:	19
August, 1865	2								9.0	N. 43 W	116	38.1	•		33
Beptember, 1865 Ootober 1865	2	29, 449	28, 986	61.	58.	. 461	830	6 1	4. 7 7. 1	8. 56 W N. 19 E	0.5	120	12, 9		25
November, 1865	2	29, 449 29, 399 29, 421	29, 249	38.	335	7. 2525 1. 179	74	0. 1.	6.6	West	0.6 5.3	160.	0.0	3L 3	100
December, 1865	<u>.</u> .	1	I .			4				1		. I . 			
Mean of spring	2	29. 317	29. 160	32, 8	30.	156	. 764	1. 04	6.6	N. 2 W	2.5	497.	5	····	10
Mean of summer Mean of autumn	2 2	29. 37	29, 103	: 51, t 345. !	142	267	794	0. 7 6	6.6	N. 47 W	1.9	943.1	BI	::::	214 250
Mean of winter	2	29, 317 29, 399 29, 370 29, 331	29. 23	20.	19.	. 092	709	1.10	7. 9	N. 2 W N. 39 W N. 47 W N. 53 W	3.3	359.	3		477
Means of all		29, 354		_		-;		2 50	6.3		-	1393.	_	1	953

		reduc	ed to	Ter Fa	mp.,	Va	por.	denow,	akyen- ersky.)			Wind			_
Data,	of observation.	, U. S. inches id decimals.	ous, U. S. tn. and decimals.	balb.	balb.	Charticity, U. S. in- ches and decimals.	dity. Satura- on = 1.000.	Am't of rain and melted an U. S. inches and decima	Am't cloudiness. (10-sky er tirely overcast, 0-clear sky.	Observed direction; from whence.	Observed velocity; miles per hour.	and		ity, iı	retion n miles
	TIES	Total, 1	Gaseous, ches and	1	Wet bulb.	Elasticity, ches and	Hum	Am't	Am't tirely	Obser fr	Obser	N.	8.	E.	w.
December, 1863 January, 1864 February, 1864 March, 1864 April, 1864 May, 1864 June, 1864 June, 1864 September, 1864 October, 1864 November, 1864 December, 1864 December, 1865 January, 1865 April, 1865 April, 1865 June, 1865 June, 1865 June, 1865 June, 1865 June, 1865 November, 1865 November, 1865 November, 1865 November, 1865 November, 1865 November, 1865 November, 1865 November, 1865 Mean of summer Mean of summer Mean of winter	A.1d. 1. 30	29. 442 29. 273 29. 361 29. 363 29. 363 29. 272 29. 366 29. 367 29. 366 29. 367 29. 36	98. 388 59. 100 29. 055 59. 100 29. 055 59. 100 29. 075 59. 076 59. 071 59. 07	26. 3 20. 8 22. 3 22. 3 24. 1 562. 3 65. 2 25. 3 26. 5 20. 6 3 20. 6 3 20. 6 3 3 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4	24. 22 19.0 5 21. 4 32. 1 40. 1 32. 1 40. 1 32. 1 32. 4 17. 5 5 5 6 1. 3 3 7 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	. 114 . 093 . 103 . 104 . 159 . 228 . 348 . 470 . 503 . 356 . 225 . 161 . 100 . 066 . 080 . 080 . 110 . 145 . 199 . 335 . 401	. 741 . 727 . 721 . 742 . 802 . 781 . 825 . 789 . 808 . 809 . 778 . 658 . 658 . 658 . 757 . 769 . 831 . 775 . 741 . 762 . 800 . 775 . 741 . 762 . 800 . 791 . 706 . 791 . 706		7.367.557.55.68111.65.99 6.42.5.99 6.42.5.99 6.42.5.99 6.43.33 6.46.67.7	o N. 56 W. N. 56 W. N. 58 W. N. 58 W. N. 53 E. N. 4 W. N. 35 W. N. 74 E. N. 42 W. N. 65 W. N. 14 W. N. 41 W. N. 41 W. N. 42 E. S. 32 W. N. 45 W. N. 16 E. S. 84 W. N. 16 E. S. 84 W. N. 16 E. S. 84 W. N. 16 E. S. 84 W. N. 15 W. W. 15 W. N. 15 W. W. 15 W. N. 15 W. W.	1.64 4.87 1.84 4.58 2.62 2.23 1.56 4.11 2.11 4.98 1.16 2.16 1.16 1.16 1.16 1.16 1.16 1.16	85. 0 40. 8 114. 3 31. 6 138. 9 86. 7 51. 9 69. 9 18. 0 79. 8 74. 4 74. 4 75. 2 14. 3 60. 2 147. 5 147. 5 290. 7 208. 7	14. 6	3.9	71. 8 137. 9 158. 9 84. 9 39. 0 3. 6
			29. 194	39. 5	37. 0	. 230	. 765		6.3	N. 34 W.	2. 2	1318. 6		<u>::</u>	967. 3
December, 1863. January, 1864. February, 1864. March, 1864. April, 1864. May, 1864. July, 1864. August, 1864. October, 1864. October, 1864. November, 1864. January, 1865. January, 1865. March, 1865. March, 1865. May, 1865. July, 1865. April, 1865. April, 1865. April, 1865. November, 1864. December, 1865. May, 1865. April, 1865. November, 1865. December, 1865. November, 1865. Macan of spring. Mean of summer Mean of summer Mean of summer		29. 444 29. 272 29. 191 29. 262 29. 350 403 29. 403 29. 364 29. 296 29. 366 29	29, 159 29, 191 29, 048 29, 071 28, 926 28, 825 29, 018 29, 072 29, 135 29, 169 29, 290 29, 290 29, 153 29, 169 29, 169 29, 169 29, 169 29, 169 29, 169 29, 169 29, 169	21. 0 22. 4 23. 4. 1 43. 2 54. 5 62. 5 65. 6 65.	19. 2 20. 6 21. 5 21. 1 40. 6 51. 1 59. 4 61. 6 51. 1 22. 2 20. 5 32. 2 20. 5 30. 8 30. 8 50. 3 55. 7 40. 6 6 5 6 7 7 8 8 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9	. 094 102 102 159 226 349 477 504 346 225 161 099 066 081 112 148 337 361 402 461 179	722 718 725 802 792 770 824 780 804 774 790 652 681 708 761 830 772 791 830 774	0. 09 0. 35 0. 02 0. 30 0. 18 0. 25 0. 07 0. 08 0. 10 0. 12 0. 42 0. 42 0. 42 0. 42 0. 09	7.67 7.79 7.40 3.43 5.78 1.15 6.49 5.80 4.35 7.64	N. 1 W N. 46 E. N. 5 W N. 32 W N. 41 W N. 42 W N. 72 E. N. 36 W N. 63 W N. 63 W N. 41 W N. 23 W N. 13 W N. 13 W N. 14 E. S. 9 W N. 44 W N. 14	3.8 3.8 3.8 3.8 4.9 5.3 4.6 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0	76. 3 46. 2 92. 7 38. 3 120. 4 71. 8 23. 0 81. 6 82. 8 93. 3 42. 7 100. 5 18. 6 69. 0	24. 4 21. 5	39. 4 71. 7 9. 3	37. 9 59. 2 141. 1 163. 8 82. 7 15. 9 42. 3 4. 3 58. 6
Mean of spring			29, 169 28, 993 29, 105 29, 239 29, 125	58. 6 45. 8 21. 4	55. 9 43. 1 19. 5	. 405 . 267 . 093	. 794 . 787 . 706	1. 10 0. 51 0. 73 0. 88 3. 22	4.7 6.6 7.8	N. 29 W. N. 42 W. N. 56 W.		439. 5 269. 2 183. 8 337. 2			17. 9 904. 6 171. 9 503. 6 897. 3

TABLE K.—Meteorological observations at Thunder Bay Island, in Lake Huron, Michigan, made under the direction of Col. W F. Raynolds, C. S. corps of engineers, superintendent of the survey of the northern and northwestern lakes.

						SPRIN	īG.							
	Pressure to 32°	reduced Fahr.	Temp.	,Fahr.	Vag	or.	d snow, mals.	(10=sky en-			Wind			
Time of observation.	Total, U. S. inches	Gassous, U. S. in- ches and decimals.	Dry bulb.	Wet bulb.	Elasticity, U. S. in- ches and decimals.	Humidity. Satura- tion == 1,000.	Am't of rain and melted anow U. S. inches and decimals.	Am't cloudiness. (10-sk tirely overcast, 0-clear	Observed direction; from whence.	Observed velocity; miles per bour.			f direction mile	
0, 30 a.m. 1, 30 a.m. 2, 30 a.m. 3, 00 a.m. 3, 00 a.m. 4, 00 a.m. 4, 00 a.m. 4, 00 a.m. 5, 30 a.m. 7, 30 a.m. 7, 30 a.m. 7, 30 a.m. 7, 30 a.m. 11, 30 a.m. 11, 30 a.m. 11, 30 a.m. 12, 30 p.m. 1, 30 p.m. 3, 30 p.m. 6, 30 p.m. 7, 30 p.m. 6, 30 p.m. 7, 30 p.m. 6, 30 p.m. 1, 100 p.m. 1, 30	29. 329 29. 329 29. 318 29. 313 29. 313 29. 313 29. 313 29. 313 29. 313 29. 313 29. 313 29. 313 29. 329 39. 325 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 327 29. 328 29. 329 29. 329 29. 329 29. 328 29. 329 29. 32	29. 163 29. 160 29. 160 29. 159 29. 159 29. 159 29. 159 29. 159 29. 148 29. 148 29. 148 29. 147 29. 147 29. 147 29. 148 29. 148 29. 143 29. 143 29. 143 29. 143 29. 143 29. 129 29. 12	0 33, 6 33, 3 33, 1 32, 6 32, 5 32, 5 32, 6 32, 5 32, 6 32, 7 34, 2 35, 2 36, 0 36, 7 36, 3 36, 0 36, 7 41, 4 41, 6 41,	31.33 30.96 30.55 30.55 30.66 30.99 31.22 32.13 33.55 34.28 35.37 36.22 36.58 37.14 37.44 37.45 37.44 37.45 37.44 37.45 37.44 37.45	. 158 . 156 . 157 . 156 . 157 . 158 . 160 . 163 . 163 . 163 . 163 . 183 . 183 . 183 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 184 . 185 . 186	. 754 . 758 . 768 . 768 . 771 . 774 . 777 . 781 . 773 . 763 . 755 . 745 . 740 . 730 . 740 . 730 . 741 . 751 . 691 . 792 . 792 . 793 . 793 . 794 . 798	1. 10 1. 04 0. 81 1. 22 0. 76 0. 82 0. 84 0. 45 0. 59 0. 42 0. 59 0. 57 0. 81 0. 49 0. 59	6.6.6.6.6.6.7.7.7.7.7.7.7.7.7.7.7.7.7.7	0 N. 2 W N. 2 W N. 2 W North N. 2 W North N. 3 W N. 3 W N. 3 W N. 3 E N. 3 E N. 3 E N. 3 E N. 14 E N. 15 E N. 15 E N. 15 E N. 15 E N. 16 E N. 17 E N. 18 E N. 18 E N. 19 N. 20 E N. 19 N. 20	224532211.191988765544411.157666707011.1.16667070223	944. 0 250. 2 250. 5 239. 3 195. 4 196. 7 235. 4 299. 1 296. 6 269. 9 301. 5 307. 1 311. 3 311. 7 392. 6 496. 4 496. 1 472. 0 448. 1 441. 5 442. 6 443. 0		2.6 20.5 30.9 19.3 32.4 18.0 20.4 37.6 52.4 97.4 54.5 56.0 70.7 51.0 87.1 92.6 130.2 97.5 92.6 130.2 97.5 92.6 130.2 130.4	21 11:105 20:4 11:
Mean of a	29, 317	29, 147	37. 1	33.4	. 170	. 716	16. 51	6.	N. 6 E.	1.5	16433, 1	7	1412.8	

SUMMER. = aky i melted snow, I decimals. Pressure reduced Vapor. Temp., Fahr. Wind. to 32° Fahr. 02.98 10.98 miles U. S. inches decimals. Saturation 000. ğ Observed direction; from whence. t cloudinoss. Resolution of direction and lime of observation. U. S. Inches declinals. PP U. S. in velocity, in miles per Observed velocity; per hour. hour. Amount of rain U. S. Inches 1 를 umidity. balb. Amount o Total, E. w. N. 8. E ٥ ۰ 0 **29. 398** 29. 799 29. 399 28, 992 28, 993 28, 995 28, 998 4.6 N. 34 W. 4.7 N. 39 W. 4.7 N. 39 W. 4.7 N. 39 W. 4.7 N. 39 W. 4.5 N. 39 W. 5.1 N. 43 W. 5.2 N. 43 W. 5.4 N. 44 W. 5.5 N. 43 W. 5.5 N. 38 W. 5.5 N. 38 W. 5.6 N. 38 W. 5.6 N. 38 W. 5.7 N. 38 W. 5.7 N. 38 W. 5.8 N. 38 W. 5.8 N. 38 W. 5.8 N. 38 W. 5.8 N. 38 W. 5.8 N. 38 W. 5.8 N. 38 W. 5.8 N. 38 W. 5.8 N. 38 W. 5.8 N. 38 E. 6.0 N. 14 E. 6.0 N. 14 E. 6.0 N. 12 E. 6.0 N. 12 E. 6.1 N. 12 E. 6.2 N. 12 E. 6.3 N. 12 E. 6.4 N. 12 E. 6.6 Nerth. 6.7 N. 12 E. 6.8 N. 12 E. 6.9 N. 12 E. 6.9 N. 12 E. 6.9 N. 12 E. 6.9 N. 12 E. 6.9 N. 12 E. 6.9 N. 12 E. 6.9 N. 12 E. 6.9 N. 13 E. 6.9 N. 12 E. 6.9 N. 13 E. 6.9 N. 12 E. 6.9 N. 13 E. 6.9 N. 990. 8 969. 9 990. 7 0. 30 a.m. 58. 9 58. 6 58. 2 57. 8 57. 6 57. 4 57. 4 55. 4 55. 9 54. 9 54. 7 54. 6 54. 5 54. 5 55. 8 55. 8 56. 5 56. 9 56. 9 60. 0 60. 6 . 796 . 794 . 800 .405 .403 .401 .401 .400 .401 .404 .405 .408 .415 1. 00 a.m. 0.51 1. 30 a.m. 2. 00 a.m. 175. 2 0. 76 29. 399 293. 1 353. 4 274. 0 214. 3 804 28, 999 28, 999 28, 909 29, 000 26, 999 28, 997 28, 996 29, 400 29, 401 29, 401 29, 401 29, 403 29, 404 29, 409 29, 416 29, 417 29, 417 29, 417 29, 417 29, 416 29, 402 29, 402 29, 402 29, 402 29, 402 29, 394 29, 394 29, 394 29, 394 29, 394 29, 394 2. 30 a.m. 3. 00 a.m. RIN 192. 9 220. 5 0.86 .814 3. 30 a.m. 271.7 243.2 228.8 . 817 0. 75 4. 00 a.m. 251.5 257. 0 239, 4 200, 6 4. 30 a.m. 57. 5 . 821 57. 5 57. 7 58. 1 58. 8 59. 9 61. 1 0.74 232.6 216.9 193.5 170.3 157.4 5. 00 a.m. . 821 189. 7 175. 5 154. 2 5. 30 a.m. 6. 00 a.m. 815 28, 991 28, 984 28, 978 0. 51 .812 6. 30 a.m. 0. 43 145. 8 147. 1 7. 00 a.m. 433 . 782 62. 2 63. 3 64. 3 65. 1 65. 8 66. 5 7. 30 a.m. 28. 974 . 439 . 766 . 439 . 449 . 445 . 458 . 460 . 462 . 461 28. 965 28. 961 28. 958 0. 21 128. 8 190. 2 8.00 a.m. 177. 2 166, 9 152, 8 **251**, 6 111, 3 8. 30 a.m. . 737 0. 02 79, 8 81, 1 66, 3 9.00 a.m. . 723 ... 28. 957 28. 955 28. 955 28. 949 28. 946 28. 946 28. 943 28. 933 28. 933 28. 933 28. 933 28. 933 28. 933 9. 30 a.m. 716 10.00 a.m. 693 0. 07 ... 10.30 a.m. 60. 8 61. 1 61. 2 61. 4 61. 6 61. 7 61. 7 61. 8 61. 7 61. 3 61. 0 60. 7 60. 2 59. 9 58. 8 58. 3 107.8 45. 6 . 676 0. 07 11.00 a.m. 11.30 a.m. 12.00 m... 67.7 . 671 58.8 664 . 656 . 642 . 634 . 627 104. 8 51. 3 18. 0 47. 5 68. 1 68. 6 68. 9 69. 3 69. 6 69. 7 69. 5 69. 5 466 38. 0 58. 3 0. 04 465 463 89. 5 89. 0 92. 8 0. 30 p.m. 1. 00 p.m. 0.30 463 1. 30 p.m. 19. 2 11. 6 1. 8 51. 9 78. 7 98. 8 0. 44 9.00 p.m. 461 623 114. 1 . 459 . 458 . 457 . 455 . 453 . 450 2.30 p.m. . 620 1.09 41. 9 22. 1 43. 0 46. 6 41. 3 3,00 . 624 p.m. 3. 30 p.m. 4. 00 p.m. 4. 30 p.m. 627 29, 369 29, 387 29, 365 29, 364 29, 363 29, 363 29, 363 29, 365 29, 365 68. 8 68. 4 67. 7 0. 91 . 632 ... 28, 932 28, 931 28, 934 28, 938 28, 941 28, 943 28, 952 28, 953 . 641 . 648 112.1 54. 6 56. 1 61. 0 0. 59 5.00 5. 00 p.m. 5. 30 p.m. 6. 00 p.m. 6. 30 p.m. 41.3 44.8 73.4 79.1 53.0 47.1 6.6 1.3 67. 0 66. 3 65. 3 64. 4 63. 7 .445 .441 .439 .432 . 659 0. 42 . 671 6. 30 p.m. 7. 00 p.m. 7. 30 p.m. 8. 00 p.m. 117.0 . 683 0. 49 . 693 . 705 . 717 . 728 122. 0 168. 1 178. 9 225. 3 . 428 63. 0 62. 4 61. 8 0. 60 28, 961 57. 9 425 8. 30 p.m. 29. 388 28, 966 57. 5 . 422 0. 90 9. 00 p.m. 57. 2 56. 9 56. 6 29. 390 28, 971 737 239. . 419 98, 975 28, 981 263, 32. 2 9. 30 p.m. 29. 393 61.3 203, 3 292, 2 276, 7 279, 5 249, 7 253, 3 10. 00 p.m. 29. 395 60. 8 60. 4 60. 0 .414 754 760 767 0.78 51.8 56. 3 56. 1 55. 9 55. 8 10. 30 p.m 104. 7 197. 3 29. 396 28. 983 . 412 0. 79 11.00 p.m. 29, 397 28, 987 . 410 28. 987 59. 5 29, 398 .410 . 773 168.0 11. 30 p.m. 28. 988 59. 3 0. 85 185. 1 12, 00 m.. 29. 397 . - - . 28, 962 58.8 . 438 .714 5.5 N. 8 W... 0.7 408.1 Mean 7.2.9 29, 400 -----N. 94 W. 1. 0 Mean of all 29.408 28.966 58. 2 . 434 . 725 13. 15 5, 4 8090.7 3175.3

AUTUMN.

		reduced Fahr.	Temp.	, Fahr.	Va	por,	d snow,	skyen- sky.)			Win	L		
Time of observation.	U. S. inches and decimals.	ous, U. S. inches and decimals.	alb.	ulb,	ity, U. S. inches nd decimals.	lity. Saturation = 1.000.	Amount of rain and melted an U. S. inches and decimals.	Amount cloudiness, (10=1) tirely overcast, 0 = clear	Observed direction; from whence.	Observed velocity; miles per hour.	Resolut veloc hour.		direction in mile	
Time	Total,	Gaseous, and	Dry balb.	Wet bulb.	Blasticity, and d	Humidity.	Amoun U.	Amour	Observ	Observ	N.	8.	E.	w.
0.30 a.m. 1.00 a.m. 1.30 a.m. 2.30 a.m. 2.30 a.m. 3.30 a.m. 3.30 a.m. 4.00 a.m. 5.50 a.m. 5.50 a.m. 6.00 a.m. 6.00 a.m. 6.00 a.m. 6.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 9.30 a.m. 9.30 a.m. 9.30 a.m. 9.30 a.m. 9.00 a.m.	29. 373 29. 372 29. 370 29. 370 29. 370 29. 370 29. 370 29. 370 29. 370 29. 372 29. 372 29. 372 29. 372 29. 372 29. 372 29. 372 29. 372 29. 372 29. 373 29. 363 29. 384 29. 385 29. 386 29. 386 29. 386 29. 386 29. 386 29. 387 29. 373 29. 373 29. 373 29. 373 29. 373 29. 373 29. 373 29. 373 29. 373 29. 373 29. 373 29. 373 29. 373 29. 373 29. 374 29. 373 29. 374 29. 373 29. 373 29. 374 29. 373 29. 374 29. 373 29. 374 29. 373 29. 374 29. 373	29. 104 29. 105 29. 103 29. 103 29. 103 29. 104 29. 104 29. 104 29. 104 29. 105 29. 104 29. 105 29. 101 29. 101 29. 101 29. 101 29. 101 29. 101 29. 101 29. 101 29. 107 29. 082 29. 082 29. 082 29. 082 29. 082 29. 082 29. 082 29. 082 29. 082 29. 082 29. 082 29. 082 29. 082 29. 083 29. 086 29. 086 29. 086 29. 086 29. 086 29. 086 29. 086 29. 086 29. 086 29. 086 29. 086 29. 086 29. 108 20. 108 20. 108 20. 108 20. 108 20. 108 20. 108 20. 108 20. 108 20. 108 20. 10	45.86 45.53 45.54 45.50 14.53 45.50 15.13 8 2.26 45.51 15.10	\$\frac{4}{4}\$\frac{1}{4}\$\frac	. 268 . 267 . 268 . 267 . 266 . 265 . 266 . 266 . 266 . 266 . 266 . 277 . 277 . 271 . 292 . 293 . 294 . 293 . 294 . 294 . 295 . 296 . 296 . 297 . 297 . 298	. 787 787 791 794 901 803 805 806 809 810 812 806 807 790 756 779 691 685 677 680 699 705 713 729 736 742 749 759 769 776 777 780 777 780 774 777 780 774 776 777 780 774 776 777 780 774 776 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 780 776 777 777 780 776 777 777 780 776 777 777 780 776 777 777 780 776 777 777 777 780 780	0. 73 0. 68 0. 64 0. 73 0. 81 0. 86 0. 77 0. 81 0. 78 0. 74 0. 73 0. 94 0. 73 0. 94 0. 33 0. 97 0. 93 0. 94 0. 95 0. 97 0. 99 0. 90	6666669112357788888998999998755555555556631108776777777777777777777777777777777777	N. 80 W W W W W W W W W W W W W W W W W W	1.46 1.46 1.46 1.55 1.16 1.55 1.16 1.16 1.16 1.16 1.1	908. 7 943. 8 233. 2 990. 4 175. 0 154. 3 141. 0 139. 4 111. 5 117. 8 180. 7 121. 0 131. 0 101. 2 147. 7 133. 6 140. 4 116. 3 109. 1 80. 9 97. 1 80. 9 97. 1 100. 6 80. 1 100. 6 100. 1 100. 1	••••		211 9 241 9 175 5 180 1 171 5 117 5
Mean 7,2,9	29. 368	29. 088	48. 4	44.9	. 279	. 749		7.3	N. 58 W.	0. 9	300.0			436.
Mean of all	29, 370	29. 092	47.7	44.6	. 282	.755	15. 55	7.3	N. 52 W.	1.1	6329.7			7307.

						WIN	TER.							
		reduced Fahr.	Temp	.,Fahr.	Va	por.	l snow,	sky en-			Wind.			
Time of observation.	J. S. inches and decimals.	U. S. inches decimals.			U. S. inches lecimals.	Humidity. Saturation = 1.000.	Amount of rain and melted snow U. S. inches and decimals.	ondiness. (10=sky en reast. 0=clear sky.)	Observed direction; from whence.	Observed velocity; miles bour.	Resolut veloc hour.	ity,	f direc in mi	tion and les per
Time of ot	Total U.	Gaseous, and d	Dry bulb.	Wet bulb.	Elasticity, U. S. and decimals	Humidity.	Amount of U. S. h	Amount cloudiness.	Observed d	Observed v	N.	s.	E.	w.
0.30 a.m. 1.00 a.m. 2.00 a.m. 2.00 a.m. 2.00 a.m. 2.30 a.m. 3.00 a.m. 3.00 a.m. 4.30 a.m. 5.00 a.m. 6.00 a.m. 7.00 a.m. 7.00 a.m. 7.00 a.m. 10.30 a.m. 10.30 a.m. 11.30 a.m. 12.00 m. 11.30 p.m. 12.00 p.m. 3.30 p.m. 4.30 p.m. 5.30 p.m. 6.00 p.m.	**************************************	29. 229 29. 23	21.6 21.4 20.9 20.8 20.8 20.7 20.7 20.7 20.7 20.7 20.1 21.1 21.1 21.1 22.1 23.1 24.1 25.7 25.7 26.2 26.2 26.2 26.2 26.2 26.2 26.2 26	19.7 19.5 19.9 1 18.9 8 18.8 18.8 18.8 18.8 19.0 19.0 19.5 19.5 20.1 19.5 20	. 094 . 093 . 092 . 092 . 092 . 093 . 093 . 093 . 093 . 095 . 095 . 096 . 097 . 096 . 099 . 101 . 102 . 104 . 104 . 104 . 104 . 104 . 104 . 104 . 104 . 104 . 104 . 106 . 106 . 099 . 099 . 093 . 093 . 095 . 095 . 096 . 097 . 096 . 096 . 096 . 096 . 096 . 096 . 097 . 096 . 096 . 096 . 096 . 096 . 097 . 096 . 096 . 097 . 096 . 096 . 096 . 096 . 097 . 096 . 096 . 096 . 096 . 096 . 097 . 096 . 096 . 096 . 096 . 097 . 096	. 707 . 706 . 708 . 709 . 712 . 713 . 714 . 715 . 725 . 723 . 719 . 720 . 721 . 723 . 719 . 720 . 721 . 702 . 693 . 691 . 663 . 665 . 667 . 663 . 704 . 705 . 702 . 702 . 702 . 702 . 702 . 703 . 704 . 705 . 702 . 705 . 702 . 705	0.88 1.10 1.07 0.96 1.24 1.23 0.94 0.93 0.90 0.76 0.51 0.56 0.61 0.63 0.60 0.52 0.64 0.43 0.39 0.55	7.7.7.99 8.8.234538.8.6666738.8.8.7.7.7.7.7.99 8.8.8.8.8.8.8.8.8.8.8.8.8.8.8.8.8.8.	N. 56 W N. 53 W N. 48 W N. 48 W N. 48 W N. 49 W N. 49 W N. 55 W N. 55 W N. 55 W N. 55 W N. 55 W N. 55 W N. 55 W N. 55 W N. 55 W N. 60 W N. 60 W N. 66	3.43.3.3.3.5.6.4.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2	337. 2 306. 7 359. 3 409. 0 439. 8 436. 5 277. 8 300. 7 274. 2 253. 5 186. 7 214. 4 181. 3 174. 9 104. 0 145. 5 114. 4 148. 2 155. 2 177. 1 181. 0 161. 0 161. 0 150. 3 15			479. 0 503. 6 479. 1 477. 5 479. 1 477. 5 477. 5 477. 5 479. 7 492. 7 492. 7 492. 7 492. 7 492. 7 492. 7 492. 7 492. 7 492. 7 308. 7 308. 7 308. 7 309. 5 32
Mean of all	29. 332	29. 234	23. 0	20. 9	. 097	. 698	18. 12	8. 2	N. 59 W.	2.3	10699. 8			17213, 1

TWO YEARS.

The lates The		Pressure to 320	reduced Fahr.	Temp	,Fahr.	Vaj	por.	melted snow, decimals.	(10 = sky clear sky.			Wind	L.		
0.30 a.m. 29.356 29.194 40.0 37.4 231 759 6.4 N.28 W. 1.9 1967.9 691.6 1.00 a.m. 29.356 29.195 39.8 37.2 231 761 3.92 6.4 N.35 W. 2.1 1297.9 687.3 2.00 a.m. 29.354 29.125 39.2 36.8 229 788 3.58 6.3 N.36 W. 2.2 1381.6 687.3 3.00 a.m. 29.354 29.125 39.2 36.6 229 788 3.58 6.3 N.36 W. 2.2 1381.6 787.3 1.00 a.m. 29.354 29.125 39.1 36.6 229 779 2 6.6 N.36 W. 2.2 1382.7 1007.0 3.00 a.m. 29.353 29.125 39.1 36.6 228 775 3.88 6.6 N.36 W. 2.2 1382.4 975.2 3.00 a.m. 29.353 29.125 39.1 36.6 228 775 3.88 6.6 N.36 W. 2.2 1382.4 975.2 3.00 a.m. 29.353 29.125 39.0 36.6 228 775 3.88 6.6 N.36 W. 2.2 1382.4 975.2 3.00 a.m. 29.352 29.121 39.0 36.6 228 777 3.00 a.m. 29.352 29.123 39.0 36.6 228 777 3.00 a.m. 29.352 29.123 39.0 36.6 228 777 3.00 a.m. 29.352 29.123 39.0 36.7 223 779 3.63 6.8 N.34 W. 2.0 1069.5 992.6 6.8 N.36 W. 2.2 1382.4 975.5 992.6 6.00 a.m. 29.352 29.123 39.0 36.8 20.7 81 7.0 N.39 W. 2.1 1170.9 994.5 5.00 a.m. 29.352 29.123 39.0 37.1 223 7783 3.59 7.0 N.41 W. 1.9 1045.4 554.6 0.00 a.m. 29.354 29.118 30.8 87.4 226 781 37.7 229 777 7.8 N.41 W. 2.1 1965.4 554.6 0.00 a.m. 29.354 29.118 40.1 37.7 229 777 7.8 N.41 W. 2.1 1965.4 554.6 0.00 a.m. 29.358 29.115 40.8 32.7 785 3.00 77.0 N.39 W. 1. 177.9 94.1 6.00 a.m. 29.359 29.115 40.8 32.7 79.5 3.00 3.00 37.1 223 777 3.00 a.m. 29.359 29.115 40.8 32.7 245.0 3.00 a.m. 29.359 29.115 40.8 32.7 245.0 3.00 a.m. 29.359 29.115 40.8 32.7 245.0 3.00 a.m. 29.360 29.118 40.1 37.7 229 777 7.7 3.0 3.00 3.00 3.00 3.00 3.0	observation.	. S. inches and scimals.	U. S. decimal			U. S. lecimal	. Saturation : 1. 000.	and and	111	direction; from	velocity; miles	veloc	ity,		
0. 30 a.m. 92. 356	Time of		Gaseous, and	Dry bulb	Wet bulk	Elasticity and	Humidity	Amount U. S.	Amount entirely	Observed	Observed	N.	8.	E.	₩.
	1. 00 a.m. 1. 30 a.m. 2. 30 a.m. 2. 30 a.m. 3. 00 a.m. 3. 00 a.m. 4. 00 a.m. 5. 00 a.m. 6. 00 a.m. 6. 00 a.m. 7. 00 a.m. 7. 30 a.m. 8. 30 a.m. 10. 00 a.m. 10. 00 a.m. 11. 00 a.m. 11. 00 a.m. 12. 00 m 11. 30 a.m. 12. 00 p.m. 13. 00 p.m. 14. 30 p.m. 5. 30 p.m. 5. 30 p.m. 6. 30 p.m. 7. 00 p.m. 9. 00 p.m. 11. 00 p.m. 11. 00 p.m.	29. 356 29. 354 29. 354 29. 353 29. 353 29. 353 29. 353 29. 352 29. 352 29. 353 29. 354 29. 356 29. 367 29. 369 29. 35	28. 125 29. 124 29. 125 29. 125 29. 125 29. 125 29. 126 29. 121 29. 121 29. 121 29. 113 29. 115 29. 115 29. 116 29. 107 29. 111 29. 111 29. 111 29. 111 29. 111 29. 111 29. 111 29. 111 29. 111 29. 111 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112 29. 112	40. 0 39. 84 39. 5 39. 2 39. 5 39. 2 38. 9 39. 0 38. 9 39. 0 140. 84 42. 1 42. 1 42. 1 42. 1 42. 1 42. 1 44. 6 45. 1 44. 6 45. 1 44. 6 46. 2 44. 6 46. 5 44. 6 46. 5 44. 6 46.	37. 4 2 3 36. 7 6 6 6 7 3 6 6 8 9 3 2 6 7 7 1 1 1 2 1 3 3 6 6 7 1 1 1 2 1 3 3 6 7 1 1 1 2 1 3 3 6 7 1 1 1 2 1 3 3 6 7 1 1 1 2 1 3 3 6 7 1 1 1 2 1 3 3 6 7 1 1 1 1 2 1 3 3 6 7 1 1 1 1 2 1 3 3 6 7 1 1 1 1 2 1 3 3 6 7 1 1 1 1 1 2 1 3 3 6 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	. 231 229 229 229 229 229 228 229 228 233 230 245 256 257 256 256 257 257 259 261 261 261 261 261 261 261 261 261 261	761 765 778 778 775 777 781 783 781 783 781 767 769 769 696 666 663 656 663 656 663 663 663 671 677 779 779 779 779 779 779 779 779 779	3.58 3.38 3.59 3.43 3.02 2.22 2.02 2.08 2.03 3.18 2.24 2.03 2.21 2.33 3.00 2.21 2.33 3.00 2.21 2.33 3.00 2.21 2.33 3.00 2.21 2.33 3.00 2.24 2.33 3.00 2.24 2.33 3.00 2.24 2.33 3.00 2.24 3.00 3.18 3.00 3.18 3.00 3.18 3.00 3.00 3.18 3.00 3.00 3.00 3.00 3.00 3.00 3.00 3.0	6.6.6.6.7.7.7.7.7.7.7.7.7.7.7.7.7.7.7.7	N. 28 W N. 35 W N. 36 W N. 36 W N. 36 W N. 38 W N. 39 W N. 41 W N. 42 W N. 42 W N. 42 W N. 42 W N. 42 W N. 42 W N. 42 W N. 42 W N. 42 W N. 42 W N. 42 W N. 42 W N. 42 W N. 48	2 12 22 22 22 22 22 22 22 22 22 22 22 22	1229, 7 1318, 6 1323, 7 1410, 7 1319, 6 1329, 5 1069, 5 1070, 9 1045, 4 1392, 5 1070, 9 1045, 4 1392, 5 1071,			87. 1 851.1 951.0 1007.
	Mean 7,2,9 Mean of all		29, 110	43. 4	39. 9	. 244	.718	63, 38	7. 0		-		=		1348. 9 96403. 1

TABLE L.—Hourly records of the direction and relative velocity of the wind at Milwaukee, Wisconsin, taken from the sheets of the anemograph kept by observer J. A. Lapham.

ion			APRIL	, 1861.		
beerva	lst.	24.	34.	4th.	5th.	6th.
Time of observation.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.		Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 7 8 9 10 11 12 12 13 4 5 6 7 8 9 10 11 12 13 14 15 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	SE. by E . 5.0 E. by S . 5.0 E. by S . 2.6 NE . 5.5 N. by E . 6.2 North . 5.0 North . 5.0 North . 3.7 NW. by N . 3.0 W. NW . 2.5 W. NW . 2.5 W. NW . 0.5 SW. NW . 0.5 SW. D. 1 SW. by S . 0.1 SW. by S . 0.1 W. by S . 0.2 W. by S . 0.2 W. by S . 0.2 W. by S . 0.2	West 0.6 West 0.5 West 0.2 W. by N. 0.4 W. by N. 0.6 South 0.2 W. by N. 0.5 NW 0.5 8. by W. 0.9 SW. by S. 0.8 SW 0.7 S.SW 0.5	W. by 8 . 0.8 N. NW . 0.9 N. NW . 0.3 North . 1.0 N. by E . 1.2 N. by W. 0.2 N. NE . 1.4 NE. by N. 1.4 NE. by N. 2.5 NE. by E . 2.5 NE . 2.5 NE . 2.5 NE . 2.7 NE . 3.6 NE . 3.6 NE . 3.6 N. 1.0 N. 1.	E. by S. 2.9 E. 8E 4.0 E. 8E 4.0 E. 8E 4.0 SE. by E 4.0 SE. by E 3.5 E. 8E 3.7 E. by N 3.7 E. by N 3.7 E. NE 3.0 E. NE 3.0 E. NE 3.5	E by N 3.5 East 1 3.0 E by N 3.0 E by N 3.0 E by N 3.0 E NE 3.0 E NE 3.2 E NE 3.2 E NE 3.2 E NE 3.2 E NE 3.2 E NE 3.2 E NE 3.2 E NE 3.2 E NE 3.6 E NE 3.6 E NE 3.6 E NE 3.6 E NE 3.6 E NE 3.6 E NE 3.6 E NE 3.6 E NE 3.6 E NE 3.6 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5 E NE 3.5	E.NE . 24 NE. by E . 3.1 E.NE . 3.6 E.NE . 3.2 E. NE . 3.2 E. NE . 2.4 E.SE . 2.0 E.SE . 2.0 E.SE . 3.7 E.SE . 4.0 E.SE . 3.7 E.SE . 4.0 E.SE . 3.7 E.SE . 5.1 SE . by E . 5.5 SE . 5.1 SE . 5.1 SE . 5.1 SE . 5.5
	7th.	8th.	9th.	10th.	11 tb.	12th.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M. 5 6 7 8 9 10 11 12	8. SE 3.4 S. SE 3.5 SE by 8 2.0 SE by 8 1.5 E by 8 0.4 SE 0.3 SE 2.2 SE 3.0	E.SE 3.8 E.SE 3.2 E. by S 3.3 East 2.2 E. by S 3.2 E. SE 3.4 E.SE 1.7 E.SE 1.6 E.SE 1.4 E.SE 1.4 E.SE 1.4	NE by N. 4.1 NE by N. 4.2 NE	E. NE	N.NE3.0 N.NE4.2	South 0. 1 S. by W 0. 1
	13th.	14th.	15th.	16th.	17th.	18th.
A. M. 1 2 3 4 5		E. by N0.6 North0.4 NW. by N0.2 NW. by W .0.9 N. by W1.6	SE	NE. by N4.1 NE. by N3.1 NE. by N4.6 NE. by N4.5 N.NE3.1	N.NW	8W

ion.			APRIL	, 1961.		
baervat	13th.	14th.	15th.	16th.	17th.	18th.
Time of observation.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.
A. M. 6 7 8 9 10 11 12 P. M. 1 2 3 4 5 6 6 7 8 9 10 11 12 12 12 12 12 12 12 13 14 14 15 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	NW. by N3.6 North2.8 E.NE2.3 E. by N1.4 East0.5	North 2.2 North 1.5 East 1.5 E. by S 2.1 East 2.5 SE 3.1 SE 3.4 SE 3.5 SE 3.5 SE 3.9 SOUTH 1.4 SOUTH 1.4	N.NE	N.NE	E by S	W. NW. 47 W. by N. 52 NW. by N. 52 NW. by N. 42 North 45 North 45 North 46 N. by E 54 North 16 N. by W. 17 North 10 N. by W. 15 North 10 N. by W. 15 North 25 North 15 North 15 North 15 North 15 North 15 North 15 North 15
	19th.	20th.	21st.	224.	23d.	914.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12	NW 3.0 NW 3.0 N.NW 2.5 N.NW 2.4 N.NW 2.4 N.NW 2.3 N.NW 3.2 North 1.7 North 1.7 North 2.0 East 2.0 East 2.1	S.SW 2.3 S.SW 2.9 S.W. by S. 3.5 SW 3.5 SW. by S. 3.1 SW. by S. 4.6 SW. by S. 5.0 SW 5.1 SW 5.5	SE	8W. by W. 3.1 8W. by W. 3.1 8W. by W. 3.7 8W. by W. 4.5 8W. 5.5 West. 4.9 8.8W. 3.8	N.NE	W. by 84 W. SW4 W. SW4 W. by 84 W. by 86 W. by 86
P. M. 1 2 3 4 5 6 7 8 9 10 11	E.SE	SW5.0 SW. by S4.5 SW. by S4.0 SW3.2	SE	West 4.5	N.NE3.0 N. by W1.3 N. by E0.7 NE. by N0.1	West 1: West 6: W. by N 6: W. by N 6: NW 2: W. NW 2: W. NW 2: W. NW 2: NW 0: NW 0:
	25th.	26th.	27th.	28th.	29th.	30th.
A. M. 1 2 3 4 5 6 7 8 9 10 11	SW0.2 SW1.3 E.SE0.2 SE by E0.8	BE. Dy B 0. 8	8 W	NW. by W .1. 1 W.NW 0. 8 W. by N .1. 3 W. NW 1. 8 W.NW 0. 5 NW. by W .0. 1 West 0. 4 W.SW 2. 2 8.SW 3.0 NW 2. 8 SW. by S .2. 9	W.N.W	

			APRIL	, 1961.		
Servatic	25th.	26th.	27th.	28th.	29th.	30th.
Time of observation.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.
P. M. 1 2 3 4 4 5 6 6 7 8 9 10 11 12	SE. by S 4. 0 SE. by S 3. 2 SE. by S 2. 2 SE. by E 1. 9 SE. by S 2. 0 S.SE. by S 2. 7 SE. by S 2. 7 SE 2. 5	SE. by E 4. 6 SE. by E 3. 6 South 5. 0 South 2. 2 South 2. 1 SE 1. 3 South 2. 7 South 4. 0 South 4. 0	N.NW	8W	West	NW 5.0 NW. by W .5.4 W.NW 5.8 NW. by W .5.1 NW 4.9 North 2.0 North 1.0 N. by E 0.9 North 0.1
			MAY,	1861.		
	lst.	2d.	3d.	4th.	5th.	6th.
A. M. 1 23 34 56 78 99 10 11 12 P. M. 1 23 44 56 78 99 10 11 12 12	N. by W. 1.4 North 0.5 NE. by N. 0.6 North 0.4 N.NW 0.9 N.NW 0.3 NE. by N. 1.3 NE 3.5 NE 3.5 NE 3.5 NE 3.5	SE 0.3 SE 1.2 SE 2.7 SE 3.3 SE 4.7 SE 5.8 SE 4.1 SE 3.8 SE 3.8 SE 3.8 SE 3.8 SE 3.9 SE 3.8		NE. by E	SE 3.4 SE 4.0 SE 4.7 SE by E 4.7 SE by E 3.9 SE by E 3.9 SE by E 4.5 E.SE 2.7 E.SE 3.1 E. by S 4.8 SE 6.0 SE 6.4 SE 7.6 SE 5.6 S. by W 3.1 S. by W 3.1 S. by W 3.1 S. by W 3.1 S. by W 3.1 S. by W 3.1 S. by W 3.1 S. by W 3.1 S. by W 3.1 S. by W 3.1 S. by W 3.1	SW. by W. 7. 1 SW
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M.	7th. W.SW W.SW W.SW W.SW W.SW W.NW W.NW W.	NW	W. by S W. by N W. NW W. by N W. by N W. by N W. NW West	10th. SE. by E	West	NE NE. by N
1 2 3 4 5	West	SE. by E SE. by E SE SE	l	W. by N West W. by S West	W.NW W. by N NW. by W	NE by E

100			MAY,	1861.		
Time of observation.	7th.	8th.	9th.	10th.	lith.	19th.
5	Wind, from	Wind, from	Wind, from	Wind, from	Wind, from	Wind, from
2	whence, and rel-				whence, and rel-	
	ative velocity.	ative velocity.	ative velocity.	ative velocity.	ative velocity.	ative velocity
_						
M.						
6	NW. by W	8E. by E	8E	West	NW. by W	N.NE
7	NW. by W	SE. by E	SE. by 8	W. by N West	NW	N.NE
8 9	NW	South	SE. by S	West	NW	N. by E
Ŏ	N. by E	8W	E. by 8	West	W.NW	N. by E
0	NW. by W	8W	8. by E	West	NW. by W	N. by E
2	NW	s.sw	SE	W. by N	N. by E	N.NE
	13th.	14th.	15th.	16th.	17 th.	18th.
						
М. 1	N. by E	NW. by W	w.sw	NW	NE. by N	E.NE
2	N.NE	NW	W. by 8	NW	NE	East
3	NE	NW. by W	NW. by W	NW. by N	NE. by E	East
4	E. by N E. by S	W.NW NW. by W	NW W. by 8	NW	E.NE	E. by N E.NE
5 6	E. by 8	NW. by W	W. by 8	NW	E. by N	E.NE
7	E.8É	NW. by W W.NW	West	North	East.	E. by N
8	8E. by E	W.NW	QUUT has UUT	N. by W	SE	E. by N
9	SE. by E	NW. by W	8W. by W	E. by S	E.SE	East
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1 2	N. by E	W.NW NW. by W	W. 8W W. by 8	SE	SE. by E East.	E. by N E. by N
M. 1	N.NE	w.xw	West	se	F 1- W	E 1070
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3	West	W. by N	W. by N	šR	E.NE	N.NE
3 4	W. by N	West	W. by N	SE	NE	N. by E
5		8W. by W	West	SE. by E	NE	N. by E
6		w.sw	W. by N	SE. by E	NE. by N NE.	NE
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9	W. by N	8W. by W	W.NW	E. by 8 E.SE	NE	E.NE
ŏ	W.NW	sw	NW. by W	East	NE	E.8E
1	W.NW	W. Dy N	NW	E. by N	NE	8E. by E
2	W.NW	W. by 8	NW. by N	NE	E.NE	E. SE
	19th.	20th.	21st.	22d.	234.	94th.
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1 2	SE. by E	N.NE	N.NE	NW	8W. by W	W.8W
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7	East		North	N.NW	8E	W. by 8
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4 5	E. by N	N.NE	North			W.SW
5 6	East	N.NE	N. by W	N.NW	8E	
5 6 7	East E. by S	N.NE	N. by W N. by E	N.NW	South	W. by 8
5 6 7 8	East E. by 8 E. by 8	N.NE N.NE	N. by W N. by E N.NE	N.NW NE East	South	West
5 6 7 8 9	East E. by S	N.NE N.NE N.NE	N. by W N. by E N.NE	N.NW NE East	South	West W. by N
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5 6 7 8 9 0 1 2	East. E. by S. E. by S. E. by S. E. SE	N.NE N.NE N.NE N.NE	N. by W N. by E N.NE. NE. by N	N.NW NE East East	8. by W 8. by W 8. by W	West W. by N 8W. by W
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5 6 7 8 9 0 1 2 M.	East. E. by S. E. by S. E. by S. E. by S. E. SE. SE. by E. E.NE N.NE N.NE NR. by N.	N.NE	N. by W. N. by E. N. NE. NE. by N. NE. NE. NE. NE. NE. NE. NE. NE. NE. N	N.NW	South S. by W S. by W S. SW S.	West. W. by N SW. by W SW SW SW SW
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567789901122M.	East E by 8 E by 8 E by 8 E by 8 E sE E sE ESE ENE NE NNE NE NE NNE NNE NNE NNE N	NNE NNE NNE NNE NNE NNE NNE NNE NNE NNE	N. by W. N. by E. N. by E. N. NE. NE. NE. NE. NE. NE. E. SE. E. by N. NE. NE. NE. NE. NE. NE. NE. NE. NE. N	N.NW NE East East E.SE SE SE SE SE SE SE SE SE SE SE SE SE S	South S. by W S. by W S. by W S. SW S. SW S. SW S. SW SE SE East E.SE East E.SE East East East East East East East East East East	West. W. by N. SW. by W. SW. SW. SW. SW. SW. SW. SW. by S. SW. by S. SW. by S. W. by S. W. by S. W. by S.
56789012 M. 12345678990	East E by S E by S E by S E by S E SE SE by E E.NE N.NE NE by N N.NE N.NE N.NE N.NE N.NE N.NE N.NE N.	N.NE N.NE N.NE N.NE N.NE N.NE N.NE N.NE	N. by W. N. by E. N. by E. NE by N. NE NE NE NE NE NE NE NE NE NE NE NE NE	N.NW NE East East Esse ESE SE SE SE SE SE SE SE SE SE SE SE SE	South S. by W S. by W S. by W S. S. W S. S. W S. S. W S. S. W S. S. W S. S. W S. S. S. W S. S. S. W S. S. S. S. S. S. S. S. S. S. S. S. S.	East
56789012M. 123456789	East E by 8 E by 8 E by 8 E by 8 E sE E sE ESE ENE NE NNE NE NE NNE NNE NNE NNE N	NNE NNE NNE NNE NNE NNE NNE NNE NNE NNE	N. by W. N. by E. N. by E. N. NE. NE. NE. NE. NE. NE. E. SE. E. by N. NE. NE. NE. NE. NE. NE. NE. NE. NE. N	N.NW NE East East E.SE SE SE SE SE SE SE SE SE SE SE SE SE S	South S. by W S. by W S. by W S. SW S. SW S. SW S. SW SE SE East E.SE East E.SE East East East East East East East East East East	West W. by N. SW. by W. SW. SW. East. SW. SW. SW. SW. by S. SW. by S. SW. by S. W. by S. W. by N.

	may, 1861.			JUNE, 1961.		•
	25th.	3d.	4th.	5th.	6th.	7th.
A TIME OF CORES VENCOL	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.
M. 123456789012	NE. by N N.NE East N. by W West South S. by E		N. NE 1. 3 N. NE 1. 0	N. NE 0.8 N. NE 1.5 N. NE 2.2 N. NE 1.5 N. NE 1.7 N. NE 1.7 N. NE 2.8 NE. by N 3.3 NE 2.0 N. NE 2.0	NE	North 0.8 N. by E . 1.8 N. NE 1.3 NE. by N 1.6 N. NE 2.8 NE 1.9
M. 1 2 3 4 5 6 7 8 9 0 1 2		W. by S0.3 W. by S0.1 W. by S1.0 NW1.8	N. by E 2.3 N. by E 2.8 N. by E 2.7 N. NE 2.2 N. NE 2.3 N. NE 2.5 N. NE 1.6 NE 1.6	NE1.1 NE1.3	E. SE	NE
	8th.	9th.	10th.	11th.	12th.	13th.
M. 1 2 3 4 5 6 7 8 9 0 1 2	N. NW 0. 1 N. NE 0. 4 N. NE 1. 0 NE 1. 0 NE 1. 0 East 1. 0	NW. by N0.1 NE. by N0.2 NE. by N0.5 N. NE. by N0.5 N. NE. 1.0	SE0.9 SE0.1 SE0.1	8W 0.1 8W 0.3 8W 0.5 8W .by W .2.0 8W .by W .2.5 8W .by W .2.6 W .8W .3.8	NW 0.5 NW 0.8 NW 0.9 E. by 8 1.3 E. SE 1.3	N. NE
M. 1 2 3 4 5 6 7 8 9 0 1 2	NE	N. by E 1. 0 E. 8E 0. 5 E. 8E 0. 5 SE. by E 0. 5 E. by S 0. 1 SE 0. 2	SE	W.NW	SE1.8 SE1.0 W. NW3.0	SE 0.8 SE 0.7 SE 0.5 SE 0.5 SE 0.5 SE 0.5 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7
	14th.	15th.	16th.	17th.	18th.	19th.
M. 1 2 3 4 5 6 7 8 9	8W. by S 0. 2 8. SW 0. 1 8. SW 0. 1 8. outh 0. 2 8. outh 0. 1 8. by W 2. 2 8. SW 3. 5 8W. by S. 4. 0	SW	N.NW 2.9 N.NW 3.0 N.NE 3.4 N. by E 2.5 N. by E 3.6 N. by E 4.5 N.NE 4.5 N.NE 4.5	North	8W0.1 S.SW0.3 SW. by S0.2 SW0.1 SW0.1 SW1.0 SW1.1 SW1.0	S.SW 0.1 South 0.5 S. by W 0.5 SW. by S 0.6 SW. by S 0.5 South 0.0

			JUNE	, 1861.		
beervati	14th.	15th.	16th.	17th.	18th.	19th.
Time of observation.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rei- ative velocity.
A. M. 10 11 12 P. M.	8W. by 84. 9 8W5. 4	W.8W9. 2	N.NE 6. 5 N.NE 5. 7	E.SE1. 1 SE. by E1. 4	8W1.6 8W1.4	SE
1 2 3 4 5 6 7 8	SW. by 8 5. 5 SW 5. 4 SW. by 8 5. 7 SW. by 8 5. 8 S.SW 6. 3 SW. by 8 5. 4 SW. by 8 3. 6 SW. by 8 3. 6 SW. by 8 3. 6	W.8W9.1 8W. by W. 6.2 8W. by W. 6.2 W.8W5.4 W.8W5.0 NW. by W. 6.3 W.NW5.9 W. by N5.1	N.NE	E.SE. 1.6 E.SE 1.5 E.SE. 1.1 SE 1.2 SE 1.2 SE 0.7 SE 0.3 South 0.3	SW	SE. by E. 16 S. by E. 15 SE. 19 North 1 8 SW. 12 SSW (13 SW. by W (13) SW. 12 SSW (14)
10 11 12	8W 4.4 8W 5.4 8W . by W 3.1	W.NW3.5 NW2.1 NW. by N4.0	North 1. 9 North 2. 1 North 2. 4	8. by W0. 1 8.8W0. 3 8.8W0. 3		8.8E
	20th.	21st.	224.	234.	94th.	25th.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M. 1 2 3 4 5 6 7 8 9 10 11 12 12 13 14 15 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	NE. by E 1. 9 E.NE 2. 0 E.SE 0. 8 SE 1. 7 SE 1. 6 SE 8 SE 8 SE 8	W.NW 1. 3 W.NW 1. 0 W.NW 3. 4 N.NW 3. 4 N.NW 2. 4 N.NW 1. 7 N. by E 1. 7 East 1. 0 NE 0. 6 NE 0. 5 SW. by W 6 W.SW 1. 8 SW. by W 1. 8 SW. by W 1. 9 W.SW 0. 1 SW 0. 1 SW 0. 1	W.SW 1.0 W.NW 2.3 NW.by W.1.1 NW 0.8 NW 1.2 N.NW 1.2 N.NE 2.0 N.NE 2.3 E.SE 1.2 E. by S 1.0 SE 0.9 SE 0.7 SE 0.8 SE 0.7 SE 0.8 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7 SE 0.7	8.8W .0.1 N.NE .3.5 N. by E .2.1 N. by E .2.5 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0 N.NE .3.0	E. by N	South
	96th.	97th.	28th.	29th.	30th.	JULY 1st.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M.	West 2.0 West 1.6 W.8W 0.7 W.8W 1.9 West 2.4 NW. by N 2.2 West 3.0 West 3.0 W. by 8.3 W.8W 3.3	W. by S 0. 8 W. by S 0. 2 W. by S 0. 1 West 0. 4 West 0. 2 North . 0. 2 SW 0. 1 SW. by S. 1. 3 S.SW 20	W.8W0.7 W.NW1.4 NW. by W.1.1	E. by S	West 0.4 W. by 8.1.8 W.NW	North

tion.		,	JUNE, 1961.			JULY, 1861.
bserva	26th.	27th.	28th.	29th.	30th.	1st.
Time of observation	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.
P. M. 5 6 7 8 9 10 11 12	W. by N2.5 NW. by W.1.9 West1.1 W.NW0.1 W.8W0.2 W.8W0.4	8.8W 1.3 8W. by 8 1.2 8W. by 8 1.3 8W 0.5 8W. by 8 0.5 8W 0.3 8W. by W 0.6	8E. by E0.5 NE	SE0.9 SE0.9 W.SW0.8	E.NE	N. by E 3. 8 N. by E 3. 2 N. by E 1. 5 North 0. 2
	2d.	3d.	4th.	5th.	6th.	7th.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M. 1 2 3 4 5 6 7 8 9 10 10 10 10 10 10 10 10 10 10 10 10 10	SW. by W. 1.0 W. 8W. 1.8 W. by N. 1.8 N. by I. 1.8 N. by E. 1.7 SE. by E. 1.7 SE. by E. 2.0 SE. 1.5 SE. 1.5 SE. 0.9 SE. 0.9 SE. 0.9 SE. 0.9 SE. 0.9	SW by S. 0.1 SW 0.6 SW 1.0 SW 2.6 SW 2.8 SW 2.8 SW 59 S. 2.2 SW by S. 2.3 SE 2.3 SE 1.7 SE 1.7 SE 0.8 SE 0.5	S. by E	East	SW 0.6 SW 0.6 SW 0.8 SW by S. 0.7 SW 1.1 SW by S. 2.3 SW 2.5 SW. by W 2.1 SE. by E 0.9 SE 1.0 South 1.1 S. SW 2.5 SW 1.7 S. by W 1.4 South 0.9 SE 0.5 SE 0.5 SW 0.5 SE 0.5 SW 0.5 SE 0.5 SW 0.5 SW 0.5 SW 0.5 SW 0.5 SW 0.5 SW 0.5 SW 0.5 SW 0.5 SW 0.5 SE 0.5	8W3.9 8W. by 83.7 8W3.9 8W3.0 8W3.0 8W2.2 8W2.2
10 11 12	S. by W0.2		South0.9	8W. by 80.2	S. by W1. 6	sw1.5
	8th.	9th.	10th.	11th.	12th.	13th.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 2 3 4 5 6 7 8 9 9	8W 1.6 8W 1.1 8W 1.0 8W by 8 0.9 8W 2.1 West 1.5 8W 1.5 8W 1.5 8W 1.5 8W by W 2.3 8W by W 2.3 8W by W 3.5 W by S 2.7 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8 8W 1.8	W. SW. 1.8 W. SW. 2.3 W. SW. 1.7 W. SW. 1.4 W. by 8. 2.0 W. by 8. 2.0 W. by 8. 2.0 W. by N. 2.9 W. NW. 2.0 W. NW. 2.0 SW. 1.5 W. SW. 2.4 W. SW. 2.4 W. NW. 2.4 S. 3.5 W. SW. 2.4 S. 3.5 S. 3.6 SE. 6.6 SE. 6.6	N. by E 3.4 N. by E 2.6 N. by E 2.5 North 2.0 N. by W 2.2 North 2.6 N. by E 3.9 N. by E 3.9 N. by E 3.0 N. by E 3.0 N. by E 4.4 N. by E 4.4 N. by E 4.4 N. by E 4.4 N. by E 4.2 N. by E 2.3 N. by E 2.3	NW 0.1 NW 0.1 NW 0.1 E. SE 0.4 SE. by E 2.0 SE. by E 2.8 SE 1.8 SE 1.8 SE 2.0 SE by E 2.8 SE 1.8 SE 2.0	N. by W 2. 4 North 2. 7 North 4. 7 North 4. 7 North 4. 7 North 4. 7 N. by E 4. 7 N. by E 4. 1 N. by E 4. 5 N. by E 4. 5 N. by E 5. 5 N. by E 5. 5 N. by E 5. 5 N. by E 5. 5 N. by E 5. 6 N. by E 5. 6 N. by E 4. 6 N. by E 5. 6 N. by E 5. 6 N. by E 5. 6 N. by E 5. 6 N. by E 5. 6 N. by E 5. 6 N. by E 5. 6 N. by E 5. 6 N. by E 5. 6 N. by E 5. 6 N. by E 5. 6	N. by E 1.6 N. by E 1.8 N. NE 0.8 NE 0.7 NE 0.6 E. by S 0.7 East 0.3 E. by S 0.7 E. by S 0.7 E. by S 0.7 E. by S 0.7
7 8 9 10 11 12	West	SW. by S	North	N. by W2.4 Fast2.0 E. by S1.1 SW0.4 NW1.0 N. by W1.1	North 2. 0 North 0. 5 North 1. 0 North 0. 5	E. by 80.7 East0.3 E. by 80.7 E. by 80.5 E. by 80.7 E. 8E0.1 SE0.5

tion.						
beerva	14th.	15th.	16th.	17th.	18th.	19 th.
Time of observation	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and re- ative velocity.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12	8. SE 0.2 8. by W 1.5 8. by W 1.5 8. SW 2.9 8. SW 2.9 8. SW 2.0	W.NW 2.1 W.NW 1.7 NW 0.8 SE. by S. 0.5 SE 1.4	W. by N	W. by S 0.8 West 2.0 W. by S 2.2 SW 1.9 SW 2.1	8W	W. by S
P. M. 1 2 3 4 5 6 7 8 9 10 11 12	SE	S.SE 1.1 S.SE 2.0 SE. by E 2.6 SE. by S 1.4 SE 0.5 East 0.4 SE by E 0.2	SE. by E 1.0 E. SE 1.3 SE. by E 1.3 SE. by E 1.3 SE. by E 0.5 SE. by E 0.5	SW 1.8 SW 2.3 SW 2.5 SW 1.5 SW 0.1 SE 0.4 SE 0.1 SW 1.0 W. SW 2.5 W. by S 2.1 W. SW 2.5 W. SW 2.5	SW. by 8. 2.4 SW . 2.1 SW. by 8. 2.4 SW . 2.0 SW . 2.2 SW . 0.3 N. NE . 4.4 N. NE . 1.4 W. by 8. 0.2 SW . 0.4	NE
	90th.	Alst.	224.	934.	94th.	95th.
A. M. 1 2 3 4 5 6 7 8 9 10 11	N. NW 0.6 N. NW 0.5 NW. by N. 0.1 NW. by N. 0.7 N. by W 0.7 N. by W 0.6 E. by S 1.3 SE. by E. 1.2 SE. 1.8	E. by S0.1 E. by S0.1 E. by S1.0 E. SE1.0 E. SE2.0	NW. by N . 0.3 N. NW . 0.6 N. NW . 0.4 NW. by N . 0.7 N. NW . 0.9 North . 2.4 N. NE . 3.9 N. NE . 4.6 N. NE . 4.6 N. NE . 5.0	N. by W. 0.5 NE 0.4 E. 8E 1.3 E. 8E 2.0	SW. by S0.5 SW. by S1.6 SW. by S1.7 SW. by S1.7 SW. by S0.8	SW
P. M. 1 2 3 4 5 6 7 8 9 10	SE by E 20 SE 17 E SE 16 E by S 12 E by S 10 E by S 0.5 East 0.4 E by N 0.5 E by N 0.5	ESE 21 ESE 1.7 E by N 1.5 E by N 1.5 E by N 1.5 NNE 1.5 NNE 1.5 NNE 0.3 NNW 0.3 NNW 0.3	N. NE	SE	SW 3.7 SW 1.8 SE 1.8 S.E 1.8 S. SE 2.2 SE 1.0 SE by S 1.7 SE by S 1.3 S. SE 1.0 South 1.1 S. SW 2.2 SW 2.2	8W. by 8. 42 8W
	26th.	27th.	28th.	29th.	30th.	31st.
A. M. 1 2 3 4 5 6 7 8	SW	0.1	NW	8W0.1	S. SW	8W. 34 8W. by W. 31 W. SW. 22 8W. 17 8W. 17 8W. 25 8W. 25 8W. by W. 34

<u>.</u>			JULY,	1861.		
beerval	26th.	27让.	28th.	29th.	30th.	31st.
Time of observation.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.
A. M. 9 10 11 19 P. M.	SW1.4 SW. by W. 2.5 W. SW1.8 SW1.3	NE0.5 E.8E0.9 E. by S0.9 E. by S1.2 E.NE0.9	NW4.1 NW4.2 NW4.0	SW. by S2.0 S. SW1.8 SW. by S1.7	8W. by 8 2. 3	8W. by W3.2 8W. by W3.3 W. 8W2.6
2 3 4 5 6 7 8 9	South 0. 7 0. 8 0. 7 0. 4 0. 3 0. 2	E. by 8	NW3.1 NW2.9 NW3.1 NW.by W.2.0 NW1.9 NW0.1 N.W0.4	SE. by S 2. 4 SE 1. 6 SE 1. 0 SW 1. 5 S. by W 1. 3 SW 1. 3	8W 3. 6 8W 3. 6 8W 2. 3 8W 2. 3 8. 8W 1. 2 8. by W 1. 3	SW. by W25 SW. by W23 SW. by W1.7 SW. by W0.6 SW04
10 11 12	0.5	N. by W0.7 N. by W1.5 N.NW2.6	0.0	8W0.1 8. 8W0.1	8W	8W0.5 W.8W1.7 8W. by W0.7
			AUGUS	г, 1961.		
	lst.	24.	34	4th.	5th.	6th.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12		SW. by W. 1. 0 SW. by W. 1. 0 W. SW. 1. 2 W. by S. 1. 9 W. by N. 3. 4 West. 2. 9 W. by S. 2. 5 W. by N. 2. 4	N. by E0. 9	SW. by S0. 4 SW. by S0. 2 S. SW0. 1 SW0. 1 SW0. 5 SW. by W1. 5 W. SW2. 5 SW. by W3. 1	N. by E 1.5 N. by E 1.5 N. by E 1.5 N. by E 1.8 N. NE 2.4 N. NE 2.4 N. by E 2.9 N. NE 2.0 N. NE 2.0	
P. M. 1 2 3 4 5 6 7 8 9 10 11 12	8W	E. by N 1. 0 NE. by E 1. 0 East 0. 5 North 3. 0 N. by E 1. 7 North 0. 5 North 0. 2 NE 0. 3 NE 0. 3 NE 0. 6	E. SE	8W. by W. 3.3 8W. by W. 3.0 West	E. SE0.9 E. by S1.4 SE0.5 E. SE0.5	SE
	7th.	8th.	9th.	10th.	11 th .	19th.
A. M. 1 2 3 4 5 6 7 8 9 10 11	N. by E 1.0 E. NE 0.5 NE by E. 0.1 NE . 0.1 N. by E 1.8 NE . 3.0 E. SE . 1.6 E. NE . 1.5 E. NE . 1.2 E. NE . 1.3 NE . 1.3	N. NE	North	W.JNW 0. 2 W.NW 0. 1 W. by S 0. 3 W. 8W 0. 1 N. NE 2. 7 N. NE 3. 2 N. by E 0. 0. N. NE 4. 4	N. NE	NE

do.			∆ UGU\$	т, 1861.		
Herrett	7th.	8th.	9th.	10th.	11 th.	19th.
Time of ebservation	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and re- attive velocity.
P. M. 1 2 3 4 5 6 7 8 9 10 11 12	NE	N. by E 2. 7 N. by E 3. 0 N. by E 2. 2 N. by E 2. 2 N. by E 2. 3	E. SE	N. NE	N. NE	N. NE
	13th.	14th.	15th.	16th.	17 th.	19th.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 2 3 4 5 6 7 8 9 10 11 12 2 3 4 10 11 12 12 12 14 14 15 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	N. NE. 3.9 N. NE. 3.5 N. by W. 2.0 N. NW. 1.2 N. NW. 1.2 N. NE. 4.0 N. NE. 4.0 N. NE. 4.0 N. NE. 4.0 N. NE. 4.0 N. NE. 4.0 N. NE. 4.0 N. NE. 4.0 N. NE. 3.5 NE. by N. 3.5 NE. by N. 3.7 NE. by N. 3.7 NE. by N. 4.2 N. NE. 4.0 N. by E. 3.5	N. NE0. 1 N. NE0. 1	SW. by 80.4 SW. by 81.1 SW. by 81.7 S. SW2.0 S. SW2.3 SW2.3	W. by N 20 NW. by W 2 0 West 1, 7 SW. by W . 2, 5 W. NW . 2, 5 NW. by W . 2, 9 NW 1, 6 NE 1, 5 NE . by E . 1, 1 East 0, 1	East. 0.6 E. SE . 0.5 SE . 1.2 SE . 2.8 SE . 2.7 SE . 2.8 SE . 3.0 SE . 3.0 SE . 3.0 SE . 3.0 SE . 3.0 SE . 3.0	SE
	19th.	20th.	21 st.	224.	234.	24th.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M. 1 2 3 4 5 6 7 8 8 8 9 10 11 12 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	NW. by N. 0. 1 N. NW 0. 5 N. NW 0. 8 North 1. 3 NW. by N. 1. 8 North 1. 7 NE 2. 2	N. by E 1. 5 N. by E 1. 1 N. NE . 1. 7 N. NE . 1. 8 N. NE . 2. 0 N. NE . 2. 5 NE . 1. 5	South 0. 3 South 0. 3 S. by W 0. 5 NW 0. 5 N. by W 0. 4 N. by W 0. 1 SE. by E 0. 5 SE 0. 5	NW. by N 0. 5 NW. by N 0. 9 NW. by N 1. 3 N. by W 2. 5 N. by W 2. 2 N. NE 4. 0 NE 4. 0 NE 3. 5		W. 8W 0.5 SW. by W. 0.1 W. SW 1.5 SE. by E 1.5

REPORT OF THE SECRETARY OF WAR.

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servation	19th.	20th.	21st.	22d.	234.	94th.
Time of observation.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.
9 10 11 12	North0, 1 North0, 1	SE.by E0.1 SE0.5	N. NE1. 8 N. by W1. 0 N. NW0. 8 NW0. 7			8. by E0.1
	25th.	26th.	27th.	28th.	29th.	30th.
A. M. 1 2 4 5 6 7 8 9 10 11 12 P. M. 2 3 4 5 6 7 8 9 10 11 12 12 13 4 5 6 7 8 9 10 11 12 12 13 14 15 16 16 17 18 18 18 18 18 18 18 18 18 18	SE. by S 1. 8 South 1. 0	SW 0. 1 SW 0. 1 SW 0. 3 SW. by 8 0. 8 S. SW 1. 7	SW. by W. 0.8 SW. by W. 0.8 SW. by W. 0.8 West 1.1 W. by N. 1.9 West 1.6 W. NW 1.9 SW. by W. 1.1 W. by N. 1.2 E. SE 1.7 SE 1.9 SE 1.9 SE 1.9	West	NW. by W. 0. 3 W. NW 0. 1 NW. by W. 0. 5 NW 0. 1 NE 1. 4 NE. by E. 1. 7 E. SE 1. 7	W. by 80.1 West0.1 West1.5 W. NW0.5 NW0.5 N. NW0.5 N. NW0.5 N. NE2.4 N. NE3.0
	AUGUST, 1861.		;	SEPTEMBER, 1961	•	
	31st.	lst.	2d.	3d.	4th.	5th.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M. 1 2 4 5 6 7 8 9 10 11 12 12 12 13 14 15 16 16 17 18 18 19 19 10 10 10 10 10 10 10 10 10 10	SE 0.8 SE 0.5 SE 1.9 SE 2.1 SE 2.0 SE 2.3 SE 2.2 SE 2.5 SE 2.8 SE 2.8 SE 2.8 SE 2.8	S. by W. 0.4 S. by W. 0.6 S. SW 0.9 S. by W. 0.6 S. SW 0.9 S. 1.1 SW. by S. 1.1 SW. by S. 1.4 NW 0.0 S. by W 1.4 NW 0.0 S. by W 1.5 SE 1.1 SE 4.4 SW 3.2 S. by W 3.2 S. SW 0.3 S. SW 0.3 S. SW 0.3 S. SW 0.3 S. SW 0.4 S. SW 0.4 S. SW 0.5 S	SE. by S	West 1.7 West 1.7 West 2.5 NW by W 2.6 NW by W 3.6 NW by W 3.6 NW by W 3.6 NW by W 3.6 NW by W 3.6 NW by W 3.6 NW by W 3.6 NW 5.5 NW 5.5 NW 5.5 NW 5.5 NW 5.5 NW 5.5 NW 5.5 NW 5.5 NW 5.5	East 0. 7 East 0. 9 E. by S 1. 3 SE. by E 1. 5 SE. by E 1. 5 SE. by E 1. 3	SW. by 8. 0.5 SW. by 8. 1.0 SW. 1.0 S.SW. 2.0 SW. 2.5 SW. by 8. 2.0 SW. 2.5 SW. 2.5 SW. 2.5 SW. 2.5 SW. 2.5 SW. 2.5 SW. 2.5 SW. 2.5 SW. 2.5 SW. 2.5 SW. 2.5 SW. 2.5 SW. 3.2 SW

ej O	•	SEPTEMBER, 1861.					
18178	6th.	7th.	8th.	9th.	10th.	11 th.	
Time of observation.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, an i rel- ative velocity.	
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M.	W. by 80. 5	N. by E 3.5 N. by E 3.2 N. by W 2.3 N. by W 1.9 N. by W 2.2 N. by W 2.9 North 2.9 N. by E 3.8 N. NE 3.7 N. NE 3.4	E. by N 2. 0 E. by N 1. 7 NE. by N 1. 1 NE. by N 1. 5 NE 2. 0 NE 2. 0 NE 2. 5	• • • • • • • • • • • • • • • • • • • •	SE. by E 1. 6 SE. by E . 1. 5 SE by E . 1. 5 SE 1. 0 SE 1. 0 SE 0. 5 SE 0. 6 SE 1. 9 SE 1. 9 SE 1. 9	NW. by N0.1 NW. by N0.1 NW. by S0.4 West	
2 3 4 5 6 7 8 9 10 11	NW 2.8 NW 2.4 W. by S. 2.1 West 2.5 W.SW 2.0 W. by S. 1.0 West 0.3 NW 0.8 NW 0.5 N. by E 3.5 N. by E 3.5	N. NE 3.7 N. NE 3.8 N. NE 3.8 N. NE 3.0 N. NE 3.0 N. NE 3.0 N. Oby E 2.5 N. by E 1.1 NW 1.1 NW 1.1 NW 1.0 NE by N 1.0	NE 2.5 NE 2.3 NE 2.4 NE 2.3 NE 2.5 NE 1.5 E.NE 1.5 E.NE 0.1 E.NE 0.1	SE 2.1 SE. by E 2.5 SE. by E 1.5 SE. by E 1.2 E.SE 1.2 SE. by E 2.0 E. by S 1.5 E.SE 1.4 E.SE 1.4 E.SE 1.4 E.SE 1.5 SE. by E 1.8	SE, by E 2. 5 E.SE 2. 0 E.SE 1. 5 E. by S 0. 6 North 0. 9 North 0. 9 North 1. 4 N. by W 2. 0 N. by W 1. 4 NW 0. 7 NW by N 0. 9	North	
	19th.	13th.	14th.	15th.	16th.	17th.	
A. M. 1 2 3 4 5 6 7 8 9 10 11 12	8W1.5	SW. by 8 0.5 SW 1.0 SW 0.4 SW. by 8 0.6 SW. by W . 0.5 SW. by W . 1.5 SW. by W . 1.7 SW. by W . 1.2 SW. by S 2.5 SW. by S 2.5	w. sw1.2	NW 0.6 N. by E 3.8 N. by E 3.5 N. by E 3.5 N. by E 2.3 N. by E 3.5		NW. by N . C: N. NW . L: N. by W . L6 N. by E . 24 N. NE . 23 NE . 1.7	
P. M. 1 2 3 4 5 6 7 8 9 10	8W. by S 1. 7 8W 1. 5 8W. by W. 1. 3 8W 1. 0 8W. by S 1. 5 8.8W 0. 5	SW. by S 2.9 S. SW 2.0 S. SE 2.0 SE. by S 1.3 SE	W. SW	N. by E4.3 N. NE3.3 N. NE3.1 N. NE3.1 N. NE2.2	NE. 1.2 N.NE 1.0 N.NE 0.9 E. NE 1.5 E. NE 1.0 E. NE 2.0	NE. by E. 1.2 E. by S. 0.5 E. SE 0.6 E. by S. 0.5 SE. by E. 0.9 SE 0.0 SE 0.0 SE 0.0 SE 0.0 SE 0.0 SE 0.0 SE 0.0 SE 0.0 SE 0.0 SE 0.0	
	18th.	19th.	90th.	21st.	224.	234.	
A. M. 1 2 3 4 5 6 7 8	South	SW. by S 4.0 SW. by S 2.6 SW 2.5 SW. by W. 2.6 SW. by W. 2.6 SW. by W. 2.6 SW. by W. 3.5	E.SE	N. by W	SE . 0.5 SW. by W. 0.5 SW . 1.5	W. by N 0.1 W. by N 0.5 W.NW 0.6 W.NW 0.7 W.NW 1.4 W.NW 1.3 W.NW 1.3	

log.	SEPTEMBER, 1861.							
beervat	18th.	19th.	20th.	21st.	224.	234.		
Time of observation	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.		
A. M. 9 10 11 12 P. M. 1 2 3 4 5 6 7 8 9 10 11 12	SW. by S	W.SW 28 W.SW 7 W.SW 15 W. by S 18 E, NE 1.0 N.E. by E 20 N.NE 20 N.NE 13 E. by N 1.7 E. by N 1.7 E. by N 1.7 N. by E 20 N. by E 20 N. by E 20	N. NE 3.4 NE. by E. 3.7 NE. by N. 1.7 NE. by N. 1.9 North 3.1 N. by W. 4.5 N. by W. 5.3 N. by W. 5.3 N. by W. 5.3 N. by W. 3.5 N.NW 3.5 N.NW 3.6 N.NW 3.6 N.NW 3.6 N. by W. 4.4 N. NW 3.6 N. NW 3.6 N. NW 3.6 N. by W. 4.2 N. NW 3.6 N. NW 3.6 N. by W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. Dy W. 4.2 N. NW 3.6 N. Dy W. 4.2 N. Dy W	N.NW 2.0 N.NW 1.0 N.NW 1.0 N.NW 1.0 E. by N 1.0 E. 8E 0.5 E. by S 0.5 E. 8E 0.1	W.8W 2.7 8W. by W. 2.8 8W. by W. 5.0 W.8W 4.2 W.8W 3.6 W.8W 3.6 W.8W 3.6 W.8W 3.6 W.8W 3.6 W.8W 3.6 W.8W 3.6 W.9W 1.7 W. by 8 1.7 W. by 8 1.7 W. by 8 1.5 W. by N. 1.5 W. by N. 1.5 W. by N. 1.6	W. by N0.8 W.NW0.3 E.SE0.8 E.SE1.1 E.SE1.4 SE1.5 SE. by E0.5 SE0.5 SE0.5 S. by E0.1		
	24th.	25th.	26th.	27th.	28th.	29th.		
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M. 1 2 3 4 5 6 7 8 9 10 11 12 9 10 11 12 12 12 13 14 15 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	S. by W 0.2 S. by W 0.3 S. by W 0.3 S. by W 0.3 S. by W 2.2 S. by W 2.2 S. S. W 3.5 S. S. W 3.5 S. S. W 3.5 S. S. W 3.5 S. S. W 3.5 S. by W 3.9 South 2.0 S. S. 1.1 S. W 1.4 South 1.7 South 1.7 South 1.8 S. S. W 1.0	SW0.7 SW0.7 SW1.1 S.SW0.8 S. by E1.2 SE1.1	W. by 80. 1 W.8W0. 3 W. by 81. 9 W. by 81. 7 W. by 82. 1 W. by 82. 0 W. by 82. 0 W. by 82. 0 W. by 82. 0	W. by N 0. 8 W. by N 0. 2 W. by N 0. 5 N. by W 2. 5 N. NW 2. 9 N. NW 2. 9 N. W 2. 5 NW 2. 5	NW. by W. 0. 4 NW. by W. 0. 4 West 0. 9 West 1. 1 West 1. 5 W. by N 2. 1 W. NW] 3. 0 W. by S. 2. 8 W.SW 2. 1	SW 1.0 SW. by W 0 SW. by W 0 SW. by W 0 W.SW 0.1 W. by S 0.5 West 0.4 W.NW 1.0 NW. by N 1.1 NW. by N 1.1 NW. by N 0.3 N. by W 0.3 N. by W 0.8 N.NE 0.2 N.NE 0.3 N. NE 0.2 N.NE 0.3 N. NE 0.2 N.NE 0.3 N. NE 0.3 N. NE 0.3 N. NE 0.3 N. NE 0.3 N. NE 0.3 N. NE 0.3 N. NE 0.3		
	SEPT., 1861.		·-	OCTOBER, 1861.				
	30th.	1st.	2d.	3d.	4th.	5th.		
A. M. 1 2 3 4 5 6 7 8 9 10 11	N. by E 1.0 N. by W 0.5 N. by E 0.5 N. by E. 0.4 N. by E. 0.9 N.NE . 0.7 SE. by E. 2.3 SE 1.5	SE. by E3.0 SE. by S2.2 SE. by S2.4 SE. by S2.4 SE. by S2.1 S. SE0.6 S. by E1.1	S.SW 2.5 S.SW 2.7 S.W. by S. 2.6 S.W. by S. 2.6 S.W. 2.7 S.W. 2.7 S.W. 1.6 S.W. 2.4 S.W. 3.6 W.S.W. 3.6 W.S.W. 4.6 W.S.W. 4.6	NW 0.1 NW 0.3 West 0.1 NW 0.5 NW 1.0 NW 0.5 North 1.0 NE 0.7 NE 1.6	NE. by E. 0.7 NE. by E. 0.7 NE. by N. 1.6 NE. by N. 1.6 NE. by N. 2.0 NE. by N. 0.1 N.NE. 0.0 NE. by N. 0.1 N.E. by N. 0.1 E. by N. 0.5 SEAST. 0.6	E.SE. 0.1 SW. 0.7 W.SW. 1.3 W.SW. 1.5 W. by S. 2.2 West. 3.1 W. by S. 2.2 West. 3.2 W. by S. 3.4 W. by S. 3.4		

do .	SEPT., 1861.			OCTOBER, 1861.	•	
beervat	30th.	ist.	24.	3d.	4th.	5th.
Time of observation.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and re- ative velocity.
P. M. 1 2 3 4 5 6 7 8 9 10 11	E.SE	SE. by S. 2.5 SE. by S. 2.1 S.SE. 2.6 S.SE. 1.1 S. by E. 1.2 S. by E. 2.9 S. by E. 3.0 South 3.3 S. by W. 2.7 S.SW. 2.7	W.SW 5.2 SW. by W 4.4 SW. by W 4.6 W.SW 3.6 W.SW . 3.2 W.SW . 2.2 W. by S 1.5 NW. by S 1.5 NW 0.6 NW 0.1	NE	NE. by N. 2.5 East. 1.0 E.NE 1.5 E.NE 0.6 E. by S. 0.6 East. 0.2 East. 0.1	W. by 8 . 34 W. by 8 . 32 W. by
	6th.	7th.	eth.	9th.	10th.	11th.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M. 5 6 7 8 9 10 11 12 12 12 12 13 4 4 5 6 7 8 9 10 11 12 12 13 14 15 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	NW. by N. 0.1 NW 0.4 NW 0.7 NW 0.3 NW. by W. 0.2 N.NW 0.1 NW 0.5 NW 0.5 NW 0.5 West 0.7 SE 1.3 SE 1.7 SE 2.3 SE 2.3 SE 2.3 SE 2.3 SE 2.3 SE 2.3 SE 2.3 SE 2.3	8W. 1.1 8W 1.7 8W 1.7 8W 1.9 8W 1.9 8W 1.1 8W 0.5 8W 2.7 8.8W 2.7 8.8W 2.7 8.8W 2.9 8.8W 3.1 8.8W 3.0 8.8W 3.0 8.8W 3.0 8.8W 3.0 8.8W 3.0 8.8W 3.0 8.8W 3.0	SW. by W. 0.2 SW. by W. 0.1 S. by E. 0.5 South 1.0 S.8W 18 SW. by S. 2.2 SW. by S. 3.1 SW. by S. 3.1 SW. by S. 3.0 SW. by S. 2.6 SW. by S. 2.6 SW. by S. 2.6 SW. by S. 2.6	SW. by S 8 S.SW 0.7 S.SW 0.3 S.SW 0.1 S. by W 0.8 S. by W 2.3 S. by E 3.0 SE. by S 2.0 SE. by S 2.0 SE. by S 2.0 SE. by S 1.6 SE. by S 1.6 SE. by S 1.6 SE. by S 1.6 SE. by S 1.6 SE. by S 1.6 SE. by S 1.6 SE. by S 1.0	8.8E 0.1 South 0.1 South 0.1 S.8W 0.1 S.8W 0.4 S.8E 0.5 SE 1.7 SE 2.2 SE 2.6 SE 1.5 SE 1.5 SUM 1.9 SUM 2.3 SUM 2.2 W.SW 2.2 W.SW 3.0	W.SW
	12th.	13th.	14th.	15th.	16th.	17th.
A. M. 1 23 4 56 7 8 9 10 11 12 P. M. 1 3 4 5 6 7 8	West 1.2 W.SW 0.5 SW 0.3 W. by S 2.0 West 3.5 W.NW 3.9 NW. by W 4.6 NW. by W 4.5 NW. by W 4.5 W.NW 4.5	W.8W 0.4 W.by 8 0.1 I.W. by 8 0.5 W.by N. 0.5 NW.by W.0.7 NW 0.1 W.by 8 0.5 West 0.5 South 0.5 E.SE 1.5 SE 1.7 SE by S 1.8 SE 0.4	W.SW 0.5 W.SW 0.5 W. by S 1.5 W. by N 1.7 W. by N 1.2 W.NW 0.9 N.NE 4.2 N. by E 5.0 N.NE 4.5	SW. by S0.5 SW0.5 S.S.E0.5	SW. by W. 1. 8 W. SW 1. 8 SW 0. 7 SW 0. 4 SW. by S 0. 1 S.SW 0. 5 SW 0. 1 SW. by W. 0. 8 SW. by W. 1. 7 SW 1. 8	SW. by S. 0.1 N.NE . 1.6 N.NE . 3.3 N.NE . 1.3 N. by E . 1.3 N. NE . 3.5

	OCTOBER, 1861.						
12th.	13th.	14th.	15th.	16th.	17th.		
Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.		
				8W. by 80. 1	North4. 7		
NW 0.1	SW. by 80.2		8W. by 80. 2 8W 0. 3 8W. by W 0. 5	8W. by 80. 1	North 5. 5 North 5. 1 North 5. 3		
18th.	19th.	20th.	21st.	224.	23d. 		
N. by E3. 1 N. by E3. 3 N. by E2. 8 N. by E2. 8 N. NE2. 7 N. NE3. 3	NW. by W. 4.3 NW. by W. 4.2 W.NW. 4.8	8. by E 0. 4 8E. by E 1. 1 8E. by E 1. 0 E.8E 1. 3 E.8E 1. 7	8.8E	West 2. 2 W. by N 2. 1 W. by N 1. 9 W. by N 3. 3 W. by N 3. 1 West 4. 0	W.NW 2. 5 W.NW 2. 4 West 2. 4 W.NW 2. 5 West 1. 7		
8W0.2 8W0.7 8W1.2	W. by 80.3 W.SW0.7 West0.8 W. by N0.6 W.NW0.8	SE	SE. By E 1. 5 E.SE 0. 5 NW. by N 0. 6	W.NW 3. 2 West 3. 5 W.NW 2. 5 West 1. 9 West 2. 4	W. by N1.5 W. by N1.5 W. by N0.5 W. by N0.5 W.NW0.1		
W b=N 00	8. 8W0. 8	W b= 8 0.1		R b= W7 0.4	8 h- F 1 2		
SW	S. SW	West	SE. by S. 2.3 SE. by S. 2.8 SE. by S. 2.6 S. SE. 2.0 S. SE. 0.2 S. by E. 0.1 South 0.4 S. by W. 0.1	South	8W. by 8. 1.9 8W. by W. 3.7 8W		

<u>.</u>	остові	r, 1861.		NOVEMB	er, 1861.	
Bervati	30th.	31st.	lst.	2 d.	3d.	4th.
Time of observation.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.
A. M. 2 3 4 5 6 7 8 9 10 11 12 P. M. 1 2 3 4 5 6 7 8 9 10 11 12 12 10 11 12 10 11 11 12 13 14 15 16 16 17 18 18 18 18 18 18 18 18 18 18	W. by N	W. by N 1. 0 W. by N 1. 6 W. NW 1. 9 W. by N 1. 8 W. NW 0. 6 W. NW 1. 0 W. by N 0. 5 W. by 8 0. 4	N. NE	NE. by N. 2.5 NE. by N. 3.0 NE. by N. 3.9 NE. by N. 3.9 NE. by N. 4.1 NE. by N. 4.4 NE. by N. 4.5 N. by E. 5.0 N. by E. 5.	N. W. N. W. N. W. by N. 1.3 NW. by N. 1.3 NW. by N. 1.5 NW. by N. 2.5 NW. by N. 2.5 NW. by N. 2.5 W. NW. 2.5 W. by N. 2.5 W. by N. 2.5 W. by N. 2.5 West 3.4 West 2.5	South
	5th.	6th.	7th.	8th.	9th.	10th.
A. M. 1 2 3 4 5 6 7 8 9 10 11	W. SW 0. 1 W. SW 0. 4 W. SW 0. 2 W. SW 0. 1 W. SW 0. 6 SW. by W. 0. 1 SW 0. 2 SW 0. 8	W. NW 0. 3 W. by N 0. 5 W. NW 0. 4 West 1. 1 W. by N 2. 0 W. NW 1. 1 NW 2. 0 NW 2. 3 NW 2. 3 NW 2. 3 NW 2. 3	S. SW1.3 SW. by S 2.5	NW. by N . 0.5 NW. by N . 1.1 N. by W . 2.2 N. by W . 3.2 N. by E . 2.8 N. by E . 2.8 N. N. E . 2.6	N. NW0.	8. by E 8. SE 8. SE 8. by E 8. by E 8. by E 8. by E
P. M. 1 2 3 4 5 6 7 8 9 10 11	8W. by S1.5 S1.0 S. by E1.1 S. 8W2.0 8W. by S0.2	NW. by W3. 0 NW. by W2. 8 W. NW	8. 8W	NE	E. by S 0. 6 SE. by E 0. 8 SE. by S 1. 0 S. SW 0. 5 S. by E 0. 1 South 0. 2 S. SW 0. 7	8W. by S 8W 8W 8W W. by W W. by S
	11th.	12th.	13th.	14th.	15th.	16th.
A. M. 1 2 3 4 5 6 7	West	SE0, 3 SE. by S1, 2	NW. by W0.5 NW	NE. by N. 1. 4 E. NE 0. 4 E. NE 0. 9 E. NE 1. 6 E. by 8. 1. 1 East 0. 9 East 1. 2	NW 0. 1 N. NW 0. 1 NW 0. 1 W. NW 0. 1 W. NW 0. 1 W. NW 0. 2 W. by N 0. 8	N. NW

g			NOVEMB	ER, 1861.		
eervatí.	11th.	12th.	13th.	14th.	15th.	16th.
Time of observation.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.
A. M. 9 10 11 12 P. M.	West3. 5 W. by 83. 2	SE. by S1. 5 SE. by S1. 7 SE. by S2. 4 SE. by S2. 1	NW. by N l. 6 NW. by N l. 1	E. by 8l. 1 E. by 80.9	NW. by Wl. 4 NW	NW0. 7
2 3 4 5 6 7	W. by 8 1. 5 W. by 8 1. 1 W. by 8 0. 9 W. by 8 0. 1	SE. by S 1. 2 SE. by S 1. 5 SE. by S 1. 3 SE. by S 0. 1 SE. by S 0. 1 SE. by S 0. 4	NW 0. 7 NW. by N 0. 8 N. NW 0. 6 N. NW 0. 3	E. by N. 1. 0 E. NE 1. 2 E. NE 1. 3 NE 0. 6 N. NE 0. 5 NE 0. 5 NE 0. 5	N. NW 2. 5 N. by W 2. 1 N. NW 1. 4 N. by W 0. 7 N. by W 0. 3 N. by W 0. 7	N. by W0.9
8 9 10 11 12			N. by W 0. 1 NE. by N 2. 5	NE. by N0. 1		NW. by N 0. 3 NW 0. 2 N. NW 0. 1
	17th.	18th.	19th.	20th.	21st.	22d.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 7 8 9 10 11 12 12 13 4 14 15 16 7 8 9 10 11 11 12 14 15 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	0.0 NW 0.2 NW 0.1 NW 0.1 NW 0.1 NW 0.1 N. NW 0.1 N. NE 0.5 NE by N 0.9 NE NE 0.2 N. NE 0.2 N. NE 0.2	N. by E . 0.6 N. by E . 0.4 NW. by N . 0.5 NW . 1.0 NE by N . 1.0 E.NE . 1.9 E.NE . 1.7 E.NE . 1.7 E.NE . 1.5 E.NE . 1.5 E.NE . 1.5 E.NE . 0.6 E. by N . 0.6 E. by N . 0.6 E. by S . 1.1 E. 8E . 1.5	SE	S. S. S. S. S. S. S. S. S. S. S. S. S. S	W.SW0.1 W.SW. 1.0 SE1.0 SE2.2 SE2.4 SE2.4 S.SE2.8 S.SE2.8	S.SE. 24 S.SE. 23 S.SE. 26 S.SE. 20 S.SE. 22 S.SE. 18 S. by E. 1.8 S. by E. 22 S. by E. 22 S. by E. 22 W. by E. 22 W. by E. 3.6 W. by S. 3.6 W. by S. 3.7 W. by S. 3.7
	23d.	24th.	25th.	26th.	27th.	28th.
A. M. 1 2 3 4 5 6 7 8 9 10 11 12 P. M.	W. SW . 4. 3 W. SW . 4. 5 W. SW . 4. 0 W. SW . 4. 0 W. SW . 4. 0 W. SW . 4. 1 W. SW . 4. 3 W. by S . 4. 3 W. by S . 4. 7 W. by S . 4. 9	NW	W.NW 4.3 W.NW 3.2 W.NW 3.0 W.NW 2.0 W.NW 2.0 W.by N 2.0 NW. by N 1.8 N.NW 2.2 N.Dy 2.9 NW. by W 2.1	W. SW . 0.4 SW . 1.1 SW. by S . 2.3 8. SW . 2.4	W. SW	SW. by 8 0. 2 W. 8W 0. 3
1 2 3 4	W. by 85.1 W. by 85.0 W. by 84.4 W. by 84.6	N. by W2.6 N. by W2.7 N.NW2.4 NW2.3	NW. by W .2.5 N. NW 2.4 N. NW 1.6 N. by W 1.6	8. SW 2. 8 8. by W 3. 0 8. SW 2. 4 8. SW 2. 6	W. by N4. 3 W. by N3. 7 West3. 0 W. by N2 9	South 0. 1 S. by E 2 2 SE. by S 2 3 SE. by S 2 8

		NOVEMB	ER, 1861.		
23d.	24th.	25th.	26th.	27th.	98th.
Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- a ive velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and re ative velocity.
West 4.9 W. by N4.6 W by N4.3	NW. by W .2.0 NW. by W .2.5 NW. by W .2.3 NW by W .3.0	N. by W0. 2 N. by W0. 1 N. by W0. 1 N. by W0. 4	8. by E 3. 6	W. by N1.3	8. SE
NOVEMB	er, 1861.		DECEMB	er, 1861.	
.2 9tb.	30th.	1st.	2 d.	34.	4th.
W. by N 3.9 W. by N 3.9 W. by N 3.9 W. NW 2.1 W. NW 3.6 W. NW 3.5 W. NW 3.5 W. NW 3.5 W. by N 3.5 W. by N 3.5 W. by N 3.5 W. by N 3.5 W. by N 3.5 W. by N 3.5 W. by N 3.0 W. by N 3.0 W. by N 3.0 W. by N 3.0 W. by N 3.0 W. by S 3.0 W. by S 3.0 W. by S 3.0 W. by S 3.0	W. NW .4.4 W. NW .4.8 W. NW .3.5 W. NW .1.5 W. by N .1.2 W. by N .1.2 W. by N .1.2 W. by N .1.2 W. by N .1.2 W. by S .1.0 W. by S .0.1 W. by S .0.1 W. by S .0.1 W. by S .0.3	NW. by W. 0. 6 NW. by W. 0. 3 NW. by W. 1. 0 NW. by W. 1. 2 NW. by W. 1. 0 NW. by W. 1. 0 NW. by W. 1. 0 NW. by W. 2. 0 W. NW. 2. 0 W. NW. 2. 1 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0 W. NW. 3. 0	NW. by W. 0.4 W.NW. 0.1 W.NW. 0.5 West. 1.0 W.NW. 1.4 W.NW. 1.5 W.NW. 1.4 N.NW. 1.3 NW. by W. 1.4 NW. by W. 0.5 W. by N. 1.0	West 1	SW 2 SW 2 SW 2 SW 2 SW 2 SW 2 SW 2 SW 2
5th.	вth.	7th.	8th.	9th.	10th.
W 0. 4 W 0. 1 W 0. 5	SW	SW 3.5 SW 3.6 SW 3.6 SW 3.6 SW 5.2 SW 5.2 SW 5.2 SW 5.2 SW 6.2 SW 6.2 SW 6.6 SW 6.6 SW 6.6 SW 6.5	W. SW 1.2 W. SW 2.0 W. SW 1.0 W. SW 1.0 W. SW 1.0 W. NW 1.1 W. NW 1.1 W. NW 2.9 SW 0.5 SW 0.5 SW 1.2 SW 58 SW 1.2 SW 58 SW 1.2 SW 58 SW 1.2 SW 58 SW 1.2 SW 58 SW 1.2 SW 58 SW 58 SW 1.2 SW 58 SW	SW. by S. 0.1 SW. by S. 0.5 SW. by S. 1.0 SW. by S. 1.0 SW. by S. 1.0 SW. by S. 1.6 SW. by S. 2.7 SW. 2.7 SW. 2.8 SW. 2.0 SW. 3.4 SW. 0.4 SW. 0.1	SW SW SW SW SW SW SW SW

i		DECEMBER, 1961.					
) Berrali	11th.	12th.	13th_	14th.	15th.	16th.	
Time of observation	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	
A. M. 1 2 3 4 5 6 7 8 9 10 11	NW. by N . 2.5 NW. by N . 2.8 NW. by N . 2.0 NW. by N . 2.0 NW. by N . 1.2 NW. by N . 1.2 NW. by N . 1.3 NW. by N . 1.3 NW. by N . 0.5 W. NW. by N . 0.5 W. NW. 5 W . 0.5	W. by 8	SW. by S 2. 1 SW. by S 1. 4 W. SW 2. 0 W. by S 1. 4 W. by S 0. 9 W. by S 0. 6 W. SW 1. 2	W. by 80.5 W. 8W1.1 SW. by W. 1.2 SW. by W. 1.3 SW. by W. 1.4 SW. by W. 1.3 SW. by W. 1.4 W. SW. 2.0 W. SW2.5 NW1.5 NW1.6 N. NW2.5	NE. by N 1. 2 NE. by E 1. 8 NE. by E 1. 6 E. by S 1. 6 SE. by E 2. 0 SE . by S 2. 2 S. by E 1. 8	W. 8W 2.5 W. 8W 2.7 W. 8W 3.9 W. 8W 3.8 W. 8W 4.4 W. 8W 4.8 SW. by W. 4.5 SW. by W. 5.5 SW. by W. 5.5 SW. by W. 4.1 W. 8W 4.5 W. 8W 4.5	
P. M. 1 2 3 4 5 6 7 8 9 10 11	W. by 8 1.5 W. 8W 0.5 W. by 8 0.8 W. by 8 0.6 W. by 8 0.1 W. by 8 0.9 W. by 8 1.1 W. by 8 1.1 W. by 8 0.5 W. by 8 0.5 W. by 8 0.5	8W 3.8 8W 3.6 8W 2.0 8W 2.5 8W 2.5 8W 1.7 8W 2.1 8W 2.1 8W 2.1 8W 2.2 8W 2.2 8W 2.2 8W 2.2 8W 2.2	SW. by W. 1.8 SW. by W. 1.7 SW. by W. 0.9 SW. by W. 0.5 SW. by W. 0.5 SW. by W. 2.2 W. SW. 2.6 W. SW. 2.4 W. SW. 2.4 W. by S. 1.1	N. NE	S. by E	SW. by W. 3.6 SW. by W. 3.8 SW. by W. 2.2 W. SW. 2.9	
	17th.	18th.	19th.	90th.	21st.	224.	
A. M. 1 2 3 4 5 6 7 8 9 10 11 12	E. by S	E. NE 0.5 E. NE 0.7 NE. by E. 0.7 NE. by E. 0.6 N. by E. 0.6 SE. by E. 0.6 SE. by E. 0.9 SE. by E. 0.7	W. NW 0.7 W. NW 0.4 W. NW 0.5 NW 0.6 NW 0.6 NW 0.8 NW 0.7 NW 1.9 N. NW 1.7 N. NW 1.7 NE 2.2 NE 2.4	N. NE 3.2 N. NE 3.7 NE 3.7 NE 4.1 NE by N 3.8 NE 4.1 NE 5 by N 3.8 NE 2.7 NE 2.8 NE 2.8 NE 2.4 NE 2.2	N. NW 0. 1 N. NW 0. 1 N. NW 0. 3 N. NW 0. 1 NW. by N. 0. 2 NW. by N. 0. 2 NW. by N. 0. 8 NW 0. 8	E. NE 0. h E. by S 0. 1 NE. by E. 0. 7 NE 0. 6. S. SW 1. 8 S. SW 1. 7 S. by W 2. 1 South 2. 4 S. SE 2. 6 SE 3. 1	
P. M. 1 2 3 4 5 6 7 8 9 10 11	8W. by W2.8	S. SW0. 7	NE. by E0. 6	' NE. by N2.4'	NW.by W0.1	8E, by E 3. 0	
	23d.	24th.	25th.	26th.	27th.	28th.	
A. M. 1 2 3 4 5 6 7 8 9 10 11	E. NE	W GW A O	W. NW 0. 1 N. NW 0. 3 North 0. 1 NE 0. 2 E. NE 0. 2	SW. by W. 3.0 SW. by W. 3.7 SW. by W. 2.3 W. SW. 2.0 W. SW. 1.9 W. SW. 1.9 W. by S. 1.4 West. 1.0 W. hy S. 3.4 NW. 1.8	W. 8W 3. 9 W. 8W 3. 9 W. 8W 1. 5 W. 8W 1. 5	S. SE	

rioj.	DECEMBER, 1861.								
baerva(23d.	24th.	25th.	26th.	27th.	98th.			
Time of observation	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.	Wind, from whence, and rel- ative velocity.			
P. M.	NE1.8	8W. by W1.4	E. by 80.1	8W. by W5.7	W. 8W0.5	SE. by 8L!			
2 3 4	NE. by N1. 1 N. NE 0. 4	8W. by W2.5 8W. by W2.9	South6.0 SW. by 82.8	8W. by W6. 4 8W. by W6. 3	SW0.1 SW0.1 S: SE0.1	SE1			
5 6 7 8	NW 0. 1	W. 8W1. 5 W. NW0. 8	8W. by 82.6	W. 8W5.4 8W. by W6.2	8. by W0. 1 8W. by 80. 1 8. 8W0. 1	S. SE			
9 10 11	N. NW0. 4	West1. 5 W. by S1. 4 West0. 6	8W. by 83.5 8W3.5 8W2.6	W. 8W8.0 SW. by W6.7 SW. by W7.3	8. by W0. 1 8. SE0. 6 South0. 8	!			
12		W. 8W0.5	8W2.7	8W. by W7. 5	South1. 7	1			
	29th.	30th.	31st.						
A. M. 1 2			8. by W2.0 8. by E1.0	•••••					
3 4 5 6	W. 8W0.5 W. 8W0.4 West0.2		8. by E0. 6 South0. 5 8. by E0. 4						
7 8	W. by N0. 5 W. by N0. 8		South 0. 5 S. by E 0. 6						
9 10 11	NW.bv W1.0	8. by E0.8	8W. bv 81. 3						
12 P. M. 1 2	NW.by W0.1		8W. by 81.8						
3 4 5	W. NW0.1	8. SE1. 2 8. SE1. 0	8. 8W1. 0 8W. by 80. 4						
6 7 8		8. 8E0.4	8. by W 0. 5 8. by W 0. 2 8W by S 1 8						
.9 10 11			8W1.2						
12		8. by E0. 4	8W1.3						

Calm.	201220000014111101111111111111111111111	22	627 hours.
W. by W.		28	28
'AN'N	QQQ-48884-14488	30.6	91
NW. by N.	4	21.6	15
.WW	ひょえきまましひまひひょうひひひとまるの	8	21
W. by W.	ddd:dd:dd:dd:dd:dd:dd:dd:dd:dd:dd:dd:dd	ایّا	15
W. WW.	000-1-0-0-1	17.6	Ξ
W. by M.	#80001100001000014	17.4	19
.W	- 4444400000	133	8
W. by St.	CCCCCC-	49.6	88
.W8.W		99.3	8
.W &d .W8	Q41111Q111Q10QQQQQQQQQQQQQQQQQQQQQQQQQQ	1.9	27
.W8	QQ-Q-Q-Q-Q-Q-Q-Q-Q-Q-Q-Q-Q-Q-Q-Q-Q-Q-Q	<u>2</u>	33
.8 vd .W8	114614441464444644444444444444444444444	49.1	21
.W8.8	QQQQQQQQQQQQQQQQQQQQQQQQQQQQQQQQQQQQQQ	6.09	31
.W vd .8	QQ14Q1QQQ44Q 40+44+046464000	13.9	13
.8	QQQQQqqqqq	8.	13
g. dy E.	Q11 148	2.7	က
8. SE.	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	7	7
gE. by 8.	0444646 mononum4	7.5	8
SE.	1400001441104441164100001144	828	83
8E. by E.		57.7	ਲ
.as.a	111111000414000000011111 0111410000000000	24.7	8
E. by 8.	1111111100110	7	13
.21	666	2.2	3
E. by M.	0000 0000 0000 0000 0000	Ğ. 2.2	8
E' NE'	144111600000011119411	8.6	ន
ME. by E.	-00011111444	5.4	=
ИE.	99199999999999999999999999999999999999	89.6	11
ME. by M.	1111011111	9.3	80
n' ne'		9:4	8
M. by E.	みみみみよみなよれなみななみよれれるなみなないのならよれのいい。 ちちらちゅうりりを得るおおろうちのあるようちのはまるののでする。	102.0	8
N.	dcidcddiddi	14.5	2

TABLE N .- Second reduction, September, 1861, Milwaukee, Wisconsin.

Obse	rved.			Reduced	L	
Points,	Hours dura- tion.	Relative ve. locity.	Amount.	Points.	Duration.	Amount
North N. by E N. N. N. N. N. N. N	10 36 30 8 17 11 23 8 3 13 92 29 8 7 3 13 13 13 13 13 27 33 27 33 27 33 27 33 27 33 27 33 27 33 27 33 27 33 27 33 27 33 27 33 31 31 31 31 31 31 31 31 31 31 31 31	1. 45 2. 83 2. 04 1. 16 1. 74 1. 07 1. 27 1. 11 1. 12 1. 80 1. 17 0. 94 0. 59 1. 55 1. 16 1. 97 1. 82 1. 64 1. 55 1. 12 1. 26 2. 07 1. 61	14. 5 102. 0 61. 4 9. 6 15. 4 24. 6 10. 2 14. 4 24. 7 7. 7 33. 9 49. 1 13. 9 60. 2 49. 1 11. 9 66. 2 49. 1 17. 6 1	North N. by E N.NE N.NE NE. by N NE NE NE NE NE NE NE NE NE NE NE NE NE	10. 3 37. 2 31. 0 9. 3 17. 6 11. 4 23. 7 8. 3 13. 4 22. 7 33. 1 13. 4 12. 1 28. 0 34. 1 28. 0 37. 2 39. 3 19. 7 19. 7 19. 7 19. 7 19. 7 19. 7 19. 7 19. 7	14.90
NW. by N N.NW N. by W	15 16 28	1. 44 1. 91 2. 10	21. 6 30. 6 58. 7	NW. by N N. NW N. by W	15. 5 16. 5 29. 0	31.5 60.5
Calm	79			Calm	81.7	
Sums	706	46, 62	1005. 9	Sums	730, 0	1010.

TABLE O.—Mean relative velocity of the wind at the several points of the compass, in miles per hour, 1861, Milwaukee, Wisconsin.

Points.	A pril.	Мау.	June.	July.	Angust.	September.	October.	November.	December,	Your.
North N. by E N.NE N.NE NE. by N NE. by E E. NE E. by S E. SE SE by S SE by E SE SE by S SSE SSE SSE SSE SSE SSE SSE SSE SSE S	2. 15 3. 24 3. 99 3. 31 2. 02 2. 79 2. 30 2. 44 3. 28 2. 85 2. 85 2. 81 2. 96 3. 64 3. 68 3. 64 3. 68 3. 68		1. 76 2. 03 2. 30 1. 72 1. 09 1. 83 1. 09 1. 83 0. 80 0. 80 0. 80 0. 82 0. 83 0. 83 1. 40 2. 32 2. 37 2. 46 2. 16	1. 88 3. 91 2. 86 0. 90 0. 90 0. 92 0. 92 0. 71 1. 24 0. 10 1. 13 1. 13 1. 13 1. 14 1. 13	0.98 2.94 2.57 2.39 2.69 0.77 1.10 1.76 6.82 0.50 0.50 0.60 1.37 1.33 1.33	1. 45 2. 83 2. 04 1. 16 1. 74 1. 40 1. 07 1. 17 0. 73 1. 11 1. 12 0. 59 0. 90 1. 16 1. 16 1. 17 1. 18 1. 19 1. 10	3.79 3.71 3.96 1.43 1.11 1.21 0.00 0.48 1.05 1.05 1.05 1.87 1.37 1.37 1.59 1.59 1.59 1.68	1.00 3.72 1.66 2.85 1.485 0.836 0.900 1.502 2.78 2.175 1.470 2.10 2.41 2.43	1. 25 0. 35 2. 62 9. 16 2. 14 2. 51 0. 10 0. 10 2. 10 2. 55 2. 55 1. 94 1. 23 1. 97 1. 07 1. 35 1. 37	

Mean relative velocity of the wind at the several points, &c.—Continued.

Points.	A pril.	May.	June.	July.	August.	September	October.	November.	December.	Year.
West	2, 65		2.04	1. 39	1.94	1, 14	2.38	2, 25	1.52	1. 83
W. by N	2, 09		3, 07	2. 25	1. 57	0.92	9.17	2,43	1. 29	1.97
W.NW	2, 25	l. 	1.10	2.03	0, 89	1. 26	2.31	2.58	1. 63	1.76
NW. by W	2.86		2, 27	2, 25	1.46	9.07	2, 25	2, 16	0.85	2.02
NW	2.09		1. 26	1.89	0.69	1, 61	0.70	1.03	1,94	1.40
NW. by N	1.89		2.83	0, 83	0.77	1.44	0.37	1, 21	2,11	1. 43
N.NW	1.82		1.54	0.85	0.77	1, 91	0,41	1.12	1.01	1. 18
W. by W	1. 13		0.00	1. 55	0.99	2. 10	1.20	1. 49	0.00	1.06
Sums	83, 00		46.50	48. 34	38, 80	46, 62	52. 80	61, 07	52, 13	53. 66
Means	2, 60		1. 45	1. 51	1. 21	1.46	1. 65	1. 91	1. 63	1. 68

Table O.—Duration and amount of wind at the several points of the compass for the year 1861, Milwaukee, Wisconsin.

		DUR	DURATION OF WIND	OF WIA	ē.								AMOUNT OF WIND.	OF WINT	ċ			
May.	Jame.	July.	Angust	September.	October,	Дочешрег.	ресешрет.	Year.	April.	May.	.ount	July.	Jengua	September.	October.	Долешрег.	.төбшэөөП	Year.
		8	61	2	=	!		82	۵ 0		40.4	37.7	18.6	l	<u> </u>	် ရ		231.4
		3	28	88	2			9/2	83	:	£.5	176.1	8			6.5		765.5
25	2 5	8 -	28	8 4	19	3 K	* 5	55.5	8.6		20 20 20 20 20 20 20 20 20 20 20 20 20 2	4.0	27.0	4.6	6.4		58	813. 974.3
		12,	;	1,	-			179	51.2		20.55	12.6	95.6			6.8		271.7
_		က	77	=1	-			æ j	8	:	8	ය ස්	2i'			8		888
		- K		g «	.			32	5.5		0 - d C	ລ ເ	Q 14		_	2 6		57.4
				· m	10			3 60	51.3		4	8	. K		_	9		76.8
		19	9	2	CR	_		88	38.7		6.4	13.6	7			63		99.7
8	_	7	ผ	ន	20	_		1:1	2	:	8. 8.	18.	8			-:		182.7
	_	8	12	8	6 0				52.4	:	16.1	8 8 8	12.1		_	21.1		833
5-2	_	42	2	R	<u>~</u>	_	_	346	133,8	:	99	S.	90.			æ æ		<u>S</u>
	_	=	2	80	<u>ಕ</u>	_		23	8	:	60	27	12.6		_	86		200
CN .	_	_	9	-	61	_		8	19.7		ر م	8	,			86		89.8
		cv	0 0	က	a			2	7.9	:	9:	o i	4.			37.8		25
ر د ده		Ξ:	9	E ;	a:		_	85	8	:	ල i ල් ල	13.7	공:		_	-		35 8
2		21	9	2	9:	_		8	6.5	:	36	2.5	7.7.			zo c		5
_	_	5	28	ਜ਼ ਵ	\$:			99	38	:		26	4 6			7		Š
_		3	R 8	S 8	7 8	_		7 2 2	ģ	:	9 9		200			20.00		100
_	_	35	3 2	38	3 2	_		1		:	9,6	2 2	5 6			25.		78.2
_	_	8 8	\$ 5	9 8	ŞŞ	_		18	3 8	:	ia	1	8			8		8
_		3 =	==	3 8	. 8	_		200	9		8	8	- C			9 711		338
. 28	_	22	: =	38	2	_		200	9		8	13.9	1.7			8		22
		1 10	=	15	\$	_	_	176	8		6		17.3			101.9		296.5
	_		12	7	3	_	_	8	6		2	1.2	11.6			128		38
_		9	2	15	8		_	112	2,		13.6	13.5	10.2			Š	_	38.0
		88	16	5	ଥ	_	_	213	36.4		20.	47.4	11.0			8	_	98
_		0	~	12	m			8	8		80	7.5	4		_	2	_	140.8
_	_	=	~	16	9	_	_	113	8:8		15.4	11.9	4			31.3	_	
2	_	2	13	88	-		_	90	11.3		ဗ	18.5	74.8	_		‡		7
_		8	119	2	88	_	_	 .:	:		:	:		:				
200	20	200	82	ğ	2. 2.	F	122	1000	1780 4		8	200	888	1005	1133 0	1194.8	1113.4	9215.9

TABLE P.—Comparative duration and amount of wind at the several points of the compass reduced to the mean month of 730 hours, at Milwaukee, Wisconsin, for the year 1861.

Ì	Year.	28 28 28 28 28 28 28 28 28 28 28 28 28 2
	.тоесешрет.	\$5383585858585838885888841584848888888888
	Иочешрег.	8-18878588888888888888888888888888888888
	October.	88888468888888888888888888888888888888
WIND.	September.	1.25 名の名は代になるよなの名とよる名は名は名は名は名は名は名は名の名とと名の名は 20 00 00 00 00 00 00 00 00 00 00 00 00
AMOUNT OF	JanguA	最初に対象を表れる。 1 を表現に対している。 1 を表現に対している。 1 を表現に対している。 1 を表現に対している。 2 を表現を表現を表現を表現を表現を表現を表現を表現を表現を表現を表現を表現を表現を
MY	July.	8.557 5.58 5.44 6.44 6.44 6.45 6.45 6.45 6.45 6.45
	.eant	48 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	May.	
	April.	F. 52 52 72 73 52 82 72 72 73 73 73 73 73 73 73 73 73 73 73 73 73
	Year.	250 25 25 25 25 25 25 25 25 25 25 25 25 25
	. Гресешрет.	94444444 000000000000000000000000000000
	Мотетьет.	4.1.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4.4
WIND.	.тәборет.	11000000000000000000000000000000000000
0	September.	GE:04112404234800000000000000000000000000000000
DURATION	JanguA	800 801 44 44 45 45 45 45 45 45 45 45 45 45 45
βΩ	July.	848-1-044-1-8-98-1-8-8-1-1-8-1-8-1-8-1-8-1-8-1-8-
}	.eant	84485;72941;9283;72649;425425;0284;1100000000000000000000000000000000000
	May.	್ದಿ ಪ್ರಜ್ಞೆ ಪ್ರಸ್ತೆ ಪ್ರಜ್ಞ ಪ್ರಜ್ಞ ಪ್ರಜ್ಞ ಪ್ರಜ್ಞ ಪ್ರಜ್ಞ ಪ್ರಜ್ಞ ಪ್ರಜ್ಞ ಪ್ರಜ್ಞ ಪ್ರಜ್ಞ ಪ್ರಜ್ಞ ಪ್ರಿಸ್ ಪ್ರಜ್ಞ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರಜ್ಞ ಪ್ರಿ ಪ್ರಜ್ಞ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರತ್ಣ ಪ್ರತ್ಯ ಪ್ರ ಪ್ರತ್ಯ ಪ್ರ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರಕ್ಷ ಪ್ರತ್ಯ ಪ್ರತ್ಯ ಪ್ರತ್
	April.	次される場合は、本式は成成化はたけによって、
	Points.	North N. by B. N. by B. N. by B. N. B. by B. E. by W. E. by

### Springs			DURATION	ō	WIND.			AMOUNT	Ö	WIND.			VELO	VELOCITY OF WIND	GAD.	
28.5 38.0 19.9 8.1 2.0 18.5 18.5 18.5 18.5 18.5 18.5 18.5 18.5	· Polnta.	Spring.	.remmu8	.amntaA	Winter.	Year.	Springs.	.Teatant 8	-amminA	Winter.	Year.	Spring.	Summer.	.ampinA	Winter.	Year,
23.5	North	84	91.9	80	9	전		34.33	19.91		30.27		1.540	080 %		
22.1 11.7 14.9 13.0 13.8 13.8 13.8 13.8 13.8 13.8 13.8 13.8	N. by E.		6.7	19.5	9 7	8 4		8 8 8 8	2.2 2.4	c, % 5 %	8.8 8.8		9 9 576 576	다 야 왕 장	드 04 전 명	이 어 당 한
11.2 87.4 87.5 11.5 87.5 11.5 87.5 11.5 87.5 11.5	NE. by M.	ន់ន	ri 8	14.9	120	15.4		21.57	83.5		88		1.553	1.743		
25.0 1.0	NE DVE	1.2	9 00	7 CE	15.9	9		3.5	11.11		16.55		1.589	38		
25.2 11.6 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7	E. NE	8,5	8 7	16.7	8 6 7	15.6	:	56	19.85		29.06		1. 240	1.233		
14.9 14.9 14.9 15.6 10.0	East	- R	11.8	۰ د د د	o o	10.0		3 SS	; 4		10.08		0.596	38		
28.9 16.9 15.4 8.0 18.8 15.9 15.4 8.0 18.8 15.9 15.4 16.4 16.4 16.4 16.4 16.4 16.4 16.4 16	E. by S	9 6 6	14.9	5.0	Ø 6	10.5		8, 8	2.5 8.8		19, 92	:	0.746	0.00		
28.3 61.2 22.1 8.0 41.2 2.2 18.0 14.2 2.2 18.0 14.2 2.2 18.0 14.2 2.2 18.0 14.2 2.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 14.2 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.0	SE. by E.	68	16.9	15.4	9 00	9 6		18	. S		3 81 3 81		1.160	. 69		
10	20E	% :	2.5	- c	8;	9.7	:	865	20.5		8	:	1.313	2000		
4.1 4.1 14.1 11.0 8. 6.4 16.4 16.4 16.4 16.4 16.4 16.4 16.4	25. 0y 32.	걸 너	# ~ i id	18.5	19.0	1 2		2 8	2 K		8 8		0.803	1.43		
6.6 8.7 8.7 1.15 1.0 6.8 8.3 1.15 1.0 6.8 8.3 1.15 1.0 6.8 8.3 1.15 1.0 6.8 8.3 1.15 1.0 6.8 8.3 1.15 1.0 6.8 8.3 1.15 1.0 6.8 8.3 1.15 1.0 6.8 8.3 1.15 1.0 6.8 8.3 1.15 1.0 6.8 8.3 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0	S. by E	40	7	17:	1.0	ص ده د		25	8 :		11.86		9	1.43		
10.6 25.2 25.3 25.5	South 8. by W	•	3 ed	11.5	20.0	, O;		38	16.22		. 4 8		38	3.5		
20.0 53.6 55.6 55.6 55.6 55.6 55.6 55.6 55.6	8.8W	_	8 8	g	15.9	8		82	2.0		45.89		1.453	1.883		
11.4 24.9 19.7 73.7 25. 18.0 18.0 18.0 18.0 18.0 18.0 18.0 18.	SW by S	o o	. 8	\$ X	113.5	4 G		12.5	3.1		\$ 88 8 55		1.530	1.856		
77.9 14.1 29.0 13.0 34.1 29.0 13.0 34.1 29.0 13.0 34.1 29.0 13.0 34.1 29.0 13.0 34.1 29.0 13.0 34.1 29.0 3	SW. by W	11:4	2,5	19.7	E	8		47.96	8		8		1.930	3		
77.9 14.1 28.0 13.0 84. 28.0 13.0 80. 28.0 13.0 80. 28.0 10.9 34.2 10.0 80. 28.0 13.3 6.7 17.9 17.9 13.0 80. 28.0 11.0 11.0 6.6 13.0 13.0 13.0 13.0 13.0 13.0 13.0 13.0	W. bw 8	48	15.5	37.7	9 8	2. 2.		20.5	38				386	25.5		
7. 3 24. 3 2	Went	6.6	14.1	8	13.0	2		25 25	38.63		4.7		1.556			
7. 13.3 6.7 17.9 17.9 17.9 17.9 17.9 17.9 17.9 17		3.8	3 0	× -	⊃ 00 ⊃ 00 ⊃ 00	3.3		2 2	3.5		5.5 2.5 3.5		2 2	5 5		
7.	NW. by W	13.3	6.7	17.9	17.9	13.1		13.16	86.99		24.1			1 of 3 3		
7. 11.4 10.8 17.0 11.0 13. 13. 13. 13. 13. 13. 13. 13. 13. 13.	WW.	ਝ ਵ ਵ	9.9	8 8 8	8 9	સં :	:	2.52 2.53	88		න : සූ :		283			
77 34.5 109.5 116.7 194.4 96.		=======================================	10.8	17.0	11.0	12		35	. S		17.8			1.146		
38 mas 730, 0 730, 0 730, 0 730, 0 730, 0 730, 0	N. by W	~ Z	9.00	180	0.0	9.5		11.30	8. 8.		19 16		0. 846	1.596		
730.0 730.0 730.0 730.0 730.						1										
	Bumi	E	730.0	730.0	730, 0	730.0		1057. 23	1129, 50	1106.64	1169, 33		44. 529	53. 18 8	52. 13	23
	Мевля												1.360	1. Get	3.	38

Table R.—Reduction of the winds to the four cardinal points of the compass, September, 1861, Milwaukee, Wis.

	No	rth.	E	ust,	801	ath.	W	est.
Points in degrees.	Duration.	Amount.	Duration.	Amount.	Duration.	Amount.	Duration.	Amount
Due	10.00	14, 500	3, 00	2, 200	13.00	20, 100	22.00	25, 100
1 +		100, 062	7.02	19, 892	11.77	13, 635	2.34	2, 711
2 +		56, 724	11.48	23. 496	28, 64	56, 262	11.86	23, 304
3 +	6. 65	7. 734	4.45	5. 167	22, 45	40, 827	15.00	27, 276
5		20.928	12 02	20. 928	23, 33	38, 259	23, 33	38, 259
6 +		8, 560	9. 15	12.811	15.00	23, 276	22. 45	34. 840
7 +	8.80	9.411	21, 25	22, 730	13.88	25. 323	33. 26	61. 158
8 +		1.989	7.85	10.006	7.41	9. 673	37. 27	48. 645
8 + 		3, 394	12.75	14. 125	2.54	2.808	18.64	17.068
7 +		6. 739	20, 33	22. 823	8. 42,	9, 449	12.94	16. 261
6 +		17. 282	26. 61	47. 983	17.78	32.058	12.48	25.865
5	14.85	23, 898	20. 50	23, 967	20. 50	23. 967	14.85	23.898
3 +		17. 961	4. 45	4. 167	6.65	6. 237	8. 34	11.999
2 +		28. 274	2. 68	1. 569	6.47	3.788	6. 13	11.710
1 + ··········	27.47	57. 553	0. 59	0. 529	2.94	2. 649	5. 48	11. 458
Sums	195, 17	375, 009	164, 13	273, 795	200.78	308, 311	246, 37	379. 552

TABLE S.—Resultant direction, mean progress, and mean relative velocity of the wind for the several months of 1861, Milwaukee, Wis.

		Fro	m the dur	ation.	· From the a	mount.
Months.	Wind	fron	whence.	Mean pro- gress.	Wind, from whence.	Mean relative velocity.
April	N. 20 N. 3 S. 39 N. 37 S. 86 S. 56 N. 79	36 35 42 05 05 04 19	52 E		N. 47 14 35 E N. 39 58 52 W.S. 75 57 32 W.N. 29 16 32 E.N. 57 45 43 W.S. 69 30 27 W.S. 69 30 27 W.S. 68 62 32 33 W.	1. 71 1. 76 1. 76 1. 63 1. 86 2. 07
Year	8. 80	17	43 W	0. 095	N. 76 36 45 W	1.71
Year omitting April	8. 75	10	35 W	0. 123	N. 87 24 54 W	1. 57



TABLE U .- Storms during the different months of 1861 at Milwaukee, Wis.

			i i		lativ locit	e ve-					ative locity	
Dat	e.	Wind, from whence.	Duration hours.	Maximum.	Minimum.	Mean.	Date.	Wind, from whence.	Duration hours.	Maximum.	Minimum.	Mean.
8, 9. 11. 15, 17. 18. 20. 23. 24. 25. 26. 30. June 15. 10. 12. 25. 24. 44. 15. 10. 11. 12. 12. 13. 14. 15. 16. 11. 19. 11. 11. 11.	9	8W. to W. N. NE. SW. to W. SE. to S. South NW. to W.NW. SW. SW. to W. N. NE. N. to N. NE. North NOrth SW N. NE. N. NE. N. NE.	9 13 9 7 3 36 4 8 9 6 3 17 3 5 5 4 8 15 14 10 10 7 11 3 6 8 8 12 7	6.6 6.2 4.4 5.5 4.5 5.5 4.5 5.5 6.8	3.9 3.4 4.1 3.4 4.5 4.1 3.6 4.1 4.1 4.1 4.1 4.1 4.1 4.1 4.1 4.1 4.1	5.02 4.213 4.13 4.53 4.95 4.95 4.65 5.21 5.21 5.21 6.34 6.34 6.34 6.34 6.34 6.34 6.34 6.34	Aug. 13 Sept. 15 18 20 22 Oct. 9 11 12 14	N. NE. S. SW North W. SW W. SW W. SW W. NE. N. NE. to N W. NW SW West. N. NE S. SW SE. to S. SE W. SW W. NW W. NW W. NW W. NW W. NW W. NW N. NE W. SW W. NW W. NW W. NW W. NW W. NW W. NW W. NW W. NW W. NW W. NW W. NE W. SW	3 4 3 4 3 5 7 9 9 4 17 4 5 3 14 5 21 4 3 6 6 4 7 7 11 3 9 3 11 17	4.833022200088324557325593155250	34343433715035098	· 1945年,1945年,1945年,1945年,1945年,1945年,1945年,1945年,1945年,1945年,1945年,1945年,1945年,1945年,1945年,1945年,19

TABLE V.—Storms in the year 1861, Milwaukce, Wis.; compiled by their direction.

			Re	1-41-			1		1		
i		Ē		locit	e ve- y.			Ë		lativ locit	
Date.	Wind, from whence.	Duration hours.	Maximum.	Minimum.	Mean.	Date.	Wind, from whence.	Duration hours.	Maximum.	Minimum.	Meau.
12. Sept. 20. July 1. Oct. 17,18. April 8,9 9. 11. 15,16. 23. June 16. Aug. 10. 11. 12. Sept. 15. Oct. 14. Nov. 2. Dec. 14. Aug. 12. Dec. 20. 22,23. April 4. 6. 25. Nov. 19.	North	7 11 4 10 17 9 7 3 36 3 10 8 8 12 3 4 4 4 14 3 7 3 1 1 9 1 1 9 1 1 9 1 1 9 1 1 1 1 1 1 1	6. 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4.04.4.4.4.3.7 3.3.4.11 4.02.4.2 3.3.6 4.02.4.2 3.3.6 3.6	4. 76 5. 67 5. 41 5. 05 4. 21 3. 93 4. 53 4. 53 5. 34 5. 71 4. 67 4. 67 4. 67 4. 67 4. 60 3. 93 4. 63 4. 63 4. 61 4. 61	April 26. Sept. 18. Nov. 10. April 20. June 14. July 25. Oct. 29. June 15. Sept. 22. Oct. 2. Nov. 22. Dec. 16. Nov. 23. Oct. 30. Dec. 10. Oct. 12. 19. Nov. 25. 29. 30. April 29. April 39. April 39. April 39. April 30. July 28.	Sonth	4 3 5 9 15 3 5 17 6 6 17 14 3 5 5 7 4 9 9 1 3 6 6 7 4 6 6 6 8	4.3 5.0 4.8 4.3 5.9	2422222444242424242424242444	5.12.13.13.13.13.13.13.13.13.13.13.13.13.13.

TABLE W.—Showing rain and snow, with different winds, at Milwaukee, for the years 1861, 1862, 1863, and 1864.

1861.	
17	0.9 0.8 0.5 0.3 1.5 0.3 0.3 0.9 1.5 0.3 0.9 1.5 0.3
17	2.0 0.8 0.5 0.3 1.5 1.5 0.9 1.5 3.6 1.0 5.5
17	0.5 0.3 0.5 0.5 1.5 1.8 0.3 0.9 1.5 3.6 1.0
17	1.5 0.5 1.8 0.3 0.3 1.5 1.5 1.5 5.5
17	
7 0.24 SW. 29 0.01 8 0.12 West. 31 0.14 11 0.58 N. by E April 1 0.53 13 0.01 North 3 0.25 16 0.12 SW 7 0.88 18 0.29 SW 11 0.74 23 0.06 SE to SW 11 0.74 25 0.27 SE 15 2.26 E.SE 26 1.04 SW 15 2.26 E.SE 28 0.02 Calm 26 0.20 N.NE 28 0.02 Calm 26 0.20 N.NE 11 0.63 NE 11 0.63 NE 11 0.63 NE 11 0.63 NE 12 0.08 SW 15 0.18 E.SE 30 0.76 SE 30 0.76 SE 30 0.76 SE 31 0.76 SE 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.77 0.57 West 31 0.78 S.SW 30 0.76 SE 31 0.79 S.SW 30 0.70 S.SW 31 0.70 S.SW 31 0.71 0.55 SW 31 0.75 S.SW 31 0.76 S.SW 31 0.77 0.58 S.SW 31 0.78 S.SW 31 0.79 S.SW 30 0.62 NE 31 0.03 NW 30 0.62 NE 31 0.03 NW 31 0.06 N.NE 31 0.03 NW 31 0.06 S.SE 32 0.06 SE 33 0.07 S.SW 34 0.07 S.SW 35 0.00 S.SE 36 0.00 S.SE 37 0.06 S.SE 38 S.SW 39 0.53 S.S 30 0.62 NE 30 0.62 NE	1.5 1.8 0.3 0.9 1.5 3.6 5.5
7 0.24 SW. 29 0.01 8 0.12 West. 31 0.14 11 0.58 N. by E April 1 0.53 13 0.01 North 3 0.25 16 0.12 SW 7 0.88 18 0.29 SW 11 0.74 23 0.06 SE to SW 11 0.74 25 0.27 SE 15 2.26 E.SE 26 1.04 SW 15 2.26 E.SE 28 0.02 Calm 26 0.20 N.NE 28 0.02 Calm 26 0.20 N.NE 11 0.63 NE 11 0.63 NE 11 0.63 NE 11 0.63 NE 12 0.08 SW 15 0.18 E.SE 30 0.76 SE 30 0.76 SE 30 0.76 SE 31 0.76 SE 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.76 S.SW 31 0.77 0.57 West 31 0.78 S.SW 30 0.76 SE 31 0.79 S.SW 30 0.70 S.SW 31 0.70 S.SW 31 0.71 0.55 SW 31 0.75 S.SW 31 0.76 S.SW 31 0.77 0.58 S.SW 31 0.78 S.SW 31 0.79 S.SW 30 0.62 NE 31 0.03 NW 30 0.62 NE 31 0.03 NW 31 0.06 N.NE 31 0.03 NW 31 0.06 S.SE 32 0.06 SE 33 0.07 S.SW 34 0.07 S.SW 35 0.00 S.SE 36 0.00 S.SE 37 0.06 S.SE 38 S.SW 39 0.53 S.S 30 0.62 NE 30 0.62 NE	0.3 0.9 1.5 3.6 1.0
7 0.24 SW	0.9 1.5 3.6 1.0 5.5
7 0.24 SW	3. 6 1. 0 5. 5
7 0.24 SW	5.5
7 0.24 SW	
7 0.24 SW	2.9 0.2
7 0.24 SW	
11	
16	1
18	
25	
Aug. 6 0.78 NE 30 0.76 SE 30 0.76 SE 31 0.64 NE 15 0.18 E.SE. 15 0.18 E.SE. 31 0.76 SSW 20 0.15 N.NE 17 0.58 WE 20 0.15 N.NE 17 0.58 WE 20 0.15 N.NE 20 0.15 N.NE 20 0.15 N.NE 20 0.15 N.NE 20 0.15 N.NE 20 0.15 N.NE 20 0.15 N.NE 20 0.16 N.NE 20 0.16 N.NE 20 0.17 N.NE 20 0.18 E.SE. 20 0.19 North 20 0.53 SW 25 0.30 S.SW 25 0.30 S.SW 25 0.30 S.SW 25 0.30 S.SW 25 0.30 N.NE 20 0.19 North 20 0.03 NW 20 11 1.08 SW 20 0.62 NE 20 0.60 NE 20 0.60 N.NE 20	4.5
Aug. 6 0.76 NE 30 0.76 SE 11 0.63 NE 15 0.18 E.SE 15 0.18 E.SE 15 0.18 E.SE 15 0.18 E.SE 16 0.57 West 17 0.58 NE 17 0.58 NE 17 0.58 NE 17 0.58 NE 17 0.58 NE 18 0.76 S.SW 20 1.12 NE 17 NE 17 NE 17 NE 17 NE 17 NE 17 NE 17 NE 17 NE 17 NE 17 NE 17 NE NE 17 NE NE 18 NE 18 NE 19 0.53 NE 19 0.53 NE 19 0.53 NE 19 0.54 NE 19 0.57 NE 11 1.08 SW 12 0.57 SW 12 0.57 SW 12 0.57 SW 12 0.57 SW 12 0.57 SW 12 0.66 N.NE 15 0.48 NE 15 0.48 NE 16 0.69 E.SE 16 0.69 SW 18 NE 16 0.69 E.SE 16 0.69 SW 16 0.69 SW 16 0.69 SW 16 0.69 SW 16 0.69 SW 16 0.66 NW 16 0.66	1.0
11	0.5
18	1.4 0.4 0.2 0.9
Sept. 1 0.76 8.8 W 20 1.12 NE 6 0.02 8W 29 0.90 8.8 W 9 0.53 8E 30 0.62 NE 10 0.03 NW June 11 1.08 8W 12 0.57 8W 12 0.16 N.NE 15 0.08 NE 13 0.41 N.NE 19 0.94 N.NE 15 0.48 NE 24 0.18 8W 16 0.09 8.8 E 25 0.02 West 17 0.02 8W 28 0.01 8W 19 0.66 NW 29 0.03 North 27 0.76 6.8 E 20 0.03 SE July 7 1.19 NW 20 0.63 NE 8 0.19 8W 3 0.03 NE 10 0.66 NW 2 0.63 NE 8 0.19 6.8 W 3 0.06 NE 12 0.06 3W 9 0.11 8E 17 0.40 NE 9 0.11 8E 17 0.40 NE 10 0.10 8E 18 1.10 8E	0.2
6 0.02 North 29 0.90 E.SE. to NE. 9 0.53 SE 30 0.62 NE. 10 0.03 NW June 11 1.08 SW 15 0.06 NE. 12 0.16 N.NE. 19 0.94 N.NE 15 0.48 NE. 24 0.18 SW 16 0.09 E.SE. 25 0.02 West 17 0.02 SW 28 0.01 SW 19 0.66 NW 29 0.90 E.SE. 0.02 SW 17 0.02 SW 18 0.05 SW 19 0.66 NW 19 0.70 O.66 NW 29 0.90 E.SE. 0.00 SW 19 0.06 NW 19 0.70 F.SE. 0.06 NE 10 0.03 SE 10 0.03 SE 11 0.03 SE 12 0.06 SW 13 0.06 SW 14 0.07 SE 15 0.06 SW 17 0.06 SW 18 0.19 SW 19 0.11 SE 17 0.00 SW	0.5
9 0.53 SE 30 0.62 NE 11 1.06 SW 12 0.67 SW 12 0.16 N.NE 12 0.16 N.NE 15 0.08 NE 15 0.48 NE 15 0.48 NE 15 0.48 NE 15 0.48 NE 15 0.48 NE 16 0.09 E.SE 17 0.06 NW 19 0.56 NW 19 0.66 NW 19 0.66 NW 19 0.66 NW 19 0.66 NW 19 0.66 NW 19 0.66 NW 19 0.66 NW 19 0.66 NW 19 0.66 NW 19 0.66 NW 19 0.66 NW 19 0.66 NW 19 0.66 NE 12 0.63 NE 19 0.66 NW 19 0.66 NE 12 0.63 NE 19 0.66 NW 19 0.66 NE 12 0.63 NE 19 0.66 NW 19 0.66 NE 12 0.66 NW 19 0.11 SE 19 0.11 SE 17 0.06 SW 19 0.11 SE 17 0.06 SW 17 0.06 SW 19 0.11 SE 18 1.00 NE 18 1.00 NE 19 0.00 NE 19 0.01 SE 18 1.00 NE 18 1.00 NE 19 0.00 NE 18 1.00 NE 19 0.01 SE 18 1.00 NE 18 18 1.00 NE 18 18 1.00 NE 18 18 1.00 NE 18 18 1.00 NE 18 18 1.00 NE 18 18 18 18 18 18 18 18 18 18 18 18 18	0.4
12	1.9
19	2.2 1.0
24 0.18 SW 16 0.09 E.SE 26 0.02 West 17 0.02 SW 28 0.01 SW 19 0.66 NW 30 0.33 North 27 0.76 E.SE Oct. 1 0.03 SE July 7 1.19 NW 2 0.63 NE 8 0.19 SW 3 0.06 NE 12 0.06 SW 9 0.11 SE 17 0.40 NE 21 0.01 SE 18 1.10 SE	3.0
28 0.01 8W	2.1
Oct. 1 0.03 SE. July 7 1.19 NW. 2 0.63 NE. 8 0.19 SW. 3 0.06 NE. 12 0.06 SW. 9 0.11 SE. 17 0.40 NE. 91 0.01 SE. 18 1.10 SE.	0.5
2 0.63 NE 8 0.19 SW 9 0.11 SE 17 0.40 NE 18 1.10 SE 18 1.10 SE	0.5
9 0.11 8E	0.6 1.2
#1 U.UI SE 18 1.10 SE	0.9
28 0.29 South	0.4
21 0.01 SE	1. 2
16 0.86 SE	0.5
20 0.15 8E. 4 0.15 8E. to W.NW. 21 0.10 8E. 8 0.05 W.NW. 22 0.08 W.SW. 10 0.16 8E.	0.6
22 0.08 W.SW	0.5
27 0.04 8. by E	0.5
29 '0.18 W.NW	0. 5 0. 4
6 0.37 8W 4 0.55 8W 21 0.85 Calm 5 0.41 Calm	0.5 0.4 0.5 0.8 2.1
21 0.85 Calm 5 0.41 Calm 22 0.02 East 10 0.59 W.8W	0.5 0.4 0.5 0.8 2.1 1.8
1862.	0.5 0.4 0.5 0.8 2.1 1.8
Jan. 2 0.45 NW 2.3 16 0.42 E.8E 7 0.12 W.SW 0.2 22 0.64 SW 10 0.21 NW 2.5 27 0.72 N.NE 11 0.16 SW 1.0 29 0.37 N.E 14 0.22 E.SE 2.0 30 1.40 NE 16 0.17 Calm Oct. 5 1.04 8.8E	0.5 0.4 0.5 0.8 2.1 1.8
10 0.21 NW	0.5 0.4 0.5 0.8 0.8 0.8 0.4 0.4 0.8 0.8 0.4 0.4 0.4 0.4 0.4 0.4 0.4 0.4 0.4 0.4
14 0.22 E.SE	0.4 0.5 0.8 2.1 1.8 0.8 3.4 0.4 0.4

Rain and snow with different winds, &c.—Continued.

Day of month.		Amount of rain, &c., in inches.	Direction of wind.	Mean relative ve-	Day of month.	Amount of rain, &c., in inches.	Direction of wind.	Mean relative ve-
186 Oct.	6	0. 13	sw	2.2	1963. Aug. 9	0. 02	w.sw	
	7	0. 46 0. 13	N.NE	3.5 2.5	12 14	0. 05 0. 28	SE	14
	22 23	0. 04 0. 06	NW	2.2 1.4	21 22	0. 15 0. 55	SW	1 14
Nov.	1	0. 14	NE	2.1	23	0. 35	SEW.SW	į į:
	5	0. 04 0. 04	S.SW	1.3 3.5	Sept. 6	0. 15 0. 10	S.SE	L Ci
	10 13	0. 28 0. 05	8W	0.8 0.1	16	0. 44 0. 42	E.NE. S.SW	1 4
	15	0. 50	Calm		20 30	0.06	N.NE	. 1.4
	26 29	0. 07 0. 16	8W 8.SW	2.1 0.6	Oct. 1	0. 04 0. 16	West. W.NW	1 :
Dec.	12 14	0. 39 0. 09	E.SE	0.7	2 3	0. 08 0. 33	West 8E. to W.NW	
	20 23	0. 04 0. 31	8.SE	1.8 1.8	7 16	0. 04 0. 35	SWE.NE	i Li
	25	0. 54	E.SE W.SW	0.4	21	0. 57	NW	
186 Jan	1	0. 26	s.sw	4.3	23 28	0. 02 1. 10	NW. W.NW. 8.8E	
	2 4	0. 70 0. 13	8.8W	3.8 2.5	Nov. 1	0. 28 0. 15	SE	1 2
	12 14	0. 77 0. 02	PNP	1 1 0	3 12	1. 93 0. 50	8.8W	
	18	0. 66	W.NW. 8.SW. Calm. W.SW.	0.9	13	0.05	East	1 2
	22 28	0. 73 0. 06	W.SW	0.4	14 18	0. 39 0. 08	N.NW 8.8W	
Feb.	8	0. 20 0. 56	CalmEast	0.8	23 28	0. 34 0. 07	E.NE. N.NW	1
	13 18	0. 58 0. 15	S.SE	0. 9	Dec. 9	0. 18 1. 15	N.NW. N.NW to 8E N.NE.	- 1
	25	0. 36	N.NW	0. 1	16	0, 98	E.NE	. 5
Mar.	1 4	0. 70 0. 03	W.NW	0.2	17 25	0. 02 0. 22	E.NE	. 0
	7 10	0. 04 0. 03	Calm	0.5	26 27	1. 19 0. 05	E.SE. to E.NE	. =
	16	0. 02 0. 50	SE E.SE	0.1	30 1864.	0. 78	N.NW	- 1
	19 21	0.51	Calm		Jan. 3	0. 11	w.sw	. 1.
	22 28	0, 64 0, 01	SE N.NE. to N.NW.	0.5 3.1	13 29	0, 14 1, 00	NE	. 1
A pril	9	0. 23 0. 23	S.SE	1.6 0.9	Feb. 4	0. 90 0. 19	Calm NW	; 1
	19	0, 43 0, 07	NE	1.1	23 25	0, 01 0, 22	West	.! 2
	20 21	0.08	8E	1.4	Mar. 3	0. 23	8.SW	. 1.
May	3	0. 32 0. 56	NE	2.8 2.1	9 10	0.76 0.56	NE	. 1
	4 9	0. 71 0. 75	NE	2.7 1.1	27	0. 92 0. 05	N.NE	1 6
	10	0. 90	NE SW. to N.NE	2.0	April 3	0. 48 0. 04	N.NE.	1 1
	12 21	0. 01 0. 36	NW	1.0	6	0. 18	N.NE.	.
	22 24	0. 06 0. 06	SE W.NW	0.7 0.5	10	0. 57 0. 73	N.NE. East NW. W.NW.	1
	27 28	0, 07 0, 92	NE	2.5 1.5	12 21	0. 11 0. 15	W.NW N.NE.	. 1 "
	29	0. 25	8E	0.6	23	0. 24	NE. N.NE.	. 1 4
June	30 4	0, 24 0, 04	NNW NE W.NW	1. 1 1. 0	28	0. 26 0. 25	E.SE	0.
	20 22	0. 03 0. 14	W.NW	1.6 0.8	30 May 5	0. 01 0. 44	N.NE	2
July	28	0. 58 0. 01	8E	0.7	11	2, 26 0, 01	N. to NE	a
uiy	18	0. 96	SE	1.3	22	0. 01	Calm	a
	23 24	0. 20 0. 25	W.NW	1.9 0.8	31	0, 01 0, 03	W.8W	1.
	27 28	0. 15 0. 01	NW	1.7 1.3	June 30 July 4	0. 62 0. 14	N.NE	0.
	30	0. 47	NE	0.1	6	0.58	N.NEW.NW	. (
Aug.	30	0. 36 0. 50	8E	1. 2 1. 3	16	0. 13 1. 47	E.SE	۱۹
-	6	0. 21 0. 36	8.8E W.8W	0.2	17	3. 20 0. 51	South	

Rain and snow with different winds, &c.—Continued.

Day of month.	Amount of rain, &c., in inches.	Direction of wind.	Mean relative ve-	Day of month.	Amount of rain, &c., in inches.	Direction of wind.	Mean relative velocity.
1864. July 30 Aug. 10 11 22 Sept. 2 8 12 13 16 21 22 28 29 Oct. 4 16 16 20 20 25	0. 42 0. 15 0. 08 0. 23 0. 42 0. 41 0. 15 0. 78 0. 09 0. 23 0. 27 0. 20	8W	1. 6 1. 6 1. 8	1864. Oct. 27 Nov. 3 5 7 13 24 26 27 Dec. 1 4 6 9 9 10 12 14 15 17 20 25 27	0. 17 0. 29 0. 12 0. 02 0. 50 0. 03 0. 04 0. 60 0. 23 0. 12 0. 18 0. 18 0. 12 0. 18 0. 02 0. 13 0. 09	8.8W N.NW S.SE N.NE Calm Calm W.8W E.SE Calm E.SE Calm Calm Calm S.SE Calm Calm NW S.SE Calm Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm NW S.SE Calm S.SE S.SE Calm S.SE S.S	0. 3 0. 5 1. 1 0. 2 0. 2 0. 1 0. 6

TABLE X.—Showing the evaporation and humidity for different winds at Milwaukee, for 1862, 1863, and 1864.

Year and day of month.	Mean temperature.	Mean humidity.	Meso evaporation.	Mean cloudiness.	Direction of wind.	Relative velocity.	Year and day of month.	Mean temperature.	Mean humidity.	Mean evaporation.	Mean cloudiness.	Direction of wind.	Relative velocity.
1862. May 1 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 90 21 22 23 - 24 25 26 27 28 29 30 31 June 1	45 44 53 50 46 49 55 55 56 65 58 72 48 57 56 66 46 46 46 46 46 46 46 46 49 50 68 49 49 50 68 49 49 50 68 49 50 68 50 68 50 68 50 68 50 68 50 68 50 68 50 68 50 68 50 68 50 68 50 68 50 68 50 50 68 50 50 68 50 50 68 50 50 50 50 50 50 50 50 50 50 50 50 50	.770 .744 .673 .477 .614 .493 .638 .816 .435 .573 .454 .517 .715 .755 .692 .464 .453 .762 .706 .649 .481 .587 .581 .581 .581 .581 .581 .581 .581 .581	.019 .059 .139 .100 .137 .188 .191 .165 .244 .202 .288 .214 .115 .281 .146 .147 .220 .232 .232 .233 .141 .164 .240 .240 .241 .240 .241 .240 .241 .240 .241 .240 .241 .240 .241 .240 .241 .240 .241 .241 .241 .241 .241 .241 .241 .241	8 7 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	W. SW. N. NE. SE. N. NE. E. SE. N. NW. E. SE. SW. N. NE. NE. NE. NE. NE. NE. NE. NE. NE. NE.	0.3 1.0 2.0 1.5 2.0 1.0 0.4 0.0 0.4 0.2 0.2 0.1 0.3 0.6 0.2 0.1 0.3 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6	1862. June 2 3 4 5 6 7 8 9 - 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 27 28 29 30 July 1 2 3	54 51 558 63 54 56 70 77 77 71 54 53 59 70 52 55 64 65 65 64 66 66 66 66 66 66 66 66 66 66 66 66	.512 .572 .420 .493 .530 .536 .584 .452 .462 .462 .462 .463 .703 .701 .706 .705 .706 .706 .706 .707 .706 .707 .706 .707 .706 .707 .706 .707 .706 .707 .707	. 235 . 168 . 232 . 175 . 217 . 215 . 193 . 395 . 174 . 160 . 085 . 210 . 085 . 210 . 085 . 210 . 085 . 210 . 085 . 210 . 230 . 230 . 200	530022000024581057668199477000377411506	N. NE	5.2 1 96 5.4 1.96 5.3 0.1 2.1 2.0 6 5.3 0.1 2.1 2.2 2.5 1.1 1.1 3.2 2.5 9 6.1 2.5 9 6.1 2.5 9 7 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.

Showing the evaporation and humidity for different winds, &c.—Continued.

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Year and day month.	Mean temperature.	Mean humidity.	Mean evaporation.	Mean cloudiness.	Direction of wind.	Relative velocity.	Year and day mouth.	Mean temperature	Mean bumidity.	Mean evaporation	Mean cloudiness.	Direction of wind.	Relative velocity
11 11 11 12 22 22 22 22 22 22 22 22 22 2	1 77 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	644 644 644 644 645	. 330 . 220 . 200 . 390 . 265 . 195 . 155 . 218 108 	4 0 3 8 5 5 2 7 4 4 5 5 5 5 10 0 10 5 4 4 4 4 3 2 2 4 1 3 0 1 0 0 0 7 7 4 4 7 0 7 0 3 5 5 2 2 0 0 7 6 0 0 0 8 0 1 1 1 7 4 4 0	South Sw. NE W. NW South N. NE East SW South W. NW South W. NW South W. NW South W. NW SE East W. SW East W. NW East W. NW East W. NW W. NW East East East East East East East East	0.0 0.4 0.5 0.9 0.0 0.5 0.5 0.5 0.5 0.5 0.7 0.4 1.5 0.7 0.4 1.7	1862. Sept. 15 16 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 Oct. 1 1 12 13 14 15 16 17 18 19 20 21 22 23 34 45 5 6 6 7 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 Nov. 1 2	1	. 799 . 647 . 749 . 646 . 695 . 816 . 672 . 640 . 737 . 610 . 739 . 758 . 816 . 865 . 847 . 758 . 646 . 749 . 759 . 759 . 654 . 654 . 607 . 735 . 628 . 670 . 654 . 614 . 619 . 654 . 619 . 653 . 521 . 758 . 635 . 635	.110 .102 .163 .055 .150 .180 .190 .190 .200 .195 .100 .195 .100 .100 .100 .100 .100 .100 .100 .10	7810300003440000068810015503105598440001553772335001090	NE E. SE South W. NW SE SW West S. SE North NE SE NORTH NE SE SW SW SW SW SW SW SW SW SW SW SW SW SW	
2 2 2 2 3 3 3 3 5 5 6 6 6 6 6 6 6 6 6 6 6 6 6 6	24 625 77 77 28 67 77 28 68 68 68 69 68 69 68 69 68 69 69 69 69 69 69 69 69 69 69 69 69 69	6	. 245 . 270 . 215 . 300 . 210 . 170 . 105 . 145 . 160 . 230 185 . 160 . 155 . 100 . 140 . 1215	6	N. NE	2.0 0.8 1.6 3.0 0.8 2.1 1.8 0.0 1.4 1.2 1.0 0.8 2.1 4.1	April 3 5 6 7 8 9 10 11 12 13 14 15 16 17 18 20 21 22 23	36 45 38 31 42 51 49 35 39 42 44 46 47 55 48 43 45 55	.778 .814 .530 .601 .547 .775 .688 .676 .624 .623 .524 .636 .690 .757 .889 .920 .885 .548	. 295 . 080 . 100 . 100 . 100 . 115 . 110 . 080 . 140 . 060 . 215 . 020 . 180 . 055 . 150 . 155 . 150	3 0 9 3 0 0 6 10 10 3 0 0 7 3 0 0 4 4 6 10 10 3 0 0 10 3 0 0 10 3 0 0 10 10 3 0 0 10 10 10 10 10 10 10 10 10 10 10 10	N. NE	

Showing the evaporation and humidity for different winds, &c.—Continued.

	į		4		-1	T _G	50	g		d		- 2	-0
ġ	peratu	nidity.	poration	diness.	Jo	relocity	d day	peratu	oldity.	poratio	diness.	20	elocity
ğ	Mean ten	Mean bur	Mean eva	Mean clo	Direction	Relative	Year and	Mean tem	Mean hur	Меви еvя	Mean clos	Direction	Relative velocity.
3. 95	47	515	155		NE	0.5	1863.	78	620	048		E NE	
26 27	47 50	. 611 . 501	.110	1 7	NE	0. 2 0. 3	8 9	79 71	. 651 . 818	. 235	5 10	North	0. 6 0. 6
29	49 52	. 457	. 205	0	N. NE	4. 8 0. 4	11 12	60 59	. 594 . 688	. 115	10	NE	0. 5 5. 3 4. 9
2 3	44 43	. 758 . 860	. 090	10	NE	2.8 2.1	14 15	69 61	. 570	. 275	5	West	0. 7 2. 3 1. 4
5 6	43 38 48	. 915 . 834 . 445	.000	10 2	N. NE	2.7 5.9 4.1	16 17 18	60 60 65	. 591 . 715 . 582	. 200 . 190 . 210	0 0	E. NE	1. 0 0. 8 1. 3
8	51 64 50	. 482	. 215	0	N. NE W. NW	2. 1 1. 1	19 20	68 64	. 802 . 845	. 000	10	NE	0. 6 4. 9
10 11	56 48	. 865	. 020	10	NE	2. 0 1. 1	22 23	64 74	. 742	. 150 . 295	0	SE	0 8
13 14	53 48	. 757 . 609	.175	5	NE	0. 9 1. 1 1. 2	25 26	66 68	. 787 . 903 . 652	.000	7 5	NE	0. 6 1. 0 1. 5
16	58 54	. 562	. 185	5 3	8W	2.3	28	68 68	. 595 . 708	.215	8	NW West	1. 7 1. 3 0. 7
18 19	56 67	. 550 . 459	. 175	0	West	0. 9 2. 6	30 31	71 77	. 745 . 795	. 190 . 160	4	SE	0.6
21	67 66	. 545	. 235	0	South	1. 5 0. 7		79 71	. 702	. 195 . 240	200	West	2. 9 1. 8 0. 8
23 24 25	62	. 791 . 651	. 135	. 9 6	SW	1.9 1.7	5 6	81 76 68	. 635 . 726 . 736	. 150	6	South W. NW	0, 9 1, 1 1, 8
27	52 58	. 729 . 668	. 025	9	NE	1.7 2.5	7 8	71 78	. 854	. 055	6	SW	0. 6 1. 9
29 30	59 62	. 776 . 925	. 055	8 10	East N. NW	0. 6 1. 1	10 11	77	. 744 . 611	.210	3	SE	1. 3 1. 0 4. 0
1	55	. 619	. 160	3	South	4. 2	13	77	. 672	. 220	2	SW	0. 6 1. 9 1. 6
3 4	57 61	. 636	. 145	6	SE	0. 7 1. 6	15 16	71 75	. 806	. 130 . 075	6 7	NE	0, 9 1, 5
6 7	51 54	. 673	. 160 . 190	4	NE	3. 3 2. 2	18 19	73 76	. 753 . 782	. 200	0 5	South	1. 2 1. 2 1. 3
9	59	, 564	. 105	0	NE East SE	0. 5	21	75	. 723	. 155	5	SW	0. 9 1. 3 1. 8
11 12	60 68	. 590 . 676	. 260 . 210	0	East	0. 5 0. 9	23 24	63 57	. 838 . 716	. 050	7	NW	1. 8 2. 6 0. 9
14 15	78 72	. 546	. 295	1 0	North	0.9 2.1	26 27	64 62	. 568 . 637	. 275 . 040	1 6	W. SW	2. 1 1. 9
17 18	74 73	. 457	. 230 . 325	4	North W. NW	1.8 2.2	29 30	49 55	. 571 . 652	. 150 . 080 . 140	6	South	3. 1 2. 5 1. 0
20	58	. 682	.110	6 9 8	W.NW	1. 2 1. 6	Sept. 1	62	. 721	. 170 . 140	3	South	1. 4 0. 9 5. 4
22 23	58 59	. 766 . 724	. 070	9	NE S. SE	1. 1 0. 9	3 4	55 62	. 648 . 775	. 120 . 195	2 4	NE 8. 8E	2. 2 1. 6
25 26	66 70	. 610	.215	0	8E	0. 5 0. 4	6 7	56 62	. 753 . 888	. 115 . 035	4 6	E. NE S. SE	2. 1 1. 8 0. 9
28	71 74 70	. 742 . 735 . 865	. 205 . 290 . 000	0 4 8	SE	0. 8 0. 7 1. 1	8 9 10	59 54 61	. 710 . 673 . 922	. 085 . 130 . 000	7 1 10	NE E. NE	3. (1. 4 1. 4
3 0	67 72	. 859 . 816	. 075	10 6	E.SE South	0. 5 1. 1	11 12	70 57	. 676 . 768	.1º0 .090	8	West	2. 5 2. 6
3 4	75 75	. 670	. 220	0 2	W.NW	1. 1 0. 5	14 15	71 75	. 648 . 649	. 240	0	8.8W	1. 4 1. 8 1. 7
5 6	74 76	. 681 . 622	. 215 . 250	0	East NE	0.6	16	75 60	. 676 . 791	. 170 . 015	8	South West	1. 7 3. 1
	\$\$ \$2.28.28.39.1 23.3 4.5 6.7 8.9 10.11.21.31.41.516.17.81.98.21.23.43.5 6.7 8.9 10.11.21.31.41.5 16.17.81.98.21.22.24.23.24.25.28.28.29.31.23.34.5 6.7 8.9 10.11.21.31.41.5 16.17.81.98.21.22.24.25.28.29.31.23.34.5 6.7 8.9 10.11.21.31.41.5 16.17.81.98.21.22.24.25.28.29.31.23.34.5 6.7 8.9 10.11.21.31.41.5 16.17.81.98.21.22.24.25.28.29.31.23.34.5 6.7 8.9 10.11.21.31.41.5 16.17.81.98.21.22.24.25.20.20.20.20.20.20.20.20.20.20.20.20.20.	3. 25 47 226 522 23 30 52 11 444 33 4 433 4 5 58 8 11 55 58 114 15 58 114 15 58 114 15 58 114 16 58 117 18 19 20 21 22 36 63 224 25 52 27 38 58 9 59 23 31 1 55 56 6 51 11 40 11 11 11 11 11 11 11 11 11 11 11 11 11	3. 25 47 .515 26 47 .611 .376 .39 .43 .840 .39 .457 .39 .457 .39 .457 .39 .457 .39 .457 .39 .457 .39 .457 .39 .457 .39 .59 .69 .457 .39 .59 .69 .457 .39 .59 .69 .457 .39 .59 .59 .69 .457 .39 .59 .59 .59 .59 .59 .59 .59 .59 .59 .5	3. 25 47 .515 .155 .26 47 .611 .110 .28 52 .459 .125 .39 .457 .205 .30 .52 .399 .170 .1 61 .376 .390 .52 .399 .170 .1 61 .376 .390 .52 .399 .170 .5 38 .834 .000 .5 38 .834 .000 .5 38 .834 .000 .6 48 .445 .215 .5 1.482 .215 .8 64 .471 .280 .9 50 .694 .095 .10 .56 .865 .020 .11 .48 .913 .000 .12 .58 .649 .175 .13 .5 .8 .649 .145 .15 .58 .609 .185 .609 .185 .16 .54 .562 .145 .17 .49 .645 .155 .58 .609 .185 .16 .54 .562 .145 .17 .49 .645 .155 .8 .609 .185 .16 .54 .562 .145 .17 .49 .645 .155 .28 .609 .185 .20 .20 .21 .25 .55 .828 .100 .20 .21 .25 .55 .619 .10 .25 .55 .619 .10 .25 .55 .619 .10 .25 .55 .619 .10 .25 .55 .617 .130 .3 .57 .636 .145 .50 .20 .20 .25 .55 .617 .130 .3 .57 .636 .145 .55 .619 .160 .55 .619 .160 .55 .619 .160 .55 .619 .160 .55 .619 .160 .25 .55 .617 .25 .000 .25 .55 .617 .25 .25 .55 .617 .25 .000 .25 .55 .617 .25 .000 .25 .55 .617 .25 .000 .25 .55 .617 .25 .000 .25 .55 .617 .25 .000 .25 .55 .617 .25 .000 .25 .55 .617 .25 .000 .25 .55 .617 .25 .000 .25 .55 .617 .25 .000 .25 .55 .617 .25 .000 .25 .55 .617 .25 .000 .25 .55 .617 .25 .25 .55 .617 .25 .25 .55 .56 .25 .25 .25 .55 .25 .25 .25 .25 .25 .25	3. 25 47 .515 .155 0 0 17 28 52 .459 .125 10 0 30 52 .399 .170 0 0 10 61 .310 .915 .915 .915 .915 .915 .915 .915 .915	3. 25 47 .515 .155 0 NE	3. 25 47 .515 .155 0 NE 2.5 26 47 .611 .110 1 East 0.2 27 50 .501 .140 7 NE 0.3 28 52 .459 .125 10 N.NE 4.6 30 .52 .399 .170 0 E.NE 0.4 1 .61 .376 .330 2 South 0.9 2 44 .758 .090 4 NE 2.8 3 43 .860 .005 10 NE 2.7 5 38 .834 .000 10 NE 2.7 5 38 .834 .000 10 NE 2.7 5 38 .834 .000 10 NE 2.1 8 64 .471 .280 3 W.NE 2.1 9 50 .694 .955 7 NE 4.1 10 .56 .865 .020 9 SW 0.1 11 .48 .913 .000 10 NE 1.1 12 .58 .642 .175 6 NW 0.9 13 .53 .757 .100 5 NE 1.1 12 .58 .642 .175 6 NW 0.9 13 .53 .757 .100 5 NE 1.1 14 .8 .609 .145 0 NE 1.1 15 .58 .609 .185 0 NE 1.1 16 .54 .562 .145 3 N.WW 2.7 17 .49 .645 .155 3 N.WW 2.7 17 .49 .645 .155 3 N.WW 0.9 19 .67 .459 .180 1 SW 2.6 20 .73 .447 0 SW 2.6 21 .67 .459 .180 1 SW 2.6 22 .651 .135 6 West 0.9 24 .62 .651 .135 6 West 0.9 24 .62 .651 .135 6 NE 1.7 25 .55 .828 .100 8 NE 1.7 27 .58 .668 .090 7 NE 1.7 28 .59 .677 .130 3 North 1.7 29 .59 .673 .055 9 NE 1.7 27 .58 .668 .090 7 NE 1.5 29 .59 .776 .055 8 East 0.6 5 .49 .854 .060 10 NE 1.7 27 .58 .668 .090 7 NE 1.5 29 .59 .776 .055 8 East 0.6 5 .49 .854 .060 10 NE 1.7 28 .59 .59 .776 .055 8 East 0.6 5 .49 .854 .060 10 NE 1.7 29 .50 .666 .135 .5 SW 1.9 24 .66 .668 .090 7 NE 1.5 25 .50 .677 .130 3 North 1.7 25 .56 .69 .150 0 NE 1.7 27 .58 .668 .090 7 NE 1.5 29 .59 .776 .055 8 East 0.6 29 .79 .70 .855 .000 8 E 0.4 21 .67 .457 .200 0 SE 0.5 22 .59 .776 .055 8 East 0.6 24 .66 .614 .150 0 East 0.9 25 .50 .677 .130 3 North 1.7 26 .66 .634 .170 0 East 0.9 27 .58 .668 .090 7 NE 1.5 28 .59 .564 .000 10 NE 1.1 29 .50 .606 .135 .000 0 SE 0.9 24 .64 .637 .200 0 SE 0.5 25 .677 .130 3 North 1.7 27 .200 .850 .000 0 SE 0.5 28 .79 .70 .805 .000 0 SE 0.5 29 .70 .691 .100 0 SE 0.5 20 .71 .74 .457 .200 0 SE 0.5 20 .72 .73 .74 .75 .200 0 SE 0.5 21 .64 .637 .200 0 SE 0.5 22 .72 .72 .72 .72 .72 .72 .72 .72 .72 .	The color The	The color of the	Section Sect	1	Section Sect	The color of the

Showing the evaporation and humidity for different winds, &c .- Continued.

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Year and day month.	Mean temperature.	Mean humidity.	Evaporation.	Mean cloudiness.	Duration of wind.	Relative velocity.	Year and day	monta.	Mean temperature	Mean humidity.	Evaporation.	Mean cloudiness.	Direction of wind.	Relative velocity
1863. Sept. 18 19 20 21 22 23 30 31 22 24 45 56 67 78 8 9 100 11 12 13 14 45 56 66 77 8 8 9 100 11 12 12 13 14 15 56 67 78 8 9 100 11 12 12 12 12 12 12 12 12 12 12 12 12	40 444 583 555 647 485 567 662 632 550 493 40 445 449 466 394 451 466 394 451 467 467 467 467 467 467 467 467 467 467	595 658 555 658 642 651 650 650 650 650 709 709 775 760 856 861 862 875 733 824 845 645 880 880 880 886 886 886 886 886 886 886	075 090 170 090 170 110 115 185 230 210 045 040 030 045 040 040 040 040 040 040 040 040 04	62120910131302477010071055320004110104777 8010101069107771064810422003557100333107743300041011015033	N. NW West W. SW S. SE S. SE SOUTH S. SE S.	7655724481787500055552756945055571076 3952857981214053593770555013260000727996630	June June	1. 13145167 189021 223 245 6 7 8 9 10 1 12 3 4 5 6 7 8 9 10 1 12 3 14 5 16 7 8	498 545 561 562 627 605 571 591 566 667 525 555 668 75 155 755 755 755 755 755 755 755 755	573 548 604 608 701 7612 612 414 414 727 612 414 414 414 5598 634 635 667 705 669 633 601 669 633 601 667 775 676 683 727 683 727 683 683 727 761 683 683 727 761 683 683 727 761 683 683 727 761 683 683 727 761 683 683 727 761 683 683 727 761 683 683 727 761 683 683 727 761 683 683 727 761 683 683 727 761 683 683 727 761 683 683 727 761 683 683 683 683 683 683 683 683 683 683	.150 .150 .255 .185 .185 .190 .250 .250 .250 .250 .250 .250 .250 .25	200210040200603724540544000000035624123223398305217850550020069850011	N. NE NE SW North N. NE NE SW North North E. SE Calm E. SE Calm E. SE E. SE Calm North N. NE N.	11445:12124

Showing the evaporation and humidity for different winds, &c.—Continued.

Year and day of month.	Mean temperature.	Mean humidity.	Evaporation.	Mean cloudiness.	Duration of wind.	Relative velocity.	Year and day of month.	Mean temperature.	Mean humidity.	Evaporation.	Mean cloudiness.	Direction of wind.	Relative velocity.
1864. July 24 25 26 27 28 30 31 Aug. 1 5 6 7 8 9 9 10 11 12 13 14 15 16 17 18 19 20 22 23 24 25 27	7275667381778375076698670717548188017756666565665665677426989	.575 .599 .737 .717 .705 .505 .505 .710 .742 .7710 .697 .772 .677 .677 .678 .661 .615 .575 .594 .628 .661 .615 .722 .697 .722 .697 .723 .734 .744 .745 .745 .745 .745 .745 .745 .74	.355 .355 .355 .310 .310 .280 .100 .080 .105 .135 .200 .105 .175 .200 .105 .180 .370 .370 .320 .145 .155 .155 .155 .155 .155 .155 .155	032524421055447533000322353227101100086667440335	W.SW West N. NE SE SW W.SW South W.NW N. NE N. NE N. NE N. NE SOUTH SOUTH SSW W.SW E.SE E.SE N. NE E.SE E.SE SSW West N. NE SS SSW West West West W.NW	1.07 0.76 0.55 1.28 0.77 1.10 0.87 1.11 0.88 0.77 0.99 0.85 1.22 0.43 0.99 0.90 0.70 2.33 0.93 0.93 0.93 0.93 0.93 0.93 0.93 0	1864. Aug. 28 29 30 31 Sept. 1 2 3 4 5 6 7 8 9 10 11 12 13 14 115 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30	65 61 62 68 68 68 66 63 62 61 65 61 65 61 62 61 65 61 61 61 61 61 61 61 61 61 61 61 61 61	.550 622 536 560 590 686 755 687 723 747 791 566 750 753 747 791 548 652 598 676 669 753 669 753 669 753 669 753 669 753 669 753 747 791 669 669 753 769 769 669 669 769 669 769 769 669 769 669 769 669 769 669 769 669 6	.180 .080 .080 .120 .135 .125 .050 .060 .060 .095 .095 .095 .095 .095 .090 .095 .115 .090 .095 .095 .090 .095 .095 .095 .09	782018999107772622089920255006677705166015	NW	1.37 0.56 0.066 1.11 2.17 2.22 2.81 1.12 1.23 2.07 2.066 0.31 1.19 2.05 2.07 2.066 0.31 1.17 0.88 1.11 1.77

TABLE Y .- Milwaukee for 1862.

Direc	tion of	win	đ.	Mean temperature.	Mean humidity.	Mean evaporation.	Very cloudings	Number of days	Mean relative ve-	Direction	on of	wind.	Mean temperature.	Mean humidity.	Mean evaporation.	Mean cloudiness.	Number of days. Mean relative ve-
North. N.NE. NE. E.NE E.SE. S.SE. South.				46 54 55 61 58 65 62 63	. 729 . 669 . 769 . 700 . 666 . 760 . 751	.05 .15 .14 .18 .13 .14	0 1 4 4	5 4 3 6 1 1 3 4 1 1 3 5 5		S.SW SW W.SW. West W.NW. N.NW Calm			66 61 58 67 65 56 56	. 644 . 680 . 691 . 756 . 632 . 632 . 732	. 205) . 161 . 008 3 . 164 3 . 164 2 . 154	3 3 3 4 3 9	7 26 25 L 8 3 L 2 13 L 2 13 L 2 6 L 3 8 L 1 8 Q 0
Mean relative vo-	locity.	Mean temperature.		Mean humidity.		Mean evaporation.	,	Mean cloudiness.	Number of days.	Mean relative ve-		Mean temperature.	Mean humidity.		Mean evaporation.	Mean cloudiness.	Number of days.
0. 0 - 0. 5 - 1. 0 - 1. 5 -	- 0. 4 - 0. 9 - 1. 4 - 2. 0	5 6 5	8 3 2 8		717 717 681 657	:	143 166 168 154		36 3 51 3 31 3 26	2.1— 2.7— 3.6— 4.6—	2.8 3.5 4.5 5.6	58 58 61 51		716 652 503 535	. 15 . 16 . 23 . 15	5	5 22 6 14 1 2 5 3
Mean cloudiness.	Mean temperature.		Mean humidity	Aces numery.		Mean evaporation.	Version of Arms	Number of days.	Mean relative velocity.	Mean cloudiness.	,	Mount tomperature.	Mean humidity.		Mean evaporation.	Number of days.	Mean relative ve-
0. 1. 2. 3. 4. 5.	6 6 6 6	0 6 5 0 6 3		. 649 . 685 . 617 . 641 . 685 . 684		. 186 . 206 . 172 . 171 . 186 . 169		47 13 14 18 19 19	1. 1 1. 2 1. 4 1. 5 1. 2 1. 7	6. 7. 8. 9. 10.		65 61 58 52 56	. 70 . 75 . 76 . 76 . 85	06 15 17 19 17	. 110 . 145 . 133 . 049 . 074	1	1 1.3
Mean temperature.	Mesn humidity.			Mean evaporation.		Mean cloudiness.	, , , , , , , , , , , , , , , , , , ,	Number of days.	Mean relative ve- locity.	Mean temperature.		Mean humidity.	Mean evenoration.		Mean cloudiness.	Number of days.	Muan relative ve-
30 35 40 45 50 55		681 634 663 705 649 679		. 086 . 065 . 106 . 15	0554	4 5 5 5 4 3		2 5 8 18 18 23	1.6 1.9 1.2 1.3 2.0 1.6	60 65 70 75 80 85		. 706 . 724 . 677 . 675 . 588 . 609	:	123 148 171 249 335 108.	4 5 4 3 0 3	9: 36 36 16	1.1 0.9 1.2 1.1 1.0 0.5

Milwaukee for 1862-Continued.

Mean evaporation.	Mean temperature.	Mean humidity.	Mean cloudiness.	Number of days.	Mean relative ve- locity.	Mean evaporation.	Mean temperature.	Mesa humidity.	Mean cloudiness.	Number of days.	Mean relative velocity.
.000 .030 .060 .090 .120 .150	47 58 52 56 57 62 63	. 753 . 759 . 714 . 707 . 684 . 703 . 656	9 7 4 5 4 4	6 12 15 15 21 26 22	1.0 1.1 1.4 1.6 1.6 0.9	. 210 . 240 . 270 . 300 . 330 . 360 . 390	64 71 70 75 80	. 675 . 655 . 586 . 668 . 659	3 3 3 2 0	27 12 7 2 2	1. 6 1. 0 1. 5 1. 3 1. 8
Mean humidity.	Mean temperature.	Mesn evsporstion.	Mean cloudiness.	Number of days.	Mean relative velocity.	Mean humidity.	Mean temperature.	Mean evaporation.	Mean cloudiness.	Number of days.	Mean relative vo- locity.
. 400 . 450 . 500 . 550 . 600 . 650	59 62 54 63 63	. 175 . 234 . 155 . 196 . 189 . 178	2 2 2 3 3 9 .	5 10 11 14 23 27	2. 2 0. 9 2. 6 1. 7 1. 4 1. 0	. 700 . 750 . 800 . 850 . 900	60 61 67 60	. 151 . 140 . 128 . 093 . 114	4 5 5 8	35 29 19 10	1. 3 1. 1 1. 2 0. 8 0. 8

Milwaukee for 1863.

Day of month.	Mean temperature.	Mean humidity.	Mean cloudiness.	Relative velocity.	Direction of wind.	Day of month.	Mean temperature.	Mean humidity.		Relative velocity.	Direction of wind.
Oct. 20 21 22 23 24 25	43 39 36 33 35 35	. 621 . 578 . 955 . 728 . 644 . 688	.075 6 .055 2 .000 9 .035 10 .050 9 .015 4	1.8 0.7 2.6 2.3	W.SW W.NW NW W.NW North	Oct. 96 27 28 29 30 31	37 44 46 36	7 .714 1 .719 5 .921 5 .787	.000 1	6 0.4 3 0.6 7 1.8 10 1.8 10 2.3 6 0.9	NE. East. S.SE. South. N.NW. NW.
Mean cloudiness.	Mean temperature.	Meso bumidity.	Mean evaporation.	Number of days.	Relative velocity.	Mean cloudiness.	Mean temperature.	Meso humidity.	Mesn evaporation.	Number of days.	Relative velocity.
0. 1. 2. 3. 4. 5.	61. 62 56 54 58 63	. 630 . 611 . 609 . 643 . 717 . 691	. 188 . 180 . 158 . 153 . 133 . 141	48 17 17 19 17	1.5 1.5 1.4 2.5 1.9 1.4	6. 7. 8. 9. 10.	57 56 65 56 51	.714 .744 .754 .763 .813	. 114 . 088 . 091 . 063 . 050	21 19 8 12 25	1. 4 2. 1 1. 4 1. 6 2. 2

Milwaukee for 1863—Continued.

Directi	on of	wind	Mean temperature.	Mean humidity.	1	mean evaporation.	Mean cloudiness.	Number of days.	Relative velocity.	Direct	ion of	wind.	Mean temperature,	Mean humidity.	Mean evaporation.		Mean cloudiness.	Relative velocity.
North N.NE E.NE East 8.SE 8.SE			58 45 54 59 59 55 62 55	. 646 . 626 . 715 . 680 . 677 . 716 . 703 . 733	.1	45 43 00 59 28 42 68 29	4 4 6 2 2 3 3 4	6 11 45 8 15 3 26 7	1. 6 3. 5 2. 2 1. 0 0. 9 0. 8 0. 8 1. 4	South S.SW SW W.SW West W.NV NW N.NW	7 7		60 62 66 62 64 59 53 49	. 720 . 750 . 659 . 696 . 646 . 683	0 .19 0 .10 0 .10 0 .10 3 .13 1 .14 2 .00	60 04	5 4 2	21 1.5 6 1.1 13 2.1 4 2.3 19 1.3 13 1.9 9 1.9 6 1.4
Relative velocity.		Mean temperature.	Meen humidity		•	Mean evaporation.		Mean cloudiness.	Number of days.	Relative velocity.		Mean temperature.	Mesn humidity.		Mean evaporation.		Mean cloudiness.	Number of days.
0. 0 — 0. 5 — 1. 0 — 1. 5 —	0, 4 0, 9 1, 4 -2, 0	56 62 62 51		. 574 . 711 . 715 . 725		. 128 . 147 . 133 . 112		4 3 5 4	12 47 43 46	2.1- 2.7- 3.6- 4.6-	-2.6 -3.5 -4.5 -5.6	55 47 55 50	· :	635 704 633 641	.1	36 96 178 123		5 3 3 4 5 1
Mean temperature.	Mesn humidity.		Mean evaporation.		Mean cloudiness.	Number of days			Relative velocity.	Mean temperature.	Mean bumidity.		Mean evaporation.		Mean cloudinem.	Variable of Action	Aumoer of days.	Relative velocity.
* 30 35 40 45 50 55	:	709 702 741 695 686 679	.1 .0 .0 .1	48 49 19 08	5 4 6 5 4 4		4 14 15 25 24 32		2.5 2.8 2.3 2.0 1.8 2.0	60 65 70 75 80		693 678 686 706 642	. 14 . 15 . 14 . 18 . 25	2 3 6 8 5	4 4 3 3 3 9		32 20 24 20 2	1.0 1.5 1.6 1.5
Mean evaporation.	Mean fammerature		Mean humidity.		Mean cloudiness.	Number of days.			Relative velocity.	Mean evaporation.	•	Mean temperature.	Mean bumidity.	•	Mean cloudiness.		Number of days.	Relutive velocity.
. 000 . 030 . 060 . 090 . 120 . 150		51 51 54 52 58 62	. 85 . 77 . 65 . 66	72	8 7 7 5 4 3		24 20 17 27 27 25		1.8 1.5 1.8 2.1 1.8 1.7	. 1	180 110 140 170	64 66 68 72 69	:	668 631 659 562 538	3 9 0 9		24 3 6 8 3	1.7 1.3 1.5 1.5

Milwaukee for 1863-Continued.

Mean humidity.	Mean temperature.	Mean evaporatian.	Mean cloudiness.	Number of days.	Relative velocity.	Mean humidity.	Mean temporature.	Mean evaporation.	Mean cloudiness.	Number of days.	Belative volocity.
. 350 . 400 . 450 . 500 . 550 . 600	62 60 59 52 61 58	. 275 . 215 . 207 . 168 1. 192 . 166	1 3 2 2 3	3 2 7 9 18 35	1. 2 3. 1 2. 6 1. 9 1. 9 1. 7	. 650 . 700 . 750 . 800 . 850 . 900	58 61 56 58 57 51	.143 .136 .090 .082 .041	4 3 6 6 8 9	40 34 26 13 15	1.8 1.4 1.7 1.6 1.5

						M	Iilu	aukee	for :	1864.	•							
Direc	tion of	wind.	Mean temperature.	Mean humidity.	Mean evaporation.	Mean cloudiness	Number of dees	Relative velocity.	Direc	ction of	wind.	Mean temperature.	Mean humidity.		Mean evaporation.	Mean cloudiness.	Number of days.	Relative velocity.
NE E.NE E.SE.	NE		46 53 65 66 60 64 63 62 68	. 707 . 705 . 673 . 600 . 691 . 635 . 675 . 605	. 135 . 116 . 125 . 187 . 142 . 177 . 121 . 080		5 1: 5 4 1: 2 4 1: 3 .1: 3 6 1:	2 2 0 1.4 2 0.5 1 0.8 0 0.9 1.2	S.SW SW. W.SV West W.N NW. N.NV Calm	w		75 71 77 67 61 49 44 60	. 604 . 581 . 507 . 596 . 626 . 698 . 933 . 648	1	243 210 335 163 127 118	3 4 2 3 5 5 10 3	8 9 5 17 8 5 1 6	1.4 1.4 2.7 1.8 1.4 0.8 1.0
Mean cloudiness.	Mean cloudiness. Mean temperature. Mean humidity.		Mean humidity.	Mean		Number of days	Author of days.	Relative velocity.	Mean cloudiness.	Mean temperature.	Mean humidity.		Mean evaporation.	•		Number of days.		Reserve velocity.
0 1 2 3 4 5	6	11 18 18 19 19 19 19 19 19	. 570 . 604 . 611 . 615 . 659 . 671		. 189 . 180 . 201 . 213 . 240 . 169		41 8 20 20 20 13 20	1.6 2.0 1.1 2.1 1.1 1.2	6 7 8 9 , 10	61 58 61 61 46	:	653 715 725 767 870	:	080 096 110 055 032		12 14 9 6 19		1. 1 1. 3 1. 1 2. 1 1. 9
Relative velocity.		Mean temperature.	Mean humidity.		Mean evaporation.		Mean cloudiness.	Number of days.	Deletine aslasite	recipitate recipitation.	Mean temperature.		Mean humidity.	Veer error	Monte overporation.	Mean cloudiness.		Number of days.
0.0- 0.5- 1.0- 1.5-	- 0. 4 - 0. 9 - 1. 4 - 2. 0	59 68 62 63	. :	643 677 691 634	.1 .1 .1	90 52 26 70	3 4 5	25 56 31 22	2.1- 2.7- 3.6- 4.6-	-2.6 -3.5 -4.5 -5.6	62 52 59 36		. 588 . 717 . 618 . 623	:	155 125 166 175	3		24 17 6 2

Milwaukee for 1864-Continued.

Mean temperature.	Mosn bumidity.	Mean evaporation.		Mean cloudiness.	Number of days.		Relative velocity.	Mean temperature.		Mean humidity.	Mean evaporation.		Mean cloudiness.	Number of days.	Palative valuality.	
35 40 45 50 55 60	. 795 . 762 . 670 . 603 . 642 . 651	. 07 . 06 . 09 . 17 . 15	36 14 12 10	7 6 4 3 5 4	1 1 1 1 2	6	2.3 1.2 1.6 2.2 1.1 1.7	65 70 75 80 85		. 625 . 641 . 609 . 594 . 574	.1 .1 .2 .5	98 87 89	4 3 4 3 3	36 26 14 10 1	; 	13 1.3 1.2 1.
Mean humidity.	Mean temperature.	Mean evaporation.	Men cloudiness		Number of days.		Relative velocity.	.Mean humidity.		Mean temperature.	Mean evenoration		Mean cloudiness.	Number of days.	Relative velocity	- funda 4 and 10
. 350 . 400 . 450 . 500 . 550 . 600	73 66 62 64 65 59 64	. 32 . 26 . 21 . 16 . 16	68 0 19 16 17	2 3 1 1 3 4 4	3 7 7 14 26 27 32		2.2 2.0 0.9 2.0 1.4 1.8 1.1		700 750 800 850 900 950	64 55 55 42 40 38		. 109 . 087 . 041 . 034 . 010	6 5 6 9 9	24 23 2 9 6		6. 1. 1.2 1.4
Mean evaporation.	Mean temperature.	Mesa humidity.	Mean cloudiness		Number of days.		Relative velocity.	Mean evaporation.		Mean temperature.	Mean humidity.		Mean cloudiness.	Number of days.	Relative velocity.	
.000 .030 .060 .090 .120 .150 .180	60 52 60 61 61 62 63 64	. 75 . 71 . 70 . 68 . 61 . 62 . 63	6 5 7 9 5	7 8 6 5 4 3 2	14 14 13 15 21 16 21 7		1.5 1.6 1.5 1.4 1.3 1.7 1.3		240 270 300 330 360 390	61 68 74 75 75 75		611 501 559 620 610 435 546	2323243	7 7 3 2 3 1 4		L7 67 10 10 10 10
		Тав	LE Z	.—.	Milw	au	kce fo	r 186	2,	1863,	and	186	4.			
Direction	on of win	Mean temperature.	Mean humidity.	Mean evaporation.	Mean cloudiness.	Number of days.	Mean relative velocity.	Direct	tion	of wind	Moan temperature.	Mean humidity.	Mean evaporation.	Mean cloudiness.	Number of days.	Mean relative ve-
North N. NE NE E. NE East		52 53 56 60 60 61	. 688 . 681 . 717 . 664 . 689	.141 .137 .112 .165 .149	4 1	19 89 70 10 39 32	2.3 2.2 1.9 0.9 0.8 0.7	8. 8W 8W W. 8V West. W. N NW	w		68 65 67 68 62 53	. 659 . 698 . 584 . 697 . 645 . 683	. 194 . 169 . 206 . 150 . 153	3 3 4 5 4	91 47 19 49 34 90	1.6 1.7 1.7 1.3

REPORT OF THE SECRETARY OF WAR.

Milwaukee for 1862, 1863, and 1864—Continued.

Mean cloudiness.	Mean temperature.	Meen humidity.	Mean evaporation.	Number of days.	Mean relative velocity.	Mean cloudiness.	Mean temperature.	Mean humidity.	Mean evaporation.	Number of days.	Mean relative ve- locity.
0. 1. 2. 3. 4. 5.	60 66 63 58 63 65	. 619 . 640 . 621 . 643 . 689 . 681	. 187 . 183 . 179 . 177 . 181 . 162	136 38 51 57 49 49	1. 3 1. 4 1. 3 1. 9 1. 4 1. 6	6. 7. 8. 9. 10.	60 58 61 61 50	. 695 . 750 . 770 . 764 . 822	. 104 . 105 . 113 . 059 . 056	43 48 27 22 63	1.3 1.5 1.3 1.7 1.8
Mean relative ve- locity.	Mean temperature.	Mean humidity.	Mean evaporation.	Mean cloudiness.	Number of days.	Mean relative ve- locity.	Mean temperature.	Mean humidity.	Mean evaporation.	Mean cloudiness.	Number of days.
0. 0 0. 5 1. 0 1. 5	58 62 62 60	. 668 . 628 . 697 . 686	. 164 . 156 . 142 . 136	****	73 153 105 94	2.1 2.7 3.6 4.6	58 52 57 48	. 644 . 697 . 611 . 607	.147 .124 .114 .138	4 5 3 5	79 46 16 13
			·								
Mean temporature.	Mean humidity.	Mean evaporation.	Mean cloudiness.	Number of days.	Mean relative ve- locity.	Mean temperature.	Mean humidity.	Mean evaporation.	Mean cloudiness.	Number of days.	Mean relative ve- locity.
30 35 40 45 50 55	.700 .737 .753 .694 .654	.118 .076 .065 .105 .138 .141	5 6 5 4	6 36 38 52 56 77	2. 2 2. 1 1. 7 1. 7 2. 0 1. 9	60 65 70 75 80 85	. 689 . 676 . 668 . 668 . 615 . 591	.131 .150 .172 .204 .287 .309	4 4 3 3 2 3	78 92 80 48 14	1. 6 1. 1 1. 2 1. 2 1. 2 0. 8
					1 .	,	r				
Meen humidity.	Mean temperature.	Mean evaporation.	Mean cloudiness.	Number of days.	Mean relative ve- locity.	Mean humidity.	Mean temperature.	Mean evaporation.	Mean cloudiness.	Number of days.	Mean relative ve- locity.
. 350 . 400 . 450 . 500 . 550 . 600	68 62 61 58 62 60 61	. 298 . 225 . 217 . 164 . 167 . 179 . 159	1 2 2 2 3 3 3 3	6 14 24 34 58 85 99	1.5 2.2 1.4 2.0 1.5 1.6 1.4	. 700 . 750 . 800 . 850 . 900 . 950	61 59 63 54 50 38	.136 .104 .102 .055 .040	4 5 5 9 9	93 78 34 34 20 2	1. 2 1. 4 1. 3 1. 2 1. 5 1. 3

REPORT OF THE SECRETARY OF WAR.

Milioaukee for 1862, 1863, and 1864-Continued.

Mean evaporation.	Mean temperature.	Mean humidity.	Mean cloudinest.	Number of days.	Mean relative velocity.	Mean evaporation.	Mean temperature.	Mean bumidity.	Mean cloudiness.	Number of days.	Mean relative va-
.000 .030 .060 .090 .120 .150 .180	53 54 55 54 59 62 63 65	. 808 . 753 . 712 . 696 . 660 . 670 . 655 . 640	8 7 6 5 4 3 2 2	44 46 45 57 69 67 67 58	1.6 1.4 1.4 1.7 1.6 1.6 1.3	. 240 . 270 . 300 . 330 . 360 . 390 . 420	68 70 72 77 75 75 75	. 640 . 557 . 578 . 640 . 610 . 555 . 546	2 3 3 3	8 4 3 3	1.3 0.9 0.1 1.7 1.0 1.1

Table AA.—Showing the wind, water, barometer, and thermometer at Milwaukee, Wisconsin, from April 1, 1861, to December 31, 1864.

		WIND		water be- of 1828.	duced	bed.			WIND		iter be- 1838.	reduced	ped.
Date.	Hour of day.	Course.	Relative ve-	Height of wate low zero of 1	Barometer reduced 320.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve- locity.	Height of wate low zero of 11	Barometer rec 32°.	Therm'r detached.
1861.							1861.						
April 1	3 a.m.	NE	4. 1		29, 193	33	April 13	3 a. m. 7 a. m.	Calm	0,0		29. 111	45
	9 a.m. 2 p.m.	North	3.8		29. 396	39		9 a. m. 2 p. m.	NW	1.5	3. 22	29. 088	51
	3 p. m.	W.NW. SW	1.9 0.2		29. 496			3 p. m.	East	1.8 1.4	3.21		
2	9 p. m. 3 a. m.	West	0, 4			39	14	9 p. m. 3 a. m.	E NE	0.8	3. 22 3. 25	29. 231	43
	7 a.m. 9 a.m.	NW	0.5		29. 560	36		7 s. m. 9 s. m.	East	1.9	3.30	29. 392	38
	2 p. m. 3 p. m.		::::		29, 516	51		2 p. m. 3 p. m.	SE	3. 2	3. 25	29. 474	41
3	9 p. m. 3 a. m.	SW North	0. 7 0. 7		29. 501	45	15	9 p. m. 3 a. m.	South NE	1.4 2.6	3. 21	29. 421	38
•	7 a. m.	NE	1.9		29. 555	38	10	7 a. m. 9 a. m.				29, 380	33
	9 s. m. 2 p. m.				29. 599	43		2 p. m.	N. NE	4.7	3. 06	29. 404	36
	3 p. m. 9 p. m.	NE	2.7		29. 596	36		3 p. m. 9 p. m.	NE	4.0 3.9	3.00	29. 434	32
4	3 a. m.	E.SE	3.7		29. 605	39	16	3 a.m.	NE	4.0	2. 95	29.435	35
	9 a. m. 2 p. m.	East	4. 1		29. 509	41		9 s.m. 2 p.m.	N. NE	5.2	2.97	29.406	36
	3 p. m. 9 p. m.	E. NE			29. 521	40		3 p. m. 9 p. m.	N. NE N. NW .	4.9 1.2	2.98 3.12	. 	31
. 5	3 a. m.	East	3. 1				17	3 a. m.	NW	1. 2		29, 409	l
	7 a. m. 9 a. m.	E. NE	3. i		29. 475	39		7 a. m. 9 a. m.	E. SE	1.4	2.21	29. 397	33
	2 p. m. 3 p. m.	E. NE	2. 3		29. 399	40		2 p. m. 3 p. m.	S. SE	2.2	3. 25	29. 199	38
6	9 p. m. 3 a. m.	E. NE			29. 431	39	18	9 p. m. 3 a. m.	8. SE West	4.0		29. 021	36
_	7 a. m. 9 a. m.	E.SE			29. 312	39		7 a. m. 9 a. m.	W. NW	4. 5		28. 831	41
	2 p. m.				29. 176	42		2 p. m.		l		29. 031	40
_	3 p. m. 9 p. m.	SE	5. 2	2. 95	29. 105	42		3 p. m. 9 p. m.	North	2.6	3.11	29. 346	36
7	3 a. m. 7 a. m.	8. SE		2.99	29. 141	43	19	3 a. m. 7 a. m.	N. NW	2.5	3.06	29. 483	35
	9 a. m. 2 p. m.	8E	0. 6	3. 01	29. 158	44		9 a. m. 2 p. m.	E. NE		3. 15	29, 472	43
	3 p. m. 9 p. m.	E. NE East	1. 6 3. 3		29. 113	42		3 p. m. 9 p. m.	SE	2.8 1.6	3, 23 3, 31	29.419	36
8	3 a. m.	E. SE	3. 2				20	3 a. m.	s. sw				
	7 a. m. 9 a. m.	E. 8E	2.4	2.98	29.048	44		7 a. m. 9 a. m.	sw	5. 1	3. 35	29. 302	42
	2 p. m. 3 p. m.	East		3.01	29, 080	40		2 p. m. 3 p. m.	sw	3. 8	3. 25	29. 194	58
9	9 p. m. 3 a. m.	N. NE	4. 2		29. 083	39	21	9 p. m. 3 s. m.	South			29. 211	48
	7 a. m. 9 a. m.	N. NE	3.4		29. 136	41		7 a.m. 9 a.m.	8E			29. 158	44
	2 p. m. 3 p. m.	N. NE	3. 6		29. 207	39		2 p. m. 3 p. m.	South .			28. 988	55
10	9 p. m.	N. NE	3. 5	3.06	29. 331	41		9 p. m.	South	1.5	2. 95	28, 930	57
10	3 a. m. 7 a. m.	NE	3. 4		29. 422	46	22	3 a. m. 7 a. m.	8W			28. 918	63
	9 a.m. 2 p. m.	E. NE	2.9	3. 14	29, 417	40		9 a. m. 2 p. m.	sw	4.5	3. 01	29. 025	73
	3 p. m. 9 p. m.	N. NE		3. 12	29. 382	42		3 p. m. 9 p. m.	W. NW North	2.5	3, 06 2, 96	29. 130	58
` 11	3 a. m. 7 a. m.	NE	. 1. 8	3.11	29, 322	.l. .	23	3 a. m. 7 a. m.	N. NE	. 3.8	3.00	29. 216	43
	9 a. m.	East	. 1. 3	3, 3, 13				9 a. m.	North .	. 2.9	2.97	1	
	2 p. m. 3 p. m.	North	. 3. 6		29. 214		1	2 p. m. 3 p. m.	NE	. 1. 1	3.01	29. 053	54
19	9 p. m. 3 a. m. 7 a. m.	North			29, 133	.	24		W. SW W. SW		3.06	29, 088	53
	7 a. m. 9 a. m.				29.078			7 a. m. 9 a. m.	w. sw	.l	3, 25	29. 138	47
	2 p. m.				29, 024	47		2 p. m. 3 p. m.			3, 25		45
	9 p. m.	N. NE .	0.4	3. 20	28, 995		li	9 p. m.	W. NW	li:	3 3. 23	29. 248	58

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND		water be- of 1838.	Inced	bed.			WIND		water bo. of 1838.	reduced o.	.pod
Date.	Hour of day.	Сопте.	Relative ve-	Height of wate low zero of I	Barometer reduced to 32°	Therm'r detached.	Date.	Hour of day.	Соптве.	Relative ve-	Hoight of water b low zero of 1838.	Barometer rec to 32º.	Theorem's detached
1861. April 25	3 a.m.	Calm	0.0	3, 25			1861. June 7	3 a. m.	Calm	a o	2.32		Į.
	7 a. m. 9 a. m.	E.SE	0.7	3, 28	29. 335	42		7 a. m. 9 a. m.	N. NE	1.8	2.36	29, 466	
	2 p. m. 3 p. m.	SE	3. 5		29. 296	48		2 p. m. 3 p. m.	N. NE	2.3		29, 458	
26	9 p. m. 3 a. m.	S. SE	2. 4 1. 5		29. 264	45	8	9 p. m. 3 a. m.	North	1.0 0.2		29, 466	
	7 a. m. 9 a. m.	E. NE	0. 5	2.86	29. 148	46	Ì	7 a. m. 9 a. m.	NE	i. i	2.40	29. 531	
	2 p. m. 3 p. m.	South	3.9		29.011	63		2 p. m. 3 p. m.	NE	i. i	2.39	29. 531	
27		South W. SW .	3. l 2. 0	2.69 2.69	28. 930	67	9	9 p. m. 3 a. m.	North	0.4	2.35 2.39	29. 511	٠.
	7 a.m. 9 a.m.	NW	2.9	2.74	28.918	51		7 a. m. 9 a. m.	N. NE	0.4	2.41	29. 546	
	2 p. m. 3 p. m.	NW	1.5		28. 960	58		2 p. m. 3 p. m.	8E	0.3		29. 508	
28		W.NW.	0. 3 0. 9		28. 979	50	10	9 p. m. 3 s. m.	Calm	0.0		29. 476	٠.
	7 a. m. 9 a. m.	s. sw	2. 1	3.07	29. 103	50		7 a. m.	8E	0.6	2. 46	29. 468	
	2 p. m. 3 p. m.	8W	3. 2		29. 055	67		2 p. m. 3 p. m.	SE		2.40	29. 383	
29		South			28.973	60	11	9 p. m. 3 a. m.	SW	0.0		29.310	
	7 a. m. 9 a. m.	West	1.6	2. 95	29, 177	63		7 a. m. 9 a. m.	sw	2.4	2.51	29. 260	٠.
	2 p. m. 3 p. m.	West			29. 176	54		2 p. m. 3 p. m. 9 p. m.	w.sw	2.8 1.1	2.48 2.49	29. 260	٠.
30		West Calm	0.0	2.96	29. 219	47	12	3 a.m.	NW	a 5	2.47	29. 305	٠.
	9 a. m.	West	1.6	2.98	29. 319	49		7 a. m. 9 a. m.	E. SE	i, i	2 53		
	2 p. m. 3 p. m.	W.NW		2.98		55 		2 p. m. 3 p. m. 9 p. m.	SE	1.5		29, 443	٠.
une 1	9 p. m. 3 a. m. 7 a. m.	North .		2.71 2.47 2.50	29. 484 29. 464	53	13		N. NE	1.5	2 44	29. 421 29. 593	٦.
	9 a. m. 2 p. m.			2. 50	29. 413	63		9 a. m. 2 p. m.	8E	0. 4		29.563	٠.
	3 p. m. 9 p. m.			2. 45 2. 40	29. 376	51		3 p. m. 9 p. m.	SE	0.8	2 43 2 40	29. 463	4.
2				2 44	29. 266	57	14	3 a. m. 7 a. m.	South	0. i		29. 238	
	9 a. m. 2 p. m.			2.40	29. 130	79		9 a. m. 2-p. m.	8. 8W .	4. 0	2.40	29. 140	
	3 p. m. 9 p. m.			2.40 2.43	29. 191	67		3 p. m. 9 p. m.	sw	5.7		29. 022	- ; .
3				2. 46	29. 246	66	15	3 a.m. 7 a.m.	sw	1.4		29.050	
	9 a. m. 2 p. m.			2 48	29. 256	77		9 a. m. 2 p. m.	w. sw	7. 0	2.57	29.002	
	3 p. m. 9 p. m.		1.5	2.56 2.51	29. 270	67		3 p. m. 9 p. m.	W. SW West	6.3		29, 165	
4	3 a. m.		3. 5	1	29. 335	50	16		North	3.0	2.43	29. 468	
	9 a. m. 2 p. m.	N. NE .	1	I	29. 373	49		9 a. m. 2 p. m.	N. NE	5. 7	2.37	29. 546	
	3 p. m. 9 p. m.	N. NE	2. 5 1. 5	2.31 2.25	29. 393	49		3 p. m. 9 p. m.	N. NE North	4.1		39. 551	-
	3 a.m. 7 a.m.	N. NE	1.4	2.31	29, 409	50	17	3 a. m.	NW		2. 35	29. 576	
	9 a. m. 2 p. m.	NE	2.3	2, 36	29. 391	59	1	9 a. m. 2 p. m.	NE	1.3	2.34	29. 526	
	3 p. m. 9 p. m.	N. NE	. 0.0	2.37	29. 367	49		3 p. m. 9 p. m.	SE	0.3	2.43	29, 404	
(3 a.m. 7 a.m.	Calm	0.0	2.42	29. 369	56	18	3 a. m. 7 a. m.	sw		2.46	29. 300	
	9 a.m. 2 p. m.	N. NE	1.4	2.36	29. 394	61		9 a. m. 2 p. m.		.		29. 255	
	3 p. m.	SE	. 1. 2	2.45	29. 494	54	ll .	3 p. m. 9 p. m.		. 1. 3	2.51	1	

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

			WIND		ter be-	luced	eđ.			WINI).	er be	reduced	hed.
Date	8.	Hour of day.	Course.	Relative ve-	Height of water low zero of 18	Barometer reduced to 320.	Therm'r detached	Date.	Hour of day.	Coarse.	Relative ve-	Height of water be low zero of 1838.	Barometer red to 32°.	Therm'r detached.
1861								1861.						
June	19	3 a.m. 7 a.m.	8. SW	0. 1	2. 47	29. 200	71	July 1	3 a.m. 7 a.m.	North	1.9	2. 30	29. 463	54
		9 a.m. 2 p.m.	South	0.8	2. 41	29. 135	79		9 a.m. 2 p.m.	North	5. 5	2. 27	29. 504	58
		3 p. m. 9 p. m.	sw	1.0 0.2	2. 35 2. 27	29. 225	73		3 p. m. 9 p. m.	N. NE North	4. 9 0. 6	2. 34 2. 39	29. 601	52
	20	3 a.m. 7 a.m.	N. NE	1.0	2.37	29. 251	58	2	3 a.m. 7 a.m.			2. 42	29, 639	57
		9 a.m.	NE	0.8	2. 29	29. 283			9 a.m.	W.NW	1.5	2.41	29. 576	68
		3 p. m.	8E	1.5	2. 29		59		3 p. m.	SE	1. 6 0. 1	2.46	l. 	
	21	3 a. m.	NW	1.8	2. 16 2. 19	29. 000	64	3	9 p. m. 3 a. m.	South	0. 0	2. 48 2. 53	29. 536	63
		7 a. m. 9 a. m.	North	1.5	2.22	29. 223	69		7 a.m. 9 a.m.	8w	1.7	2.46	29. 577	66
		2 p. m. 3 p. m.	w.sw	i. i	2.39	29. 298	74		2 p. m. 3 p. m.	SE	1.9	2. 44	29. 440	80
	22	9 p. m. 3 a. m.	W.NW.	0. 1 1. 0	2.40	29. 278	71		9 p. m. 3 a. m.	Calm South	0. 0 0. 2	2. 40 2. 44	29. 421	67
		7 a. m. 9 a. m.	NE	1.4	2.51	29. 343	70	-	7 a. m. 9 a. m.	8. 8W	2.3	2. 40	29. 391	65
		2 p. m.				29. 320	72		2 p. m.	 -	2.7		29. 307	85
	!	9 p. m.	W. SW	0.8 0.2	2.42	29. 268	68		3 p. m. 9 p. m.	SE	0.8	2. 42 2. 38	29. 272	74
	23	3 a.m. 7 a.m.	s. sw	1.0	2. 35	29, 308	 59	5	3 a.m. 7 a.m.	North	0.8	2. 51	29. 283	64
		9 a.m.	N. NE	2.6	2.40	29, 333	63		9 a.m. 2 p.m.	8W	1.7	2. 45	29, 255	79
		3 p. m. 9 p. m.	N. NE N. NE	3. 2 0. 7	2.31 2.36	29, 140	55		3 p. m. 9 p. m.	8W 8. SE	1.4 0.1	2. 45 2. 39	29, 230	70
	24	3 a.m.	Calm	0.0		29, 328		6] 3 a.m.			2. 46	29, 258	75
		9 a. m.	8E	0. 4	2.32		63	1	7 a. m. 9 a. m.	sw	0.4	2. 42		
		2 p. m. 3 p. m.	8E	1. 2	2. 23	29. 296	67	ĺ	2 p. m. 3 p. m.	8W	1.7	2. 43	29. 210	80
	25	9 p. m. 3 a. m.	8E	C. 3 0. 2	2. 25 2. 18	29. 251	••••	7	9 p. m. 3 a. m.	8.8W 8.8W	1.4 1.0	2. 44 2. 45	29. 195	73
		7 a. m. 9 a. m.	s. sw	2.1	2.32	29. 245	74		7 a.m. 9 a.m.	s. sw	2.9		29. 240	78
		2 p. m.	SE	1.9		29. 107	80		2 p. m.	8W	3. 5		29. 224	88
		3 p. m. 9 p. m.	SW	1.5	2. 29 2. 37	29, 208	70		3 p. m. 9 p. m.	8W	1.5		29. 224	78
	26	3 a.m. 7 a.m.	West	1.9		29. 351	63	8	3 a. m. 7 a. m.	sw	1.3	· • • • • • • • • • • • • • • • • • • •	29. 175	72
		9 a.m. 2 p.m.		3.0	2. 52	29, 398	71		9 a.m. 2 p.m.	sw	2. 2		29. 182	77
		3 p. m. 9 p. m.	West	3.0 0.4	2. 51 2. 50	29. 433	64		3 p. m. 9 p. m.	SW West	1.6		29. 170	69
	27	3 a.m.	West	0.5	2.40	29. 481	65	9	3 a. m. 7 a. m.	w.sw.	1.8		29. 208	69
		9 a.m.	sw	0.7	2.42	29. 465	74		9 a. m. 2 p. m.	NW	2. 1	2.48	29. 167	79
		3 p. m.	s. sw sw	1.5					3 p. m.	N. NE	2.0	2.38]. .	
	28	9 p. m. 3 a. m.	w.sw.	0. 5 1. 0	2.38 2.36	29. 458	66	10	9 p. m. 3 a. m.	North	1.0 2.7	2. 36 2. 29	29. 150	69
		7 a.m. 9 a.m.	E.SE	1.3	2.37	29, 461	67		7 a.m. 9 a.m.	North	2.9	2. 39	29, 291	58
		2 p. m. 3 p. m.	East	1.1	2.34	29. 428	68	}	2 p. m. 3 p. m.	North	3. 8	2. 18	29, 343	64
	29	9 p. m. 3 a. m.	Calm	0.0	2. 37	29. 428	61	n	9 p. m. 3 a. m.	NW Calm	0. 5 0. 0	2. 26 2. 24	29. 366	56
		7 a. m. 9 a. m.	SE		. 	29. 403	63	"	7 a. m. 9 a. m.	8E	0.7		29, 364	59
		2 p. m.		1. 1	2.38	29. 291	71		2 p. m.	l. 		2. 36	29. 284	67
		3 p. m. 9 p. m.	SE	1.5 0.0	2.37 2.41	29. 251	61		3 p. m. 9 p. m.	8E	1. 9 1. 3	2. 25 2. 33	29. 386	59
	3 0	3 a.m. 7 a.m.	West	0.9	2.40	29, 256	66	12	3 a.m. 7 a.m.	North	2.3	2. 27		56
		9 a. m. 2 p. m.	NW	1.8	•	29, 275	70		9 a.m. 2 p.m.	North	4, 6	2. 22	29, 453	62
		3 p. m. 9 p. m.	NE North	1.3	2.33 2.24	29. 338	56		3 p. m. 9 p. m.	North	4.5	2. 18 2. 23	29. 526	56
		~ p. m.	7101 PH	. ~ .	~ ~ ~ 7	~~. 000	, ~	1	, p			~ ~	, ~~. 040	, ~

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND	٠.	1. bg.	luced	5				WIND).	water be- of 1838.	roduced.	힣
Date.	Hour of day.	Course.	Relative ve-	Height of water be- low zero of 1838	Barometer reduced to 32°.	Therm'r detached.	Date	٥.	Hour of day.	Course.	Relative ve-	Height of water low zero of 1	58	Therm'r detached.
1861.							1861							1
July 1.	3 a.m. 7 a.m.			2. 21	29. 516	57	July	25	3 a.m. 7 a.m.	8W	1.8		29.323	
	9 a. m. 2 p. m.	NE	1.1	2. 27	29, 621	63			9 a. m. 2 p. m.	sw	3. 6	2.45	29. 259	
	3 p. m.	East	0. 5 0. 0		29. 586	59			3 p. m. 9 p. m.	8W		2.41 2.43	29. 257	٠
14	3 a. m.	Calm	0.0				,	2 6	3 a. m.	sw	Li			
	7 a. m. 9 a. m.	8. 8W	1.4	2.26	29. 536	61			7 a. m. 9 a. m.	w. sw	i. i	2.40	29. 256	
	2 p. m. 3 p. m.	8. 8W	1. 2	2. 23	29, 468	70			2 p. m. 3 p. m.			2.39	29.260	
1	9 p. m.	8. 8W 8W	1.7 0.5	2. 24	29. 380	65	١.	27	9 p. m. 3 a. m.			2.35 2.34	29. 32	•
-	7 a. m.	NW	 .	 	29. 296	67		~	7 a. m. 9 a. m.	E. 8E	0. 5		29. 336	6
	9 a. m. 2 p. m.		1.3	 .	29. 238	77			2 p. m.				29, 333	
	3 p. m. 9 p. m.	SE	1. 6 0. 1	2.29	29. 261	64			3 p. m. 9 p. m.	SE	1.6 1.5	2.11	29. 027	6
10	3 a.m. 7 a.m.	NW	9.0	2. 33	29, 281	64		28	3 a. m. 7 a. m.	NW	3.0	2.03	29.062	1
	9 a. m. 2 p. m.	NE	1.6	2. 3 6	29. 221	67			9 a. m. 2 p. m.	NW	4.3	1. 97	29. 361	
	3 p. m.	8E	1. 1	2.43					3 p. m.	NW		1.93	1	
17		West	0. 0 0. 5		29. 316	63	,	29	9 p. m. 3 a. m.	Calm	0.0		29, 225	
	7 a. m. 9 a. m.	West	1.6	2.45	29. 346	61			7 a. m. 9 a. m.	8w	1. o	2. 23	29. 321	٠
	2 p. m. 3 p. m.	sw	1.4	2.45	29. 283	74			2 p. m. 3 p. m.	8E	1.4	2.31	29.303	1
7.	9 p. m.	W. 8W.	1. 2	2.38	29. 278	73	Ι.	90	9 p. m.	8W	0.8	2.28	29. 233	ı
10	7 a. m.	sw	0.6		29. 288	69		30	3 a.m. 7 a.m.	sw		2. 25	29. 293	1
	9 a. m. 2 p. m.	sw	1.5	2.39	29. 227	82			9 a.m. 2 p.m.	8E	1.2	2.21	29, 231	•
	3 p. m. 9 p. m.	8W N. NE	2.0 1.1	2. 36 2. 25	29. 187	65	1		3 p. m. 9 p. m.	8W	2.8 1.9	2.12 2.08	29. 921	١,
19	3 a.m.	N. NW	0.3	2. 31	 .	63	:	31	3 a. m.	8W	2.5	2.04	29. 247	٠.
	7 a. m. 9 a. m.	N. NE	1.4	2. 32	29. 215				7 a. m. 9 a. m.	w.sw.	3.0	2. 15		
	2 p. m. 3 p. m.	East	0.4	2. 32	29. 200	75]	2 p. m. 3 p. m.	8w	2.1	2.06	29, 206	٠.
20	9 p. m.	N. NE N. NW.	1. 1 0. 4	2.23	29. 223	62	Aug.	1	9 p. m. 3 a. m.	w.sw.	0.5 1.6	2. 01 1. 95	29. 211	٠.
_	7 a. m. 9 a. m.	NE	1.0	2. 39	29. 363	61			7 a. m. 9 a. m.	w.sw	1.6		29. 299	į
	2 p. m.			. .	29. 376	70			9 p. m.	• • • · · • • • •			29. 333	Ï:
	3 p. m. 9 p. m.	BE	1.3 0.2	2, 40 2, 40	29. 403	60			3 p. m. 9 p. m.	8W	1.6 0.4	2·18 2·14	29, 231	1
21	3 a. m. 7 a. m.	Calm	0.0	2.40	29. 443	62		2	3 a.m. 7 a.m.	sw	0.8	2. 19	29, 287	٠.,
	9 a. m. 2 p. m.	E. SE	1.0	2. 36	29, 438	65			9 a.m. 2 p.m.	West	2.5	2. 23	29. 255	ŀ
	3 p. m.	East	1.5 0.4	2.30		57			3 p. m.	North NE	1.3 0.4	2. 20 2. 19	29. 319	1
25		N. NE N. NW.	0. 5	2.30 2.23	29. 486			3	9 p. m. 3 a. m.	North	0.6	2. 23		1
	7 a. m. 9 a. m.	N. NE.	4. 1	2. 28	29. 539	57			7 a.m. 9 a.m.	North	1.3	2, 20	29, 370	J.,
	2 p. m. 3 p. m.	North	4.3	2, 26	29. 556	63		1	2 p. m. 3 p. m.	SE	0.5	2 18	29, 330	
05	9 p. m.	N. NW.	0.8	2.30	29. 526	56			9 p. m.	8E 8. SW	0.1	2 10 2 15	29. 286	ľ
*	7 a. m.	Calm	0.0		29. 571	59		•	3 a.m. 7 a.m.		l l.	1	29, 217	7
	9 a. m. 2 p. m.	E. SE		2.39	29. 533	71			9 a.m. 2 p.m.	w. sw	3. Z	2. 19	29, 166	
	3 p. m. 9 p. m.	SE	1.5 0.0	2. 34 2. 37	29. 523	62		ļ	3 p. m. 9 p. m.	West North	3.2 2.1	2 14 2 18	29, 254	9
	3 a. m. 7 a. m.	Calm	0.0		29. 406	61		5	3 a.m. 7 a.m.	North	1.4	2.21	29. 335	
	9 a.m.	8W	1. 2	2.40					9 a. m.	N. NE	2.2	2.24		٠
	2 p. m. 3 p. m.	8E	2.0	2.31	29. 451	65		ļ		E. 8E			99. 32 2	
	19 p. m.	8. SE	1.6	2.41	29. 373	64	ł	- 1	9 p. m.	Calm	0.0	2.90	29, 335	•

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

	1	WINI) .	۳. 88.	reduced	Pg.			WIND		er be 838.	reduced o,	13
Date.	Hour of day.	Course.	Relative ve-	Height of water be- low zero of 1838.	Barometer rec to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low zero of 1838	Barometer red to 32º.	and the second
1861.							1861.		١				ľ
.ug. 6	3 a. m. 7 a. m.	Calm	0.0		29. 320	70	Aug. 18	3 a.m.	Calm	0.0	2.21	29.504	1
	9 a.m. 2 p.m.	8. SE	0.4	2. 18	29. 272	80		9 a.m. 2 p.m.	8E	1.4	2. 16	29. 440	1
	3 p. m.	8E	1.0 0.9		29. 463			3 p. m.	8E	2.1 0.1	2.15		
7	3 a. m.	North	1.1	2. 05 1. 99		74	19	9 p. m.	South NW	0.3	2.06 2.08	29. 420	١.,
	7 a. m. 9 a. m.	E. NE.	1.4	2.01	29. 255	68		7 a. m. 9 a. m.	North	1.3	2.09	29. 495	
	2 p. m. 3 p. m.	North	3.0	1. 95	29. 238	71		2 p. m. 3 p. m.		2.4	2.06	29. 485	1
_	9 p. m.	North	2.0	1.94	29. 285	66		9 p. m.	NE North	0.3	2.03	29. 539	1
8	3 a. m.	N. NE	0.4		29. 268	70	20	3 a.m.	North	0.7	2.04	29. 563	ľ
	9 a. m. 2 p. m.	NE	0.3	1.96	29. 260	71		9 a.m. 2 p.m.	N. NE	1.7	2.09	29. 533	ŀ
	3 p. m.	North	2.7	1.89				3 p. m.	East	0.8	2. 10		J.
9	9 p. m. 3 a. m.	North N. NW.	1. 5 0. 9	1. 94 1. 92	29. 300	66	21	9 p. m. 3 a. m.	SE South	0.1	2. 13 2. 13	29. 518	[.
	7 a. m. 9 a. m.	N. NE.	1.9	2.08	29. 328	67		7 a. m. 9 a. m.	NW	0. 5	2.20	29. 423	ľ
	2 p. m.	E. 8E	0.8		29. 330	72		2 p. m.		1.0	2.11	29. 330	1
	9 p. m.	Calm	0.0	2.19	29. 275	67		3 p. m. 9 p. m.	North	1.9	2.17	29.408	ľ
10	3 a.m.	W.NW.	0.1	2. 12	29. 233	67	22	3 a.m.	NW	0.7	2.17	29.488	ŀ
	9 a. m. 2 p. m.	N. NE	2.5	2.16	29. 268	70		9 a. m. 2 p. m.	N. NE	3. 2	2. 22	29. 553	ŀ
	3 p. m.	N. NE	4.3	2.01				3 p. m.	NE	2.0	2.20		
11	9 p. m. 3 a. m.	N. NE N. NE	3.9	1. 99 1. 89	29. 303	64	23	9 p. m. 3 a. m.	Calm	0.0	2.28 2.30	29. 588	
	7 a.m. 9 a.m.	NE	2.7	1. 90	29. 328	62		7 a. m. 9 a. m.	W. NW.	0.6	2.31	29. 621	ŀ
	2 p. m.		5. 2	1, 74	29.310	66		2 p. m.		1.9	2. 25	29, 591	T
	3 p. m. 9 p. m.	N. NE NE	3.6	1.73	29. 311	65		9 p. m.	SE	0.0	2.30	29. 581	1
12	3 a.m.	NE	3.9	1. 73	29. 266	57	24	3 a. m. 7 a m.	Calm	0.0	2. 25	29. 596	ŀ
	9 a.m. 2 p.m.	NE	3.9	1. 72	29. 336	63		9 a.m. 2 p. m.	w. sw.	1.0	2. 29	29. 565	ŀ
	3 p. m.	N. NE	6. 2					3 p. m.	8E	2.3	2. 23		١.
13	9 p. m. 3 a. m.	N. NE N. NE	4.0 2.7		29. 418	60	95	9 p. m. 3 s. m.	Calm	0.0	2. 26 2. 23	29. 514	ľ
	7 a. m. 9 a. m.	N. NE	3.3	1. 89	28. 551	56		7 a.m. 9 a.m.	sw	ŭ.3	2, 29	27. 531	
	2 p. m.		3.7		29. 576	63		2 p. m. 3 p. m.	8E	0.8	2. 27	29. 463	
	3 p. m. 9 p. m.	N. NE	1.0	1.98	29. 598	55		9 p. m.	8. SW	0. 1	2, 31	29. 451	1
14	3 a. m.	Calm	0.0		29. 621	54	26	3 a.m. 7 a.m.	Calm	0.0	2. 35	29. 448	ŀ
	9 a.m. 2 p. m.	E. SE	0.8	2.05	29. 601	67		9 a.m. 2 p.m.	8W	1.8	2. 39	29. 400	ŀ
	3 p. m.	8E	2.3 0.3	2.06 2.03				3 p. m.	8W		2.40		ŀ
15	9 p. m. 3 a. m.	8E		2 10	29. 576	63	27	9 p. m. 3 s. m.	8W	1.0	2. 44 2. 48	29, 368	
	7 a. m. 9 a. m.	sw	1.9	2. 13	29. 543	61		7 a. m. 9 a. m.	w. nw.	1.6	2.42	29. 385	l.
	2 p. m. 3 p. m.	sw	l l		29. 478	73		2 p. m. 3 p. m.	8E	1.5		29. 390	
	9 p. m.	S. SW	0.7	2 20 2 26		65		9 p. m.	Calm	0.0	2.32	29, 425	T
16	3 a. m.	w.sw.			29. 433	64	28	3 a.m. 7 a.m.	Calm		2. 34	29. 443	ŀ
	9 a.m. 2 p. m.	West	2.2	2. 20	29. 415	77		9 a. m. 2 p. m.	W. NW.	1.0	2. 35	29. 443	ŀ
	3 p. m.	NE Calm	1.3	2. 19	29. 460	65		3 p. m.	W. SW. West	1.2	2.34 2.33	29. 435	ŀ
17	9 p. m. 3 a. m.	Calm	0.0	2.13			29	9 p. m. 3 a. m.	W. NW.	0.5	2.35		J.
	7 a. m. 9 a. m.	8E		2. 17	29. 473	62		7 a. m. 9 a. m.	NE	0.9	2. 35	29. 471	
	2 p. m.	1	1 1		29. 480	72		2 p. m.	E SE	1.2	2 33	29. 478	
	9 p. m.	8E	0.0	2.20	29. 463	68		9 p. m.	Calm	0.0	2.35	29. 471	T

TABLE AA -Showing the wind, water, barometer, &c .- Continued.

		WIND		water be of 1838.	duced	peq.			WIND		water be-	reduced
Date.	Hour of day.	Course.	Relative ve-	Height of water low zero of 183	Barometer reduced to 32º.	Therm'r detached.	Date.	Hour of day.	Course,	Relative ve-	Height of water low zero of 183	Barometer reduce to 32°,
1861. Lug. 30	3 a. m.	West	0. 5	2. 28	29. 523	57	1861. Sept. 11	3 a.m.	NW	0.4	2.17	29, 303
	9 a, m. 2 p. m.	N. NE	1.7	2. 35	29. 546	67		9 a. m. 2 p. m.	West	0, 1	2.24	29, 306
	3 p. m. 9 p. m.	E.NE	1.7 0.2	2.35 2.32	29. 526	63		3 p. m. 9 p. m.	North Calm		2.34 2.41	29.333
31		SE	0. 9		29. 523	65	12	3 a. m.	Wost	0.3		29.366
	9 a. m. 2 p. m.	8E	2.2	2.31	29. 450	71		9 a. m. 2 p. n.	8w	1.0	2.54	29.368
	3 p. m. 9 p. m.	8E	2.6 1.0		29. 398	69		3 p. m. 9 p. m.	8W	1. 2 0. 2	2.59 2.54	29.343
ept. 1		8.8W	0. 7	2.31	29. 325	67	13	3 a. m.	8w	0.5		29.356
	9 a. m. 2 p. m.	sw	1.4	2.30	29. 258	69	ŀ	9 a. m. 2 p. m.	sw	2.0	2. 57	29.308
	3 p. m. 9 p. m.	S.SW Calm	2. 0 0. 0		29. 305	67		3 p. m. 9 p. m.	8E E.SE	1.6	2.58 2.49	29.295
2		8E	0. 9		29. 308	65	14	3 a. m. 7 a. m.	8w	0.4	2.47	29.90
	9 a. m. 2 p. m.	SE	1, 1	2.39	29. 235	75		9 a. m. 2 p. m.	North	1. 5	2.40	29, 260
	3 p. m. 9 p. m.	E. NE	1. 1 0. 0	2. 56 2. 39	29. 285	66	ļ	3 p. m. 9 p. m.	E.SE	0.8	2.43 2.35	
3		Calm West	0. 5		29. 453	62	15	3 a. m.	North	2.7	2.30	29.590
	9 a. m. 2 p. m.	NW	3.0	2.50	29. 483			9 a. m. 2 p. m.	North	3.9		29.63
	3 p. m. 9 p. m.	NW	1.7	2. 49 2. 44	29. 595	73 62		3 p. m. 9 p. m.	N. NE	3. 4		.]
4		Calm E.NE	1.4		29, 691	62	16	3 a. m. 7 a. m.	E.NE	0. 6		
	9 a. m. 2 p. m.	8E	1.3	2. 47	29. 649	65	'	9 a. m. 2 p. m.	NE	i. i		29.43
	3 p. m.	SE	1. 5 0. 0		29, 646	 59		3 p. m. 9 p. m.	E. NE	1.1		
5	3 a. m.	Calm	0. 0		29. 421		17	3 a. m. 7 a. m.				29.38
	9 a. m.	8w	2.0	2.51	29. 121	57		9 a.m.	N. NE	1.6	š	
	3 p. m.	sw	2.4	2.54		71		3 p. m.	E. SE	0. 7		
6	9 p. m. 3 a. m. 7 a. m.	W. NW. West	0. 6 0. 6		29. 191	65	18	9 p. m. 3 a. m. 7 a. m.	8. SE 8. SW	0.1	3	
	9 a. m.	West	2.9	2.61		58		9 a.m.	sw	2. 8	3,	
	3 p. m.	West	2. i	2. 53	29. 278	65		3 p. m.	8.8W		l	. ¹
7	9 p. m. 3 a. m. 7 a. m.	NW North	1.7 2.8	2.48 2.31	29. 356	60	19	9 p. m. 3 a. m. 7 a. m.	South	3. 0) ,	29. 13
	9 a. m. 2 p. m.	N. NE	3. 3	2.31	29. 571	56		9 a. m. 2 p. m.	w.sw	2.9	3	29, 21
	3 p. m. 9 p. m.	N. NE	3. 4 1. 4	2. 27 2. 23	29. 682	58 		3 p. m. 9 p. m.	E. NE	1.5	2. 56 2. 51	29.30
8	3 a. m.	East	1. 7	2 22		58	20	3 a. m. 7 a. m.	East	i. i	2. 45	29.24
	9 a. m.	NE	1.7	2. 33	29. 730	62		9 a. m. 2 p. m.	N. NE	2.8	2. 20	29.16
	3 p. m.	NE	2.2	2.28	29. 729			3 p. m.	North	4.4	1.97	
9	3 a.m.	E. NE Calm	0.0	2.34	29. 729	57	21	3 a.m.	North	. 22	2.05 2.03	29. 51
	7 a. m. 9 a. m.	E.SE	1.4	2.38	29. 745	55		7 a.m. 9 a.m.	N. NW	1.5	2.18	29. 50
	2 p. m. 3 p. m.	SE	1.3	2. 35	29.692	64		2 p. m. 3 p. m.	E.SE	. 0. 5	2 22	
10		8E	. 1. 3	2.33	29.588	62	22	9 p. m. 3 a. m.	Calm	0.0	2 37 2 40	29. 374
	7 a. m. 9 a. m.	8E	1.3	2.31	29. 428	63		7 a. m. 9 a. m.	w.sw	. 127	2, 56	
	2 p. m.		1		29. 248	64	l!	2 p. m.	W.8W West	· ·:·:		29, 967

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

			WINE	٠.	er be-	reduced	bed.			WINI),	er be-	luced	7
Da	ste.	Hour of day.	Course.	Relative ve-	Height of water be- low zero of 1838.	Barometer red to 320.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water be low zero of 1838.	Barometer reduced to 32°.	Thorn's detached
186 Sept.		3 a. m.	w.nw.	0.8	2. 44			1861. Oct. 5	3 a. m.	8W	0.7	2. 13		
		7 a.m. 9 a.m.	W.NW.	0.9	2. 46	29. 406	51		7 a. m. 9 a. m.	W. NW	2.9	2.29	29. 113	6
		2 p. m. 3 p. m.	SE	0.8		29, 406	65		2 p. m. 3 p. m.	West	2.1	2.34	29. 181	6
	24	9 p. m. 3 a. m.	Calm	0.0	2. 37 2. 34	29. 393	64	6	9 p. m. 3 a. m.	NW	0. 3 0. 4		29. 258	5
		7 a.m. 9 a.m.	s. sw	1. 9	2.40	29. 424	56		7 a. m. 9 a. m.	NW	0.4	2. 45	29. 252	4
		2 p. m. 3 p. m.	South	2.7	2. 39	29. 328	72		2 p. m. 3 p. m.	8E	1.5	2.38	29. 279	5
	25	9 p. m. 3 a. m.	South 8, 8W	2.7 1.2 0.5	2. 46 2. 44	29. 298	65	7	9 p. m. 3 a. m.	8W	0. 3 1. 5	2. 45 2. 44	29. 354	5
		7 a. m. 9 a. m.	sw	1. 2	2. 47	29. 228	60		7 a.m. 9 a.m.	s. sw	2.0	2.44	29. 397	5
		2 p. m. 3 p. m.	s. sw	2.0		29. 156	64		2 p. m. 3 p. m.	s. sw		2.42	29. 351	6
	26	9 p. m. 3 a. m.	West W. SW.	1.3 0.7	2. 42 2. 39	29. 176	55	8	9 p. m. 3 a. m.	sw	0. 3 0. 1	2. 42 2. 44	29. 377	5
		7 a. m. 9 a. m.	w.sw.	2.2	2. 44	29. 157	48		7 a.m. 9 a.m.	South	0.9	2. 44	29. 471	4
		2 p. m. 3 p. m.	W. 8W.	2.3	2.54	29. 156	55		2 p. m. 3 p. m.	sw	2. 2 1. 2	2.41	29. 428	7
	27	9 p. m. 3 a. m.	W. SW. West	1.0 0.4		29. 188	49	9	9 p. m. 3 a. m.	South 8. 8W		2. 42 2. 38	29. 448	6
		7 a.m. 9 a.m. 2 p. m.	N.NW.	2.6	2. 53	29. 204	45		7 a.m.	South	1.9	2.40	29. 531	5
		3 p. m.	NW	2. 4 1. 3	2.54	29. 179	50		2 p. m. 3 p. m.	SE	2.0		29. 458	6
	28	9 p. m. 3 a. m. 7 a. m.	W.NW.	0.7	2. 50 2. 53	29. 288	46	10	9 p. m. 3 a. m.	S. SE	0. 2	2. 38 2. 38	29. 416	5
		9 a. m. 2 p. m.	W.NW	2.0	2, 51	29. 344	41		7 a. m. 9 a. m.	8E	1. 2	2. 35	29. 386	5
		3 p. m. 9 p. m.	W.SW.	1.5	2. 56	29. 431	51		2 p. m. 3 p. m.	South	21	2. 36	29. 263	6
	29	3 a. m. 7 a. m.	W.SW.	0. 2 0. 5	2. 48 2. 57	29. 519	44	11	9 p. m. 3 a. m.	West W. SW.	3.0 2.3	2. 42 2. 44	29. 258	4
		9 a. m. 2 p. m.	NW	0.7	2, 47	29. 640	45		7 a. m.	w.sw.	0.4	2. 56	29. 294	4
		3 p. m. 9 p. m.	North.	0. 2 0. 4	2.47 2.36	29. 702	53		2 p. m. 3 p. m.	sw		2. 69	29. 297	5
	30	3 a. m. 7 a. m.	N. NE		2.38	29. 755	48	12	9 p. m. 3 a. m.	West	1.1	2.81 2.70	29, 040	5
		9 a. m. 2 p. m.	N. NE	1.0	2.30	29. 815	46		7 a. m. 9 a. m.	NW	4. 2	2, 77	29. 046	4
		3 p. m. 9 p. m.	8E	1.8 2.6	2. 36 2. 29	29. 774	55		2 p. m.	W.NW.		2. 66	29.088	5
Dct.	1	3 a. m.	8E	2.4	2.31	29. 499	55	13	9 p. m. 3 a. m. 7 a. m.	Calm		2. 63 2. 75	29. 234	4
		9 a. m. 2 p. m.			2.30	29, 308	65	1	9 a. m. 2 p. m.	West	0.3	2.72	29. 342	3
		3 p. m. 9 p. m.	8. SE	1.9 2.8	2.35 2.33	29. 218	65		3 p. m. 9 p. m.	SE	0. 9		29, 359	5
	2	3 a. m. 7 a. m.	sw	2.6	2 40	29. 186	62	14	3 a. m. 7 a. m.	w.sw.	0. 7		29. 292	. 5
		9 a.m. 2 p.m.	w.sw.	3, 1	2.48	29. 218	66		9 a. m. 2 p. m.	N. NE	3, 4	2.44	29. 384	5
		3 p. m. 9 p. m.	w.sw nw	3.8 1.0		29. 391	53		3 p. m. 9 p. m.	N. NE	1. 1 0. 0	2. 54 2. 49	29, 404	1
	3	3 a.m.	Calm	0.0		29. 504	44	15	3 a. m. 7 a. m.	Calm	0.0	2.50	29. 387	
		9 a. m. 2 p. m.	NE	0.8		29. 463	52		9 a. m. 2 p. m.	8W	0.3		29. 351	6
		3 p. m. 9 p. m.	NE E. NE	2.0 1.6	2.49 2.37	29. 331	54		3 p. m. 9 p. m.	Calm	0.0	2. 40 2. 31	29. 357	6
	4	3 a.m. 7 a.m.	NE	1.0		29. 326	56	16	3 s. m. 7 a. m.	8W	1.4	2.36	29. 356	
		9 a.m. 2 p. m.	N. NE	0.6	2. 22	29. 218	. 58		9 a. m. 2 p. m.	sw		2. 35		5
		3 p. m.	E. NE East	1.1	2.14		59		3 p. m. 9 p. m.	8.8W	1.8	2.49	29. 311	6

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

	1	WIND		water be- of 1838.	li con	ped.				WIN). 	water be-	Anced	bed.
Date,	Hour of day.	Course.	Relative ve-	Height of water low zero of 183	Barometer reduced to 63°.	Therm'r detached.	Date	٠.	Hour of day.	Course.	Relative ve-	Height of water low sero of 1	Barometer reduced to 62°.	Therm'r detached.
1861.		a .					1961	•					98. 86 3	4
Oct. 17	3 a. m.	Calm	0.0		29, 284	56	Oct	29	7 a.m. 9 a.m.	8W	3.4	264		
	9 a. m. 2 p. m.	N.NE.	2.5	2.39	29. 301	50			2 p. m. 3 p. m.	8.8W	3.8	2.61	28.718	.
	3 p. m. 9 p. m.	N.NE North	5.4 5.2 4.0	2. 28 2. 21	29. 368	48		30	9 p. m. 3 a. m.	West	2.8 2.3	2.44 2.57	28.658	3
18	3 a. m. 7 a. m.	North			29.374	46	ļ		7 a. m. 9 a. m.	West	3.9	2.65	28.826	4 -
	9 a.m. 2 p.m.	North	3.0	2.25	29. 324	50			2 p. m. 3 p. m.	West	3.2	2.74	28.966	١
	3 p. m. 9 p. m.	W.SW	1.9 0.3	2. 29 2. 43	29.318	43		31	9 p. m. 3 a. m.	West	3.2 2.0 1.4	2.71 2.76	29. 169	1
19	3 a.m.	w.sw.	1.8	2.51	29. 297	44		•-	7 a.m. 9 a.m.	w.nw.	1.0		29. 302	د ا
	9 a. m.	w. nw.	3.6	2.61	29. 312	58			2 p. m. 3 p. m.	w.sw	0. i	2.74	29. 484	6
	3 p. m.	West	3.4	2.65			W	,	9 p. m.	Calm	0.0	2.60	29. 564	4
90	9 p. m. 3 a. m.	W.8W.	0. 5 0. 1	2.70 2.68	29.416	48	Nov.	1	7 a.m.	N.NE	- <i>-</i>	l	29.617	
	7 a. m. 9 a. m.	South	0.4	2.71	29. 557	37			9 a.m. 2 p.m.	NE	1. 9 3. 0		29. 546	٠, ٠
	2 p. m. 3 p. m.	8E	1.4	2.59	29. 534	54			3 p. m. 9 p. m.	NE	1.8	2 40	29. 458	
91	9 p. m. 3 a. m.	SE S.SE	0.8 1.0	2 49 2 42	29. 469	49		2	3 a. m. 7 a. m.	NE	3.4		29.317	.,
	7 a. m. 9 a. m.	SE	1.6		29. 477	45			9 a.m. 2 p.m.	N.NE	4. 5	2.31	29.251	Ti
	2 p. m. 3 p. m.	8E	1.4	2.33	29. 349	54			3 p. m. 9 p. m.	North North	4.7		29. 254	. (
92	9 p. m.	Calm	0.0	2, 33	29. 293	48		3	3 a.m.	N.NW	1.4	2 49	29, 290	-
*2	7 a. m.		0.0		29. 207	44			9 a. m. 2 p. m.	NW	2.4	2.65	29. 171	.j.,
	9 a. m. 2 p. m.	West	2.0	2. 46	29. 229	48			3 p. m.	West	2 4 0.6	2.76	I	
	3 p. m. 9 p. m.	West	2.9 2.7	2.50 2.67	29. 314	40		4	9 p. m. 3 a. m.	West	u.o	2.85 2.88	29. 186	
23	3 a.m.	W.NW.	2.3	2.71	29. 337	39			7 a. m. 9 a. m.	• • • • • • • • •		2.94	29, 192	٠.,
	9 a.m. 2 p. m.	W.NW.	22	2.77	29. 389	41			2 p. m. 3 p. m.	8.8W	0.6		29, 043	
	3 p. m. 9 p. m.	NW West	2.4 0.6	2 77 2 77	29. 547	36		5	9 p. m. 3 a. m.	W.SW.	1.0 0.3	2.93 2.81	28.996	4-
24	3 a. m. 7 a. m.	Calm	0.0	2.70	29.660	31			7 a. m. 9 a. m.	8W	0.4	2.84	29.036	13
	9 a. m.	8.SW	0.8	2.70					2 p. m.	8.8W .			28. 863	
	2 p. m. 3 p. m.	South	1.7	2.63	29. 571	51			9 p. m.	Calm	0.9	274	28.891	
95	9 p. m. 3 a. m.	South S.SW	1.7 1.5 2.0	2.75 2.78	29. 543	45		6	3 a.m. 7 a.m.	W.NW.	0.9		29, 169	
	7 a. m. 9 a. m.	sw	2.1	2.83	29. 462	43			9 a. m. 2 p. m.	NW	2.3		29, 298	
	2 p. m. 3 p. m.	sw	0.9	2.78	29, 466	59		İ	3 p. m. 9 p. m.	W.NW.	1.7 0.0	2.80 2.85	29. 374	
26	9 p. m. 3 a. m.	Calm West	0.0	2.78	29.528	50		7	3 s. m. 7 s. m.	Calm	0.0		29, 385	٠'٠
•	7 a. m. 9 a. m.	NW	1.2		29. 644	39			9 a.m. 2 p.m.	8.S₩	1.0		29, 234	
	2 p. m. 3 p. m.	NW	1.4	2.69	29. 682	50			3 p. m. 9 p. m.	SW	1. 0 0. 0	2.87 2.75	29, 199	ŀ
27	9 p. m.	Calm	0.0	2.67	29. 744	36		8	3 a. m.	NW	0.5	2.68	29, 909	'n
*7	3 a.m.	Calm			29.786	29			9 a.m.	North		2.58	29, 250	١.,
	9 a. m. 2 p. m.	Calm	0.0		29. 662	49			3 p. m.	NE	0.6	2.67 2.74	29. 318	: ;
	3 p. m. 9 p. m.	SE South		2.69	29. 559	42	Ì	9	9 p. m. 3 a. m.	Calm	0.0	2.74		1
28	3 a. m. 7 a. m.	8W	0.1	2.74	29, 447	38	1		7 a. m. 9 a. m.	Calm	a.o	2, 89	29.370	
	9 a. m. 2 p. m.	8.8W	1.6		29. 316	55			3 p. m. 3 p. m.	8.8W	0.5	2.90	29. 294	.
	3 p. m. 9 p. m.	SE South	2.0	2.75 2.50	29. 224	51		10	9 p. m. 3 a. m.	8.8W South	0.2	2.90 2.89 2.88	29, 291	
99	13 a. m.	South	1.6	2 65	25. 227	l			7 a.m.				29. 001	#

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WIND	٠.	58	1 Pes	뒇			WIND		7 P. 20	reduced	3
Date.	Hour of day.	Course.	Relative ve-	Height of water be low zero of 1838.	Barometer reduced to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve- locity.	Height of water be- low zero of 1838.	Barometer red to 62°.	Therm'r detached
1861.							1861.						
Nov. 10	9 a.m. 2 p.m. 3 p.m.	South	3. 5		28. 800	54	Nov. 20 21	3 p. m. 9 p. m. 3 a. m.	W.8W W.8W W.8W	3. 2 1. 0 0. 2	2.83	29. 326	4
11	9 p. m.	West	3.1	2 64 2 87	28. 998	47	21	7 a. m. 9 a. m.	8E	0.5		29. 501	3
11	7 a. m.				29. 335	.39		2 p. m.	8.8E		2.90	29. 441	4
	9 a.m. 2 p. m.	West	3.4	2.92	29. 536	46		9 p. m.	8.8E	2.4 2.4	2.93	29. 334	4
	3 p. m. 9 p. m.	West	0.6	3, 06	29, 604	35	29	3 a. m.	8.8E		2.78	29.063	4
12	3 a.m.	Calm	0.0		29. 570	35		9 a.m. 2 p.m.	8.SE	5.5		28. 891	3
	9 a. m. 2 p. m.	SE	1.5		29. 349	48		3 p. m. 9 p. m.	West W.SW	3.5 3.9	2. 67 2. 73	28. 856	3
	3 p. m. 9 p. m.	SE	0.8	2.81 2.64	20.318	47	23	3 a.m.	w.sw .	4.2		28. 751	2
13	3 a.m. 7 a.m.	Calm	0.0		29. 467	38		9 a.m. 2 p.m.	w.sw	4.4	3.00	28. 703	2
	9 a.m. 2 p.m.	NW	0.7	2.72	29, 504	50		3 p. m. 9 p. m.	W.8W West	3.7	2.93 2.79	28. 753	2
	3 p. m. 9 p. m.	N.NW	0.4	2.83 2.85	29. 588	38	94	3 a.m.	W.NW.	2.3	2.70	28. 961	3
14	3 a. m. 7 a. m.	E.NE	1, 1	2.88	29. 497	40		9 a.m. 2 p.m.	N.NW	2.8	2.60	28, 958	1.3
	9 a. m. 2 p. m.	East	0.4	2.89	29, 306	40		3 p. m. 9 p. m.	W.NW	2.4	2.65 2.74	29. 045	3
	3 p. m. 9 p. m.	NE	0.4	2.80 2.74	29, 258	39	25	3 a.m.	W.NW.	3.9	2.83	29, 086	. 3
15	3 a.m.	NW	0. 2 0. 1	2.65	29, 322	34		9 a. m. 2 p. m.	N.NW	2.1	2.76	29. 307	. 3
	9 a.m.	NW	1. 9	2.68	29, 429	38		3 p. m. 9 p. m.	N.NW	1.5 0.2	2.78 2.77	29. 328	1.3
	3 p. m. 9 p. m.	N.NW .	1.3	2.68 2.72	29. 587	30	26	3 s. m.			2.81	29. 412	13
16	3 a.m.	N.NW	0.4	2 78	29, 675	24	•	9 a.m. 2 p. m.	8W	1.0	2.82	29. 367	3
	9 a.m. 2 p. m.	NW	0.7	2.87	29. 666	35		3 p. m. 9 p. m.	S.SW South	2.3 2.7	2.92 2.88	29. 245	.3
	3 p. m. 9 p. m.	N.NE	0.7	2.96 2.92	29. 624	30	27	3 a. m.	w.sw.	2.8	3.07	29, 137	-
17	3 a. m.	NW	0.1	2.88	29, 581	32		9 a. m.	W.NW.	4.9	3,00	29. 246	i
	9 a. m. 2 p. m.	N.NE	0. 2	2.81	29.517	39		3 p. m.	West	2. 7 0. 9	3.02 2.98	29. 367	ŀ
	3 p. m.	Calm	0.0	2.77			28	3 a. m.	Calm	0.0	3, 12	89. 318	2
18	9 p. m. 3 a. m.	N. NW.	0.0	2.77 2.74 2.74	29. 593	34		7 a. m. 9 a. m.	8.8W	0.i	2.99		١.,
	7 a.m. 9 a.m.	E.NE	1.2	2.78	29. 572	33		2 p. m. 3 p. m.	8.8E	2 2 2 3	2.91	29. 118	3
	2 p. m. 3 p. m.	E.NE.	1.3	2.80	29. 606	40	29	9 p. m. 3 a. m.	W.SW.	3.6	2.99 2.96	28. 958	3
19	9 p. m. 3 a. m.	E.SE	1.3 1.6 3.6	277	29. 581	40		7 a. m. 9 a. m.	W.NW.	3.3	2.89		2
	7 a.m. 9 a.m.	8E	4.9		29. 470	41		2 p. m. 3 p. m.	W.NW	i. 9	3, 00	29, 304	2
	2 p. m. 3 p. m.	8E	5. 5	2. 53	29. 209	43	30	9 p. m. 3 a. m.	W.NW.	2.9 3.3	3. 13 3. 14	29. 346	1
20	9 p. m. 3 a. m.	S.SE South	4. 4 2. 9	2.65	29. 043	43		7 a. m. 9 a. m.	West	i.i		29. 411	0
	7 a. m. 9 a. m.	8W	J		28.963	43		2 p. m. 3 p. m.	West	0.2		29. 372	1
	2 p. m.				29, 126	43		9 p. m.	Calm	ãõ		29. 315·	ì

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND	WIND.		duced	shed.			WINE) .	above suge.	queeq	bed.
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-guage.	Barometer reduced to 32°,	Therm'r detached.	Date.	Hour of day.	Соптво.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 32°.	Therm'r detached.
1861. Dec. 1	3 a. m.	NW	0. 6				1861. Dec. 13	9 a m	w.sw.	1.4			
Dec. 1	7 a. m. 9 a. m.	w.nw.	2. 2		29. 306	5	Dec. 13	3 a. m. 7 a. m. 9 a. m.				29. 734	
	2 p. m. 3 p. m.	West	2.5		29. 330	12		2 p. m. 3 p. m.	8W	0.8		29. 644	
2	9 p. m. 3 a. m.	NW	2.3 0.3		29. 472	2	. 14	9 p. m.	w.sw.	21		29. 583	Ī
~	7 a. m. 9 a. m.	West	0.7		29. 529	4	14	3 a. m. 7 a. m. 9 a. m.	w.sw.	1.6		29.497	T
	2 p. m.				29. 495	9		2 p. m.				29. 648	1
_	9 p. m.	NW Calm	1. 0 0. 0		29. 555	2		3 p. m. 9 p. m.	N. NE N. NE	3.5 0.9		29. 534	1
3	3 a. m.	Calm	0.0		29. 564		15	3 a. m. 7 a. m. 9 a. m.	NE	1.7		29, 879	٠.
	9 a. m. 2 p. m.	West	0, 5		29. 507	23		2 p. m.	8E	2.0		29.780	
	3 p. m. 9 p. m.	8W	1.6		29. 440	20		3 p. m. 9 p. m.	8.8W	1.1	' 	29. 679	٦.
4	3 a. m. 7 a. m.	8W	2.0		29. 283	21	16	9 p. m. 3 a. m. 7 a. m.	w.sw.	3.7		29. 495	J.
	9 a. m. 2 p. m.	8W	2. 4		29. 210	38		9 a. m. 2 p. m.	8W	4. 6		29. 454	٠.
	3 p. m. 9 p. m.	8W	2. 1 0. 0		29. 300	30		3 n. m.	W.8W.	2. 1 0. 4		29. 551	١.
5	3 a. m. 7 a. m.	Calm	0.0		l. 		17	9 p. m. 3 a. m.	8E	0.3		ļ	-1-
	9 a. m.	Calm	0.0		29. 422	27	Ì	7 a. m. 9 a. m.	West	1. 2		29, 492	÷
	2 p. m. 3 p. m.	Calm	0.0	••••	29. 427	39	l	2 p. m. 3 p. m.	8W	2.2		29. 291	
6	9 p. m. 3 a. m.	SW	0.0 2.8		29. 412	40	18	9 p. m. 3 a. m.	8W	2. 3 0. 3		29. 381	
	7 a. m. 9 a. m.	8W	2.9		29, 464	47		7 a. m. 9 a. m.	8E	0. 5		29. 587	j
	2 p. m. 3 p. m.	8W	2.5		29. 406	55		2 p. m. 3 p. m.		1.0		29. 554	ľ
7	9 p. m. 3 a. m.	8W	2.6		29. 426	54	19	9 p. m. 3 a. m.	w	1.0		29. 431	
•	7 a. m.				29. 307	54	1.5	7 a. m.	.			29. 203	i,
	9 a. m. 2 p. m.	8W	1.3		29. 254	54		9 a. m. 2 p. m.	N. NW	1.8		29. 329	,
	3 p. m. 9 p. m.	8W W. 8W	0. 7 0. 5 0. 8		29. 304	52		3 p. m. 9 p. m. 3 a. m.	N. NW.	1.6 2.8 3.7		59.50	i
8	3 a. m.	W. 8W.		· • • • • • • • • • • • • • • • • • • •	29. 340	44	90	7 a. m.	NE			29.771	i
	9 a. m. 2 p. m.	W.NW.	0.6		29. 309	44		9 a. m. 2 p. m.	NE	2.9		29.743	
	3 p. m. 9 p. m.	8W	0.6		29. 299	41		3 p. m.	N. NE NE	2.3 0.9		29, 939	-;
9	3 a. m.	8W	0, 1		29. 151	48	21	9 p. m. 3 a. m. 7 a. m.	Calm	0.0		29. 958	-1
	9 a. m. 2 p. m.	8W	1.4		29. 030	57	1	9 a. m.	NW	0.4		29. 870	-1
	3 p. m. 9 p. m.	8W	1. 1 0. 6					2 p. m. 3 p. m. 9 p. m.	W.NW.	0.3		29. 784	J.
10	3 a. m.	8w	0.8		29.050	52	223	3 a. m.	Calm NE	0.5			-1
	7 a. m. 9 a. m.	8W	0.3		28. 953	55		7 a. m. 9 a. m.	8.8E	2.4		29, 584	
	2 p. m. 3 p. m.	NW	5. 1 3. 7		29, 075	47		2 p. m. 3 p. m.	8E	3.4		29, 362	.1.
11	9 p. m. 3 a. m.	W. NW.	3.7		29. 523	31	23	9 p. m. 3 a. m.	E. SE E. NE	5. 2 4. 1		29, 229	
	7 a. m. 9 a. m.	NW	1.3		29, 897	18		7 a. m. 9 a. m.	NE	2.9		29, 413	
	2 p m. 3 p. m.	West			29, 895	29		2 p. m. 3 p. m.	NE	0.8		29, 461	, .'.
12	9 p. m.	West W.SW	. 0. 7		29. 879	26	94	9 p. m. 3 a. m.	N. NW.			29.638	1
	7 a. m.				29. 892	27		7 a. m.	l			29, 433	
	9 a. m. 2 p. m.	W.SW.			29. 815	40		9 a. m. 2 p. m.	W. SW			29.306	į.
	3 p. m. 9 p. m.	8W	2.8		29.789	33		3 p. m. 9 p. m.	8W West	2.1 0.9		29. 316	ľ

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

Date.		WINI).	above suge.	dueed	bed.			WIND		above inge.	reduced	bed.
	Hour of day.	Course,	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 320.	Barometer reduce to 32º. Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer re- to 32°.	Therm'r detached.
1861. 25	3 a. m.; 7 a. m.	N. NW.	0.1		29, 363	27	1862. Jan. 6	3 a. m.	NE	0. 4			
	9 a. m. 2 p. m.	E. NE.	0. 1		29. 319	33		7 a. m. 9 a. m. 2 p. m.	North	0.7		29. 241 29. 331	18
	3 p. m. 9 p. m.	South SW W.SW.	1.4 3.0 2.3		29. 226	36		3 p. m. 9 p. m.	N. NW. N. NW	1.6 0.3		29. 541	21
26	3 a. m.				28. 939	38	7	3 a. m. 7 a. m.	Calm	0.0		29, 707	6
	9 a. m. 2 p. m. 3 p. m.	sw	2.2 5 9		29. 003	29		9 a. m. 2 p. m. 3 p. m.	Calm	0.0	•••••	29. 685	21
27	9 p. m.	W.SW.	7.1		29. 372	19	8	3 p. m. 9 p. m. 3 a. m.	West W.8W. W.8W.	0.3 0.3 1.7	•••••	29. 614	20
	3 a. m. 7 a. m. 9 a. m.	sw	1.0		29. 862	10		9 a. m.	W.NW.	1.0		29. 263	27
	2 p. m. 3 p. m.	8. SE	0. 1	· · · · · · ·	29. 940	19		2 p. m. 3 p. m.	NW	0.7		29. 222	37
28	9 p. m. 3 a. m.	8. SE 8. SE	0. 5 1. 3		29, 899 29, 725	19 21	9	9 p. m. 3 a. m.	Calm N. NW.	0.0 0.1		29. 329	36
	7 a. m. 9 a. m. 2 p. m.	8E	2.2		29. 500	29		7 a. m. 9 a. m. 2 p. m.	W.NW.	0. 5	•••••	29. 393 29. 344	33
	3 p. m. 9 p. m. 3 a. m.	8E 8. SE	1.5 0.5		29. 437	30		3 p. m. 9 p. m.	NW W. SW.	0, 2 0, 1		29. 339	32
29	7 a. m.	West	0.3	••••	29. 516	22	10	3 a. m. 7 a. m.	North	1.9		29. 192	22
	9 a. m. 2 p. m. 3 p. m.	West W. NW.	0.5		29. 636	31		9 a. m. 2 p. m.	North	4, 1		29. 459	10
30	9 p. m. 3 s. m.	Calm Calm	0.0		29. 724	21	11	3 p. m. 9 p. m. 3 a. m.	West NW N. NW.	2.3 0.7 0.1		29. 583	6
	7 a. m.	South	0.9		29. 704	16	•	7 a. m. 9 a. m.	Calm	0. 0		29. 347	13
	2 p. m. 3 p. m.	8. SE	i. i	•••••	29.609	37		2 p. m. 3 p. m.	8.8W	0. 7		29. 028	26
31	9 p. m. 3 s. m. 7 s. m.	S. SE South	0.4 0.7		29, 472 29, 231	29 31	12	9 p. m. 3 a. m.	W.SW.	1. 1 1. 1	•••••	29. 028	19
	9 a. m. 2 p. m.	8.8W	1.1		29. 041	41		7 a. m. 9 a. m. 2 p. m.	NW	1.9	•••••	29. 050 29. 309	11
1862.	3 p. m. 9 p. m.	8.8W 8W	0. 8 1. 0		28. 935	40		3 p. m. 9 p. m.	W.NW. West	4.0 2.2		29. 652	li
Jan. 1	3 a. m.	8W	4.3		29. 098	26	13	3 a. m. 7 a. m.	W.NW.	2.4		29. 972	i7
	9 a. m. 2 p. m. 3 p. m.	W.SW.	6.6		29. 426	20		9 a. m. 2 p. m. 3 p. m.	W.SW.	0.9	· · · · · ·	29. 989	i9
2	9 p. m. 3 a. m.	West W.NW.	0. 5 1. 0		29. 684	15	14	3 p. m. 9 p. m, 3 a. m.	W. 8W. W. 8W. Calm	0. 6 0. 1 0. 0		30. 011	i2
_	7 a. m. 9 a. m.	N. NW.	1.3		29. 762	14		7 a. m. 9 a. m.	Calm	0.0		29. 934	-15
	2 p. m. 3 p. m. 9 p. m.	8E	2.1		29. 718	27		2 p. m. 3 p. m.	E. SE	1, 2	•••••	29. 721	13
3	9 p. m. 3 a. m. 7 a. m.	S. SE S. SE	23 21		29. 701 29. 567	26 24	15	9 p. m. 3 a. m. 7 a. m.	E. SE South	3 0 0, 1	· • • • • • • • • • • • • • • • • • • •	29. 423	22
	9 a. m. 2 p. m.	South	2.7		29. 401	24		9 s. m. 2 p. m.	w.sw	2.1		29, 102 29, 173	17
-	3 p. m. 9, p. m.	8. SE 8. SE	2.8 1.9		29. 431	25		3 p. m. 9 p. m.	West W.NW.	3. 4 1. 8		29. 487	17
4	3 a. m.	SE	1.4		29. 487	23	16	3 a. m.	Calm	0.0	· • • • • • • • • • • • • • • • • • • •	29. 798	<u>-</u> 5
	9 a. m. 2 p. m. 3 p. m.	SE	1. 5 0. 1		29. 497	26		9 a. m. 2 p. m. 3 p. m.	West			29. 853	8
5	9 p. m.	South S. SW	0.6		29. 486	24	17	9 p. m. 3 a. m.	Calm Calm	0. 0 0. 0		29. 833	5
	7 a. m. 9 a. m.	South	1.5		29. 376	24	<u>.</u>	7 a. m. 9 a. m.	Calm			29. 643	io
	2 p. m. 3 p. m.	8E	1.5		29. 212	26		2 p. m. 3 p. m.	N. NW	0. i		29. 634	24
	19 p. m.	Calm	0.0	١	29. 134	126	I	∣9 p. m.	South			29, 652	29

TABLES AA .- Showing the wind, water, barometer, &c .- Continued.

Date.			WIND		above	poor	Ę.			WINI).	gauge.	1	3
		Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide gauge.	Barometer reduced to 32°.	Therm'r detached.	Date.	Hour of day.	Соптие.	Relative ve-	Heig't of water above sere of tide gauge.	Berometer reduced to 39°.	Miles de la Constitución de la C
186								1862.						Τ
m.	18	3 a.m. 7 a.m.	Calm	0.0		29. 411	17	Jan. 30	3 a. m. 7 a. m.	W.NW.	0.7			<u> </u>
		9 a.m. 2 p.m.	Calm	0.0		29. 364	26		9 a.m. 2 p.m.	NW	0. 3			-
		3 p. m.	E. SE	0. 2 0. 3		 			3 p. m.	NW Calm	0.9			
	19	9 p. m. 3 a. m.	NW N. NW.	0.7		29. 273	24	31	9 p. m. 3 a. m.	Calm	0.0			
		7 a.m. 9 a.m.	NE	i. i		29. 198	31		7 a.m. 9 a.m.	South	1.0			
		2 p. m. 3 p. m.	N. NE	1.7		29. 212	34		2 p. m. 3 p. m.	8.8W	1. 2			-
		9 p. m.	NE	2.4		29. 201	32		9 p. zn.	West	22			-;
	20	3 a.m.	N. NE	2.7		29.300	32	Feb. 1	3 a.m.	West	1.7		29, 531	
		9 a. m. 2 p. m.	N. NE	3.8		29.376	34		9 a.m. 2 p.m.	West	2. 1		29. 623	l
		3 p. m.	N. NE	4. 3		l			3 p. m.	West	2.1		29. 792	1
	21	9 p. m. 3 a. m.	N. NE East	2.5 0.1	· • • • • • • • • • • • • • • • • • • •	29. 482	32	2	9 p. m. 3 a. m.	West Calm	0.4		29. 873	•
		7 a. m. 9 a. m.	Calm	0.0		29, 469	19		7 a. m. 9 a. m.	Calm	à. o		29, 805	ŀi
		2 p. m.				29. 409	32		2 p. m.	Calm	0.0			
		3 p. m. 9 p. m.	E. NE	0. 2					9 p. m.	Calm	0.0		29.707	11
	22	3 a. m. 7 a. m.	Calm	0.0				3	3 a.m.	E.SE	1.2		\$9.427	2
		9 a.m.	8W	0.7					9 a. m. 2 p. m.	8E	0.9		29, 477	2
		2 p. m. 3 p. m.	w.sw.	0.2					3 p. m.	West	2.3		29, 501	Ţ
	23	9 p. m. 3 a. m.	W.SW.	0.4				4	9 p. m. 3 a. m.	W.NW. West	1.4		29.660	-
		7 a.m. 9 a.m.	S. SE	0. 2	•••••		ļ		7 a.m. 9 a.m.	Calm	0.0		29.642	٠.
		2 p. m.					- 		2 p. m.					
		3 p. m. 9 p. m.	8. SE	0. 4 1. 3					3 p. m. 9 p. m.	8.8W	0.2		29.611	1
	24	3 a. m. 7 a. m.	8. SE	0.4	••••			5	3 a.m.	Calm	a o		29. 472	.!.
		9 a.m.	8. SE	1.4				ļ	9 a. m. 9 p. m.	8.8W	0.4		29.394	3
		3 p. m.	8. SE	0.4					3 p. m.	South	0. 5		99, 236	3
	25	9 p. m. 3 a. m.	W.NW. West	1.3				6	9 p. m. 3 a. m.	8. SE W.SW .	0.3		29.027	2
		7 a. m.	W.NW.	3.4		· · · · · · · · · · · ·	····		7 a. m. 9 a. m.	w.nw	3.8		99. 219	-3
		2 p. m.					 :		2 p. m.		3.9		}	١
		3 p. m., 9 p. m.	West	4.0 3.1					9 p. m.	W.NW.	2.4		29. 446	. 1
	26	3 a. m. 7 a. m.	West	2.4			· · · ·	7	3 a.m. 7 a.m.	NW	1.4		29.587	
		9 a.m. 2 p.m.	West	1.9		•••••			9 a. m. 2 p. m.	N.NW	1.5		\$9.616	2
		3 p. m.	8W	0.5					3 p. m.	N.NW.	0.6 1.2		99.621	i
	27	9 p. m. 3 s. m.	E.SE	0. 0 0. 6				8	9 p. m. 3 a. m.	W.NW.	0.3		29.551	i
		7 a. m. 9 a. m.	E.8E	4, 1	•••••		••••		7 a.m. 9 a.m.	W.NW.	0.7		99.500	h
		2 p. m. 3 p. m.	E.8E	4.3	•••••				2 p. m. 3 p. m.	West	2.2	•••••	29.518	
	~~	9 p. m.	8E	2.7	•••••		: :		9 p. m.	West	1.1			
	28	3 a. m. 7 a. m.	E.SE	0.9				9	3 a. m.	West	0.2	•••••	29. 602	i
		9 a.m. 2 p.m.	Calm	0.0			••••		9 a. m. 2 p. m.	West	0.3		29. 576	1
		3 p. m.	Calm	0.0					3 p. zo.	W.SW	0.6		99. 474	1
	29	9 p. m. 3 a. m.	W.NW. West	1.0 3.4	•••••		••••	10	9 p. m. 3 a. m.	8W 8.8W	0.5 0.5		99. 960	i
		7 a.m. 9 a.m.	Calm	0.0			••••		7 a. m. 9 a. m.	8. 8W	1.0		99.058	3
		2 p. m.							2 p. m.					١
		3 p. m. 9 p. m.	West W.NW.	2.6 1.5			••••		3 p. m. 9 p. m.	8. 8W 8. 8W	1.2		28.865	•

TABLES AA .- Showing the wind, water, barometer, &c .- Continued.

		WISI).	above	duced bed.	bed.			WIND		above ange.	reduced.	bed.
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 32°.	Barometer reduce to 32°. Therm'r detached.	Date,	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gange.	Barometer rec	Therm'r detached.
1862. Feb. 11	3 a. m. 7 a. m.	S. SE	0, 3		28. 642	29	1862. Feb. 23	3 a. m. 7 a. m.	Calm	0.0		28. 916	36
	9 a. m. 2 p. m.	NW	3.3		98. 612	29		9 a.m.	w.sw.	0.3		28, 918	41
	3 p. m. 9 p. m.	W.NW. W.8W. Calm	3.3		98. 867	25		3 p. m. 9 p. m.	Calm N. NE	0.0 3.5 2.3		29. 043	33
12	3 a. m.	Calm	0.0		29. 101	17	24	3 a.m.	N. NE N. NW.			29. 492	ie
	9 a. m. 2 p. m.	W.NW.	0. 2		29. 211	33		9 s. m.	N. NW .	0, 5		29. 642	2
	3 p. m. 9 p. m.	NE	1. 2 0. 7		29, 304	29		3 p. m.	W.NW.	0. 4 0. 0		29. 717	ie
13	3 a. m. 7 a. m.	E. NE	0. 2		29. 291	27	25	9 p. m. 3 a. m. 7 a. m.	8. SE	0.4		29.715	i
	9 a. m. 2 p. m.	West	2.3		29. 327	20		9 a. m. 2 p. m.	8. SE	2.4		29. 384	33
	3 p. m. 9 p. m.	W.NW. W.NW. W.NW.	4.7		29, 490	2		3 p. m. 9 p. m.	8. SE 8. SE	3.1 0.7		29, 191	31
14	3 a.m.	W.NW	0.8		29, 624		26	3 a.m.	Calm	ão		29. 013	20
	9 a. m.	Wost	0.9		29, 631	-13		9 a. m.	W.NW.	2.0		1	35
	2 p. m. 3 p. m.	w.sw.	1.4			-3 :		2 p. m. 3 p. m.	W.NW.	4.8	•••••	29. 113	2
15	9 p. m. 3 a. m.	W. SW. W. SW. SW	0.4	•••••	29. 568	-8	27	9 p. m. 3 a. m.	W.NW. N.NW. NW	3.0 2.5		29. 344	
	7 a.m. 9 a.m.	w.sw.	21		29. 321	_3 		7 a.m. 9 a.m.	NW	1.9		99. 455	
	2 p. m. 3 p. m.	w.sw.	2.6		29. 320	2		2 p. m. 3 p. m.	w. 8w.	1.4		29. 487	3
16	9 p. m. 3 a. m.	Calm	0.0		29. 468	_3	98	9 p. m. 3 a. m.	West N. NW .	0.7		29. 492	1
	7 a. m. 9 a. m.	8. SE	0.3		29. 550	-1		7 a.m. 9 a.m.	N. NW .	1.2		29, 613	1
	2 p. m. 3 p. m.	S. SE	0.8	•••••	29 . 472	30		2 p. m. 3 p. m. 9 p. m.	N. NE	1.6		29. 605	1
17	9 p. m. 3 a. m.	8. 8E Calm	0.0		29. 452	26	March 1	9 p. m. 3 a. m.	North N. NE	0. 4 1. 5		29. 589	Į į
1.	7 a. m. 9 a. m.	8. SE	0.8	•••••	29. 392	10		3 a. m. 7 a. m. 9 a. m.	NE	1.5		29. 522	ï
	2 p. m.	8. SE	0.6			32		2 p. m.	1	2.5		29. 404	2
	9 p. m.	w.sw.	5.9 2.6	· • • • • • •	29, 217	30		9 p. m.	N. NE E. NE	1.1		29. 354	2
18	7 a. m.	West	J	•••••	29. 574	Ö	2	7 a. m.	NE			29. 267	2
	9 a.m. 2 p.m.	West	1.5	•••••	29. 651	15		9 a. m. 2 p. m.	E. NE	4. 2		29. 130	2
	3 p. m. 9 p. m.	W. 8W. W. 8W.	1.3 0.2		29. 663	14		3 p. m. 9 p. m.	NE E. NE	3.9 2.7		28. 938	2
19	3 a. m. 7 a. m.	N.NW	0.3	· · · · · ·	29. 53 6	14	3	3 a.m. 7 a.m. 9 a.m.	E. SE	2.1		28. 689	2
	9 a. m. 2 p. m.	E. NE	1.6	•••••	29. 351	26		2 p. m.	8. 8W	2.9		98. 655	2
	3 p. m. 9 p. m.	N. NE N. NW . N. NW .	4.8		29. 336	25		3 p. m. 9 p. m.	8.8W W.8W.	2.3 2.3		28. 695	2
90	3 a. m. 7 a. m.	N. NW.	0.6		29. 601	12	4	3 a. m. 7 a. m.	w.sw.	2.8		28, 721	i
	9 a. m. 2 p. m.	West	0.4		29. 667	24		9 a.m.	W. 8W.	2.8		98.788	2
	3 p. m. 9 p. m.	West Calm	0.7		29. 697	18		2 p. m. 3 p. m. 9 p. m.	W. 8W. 8W	2.3 1.3		28, 886	i
21	3 a. m.	Calm	ãŏ		29. 467	22	5	3 a. m.	w.sw.	0. 2		28. 937	
	9 a. m.	8W	1.0		29, 320	31		9 a.m.	8.8W	0.1		29. 013	
	2 p. m. 3 p. m.	w.sw.	0.6					2 p. m. 3 p. m.	Calm	0.0		1	١. ـ
22	9 p. m. 3 a. m.	Calm	0.0		29. 286	29	6	9 p. m. 3 a. m.	Calm	0. 0 Q. 0		29, 202	1
	7 a. m. 9 a. m.	Calm	0.0		29. 116	30	1	7 a. m. 9 a. m.	w.sw.	ï.i		29.099	1
	2 p. m. 3 p. m.	Calm	0.0		28. 978	38	1	2 p. m. 3 p. m.	West	ā.3		29. 113	3
	9 p. m.	Calm	0 0		28.988	35	!	9 p. m.	Calm	0.0		29. 226	3

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND.		above	reduced.	hed.			WINI		above	reduced.	.paq
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gange.	Barometer rec	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer red to 32°.	Therm'r detached.
1862.	7. 2	200					1862.						
Mar. 7	3 a. m. 7 a. m.	Calm	0.0	•••••	29. 347	32	Mar. 19	3 a. m. 7 a. m.	E.NE	0.9	•••••	29. 361	30
	9 a. m.	Calm	0.0					9 a. m.	E.NE	2.7			
	2 p. m. 3 p. m.	Calm	0.0		29. 316	42		2 p. m. 3 p. m.	E.NE	3.3	*****	29, 247	3
8	9 p. m.	Caim	0.0		29. 314	32	00	9 p. m.	E.NE	4.4		29, 171	3
	3 a. m.	Calm	0.0		29. 287	32	20	3 a. m. 7 a. m.	E.NE	1.8	******	29, 121	3
	9 a. m. 2 p. m.	8.8W			29. 254			9 a. m.	N.NE	2, 9			
	3 p. m.	S.SE	0.9		29. 204	42		2 p. m. 3 p. m.	N.NE	6, 6		29, 003	3
9	9 p. m. 3 a. m.	E.SE	1.5		29. 266	43	21	9 p. m.	N.NE	7.3		29,008	3
	7 a. m.				29. 021	36		3 a. m. 7 a. m.	N.NE		******	29.012	34
	9 a. m. 2 p. m.	SE	1.8		28. 918	47		9 a. m. 2 p. m.	N.NE	9.4		29, 101	3
	3 p. m.	8.8W	2.4					3 p. m.	N.NE	7.0			100
10	9 p. m. 3 a. m.	W.SW. West	1.0		29, 000	40	22	9 p. m. 3 a. m.	N.NE	3.4 0.6		29, 181	3
	7 a. m.				29. 139	35		7 a. m.	********			29, 109	3
	9 a. m. 2 p. m.	West	2.9		29. 281	35		9 a. m. 2 p. m.	N.NW	0, 5		29, 071	3
	3 p. m.	W.8W.	3.0					3 p. m.	N.NE	0. 2			1
11	9 p. m. 3 a. m.	W.SW.	0, 9		29, 431	30	23	9 p. m. 3 a. m.	NW North	0.2		29, 121	3
	7 a. m.	********			29. 423	26		7 a. m.			******	29, 181	3
	9 a. m. 2 p. m.	w.sw.	1.1	77.77	29, 402	35		9 a. m. 2 p. m.	N.NE	2,0	*****	29, 214	1.3
	3 p. m.	W.SW.	3. 2					3 p. m.	N.NE	3.8			1
12	9 p. m. 3 a. m.	W.SW.	1.9		29, 404	31	24	9 p. m. 3 a. m.	N.NE North	3.6	*****	29, 296	3
	7 a. m.				29.448	24		7 a. m.				29. 312	2
	9 a. m. 2 p. m.	N.NW .	2.0		29, 459	28		9 a. m. 2 p. m.	N.NW	0. 6	*****	29, 294	3
	3 p. m.	NE	3. 2				- 1	3 p. m.	NE	0.4			
13	9 p. m. 3 a. m.	E.NE	4.2		29, 388	29	25	9 p. m. 3 a. m.	Calm	0.0		29, 326	12
	7 a. m.	*******			29. 270	32		7 a. m.				29.343	92
	9 a. m. 2 p. m.	NE	4.1		29. 276	34		9 a. m. 2 p. m.	Calm	0.0		29, 307	13
	3 p. m.	N.NE	4.3					3 p. m.	SE	0.4			
14	9 p. m. 3 a. m.	N.NE	6.0		29. 316	33	26	9 p. m. 3 a. m.	Calm	0.0		29, 291	13
	7 a. m.				29. 348	31		7 a, m.				29. 330	13
	9 a. m. 2 p. m.	N.NE	6.0		29. 279	31		9 a. m. 2 p. m.	N.NE	1.0		29, 434	17
	3 p. m.	N.NE	5. 2 4. 9				100	3 p. m.	N.NE	0.5			
15	9 p. m. 3 a. m.	N.NE	4.6		29, 228	31	27	9 p. m. 3 a. m.	Calm	0.0		29, 526	13
	7 a. m. 9 a. m.	N.NE	6.0		29, 126	32		7 a. m.	N.NE	0.1		29, 590	1
	2 p. m.				29, 061	33		9 a. m. 2 p. m.				29, 476	1
	3 p. m. 9 p. m.	N.NE	6. 8 5. 7		29, 088	34		3 p. m. 9 p. m.	S.SE	0.1		29.531	
16	3 a. m.	N.NE	2, 6				28	3 a. m.	Calm	0.0		*******	1
	7 a. m. 9 a. m.	N.NE	2.6		29. 124	30		7 a. m. 9 a. m.	Calm	0.0	****	29, 454	13
	2 p. m.				29, 108	35	1	2 p. m.				29, 342	1
	3 p. m. 9 p. m.	N.NE	2.2		29, 181	30		3 p. m. 9 p. m.	Calm	0.0	*****	29, 266	1
17	3 a. m.	N.NW	0. 2				29	3 a. m.					
	7 a. m. 9 a. m.	N.NE	0.8		29, 229	30		7 a. m. 9 a. m.				29, 139	3
	2 p. m.				29.264	38	1 3	2 p. m.				29, 088	3
	3 p. m. 9 p. m.	Calm	0.3		29. 316	32		3 p. m. 9 p. m.		****	2000	29, 085	3
18	3 a. m.	East	0, 1				30	3 a. m.				*******	
	7 a. m. 9 a. m.	E.NE	1.2		29, 397	35		9 a. m.		****		29, 936	3
	2 p. m.				29, 479	38		2 p. m.		***	*****	29, 016	4
	3 p. m. 9 p. m.	E.NE	1.0		29, 484	34		3 p. m. 9 p. m.			188.	29, 284	3

^{*}Anemograph out of order from the 29th of March until the 15th of April.

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

			WINI).	above	reduced.	bed.			WIND		above mge,	reduced	bed.
Date.		Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer re to 32°,	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gange,	Barometer re- to 320.	Therm'r detached.
186 Mar.	31	3 a. m.						1862. April 12	3 a. m.			2.78	1	
		7 a. m. 9 a. m.		1		29. 561	35	127 22	7 a. m.			2.73	29. 413	41
		2 p. m. 3 p. m.		1		29. 633	43		2 p. m. 3 p. m.			2.75	29, 320	40
April	1	9 p. m.		:	3.43	29. 622	38	13	9 p. m. 3 a. m.		••••	2.65 2.67	29. 254	4
		7 a. m. 9 a. m.		::::	3.48	29. 573	38		7 a. m. 9 a. m.	- 		2.82	29. 171	4
		2 p. m. 3 p. m.			3. 24	29. 444	41		9 n m	1	1 !	2. 85	29. 206	4
	2	9 p. m. 3 a. m.			3. 15 3. 05	29, 324	42	14	9 p. m.		••••	2.90 3.00	29. 263	4
		7 a. m. 9 a. m.			3.08	28. 942	38		7 a. m. 9 a. m.			3.06	29. 317	4
		2 p. m. 3 p. m.	1	1	3. 17	28. 738	47		2 p. m. 3 p. m.	E.SE	1.6	3.02	29, 332	5
	3	9 p. m. 3 a. m.			3. 25 3. 26	29. 264	40	15	9 p. m. 3 a. m.	E.SE Calm E.SE	0.0	3.00	29. 382	4
		7 a. m. 9 a. m.			3. 40	29. 553	34		7 a. m. 9 a. m.	E.SE	1.2		29. 235	4
		2 p. m. 3 p. m.		• • • •	3.31	29. 564	43		2 p. m. 3 p. m.	E. SE	1	1	29. 193	5
	4	9 p. m. 3 a. m.			3. 26	29. 544	47	16	9 p. m. 3 a. m.	8E	1.9 1.5 2.6	2.88	29. 121	15
	•	7 a. m. 9 a. m.			3.03	29. 425	36	1	7 a. m. 9 a. m.	sw	0.9		29. 143	
		2 p. m. 3 p. m.	1		2.88	29, 182	39		2 p. m.	w.sw.		l	29. 065	1
	5	9 p. m. 3 a. m.			2.97	28.946	38		3 p. m. 9 p. m.	w.sw.	1.9 0.7	3.01	29. 308	1
	S	7 a. m.			2.92		35	17	3 a. m. 7 a. m.	Calm	0.0		29. 622	4
		9 a. m. 2 p. m.			3. 15	29. 326	43		9 a. m. 2 p. m.	W.NW.	0. 2	-	29. 530	4
		3 p. m. 9 p. m.			3. 26 3. 29	29. 462	36		3 p. m.	8.8E E.NE	0. 3 0. 1	3.06	29. 449	4
	6	3 a. m. 7 a. m.			3. 31	29. 702	24	18	3 a. m. 7 a. m.	Calm	0.0		29. 332	4
		9 a. m. 2 p. m.			3. 42	29. 685	33	}	9 a. m. 2 p. m.	w	0. 4	3. 06	29. 452	
		3 p. m. 9 p. m.			3.44	29. 627	30	Ì	3 p. m. 9 p. m.	W.NW.	2. 2 0. 2	2.85 2.93	29. 582	- 4
	7	3 a. m.			3. 21	29. 483	34	19	3 a. m. 7 a. m.	N.NE	0, 6	3. 05	29. 652	
		9 a. m. 2 p. m.			3. 05	29, 394	34		9 a. m. 2 p. m.	NE	0.4	3.09	29. 672	
		3 p. m. 9 p. m.			2.83 2.65	29. 274	31		3 p. m. 9 p. m.	NE N.NE	1. 1 0. 5	3.01 2.97	29. 641	3
	8	3 a. m. 7 a. m.			2.60	29. 158	32	20	3 a. m.	N.NE	1.5	2.87	29. 600	. 3
		9 a. m. 2 p. m.			2, 55	29. 131	33		9 a. m. 2 p. m.	N.NE	3.3	2.87	29. 500	4
		3 p. m. 9 p. m.			2,58 2,41	29. 186	33		3 p. m. 9 p. m.	N.NE	4. 2 3. 6	2.79 2.73	29. 364	. 3
	9	3 a. m.			2.50	29. 182	32	21	3 a. m.	N.NE	3.1	2.69	29, 180	١.,
		9 a. m.			2.41			1	7 a. m. 9 a. m.	N.NE	4.1	2.67		4
		3 p. m.			2.57	29. 214	35		2 p. m. 3 p. m.	N.NE	4.1	2.64	29, 153	
	10	9 p. m. 3 a. m.			2.68 2.88	29. 379	33	22	9 p. m. 3 a. m.	N.NE N.NW .		2.75	29. 133	١.,
		7 a. m. 9 a. m.			2.88	29. 557	35	1	7 a. m. 9 a. m.	NW	2.4	2.85	29. 166	4
		2 p. m. 3 p. m.			2.96	29. 626	40		2 p. m. 3 p. m.	E.SE	1	2.92	29, 230	1.1
	11	9 p. m. 3 a. m.			2.93 2.94	29. 639	35	23	9 p. m. 3 a. m.				29. 356	13
		7 a. m. 9 a. m.			2.91	29. 701	36		7 a. m. 9 a. m.	NE	1.4		29. 562	3
		2 p. m.			2.93	29. 629	46						29, 617	3
		9 p. m.	l		2.75		42	į	9 p. m.	N.NE	1.8	3.03	29. 651	3

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

			WINI	Э.	er be-	luced	red.			WINI).	or be-	reduced.	1
Da	te.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer reduced to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer red to 32°.	Thomas 's defeathed
186						7		1862.						1
April	24	3 n. m. 7 n. m.	NE	1.8		29. 631	35	May 6	3 a. m. 7 a. m.	Calm	0.0	2.92	29.507	4
		9 a. m. 2 p. m.	NE	2.2	2.83	29, 624	40	11.	9 a. m. 2 p. m.	E. NE	1	2.97	29, 462	1:
		3 p. m.	N. NE	2.0	2.76				3 p. m.	E. SE		2.99		1
	25	9 p. m. 3 a. m.	Calm	0.0		29. 591	34	7	9 p. m. 3 a. m.	N. NW		3.00	29. 401	1
		7 a. m. 9 a. m.	8. SW	1.9		29, 547	36		7 a. m. 9 a. m.	N. NW		2.97	29, 464	
		2 p. m.				29, 451	59		2 p. m.				29, 494	T,
		3 p. m. 9 p. m.	N. NE	2.6	2.68	29. 501	43		3 p. m. 9 p. m.	Calm		2.92 2.82	29. 518	Ť,
	26	3 a. m.	N. NE	1.5				8	3 a. m.	Calm		2, 84		J.
		7 a. m. 9 a. m.	N. NE	1.4	2.74	29. 599	38		7 a. m. 9 a. m.	E. SE	0.2	2,80	29, 632	
		2 p. m. 3 p. m.	E. NE	0.6	2.72	29, 611	43		2 p. m. 3 p. m.	E. SE	17	2.88	29. 576	P
		9 p. m.	Calm	0.0	2.79		41		9 p. m.	Calm	0, 0	2.87	29, 539	1
	27	3 a. m. 7 a. m.	S. SE	0.8	2.88	29, 405	53	9	3 a. m. 7 a. m.	Calm	0.0	2,94	29, 566	17
		9 a. m. 2 p. m.	W.SW.	2.7	2.84	29, 411	53		9 a. m. 2 p. m.	w.sw.	1.1	2,90	29. 518	٠١.
	4.7	3 p. m.	sw	1.4	2.79				3 p. m.	SE	1.0	2.95		-1-
	28	9 p. m. 3 a. m.	W.SW.	0.5	2, 82 2, 85	29, 413	44	10	9 p. m. 3 a. m.	West	0.0	2.90 2.96	29, 481	1
		7 a. m.				29, 414	44	- 50	7 a. m.				29. 508	
		9 a, m. 2 p. m.	West	0.5		29, 451	52		9 a. m. 2 p. m.	NW		2.88	29, 473	j.
		3 p. m. 9 p. m.	W.NW.	0.6 1.6	2.89	29. 548	42	11111	3 p. m. 9 p. m.	Calm	0.0	2.90 2.76	29. 501	d.
	29	3 a. m.	Calm	0.0				11	3 a. m.	N. NE	0.2	2.79		
		7 a. m. 9 a. m.	E. SE	0.5	2.97	29. 627	42		7 a. m. 9 a. m.	E.SE	0.8	2.71	29.608	13
		2 p. m.	SE	0.7	2.94	29. 577	49	-	2 p. m. 3 p. m.	E. SE		2.76	29.594	1
		3 p. m. 9 p. m.	Calm	0.0	2.88	29. 534	46		9 p. m.	Calm	0, 0	2, 68	29, 551	
	30	3 a. m. 7 a. m.	Calm	0.0	2, 83	29, 432	47	12	3 a. m. 7 a. m.	sw	0. 1	2.78	29. 484	÷,
		9 a. m.	S. SW	1.1	2.85				9 a. m.	8W	2.7	2. 81		L.
		2 p. m. 3 p. m.			2.74	29, 277	54		2 p. m. 3 p. m.	8W	2.7	2, 89	29, 400	1.
ву	1	9 p. m. 3 a. m.	SE	0,6	2.67 2.72	29. 179	48	13	9 p. m. 3 a. m.	8W W. 8W.	0.1	2.84	29. 328	
ьу		7 a. m.				28, 906	48	10	7 a. m.				29, 271	-
		9 a. m. 2 p. m.	sw	0.3	2.68	28, 920	49		9 a. m. 2 p. m.	N. NE	3.4	2.68	29, 391	i
		3 p. m. 9 p. m.	w.sw.	0.7	2.60 2.51	28. 983	43		3 p. m. 9 p. m.	N. NE	3.5	2.56 2.50	29. 411	'n
	2	3 a, m.	N. NE	0.9	2. 50			14	3 a. m.	N. NE	27	2.54		
		7 a. m. 9 a. m.	N. NE	1,0	2.67	29. 143	40		7 a. m. 9 a. m.	N. NE	0.4	2.73	29.471	1.4
	- 1	2 p. m.	NW	0, 1	2.80	29, 289	49		2 p. m.	Calm		2.71	29, 461	5
		3 p. m. 9 p. m.	Calm	0, 0	2.89	29, 352	44		3 p. m. 9 p. m.	Calm	0, 0	2.74	29, 451	4
	3	3 a. m. 7 a. m.	Calm	0.0	2, 93	29. 382	46	15	3 a. m. 7 a. m.	Calm	0.0	2.71	29. 447	5
		9 a. m.	E. SE	0. 2	3. 04				9 a. m.	8E	0.4	2.77		
		2 p. m. 3 p. m.	SE		3. 04	29, 369	56		2 p. m. 3 p. m.	E.SE	1.3	2.65	29, 407	
	4	9 p. m. 3 a. m.	W.SW.	0.1	3.00 2.94	29. 306	50	16	9 p. m. 3 a. m.	Calm	0.0	2 73 9 71	29. 384	5
		7 a. m.				29. 302	47		7 s. m.				29. 399	6
		9 a. m. 2 p. m.	NW	0.6	2.95	29. 209	61		9 a. m. 2 p. m.			2.78	29.348	7
		3 p. m. 9 p. m.	SE N. NE	1.3	2, 80 2, 82	29. 304	40		3 p. m.	E. SE	0,1	2.63	29. 363	+ 14
	5	3 a. m.	N. NW		2, 85			17	9 p. m. 3 a. m.	W. NW.	0.0			
		7 a. m. 9 a. m.	N. NE		2.87	29, 360	43		7 a. m. 9 a. m.	E. SE	0.6	2.78	29. 276	63
		2 p. m.			2.90	29, 427	51		2 p. m.		.		29, 196	66
	- 1	3 p. m. 9 p. m.	N. NE N. NE	0.7	2.90	29. 433	43		3 p. m. 9 p. m.	N. NE N. NE	1. 6	2.07 2.58	29, 151	49

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

			WINI).	ar be-	duced	bed.			WIND		er be-	reduced	bed.
Dat	te.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer reduced to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer re to 320,	Therm'r detached.
186 May	18	3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	N. NE NW NW N. NE	18 2.5 1.3 0.1	2.46 2.60	29. 103 29. 318 29. 381	46 49 40	1862. May 30	3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	NE N. NE N. NE N. NE	1.1 1.6 2.9 1.8	3. 49 2. 41 2. 36 2. 38	29. 098 29. 115 29. 141	49
	19	3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m.	North NE E. SE	0. 1 0. 6 0. 6	2.77 2.84	29. 381 29. 386	38	31	3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m.	N. NE	2.0	2. 43 2. 41	29: 190 29: 184	47 50
	90	9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m.	Calm Calm	0.0	2. 80 2. 78	29. 347 29. 355 29. 292	39 45 48	June 1	9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m.	N. NE N. NE N. NE	2.8 1.6	2.51 2.41 2.48 2.45	29. 164 29. 143 29. 145	53 53 59
	21	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m.	NE NE N. NE	1.5 2.2 3.7	2.52 2.43 2.28	29. 166 29. 941 28. 910	46 43 47	2	9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m.	N. NE	0. 0 0. 9 6. 5	2. 53 2. 45 2. 47	29. 135 29. 182 29. 234	54 55 69
	22	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	West West W. SW	2.5 0.8 0.9	2.61	29. 193 29. 362	47 56	3	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE N. NE N. NE	6.8 7.7 5.5 5.5	2.41 2.40 2.34 2.34	29. 304 29. 410 29. 436	51 50
	23	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	SW NW Calm	3.6 0.7 0.0	2 73 2 94	29. 316 29. 476 29. 642	73 58 54	4	9 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE N. NE N. NE	7.1 3.3 2.5 5.0		29. 486 29. 537	5 4
	94	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	NE N. NE N. NE	0.7 1.0 0.9	2.85 2.68 2.69	29. 672 29. 682 29. 743	49 43 48	5	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE N. NE Calm	6.2 2.9 0.0	2.36 2.39 2.44 2.50	29. 529 29. 509 29. 594	5 5 5
	25	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	E. SE Calm Calm	0.7 0.0 0.0	2 64 2 61 2 67	29. 659 29. 589 29. 444	53 48 57	6	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE N. NE Calm	0.5 6.1 0.0	2. 48 2. 50 2. 53 2. 54	29. 524 29. 486 29. 451	5 6
	26	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	8.8W 8.8W 8.8W	2. 2 1. 6 5. 9	2.74 2.58 2.74	29. 369 29. 150	68 61 60	7	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	N. NE N. NE N. NE	3.3 6.1 3.5	2. 44 2. 40 2. 46	29. 330 29. 436 29. 504	4
	27	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	SW SW N. NE N. NE	4.0 2.6 2.8 5.9	l	29. 065 29. 050 29. 211	76 68	8	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	N. NE N. NE N. NE	1.1 5.0 3.5 0.5	2.41 2.40 2.51	29. 512 29. 534 29. 622	5' 5(
	28	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	N. NE N. NE N. NE N. NE	4.6 3.9 3.2	2. 65	29. 283 29. 306	57 47	9	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	N. NE East Calm Calm	0.2 0.0 0.0	2.55	29. 607 29. 589	5
	29	7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	N. NE N. NE Calm Calm	0.0	2. 59 2. 60 2. 60 2. 54	29. 447 29. 424 29. 504	53 45	10	7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	W. SW. W. NW. W. SW. West	2.1 2.1 0.5 0.4	2. 56 2. 59 2. 53 2. 57	29. 614 29. 533 29. 513	8
		7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	E. SE East NE	0. 7 0. 2 0. 5	2.59	29. 387 29. 312 29. 263	53 54 50		7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	W.NW.	1.3 1.2 0.0	2, 56	29. 538 29. 420 29. 350	66 84

TABLES AA.—Showing the wind, water, barometer, &c.—Continued.

		WINI),	er be-	need	peq.		317	WINI		r be	poon	7
Date.	Hour of day.	Course,	Relative ve-	Height of water low 1838.	Barometer reduced to 320.	Therm'r detached.	Date.	Hour of day.	Courso.	Relative ve-	Height of water low 1838.	Barometer reduced to 39°.	barbartat a 'manual'i'
1862, une 11	3 a. m.	Calm		2.56	29. 314	70	1862. June 23	3 a. m. 7 a. m.	N. NE		2. 25	29. 386	
	9 a. m. 2 p. m.	W. SW.			29. 192	84		9 a. m. 2 p. m. 3 p. m.	N. NE		2.30	29. 396	
12	3 p. m. 9 p. m. 3 a. m.	S. SW SW W. SW.	1.4	2.62	29. 117	77	24	3 p. m. 9 p. m. 3 a. m.	N. NE Calm	0.4	2.34 2.33 2.41	29. 377	•
	7 a. m. 9 a. m.	w.sw			29. 122	76		7 a. m. 9 a. m.	N. NE .		2. 43	29. 391	
	2 p. m. 3 p. m.	N. NE	3.7	2.47	29, 109	86		2 p. m. 3 p. m.	E. NE .	0. 2	2.44	29.389	
13	9 p. m. 3 a. m.	N. NE	0.9	2. 53 2. 48	29. 238	51	25	9 p. m. 3 a. m. 7 a. m.	Calm	0.0	2 41 2 44	29. 413	
	7 a. m. 9 a. m. 2 p. m.			2. 54	29. 366 29. 278	51 59		9 a. m. 2 p. m.	E. NE .	0.3	2.45	29. 464 29. 458	 6
	3 p. m. 9 p. m.	N. NE	1.3	2. 44 2. 45	29. 281	52		3 p. m. 9 p. m.	E.SB	0.0	2.40	29. 436	
14	7 a. in.	N. NE			29. 269	55	26	3 a. m. 7 a. m.	Calm		2.40	29. 443	ě
	9 a. m. 2 p. m. 3 p. m.	N. NE			29. 251	54		9 a. m. 2 p. m. 3 p. m.	E.SE.	!	2.43	29, 393	•
15	9 p. m.	N. NE	2.7	2. 27	29. 323	50	. 27	9 p. m. 3 a. m.	Calm	0.0		29. 356	•
	7 a. m. 9 a. m.	NE			29. 512	50		7 a. m. 9 a. m.	E.SE			29. 291	
	2 p. m.	N. NE	3.7		29. 566	56		2 p. m. 3 p. m.	E. SE		2.35	29. 268	
16	9 p. m. 3 a. m. 7 a. m.	E. NE E. SE	2.3 2.3	2.27	29. 602 29. 574	53	28	9 p. m. 3 a. m. 7 a. m.	Calm N. NE	0. 0 0. 1		29. 218	
	9 a. m. 2 p. m.	E. SE	2.2	2.32	29. 486	63		9 a. m. 2 p. m.	N. NE	1. 6	2.41	29. 236	٠.,
	3 p. m. 9 p. m.	E.SE	2.3 0.8	2. 24	29. 344	59		3 p. m. 9 p. m.	NE	1. 4 0. 1	2.37 2.41	29. 223	
17	3 a. m.	s. sw		2, 31	29. 196	67	29	3 a. m.	Calm	0.0		29. 211	-
	9 a. m. 2 p. m. 3 p. m.	8W	3. 6		29. 077	76		9 a. m. 2 p. m. 3 p. m.	W.NW.	2.0		29. 202	•
18	9 p. m.	W. SW.	2. 1 1. 1	2. 54	29. 040	68	30	9 p. m. 3 a. m.	West N. NW	1.4	2.51	29. 198	
	7 a. m. 9 a. m.	N. NE	4.2	2.40	29. 008	62		7 a. m. 9 a. m.	N. NE	5. 1	2. 52	29. 331	
	2 p. m. 3 p. m. 9 p. m.	N. NE		2.31 2.41	29. 166	47		2 p. m. 3 p. m. 9 p. m.	NE		2.53 2.56	29. 371	
19	3 a. m. 7 a. m.	NW			29. 350	50	July 1	3 s. m. 7 s. m.	Calm	0.0	2.69	29. 326 29. 294	
	9 a. m. 2 p. m.	N.NW.	0. 6		29, 320	59		9 a. m. 2 p. m.	SE		2.74	29, 165	
•	3 p. m. 9 p. m.	E.SE		2.55	29. 282	57		3 p. m. 9 p. m.	w. sw	0.3	2.69 2.66	29. 100	
20	3 a. m. 7 a. m. 9 a. m.	w.sw	0.4	2. 59 2. 56	29. 279	59	2	3 a. m. 7 a. m. 9 a. m.	Calm		2.54 2.58	29. 143	Ì
	2 p. m. 3 p. m.	West		2.51	29. 241	72		2 p. m. 3 p. m.			2.51	29. 221	•
21	9 p. m. 3 a. m.	NW	0. 5 0. 2	2. 39 2. 45	29. 303	61	3	9 p. m. 3 a. m.	Calm	0.0	2.54 2.50		
	7 a. m. 9 a. m.	w.sw.		2. 45	29. 361	57		7 a. m. 9 a. m.	E. 8E	0.8	2.60	29. 43 1	
	2 p. m. 3 p. m. 9 n. m.	N. NE N. NE	2.1	2. 40 2. 36	29. 385 29. 491	75 52		2 p. m. 3 p. m. 9 p. m.	SE 8. SW	113	2.58 2.60	29. 481 29. 501	
22	3 a. m. 7 a. m.	N. NE	0.3	2.39	29. 536	57	4	3 a. m. 7 a. m.	8. SE	0.2	2, 60	29.623	7
	9 a. m. 2 p. m.	E. NE			29. 524	57		9 a. m. 2 p. m.	s. sw	2.0	2.69	29. 557	
	3 p. m.	N. NE N. NE	2.6	2. 29			1	3 p. m.	South .	2.8	2.64	29. 547	

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WIND		r be-	need	bed.			WIND		r pe-	reduced	Pod.
Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer reduced to 32°.	Therm'r detached.	Date.	Hour of day,	Course.	Relative ve-	Height of water low 1838.	Barometer red	Therm'r detached.
1862. July 5	3 a.m. 7 a.m. 9 a.m.	8. 8W	0.1		29. 568	74	1869. July 17	3 a. m. 7 a. m. 9 a. m.	NE	0.1	2.43 2.49	29. 386	6
6	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m.	8W 8. SW N. NE	0. 4		29. 499 29. 451 29. 502 29. 527	91 80 81 74	18	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m.	NE E. NE SE	2.5 0.6 0.3	2. 43 2. 39 2. 49	29. 378 29. 343 29. 318 29. 253	6
7	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE Calm Calm	3. 0 0. 0 0. 0	2.71	29. 470 29. 333	72 75	19	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	8W 8W	0. 4 0. 5 0. 7	2. 46	29. 201 29. 176	6
8	9 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	W. SW. Calm Calm	0.0	2.75	29. 225 29. 212 29. 200	71	20	2 p. m. 3 p. m. 9 p. m, 3 a. m. 7 a. m.	W. NW. NW Calm	1. 5 0. 2 0. 0	2. 52 2. 50 2. 60	29. 254 29. 185 29. 286	7
9	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	E. SE SW West	1. 0 1. 3 0. 3	2.68	29. 132 29. 082 29. 168	78 68 69	21	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	NW N. NW NW E. NE	0.9 2.3 0.1 0.1	2.70	29. 272 29. 285 29. 305	7
10	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE Calm	1. 4 0. 5 0. 0	2 65 2 75 2 70	29. 263 29. 328 29. 448	64 64	22	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	SE E. SE SW	1. 3 0. 3 0. 6	2, 58 2, 50 2, 53	29. 338 29. 278 29. 218	7
11	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE Calm Calm		2. 58 2. 59	29. 468 29. 483 29. 528	69 61 64	23	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	SE W. NW. W. NW.	1. 1 1. 1 1. 2	2. 57 2. 52	29. 210 29. 205 29. 248	
. 12	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	SECalm	0. 5	2. 65	29. 498 29. 423 29. 368	78 69 69	24	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	W. NW. W. NW. W. NW.	3. 1 0. 9 0. 6	2. 73 2. 68 2. 68 2. 68	29. 245 29. 215 29. 263	1
13	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	8W 8W W. 8W	2. 3 1. 1 1. 9	2. 77 2. 82	29. 180 29. 117 29. 050	75 74 72	25	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NW SE Calm	2. 1 0. 1 0. 0	2. 59 2. 60 2. 62 2. 70	29. 280 29. 300 29. 363	
14	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	Calm Calm	0. 0	2.70 2.72	29. 060 29. 047 29. 082	74 69 67	26	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 s. m.	' . .	1. 4 0. 0 0. 0	2. 67 2. 66 2. 61 2. 64	29. 293 29. 353	6
15	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	SW SW Calm	1. 6 0. 3 0. 0	2. 48 2. 48	29. 072 29. 094 29. 213	76 73 69	27	2 p. m. 3 p. m.	8. 8E 8. SW W. SW.	1. 5 0. 2 0. 2	2. 57 2. 60 2. 59	29, 360 29, 380 29, 411	7
16	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	W.NW. N. NE. N. NE.	1. 9 0. 8 3. 0	2.57 2.64 2.60 2.66	29. 255 29. 330 29. 403	78 62 58	28	2 p. m. 3 p. m. 9 p. m. 3 s. m. 7 a. m.	8W 8. SW W. SW.	3. 0 0. 1 1. 1		29. 327 29. 310 29. 273	7
	2 p. m. 3 p. m.	N. NE N. NE	2.5	2. 55	29, 436	61 56		9 a. m. 2 p. m. 3 p. m.	W. NW. W. NW. W. NW.	1	2.73	29. 189 29. 247	8

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

			WIND		er be-	reduced o.	ped.			WIND		or be-	luced	7
Da	te.	Hour of day.	Coarse.	Relative ve-	Height of water low 1838.	Barometer red to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1638.	Barometer reduced to 32°,	Thorn'r dutached.
186 July	12. 29	3 a. m. 7 a. m.	NW	0. 2	2, 66		67	1862. Aug. 10	3 a. m. 7 a. m.				A 212	
		9 a. m. 2 p. m.	N. NW	1.4	2.70	,29, 333 29, 325	76		9 a. m. 2 p. m.	8E	0.3		29. 313 29. 983	1
•		3 p. m. 9 p. m.	8E	1.6 0.0	2,68	29. 380	68		3 p. m. 9 p. m.	SE S. SE	0.7	2 43	29. 200	-
	3 0	3 a. m. 7 a. m.	Calm	0.0		29. 463	66	11	3 a. m. 7 a. m.	8W	0. 1	2.42	29. 153	7
		9 a.m. 2 p.m. 3 p.m.	E. SE	0.9		29. 473	77		9 a. m. 2 p. m. 3 p. m.	w.sw.		2.49 2.52	29. 229	3
	31	9 p. m. 3 a. m.	Calm	0.0	2.63	29. 358	67	19	9 p. m. 3 s. m.	NW	0.1	2.62	29. 392	•
		7 a. m. 9 a. m.	NE	0.4	2. 59	29. 483	68		7 a.m. 9 a.m.	North		2.67	29. 54 8	6
		2 p. m. 3 p. m.	E. SE	0.7	2.51	29. 495	79		2 p. m. 3 p. m.	E. 8E	0.4	2.55	29. 578	7.
ug.	1	9 p. m. 3 a. m. 7 a. m.	Calm	0.0		29. 463 29. 483	70 	13	9 p. m. 3 a. m. 7 a. m.	Calm	0.0	2 60 2 53	29. 583	
		9 a. m. 2 p m.	N. NE	1.2	2.64	29. 492	80		9 a. m. 2 p. m.	8E	0.2	2.55	29. 571 29. 505	-
		3 p. m. 9 p. m.	SE Calm	0.5	2.62 2.60	29. 467	70	ĺ	3 p. m. 9 p. m.	8E W. 8W.	0.2	2.43 2.48	29. 368	
	2	3 a. m. 7 a. m.	Calm			29. 478	68	14	3 a. m. 7 a. m.	8W	0.2	2.46	29. 948	-
		9 a. m. 2 p. m.	SE	0.7	2.64 2.59	29. 370	76		9 a. m. 2 p. m. 3 p. m.	N. NE	ll	2.53	29. 458	i-;
	3	3 p. m. 9 p. m. 3 a. m.	8E 8. SE	0.1	2. 53	29. 610	71	15	9 p. m. 3 a. m.	Calm N. NW	0.0		29. 448	!
	Ī	7 a. m. 9 a. m.	s. sw	0.6		29. 097	73		7 a. m. 9 a. m.	E. NE		2.69	29. 583	
		2 p. m. 3 p. m.	W. NW.	1.7	2. 52	29. 054	81		2 p. m. 3 p. m.	East	i. i	2.63	29. 593	
	4	9 p. m. 3 a. m.	Calm	0.0	2.60	29. 135	72	16	9 p. m. 3 a. m.	Calm	0.0	2 63 2 57	29. 598	ا.
		7 a. m. 9 a. m. 2 p. m.	SE	0, 7	2.66	29. 210	72		7 a.m. 9 a.m. 2 p.m.	8W	Li	2.56	29.702	
		3 p. m. 9 p. m.	8. SE W. NW.	1. 2	2.65 2.50	29. 225	68		3 p. m. 9 p. m.	8W 8. 8W	1.4	2.55 2.56	29. 680 29. 639	ï
	5	3 a. m.	Calm	0, 0	2, 54	29. 405	63	17	3 a. m. 7 a. m.	sw	0.7	2.56	29. 661	
		9 a. m. 2 p. m.	NW	0. 2	. 	29. 433	70		9 a.m.	8W	2.7	2.73	29. 495	١.
	` 6	3 p. m. 9 p. m. 3 a. m.	Calm	0.0		29. 458	66	18	3 p. m. 9 p. m. 3 a. m.	8W 8W N. NE	2.6 0.2 1.0	2.75 2.65	29. 478	1
	Ů	7 a. m. 9 a. m.	Cam		2.60	29. 543	65	10	7 a. m. 9 a. m.	NE	2.5	2.68 2.65	29. 535	İ
		2 p. m. 3 p. m.			2, 65	29, 473	72		2 p. m. 3 p. m.	NE	2.8	2.58	29. 493	
	7	9 p. m. 3 a. m.	sw		2.48	29. 373	68	19	9 p. m. 3 a. m.	N. NE	0.1	2.53 2.57	29. 451	
		7 a. m. 9 a. m. 2 p. m.	sw		2. 57	29. 340	79		7 a. m. 9 a. m.	8E	0.6	2 59	29. 426	• •
		3 p. m. 9 p. m.			2.53	29. 247	90		2 p. m. 3 p. m. 9 p. m.	8E	1.7 0.3		29. 378 29. 393	7
	8	3 a. m. 7 a. m.				29. 260	81	20	3 a. m.	w. 8w.	0, 1	2 63		٠.
		9 a. m. 2 p. m.			· • • • • • • • • • • • • • • • • • • •	29. 569	90		9 a.m. 2 p.m.	West	1.5	2. 57	29. 418 29. 387	8
	9	3 p. m. 9 p. m. 3 a. m.	W. NW.			29. 104	91	6.	3 p. m. 9 p. m.	South	0.0	2.59 2.49	29. 391	Ġ
	ð	7 a. m. 9 a. m.	W. NW.	0.2	•••••	29. 180	66	มเ	3 a.m. 7 a.m. 9 a.m.	Calm SE		2.51 2.48	29. 300	63
		2 p. m. 3 p. m.				29. 258	70		2 p. m. 3 p. m.	SE			29.35 8	73
		9 p. m.	1	ll		29. 278	65	l .	9 p. m.	8. 8W	0.5	2 45	29. 153	

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WINI	o	38. reduced	hed.			WIN	D.	er be-	reduced	뒇
Date	Hour of day.	Course.	Relative ve- locity.	8 2	Therm'r detached.	Date,	Hour of day.	Course.	Relative ve-	Height of water low 1839.	Barometer rec	Therm'r detached.
1962 Aug.	22 3 a. m. 7 a. m. 9 a. m.	.	l	52 29. 15:	3 69	1862. Sept. 3	3 a. m. 7 a. m. 9 a. m.	8. 8W .	0.4	2.75 2.79	29. 379	57
	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	W.NW. W.NW.	2.4 2	29, 246 62 70 29, 32	5 66	4	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	8. SW . 8. SW . 8W	3.0 2.6 2.4	2.86 2.85 2.92	29. 273 29. 278 29. 330	75 72
	9 a. m. 2 p. m. 3 p. m. 9 p. m. 9 a. m.	N. NE N. NE N. NE	4.2 2 4.7 2 1.1 2 0.2 2	62 29. 568 51 48 29. 596	66	5	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	8W 8. 8W Calm	2.7 1.9 0.2 0.0	2.98 3.05 2.97 2.99	29. 342 29. 355	63
	7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	8E	1.9 2 2.4 2 0.5 2	29, 596 42 29, 536	73	J	7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	Calm	0. o	2.86 2.65 2.39	29. 350 29. 275 29. 315	79 76 60
	25 3 a. m. 7 a. m. 9 a. m. 2 p. m.	8W	202	55 29. 421 60 29. 327	69	6	3 a. m. 7 a. m. 9 a. m. 2 p. m.			2.32	29. 359 29. 389	57
,	9 p. m. 3 a. m. 7 a. m. 9 a. m.	8W 8.8W . 8W	2.2 2 0.2 2 0.2 2 1.1 2	66 29, 325 65 29, 265 61	73	7	3 p. m. 9 p m. 3 a. m. 7 a. m. 9 a. m.	8W	0. 1	2.62	29, 213 29, 293	62 62
•	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	s.sw nw. w.nw.	2.5 2. 0.3 2. 0.8 2.	59 55 29. 26 0	. 73	8	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	sw NW W.sw.	2.2 2.1 0.4	2.76 2.81 2.82	29, 203 29, 200 29, 369	76 73 58
•	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	N. NW. NE W. NW. NW	1. 2 2. 4 0. 4 2. 1 1. 0 2. 5	29, 262 18 57 29, 230	66	9	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	NW N. NW. Calm Calm	0.0	2.88 2.88 2.82 2.72	29, 476 29, 556 29, 636	79 59 57
•	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	W.NW. NW N.NE.	1.9 2.6 2.9 2.6 1.2 2.5	57 29, 334 57 29, 352	84	10	9 a. m. 2 p. m. 3 p. m. 9 p. m.	S. SE Calm Calm	2.3 0.0	2.70 2.65 2.57 2.58	29. 576 29. 531	69 60
•	7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	N. NE NE	3.2 2.4 3.1 2.4 0.7 2.4	29. 483 8 29. 553	70		7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	8E 8E W. 8W.	1.1	2. 62 2. 58 2. 57	29. 428 29. 368	58 72 67
\$	00 3 a. m. 7 a. m. 9 a. m. 2 p. m.	E. NE	0.5 2.4 1.7 2.5 1.0 2.4	29. 581 29. 524	66	11	3 a. m. 7 a. m. 9 a. m. 2 p. m.	8. SE 8. SW .	0. 5 2. 0	2. 52 2. 62 2. 61	29. 228 29. 198	70
3	9 p. m. 3 a. m. 7 a. m. 9 a. m.	Calm 8.8W	0.0 2.5 0.5 2.5 1.3 2.6	3 29.440 3 29.303	67 68	19	9 p. m. 3 a. m. 7 a. m. 9 a. m.	W. NW. NW	3. 6 1. 3	2 72 2 66 2 72	29. 305 29. 591	59 50
Sept.	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	8W W.NW. W.NW.	3.3 2.5 1.9 2.6 1.9 2.7	0 29.052 0	78 71 63	13	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	N. NE NE E. 8E	2. 6 2. 0	2. 73 2. 60 2. 48	29. 697 29. 717 29. 705	54 53 55
	9 a. m. 2 p. m. 3 p. m. 9 p. m. 2 3 a. m.	NE NE N. NE N. NW	2.8 2.6 4.2 2.5 3.3 2.6 0.4 2.6	29. 183 6 1 29. 363	61 53	14	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	SE SE Calm Calm	1. 9 0. 0	2. 54 2. 46 2. 44 2. 49	29. 611 29. 558	62 53
	7 a. m. 9 a. m. 2 p. m. 3 p. m.	E.SE	0.8 2.6 1.9 2.6	3	60 54		7 a. m. 9 a. m. 2 p. m. 3 p. m.	SE S. SE S. SE	0. 9		29. 551 29. 481 29. 498	58 69 65
	9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	E.SE	0.4 2.6 0.8 2.6 1.9 2.6	8 29. 489 1 29. 452	47	14	9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m.	SE	0. 9	2. 49 2. 61 2. 61		29. 551 29. 481

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WINE).	å Ç	peon	poq.			WINI).	r pe-	reduced	Ą
Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer reduced to 38°.	Therm'r detached.	Date.	Hour of day.	Course,	Relative ve-	Height of water low 1838.	Barometer red to 32°.	Therm's detached
/1862.							1862.						-
é pt. 15	3 a. m. 7 a. m.	N. NE	0.3		29. 578	59	Sept. 27	3 a. m.	Calm	0.0		29. 336	5
	9 a.m. 2 p.m.	N. NE	4.7		29. 611	60		9 a. m. 2 p. m.	8. SE	1.1	2.64	29. 248	. 6
	3 p. m. 9 p. m.	NE	5.4 3.9	2.41 2.38	29. 628	58		3 p. m. 9 p. m.	8. SE	1.7 0.7	2.59 2.60	29. 231	1
16	3 a. m. 7 a. m.	East	1.1	2.39	29. 541	60	28				2.61	29. 253	
	9 a. m.	E. 8E	0.9	2.46	29. 428	64		9 a. m.	8W	0.7	2.64	29. 248	
	2 p. m. 3 p. m.	E. SE	0.4	2. 45			1	3 p. m.	North	0.8		l	
. 17	9 p. m. 3 a. m.	South	0. 1	2.42 2.38	29, 321	64	29		N. NE NE	3.3		29. 338	1 2
	7 a. m. 9 a. m.	South	0.2	2.37	29. 256	65		7 a. m. 9 a. m.	NE	3. 2	2. 59	29. 436	. 5
	2 p. m.	South	0.3		29. 132	66		2 p. m.	NE	2.9		29. 410	13
	3 p. m. 9 p. m.	Calm	0.0	2.40	29. 075	64		9 p. m.	N. NE	0.5		29. 416	, 5
18	3 a. m. 7 a. m.	W.NW.	1.5	. :	29. 268	52	30	7 a. m.	North	.	· • • • • • • • • • • • • • • • • • • •	29. 418	٠.
	9 a.m. 2 p.m.	W.NW	1.5	2. 55	29. 398	63		9 a. m. 2 p. m.	N. NE .	0.2		29. 363	١.,
	3 p. m. 9 p. m.	NW	0.7		29, 501	56		3 p. m. 9 p. m.	E. NE	0. 1 0. 0		29. 330	٠:-
19	3 a. m.	Calm	0. 0				Oct. 1	3 a. m.					
	7 a. m. 9 a. m.	South	0.5	2.58	29. 612	52		9 a. m.	Calm	0.0	2.42	29. 383	٠.
	2 p. m. 3 p. m.	SE	1.4	2.54	29. 578	64		2 p. m. 3 p. m.	8E	0.2	2.47	29. 398	·
20	9 p. m. 3 a. m.	Calm W. 8W	0.0	2. 47 2. 59	29. 548	56	9	9 p. m.	E. 8E W. NW.	0.2	2.50 2.52	29. 283	
	7 a. m.	1			29. 489	58	-	7 a. m.		0.5		29. 323	•
	9 a. m. 2 p. m.	West	1.7		29. 455	76	ł	2 p. m.	N. NE .			29. 376	;-
	3 p. m. 9 p. m.	8W	1.0 0.0		29. 488	65		3 p. m. 9 p. m.	E. SE	0.5	2.62	29. 368	- -
21	3 a. m. 7 a. m.	Calm	0.0	2. 54	29. 513	61	3	3 a. m.	8. SW	0.1	2.67	29. 343	-;-
	9 a. m. 2 p. m.	8. 8W	2, 6	2.51	29. 405	78		9 a. m. 2 p. m.	8. SE	0.7	2 55		٠.
	3 p. m.	8.8W	2.7	2. 56			1	3 p. m.	8. 8W.	3.4	2.56		1.
22	9 p. m. 3 a. m.	8.8W	2.6 2.1		29.400	68	4		8.8W	4.3 3.6	2.61 2.76	29. 070	
	7 a. m. 9 a. m.	8W	2.5	2.70	29. 429	64		7 a. m. 9 a. m.	W. NW	5.0	2.88	29. 268	
	2 p. m. 3 p. m.	8W	0. 5		29. 385	67		2 p. m. 3 p. m.	W. NW	3.9		29. 478	İ
23	9 p. m.	Calm	0.0	2.68	29. 403	65	Oct. 5	9 p. m.	W. NW.		3.00	29. 666	- 1
20	3 a. m. 7 a. m.	8W	1.0		29. 335	66	Oer 3	7 a. m.				29.777	
	9 a. m. 2 p. m.	W.SW	2.5		29. 303	68	1	9 a. m. 2 p. m.	8. SE	1.8		29. 651	
	3 p. m. 9 p. m.	W. NW.	2.8	2.39 2.46	29. 480	54		3 p. m. 9 p. m.	8. SE	2.8 1.6	2.78	29. 516	
24	3 a. m. 7 a. m.	N. NW	0.5		29. 619	46	6		8. 8W.		2.84		- ;-
	9 a. m.	NW	1.0	2.66				9 a. m.	8W	3.4	2.81		٠,
	2 p. m. 3 p. m.	West		2.68	29. 578	62		2 p. m. 3 p. m.	8W	2.5 1.8	2.83	29.061	
25	9 p. m. 3 a. m.	8W W. 8W	0. 1 0. 2	2 75	29. 521	51 	7	9 p. m. 3 a. m.	8W	1.8	2.77	29. 070	i.
	7 a. m. 9 a. m.	8w	2.5		29. 494	49		7 a. m. 9 a. m.	8W	3.4		29.060	
	2 p. m.				29. 371	65		2 p. m.	8. 8W.		J	28. 977	,
	3 p. m. 9 p. m.	sw	0.3	2.91 2.92	29. 396	53	_	9 p. m.	8W	5.5	2.70 2.77 2.73	98. 930	-
26	3 a. m.	8W	0. 1		29. 419	50	8	7 a. m.	sw			98. 955	•
	9 a. m. 2 p. m.	sw	1. 1	2.87	29.378	70		9 a. m. 2 p. m.	8W	4.8	l	29, 105	
	3 p. m. 9 p. m.	8.8E 8.8W	2.0	2.76	29. 368	57	l	3 p. m. 9 p. m.	W. NW	. 1.8	2.75	29. 273	٠

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WINI).	er be-	peoup	pod.			WIN).	ş	reduced.	ped.
Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer reduced to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer re- to 32°.	Therm'r detached.
1862. Oct. 9	3 a. m.	Calm	0. 0	2.84	29. 509		1862 Oct. 21	3 a.m.	W.NW.	2.0	2. 89	00 001	
	7 a.m. 9 a.m. 2 p.m.	N.NW.	0. i	2.85	29. 523	49 		7 a. m. 9 a. m. 2 p. m.	W.NW	3. 2	2.96	28. 981 28. 979	49 60
10	3 p. m. 9 p. m.	Calm	0. 0 0. 0 0. 0	2.67 2.68	29. 568	54		3 p. m. 9 p. m.	W.NW'.	4.9 3.4	3. 05 2. 95	29. 139	46
10	3 a.m. 7 a.m. 9 a.m.	N.NW	2.3	2.58 2.58	29. 526	51	22	3 a. m. 7 a. m. 9 a. m.	W.NW.	3.9	3. 05 3. 15	29. 369	37
	2 p. m. 3 p. m.	N.NW	1.3	2. 50	29. 559	48		2 p. m. 3 p. m.	NW	3.0	3. 10	29. 419	46
11	9 p. m.	N.NW	0.2	2. 52 2. 54	29. 626 29. 686	43 38	23	9 p. m. 3 a. m. 7 a. m.	N.NW Calm	0. 2 0. 0	3. 04 3. 03	29. 583	35 36
	7 a. m. 9 a. m. 2 p. m.	N.NE	0. 2	2. 62	29, 629	49		9 a. m. 2 p. m.	NE	0.7	2,90	29. 501	49
	3 p. m. 9 p. m.	E.SE Calm	0.3	2.71	29. 566	41		3 p. m. 9 p. m.	8.8E 8.8W	2.8 3.3	2.81 2.83	29. 341	46
	3 a. m. 7 a. m. 9 a. m.	Calm 8.8W	0.0	2.69 2.73	29. 475	38	24	3 a. m. 7 a. m. 9 a. m.	W.SW	3.7	2.71 2.71	20, 294	47
•	2 p. m. 3 p. m.	8.8E	1.9		29. 353	57		2 p. m. 3 p. m.	1	3.9	2.88	29. 514	39
13	9 p. m. 3 a. m.	Calm	0. 0 0. 0	2.72 2.72 2.71	29, 316	45	25	9 p. m. 3 a. m.	W.NW NW N.NW	3.6	2.93 2.96	29. 734	33
	7 a. m. 9 a. m. 2 p. m.	8w	1.0	2.70	29. 312 29. 283	63		7 a. m. 9 a. m. 2 p. m.	North	3. 2	2.87	29. 945 29. 937	26
	3 p. m.	SW	1.0	2. 58 2. 46	29. 426	51		3 p. m. 9 p. m.	N.NE N.NW	2.0	2.79 2.77	29. 904	28
14	3 a. m.	N.NE	2.9		29. 587	40	26	3 a. m. 7 a. m. 9 a. m.	Calm	0.0	2.75	29. 673	27
	9 a. m. 2 p. m. 3 p. m.	N.NE	3, 3		29. 591	47		9 a. m. 2 p. m. 3 p. m.	w.sw.	1.8	2.84	29, 491	39
15	9 p. m. 3 a. m.	NE	2.2 1.8 0.6	2. 52 2. 51	29. 648	45	27	9 p. m. 3 a. m.	Calm	0.0	2. 94 2. 92	29. 411	32
	7 a. m. 9 a. m.	North	0. 3	2.54	29. 642	39		7 a. m. 9 a. m.	8.8W	0.2	2.87	29. 352 29. 264	29
	2 p. m. 3 p. m. 9 p. m.	w.sw	0. 5 0. 6	2.54 2.66	29. 197	50 		2 p. m. 3 p. m. 9 p. m.	8.8E 8.8W	1. 0 1. 6	2.73 2.69	29. 198	45
16	3 a. m.	8W	1.9	2.80	29. 118	46	28	3 a. m. 7 a. m.	8.8W	1.5	2.69	29. 143	40
	9 a. m. 2 p. m. 3 p. m.	NW	0.3	2.70	29. 117	58		9 s. m.	W.NW.	2.6	2.71	29. 194	50
17	3 p. m. 9 p. m. 3 a. m.	N.NW N.NW N.NW	3. 5 0. 7 0. 3	2.71 2.65 2.74	29. 374	44	29	3 p. m. 9 p. m. 3 s. m.	W.NW. NW W.NW.	0.7 0.4	2. 61 2. 65	29. 371	42
	7 a. m. 9 a. m.	NE	0.8		29. 555	36		7 a. m. 9 a. m.	8.8W	0.4	2.74	29. 512	34
	2 p. m. 3 p. m.	8.8E 8.8W	1.5 0.9	2.72 2.74	29. 556	50 		2 p. m. 3 p. m. 9 p. m.	NE	0.5 2.8	2.86 2.91	29, 480 29, 414	51 54
18	9 p. m. 3 a. m. 7 a. m.	8.8W	1.6	2.85	29, 444	49	30	3 a. m. 7 a. m.		2.0	2.97	29. 287	53
	9 a. m. 2 p. m.	8W	3.9	3. 01	29. 268	70		9 a. m. 2 p. m.	w.sw	3.4	2.83	29. 224	68
19	3 p. m. 9 p. m. 3 a. m.	NW W.NW.	3.4 2.9 2.2	3.00 2.86 3.03	29. 328	63	31	3 p. m. 9 p. m. 3 a. m.	8W 8W	2. 0 0. 6 2. 9	2.83 2.76 2.92	29. 241	52
1.0	7 a. m. 9 a. m.	N.NW.	3.3		29. 509	43	31	7 a. m. 9 a. m.	sw	4.6	2. 89	29, 179	55
	2 p. m. 3 p. m.	N.NW.	2.1	3.04	29. 616	51		2 p. m. 3 p. m.	W.8W . N.NE	2.6	2. 83	29. 140	71
20	9 p. m. 3 a. m. 7 a. m.	Calm	0.0	2. 94 2. 96	29. 647 29. 457	39 	Nov. 1	9 p. m. 3 a. m. 7 a. m.	N.NE NE	2. 5 2. 3	2. 67 2. 49	29, 243 29, 286	53 45
	9 a. m. 2 p. m.	8.8W	3. 6		29, 120	56		9 a. m. 2 p. m.	NE	1.7	2. 60	29. 268	45
	3 p. m. 9 p. m.	8.8W	4. 3 4. 6	3. 13 2. 94	28, 945	55		3 p. m. 9 p. m.	N.NE N.NW	2.6 1.7	2.72 2.70	29. 394	39

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND		- pe	need	ped.		- 1	WINI) ₁₀	er be	1	7
Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer reduced to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1636.	Barometer reduced to 38°.	Produced a second
1862.	3 a. m.	N.NW		0 75		H	1862.	20.00	Colon		2.98		1
Nov. 2	7 a. m. 9 a. m.	NW	i.i	2.75	29. 330	35	Nov. 14	3 a. m. 7 a. m. 9 a. m.	N.NW.		3.06	29.778	, :
	2 p. m. 3 p. m.		2.0		29. 275	38		2 p. m. 3 p. m.	N.NW.		3.04	29.827	٠.
_	9 p. m.	W.NW.	2.9	3.04	29. 298	36		9 p. m.	North	. 0. 1	3.07	39.949	•
3	3 a. m.	West	0.4		29. 394	322	15	3 a. m.	B.NE	.[30.034	•
	9 a. m. 2 p. m.	N.NE	2.6		29. 521	41		9 a. m. 2 p. m.	8E	.1		29. 955	,
	3 p. m. 9 p. m.	N.NE E.SE	1.7 0.6	3,00	29. 506	40		3 p. m. 9 p. m.	E.SE	. 1.7		29. 923	!
4	3 a.m.	Calm	0.0	2.97	29. 382	36	16	3 a. m. 7 a. m.	88	007		29.803	
	9 a. m.	8.8W	1.5	2.99	29. 254	55		9 a. m. 2 p. m.		· · · ·	2.64	29.71	
	3 p. m. 9 p. m.	8.8W	2. i 1. 8	2.94 2.82	29. 276	48	į	3 p. m. 9 p. m.	Calm	0.0	2.70 2.70	29.67	
5	3 a.m.			2 77	29. 454	38	17	3 a. m. 7 a. m.	Calm		2.86	29.77	
	9 a.m. 2 p. m.	North	3.9	2.82	29. 560	42		9 a. m. 2 p. m.	Calm	a	2.92	29.73	
	3 p. m.	N.NW.	3.9	2.93	1			3 p. m.	Calm	Q.	2.97	29.77	
	3 a. m.	Calm	Q O		29. 688	36	18	9 p. m. 3 a. m.	Calm	. 0.0	2.93 2.96	J	
	7 a. m. 9 a. m.	8W	0.6	3. 16	99. 474	96		7 a. m.	Calm	. a.c	2.89	29. 61	
	2 p. m. 3 p. m.	8W			29. 133	35		2 p. m. 3 p. m.	Calm	. ac	2.94	29.53	٠.
7		North	0.2	2.77 2.56	29. 224	38	19	9 p. m.	Calm N.NE .	123	2.94 2.90 2.93	29.41	٠.
	7 a.m. 9 a.m.	E.NE	2.5	2.64	99. 500	37		7 a.m.	N.NB .	. l .	2.86	29.37	
	2 p. m. 3 p. m.	NE	1		29. 586	39		2 p. m. 3 p. m.	N.NE			29. 33	
8	9 p m.	Calm	0.0	2.70 2.77 2.85	29. 594	31	90	9 p. m.	N.NW.	0.7	2.95 2.86 2.88	29.27	
·	7 a.m. 9 a.m.	N.NW.		2.97	29. 595	27	-	7 a. m.		.	2.67	29.22	
	2 p. m.				29. 541	41		2 p. m.	N.NE.		1	29. 31	
_	3 p. m.	SE	. 0.0		29. 558	33		3 p. m. 9 p. m.	N.NE N.NW.	. 2 1 0.7	2.75	29.40	Ŗ
9	7 a.m.	Calm			29.625	34	21	3 a.m.				29.35	í
	9 a. m.	W.NW		3.04	29. 631	46		9 a. m. 2 p. m.	W.8W		2.96	29. 22	2
	3 p. m. 9 p. m.	Celm		2.96 2.98	99. 661	35		3 p. m. 9 p. m.	W.SW. West	14	3.03	39.94	5
10	3 a.m.	Calm	0.0		29. 592	30	22	3 a. m. 7 a. m.	N.NW.			29. 40	3
	9 a. m. 2 p. m.	E.SE	0.9	3. 01	39, 491	46		9 a. m. 2 p. m.	North	. 3.0	2.87	29.50	2
	3 p. m. 9 p. m.	8E		2. 97 3. 00	29. 263	50	i.	3 p. m. 9 p. m.	N.NW. N.NE.		9.78 9.87	20.65	
11	3 a. m.	8W		3.02	29, 101	49	93	3 a.m.	N.NW.		2.88	29.72	
	9 a. m.			2.94				9 a.m.	N.NW.	. L.	9.97	29. 614	
	2 p. m. 3 p. m.	NW	2. i	2.94	29. 065	45		2 p. m. 3 p. m.	W.SW	. Li	3.07 3.26	29. 463	
12		W.NW	0. 9	3.04 3.16	29. 341	34	94	9 p. m. 3 a. m.	W.SW.	7.6	7.70	l	
	7 a. m.	W.SW	0.7	3. 94	29. 542	95		7 a.m.	8W	.]		20.22	١.
	2 p. m. 3 p. m.	West	0.9	3, 21	29. 484	35		2 p. m. 3 p. m.	w.sw	.]		284, 135	٠.
13	9 p. m.	w.sw	. 0.1	3. 25 3. 25	29. 535	39	25	9 p. m. 3 a. m.	W.NW W.NW	. 1.4	l. .	29, 194	
-0	7 a. m. 9 a. m.		0.4		29. 600	26	-	7 a. m. 9 a. m.	N.NW.	.]		29, 277	
	2 p. m.	8.8W		3. 15	29. 523	38		2 n. m.	1			29, 325	•
	9 p. m.	Calm	امّا	3.06	29. 486	38	l	9 p. m.	N.NW.	. L8	l:	29, 436	

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WINI).	er be-	duced	sed.			WIND	0	er be-	reduced	1
Date.	day.		ve.	f water r 1838.	820.	detacl	Date.	day.		ve-	water 1838.		1
	Hour of	Course,	Relative locity.	Height of low	Barometer reduced to 32°.	Therm'r detached.		Hour of day.	Course.	Relative	Height of low	Barometer to 39	
1862.	5						1862.		127				
to v. 96	3 a. m. 7 a. m.	NW	2.0		29. 453	27	Dec. 8	3 a. m. 7 a. m.	8W		3. 21	29. 303	ŀ
	9 a. m. 2 p. m.	W. NW.	2.5		29. 346	34		9 a. m. 2 p. m.	NW	1.6	.	29. 361	
	3 p. m. 9 p. m.	W.8W.	1.8 2.1		29. 238	30	_	3 p. m. 9 p. m.	W. NW.	0, 3		29. 542	
27	7 a. m.	8.8W	1.5	· • • • • • •	28, 969	30	9	3 a. m. 7 a. m.	N. NW.	6.2		29. 645	ŀ
	9 a. m. 2 p. m.	8W	0.7	3. 17	28.933	32	İ	9 a. m. 2 p. m.	West	1.3	3. 36	29. 549	ŀ
	3 p. m. 9 p. m.	NW	1.5	3. 04 3. 60	29.001	31		3 p. m. 9 p. m.	W.8W.	1.4 0.5	3. 44 3. 59	29. 502	ŀ
28	3 a. m.	W.NW.		2. 98	98. 946	96	10	3 a. m. 7 a. m.	w.sw.	0.5 1.5	3, 54	29. 388	ŀ
	9 a. m. 2 p. m.	W. NW	1.8	2.97	28. 910	29		9 a. m. 2 p. m.	W.SW.	2.8	3. 59	29. 287	ŀ
	3 p. m. 9 p. m.	W. NW.	0.8	3.03 3.07	98. 985	25		3 p. m. 9 p. m.	8W	2.5	3. 55 3. 56	29. 256	ŀ
29	3 a. m. 7 a. m.	NW	0.7	3.06	29.072		11	3 a. m.	8W	3.2 2.9	3. 51	1	ŀ
	9 a. m.	иw	0.7	3.07		27	1	y B. III.	W.SW.	3.5	3. 61	29. 237	ŀ
	2 p. m. 3 p. m.	8W	0.6	3.04	29. 113	35	l	2 p. m. 3 p. m.	West	2.6	3. 57	29. 218	ļ.
30		8.8W.	0. 5 0. 2	3. 05 3. 01	29. 156	30	19	9 p. m. 3 a. m.	W.NW. E. NE	0.6	3. 45 3. 36	29. 361	l.
	7 a. m.	N. NE.	1.3	2.91	99 . 134	30	ł	7 a. m.	8E	0.7	3. 28	29. 394	١.
	2 p. m. 3 p. m.	N NW	1.5		29, 199	33	ł	2 p. m. 3 p. m.	8E	0.3		29. 228	١
ec. 1	9 p. m. 3 a. m.	NW	1. 5 1. 3 1. 8	2.97 2.98	29. 231	30	13	9 n. m.	8. SE 8. SW .	1.8	3.06	29. 196	ľ
-	7 a. m. 9 a. m.	n. nw	2.4	2.97	29. 574	98		3 a. m. 7 a. m. 9 a. m.	8. 8W	3.0	3. 19	39. 111	ľ
	2 p. m.	NW	1		29. 662	98	1	2 p. m.				29. 092	ľ
	3 p. m. 9 p. m.	W. NW.	1. 7 1. 5 1. 3	3. 06 3. 10		25		19 n. m.	W.SW.	1.4	3. 20	29. 236	ľ
9	7 a. m.	W.NW.		3. 93	29. 799	18	14	3 a. m.	W.NW.	0.1		26. 260	ľ
	9 a. m. 2 p. m.	w.sw	1.7	3. 31	29. 594	24		9 a. m. 2 p. m.	8. SE	0.3		29.063	ľ
	3 p. m. 9 p. m.	8W	1.5	3. 34 3. 33	29. 447	20		3 p. m. 9 p. m.	w.nw.	1. 2 3. 5	3.34	29. 080	ŀ
3		w.sw.	1.9	3. 34	29. 315	25	15	3 a. m. 7 a. m.	W.NW.	2.0		29. 163	ŀ
	9 a. m. 2 p. m.	W.NW	1.5	3.98	29. 394	24		9 a. m. 2 p. m.	W. NW.	1.6	3, 40	29. 111	١
	3 p. m. 9 p. m.	W. NW.	1.4	3. 27 3. 18	29. 423	22		3 p. m. 9 p. m.	W.NW. W.SW.	1.3 1.4	6, 48 3, 46	29. 095	ŀ
4	3 a. m.	w.sw		3 17			16	3 a. m. 7 a. m. 9 a. m.	W.NW.		3.48		1
	7 a. m.	8w	0. \$		29. 378	30	i	9 a. m.	NW	3.5	3. 39	29. 123	
	2 p. m. 3 p. m.	NW	1.5		29. 262	26		2 p. m. 3 p. m.	W. NW.	3.5	3. 45	29. 297	. .
5		W.NW.	0.5	•••••	29. 344	21	17	9 p. m. 3 a. m.	NW N. NW.	3.7 2.6	3.34 3.37	29. 496	. .
	7 a. m. 9 a. m.	8w	1.2		29, 390	18		7 a. m. 9 a. m.	N. NW	2.2	3.98	29. 812	ŀ
	2 p. m. 3 p. m.	W. NW.	2.9		29. 174	26		2 p. m. 3 p. m.	8W	0.5	. .	29. 895	1.
6	9 p. m.	N. NW N. NW	3.2		29. 282	20	18	9 p. m.	8.8W .	2.4	3 36	29. 882	
•	7 a. m. 9 a. m.				29. 536	4		7 a. m. 9 a. m.				29. 605	ľ
	2 p. m.	N. NW			29. 607	10		2 n. m.	8W		3,81	29. 362	ľ
_	3 p. m. 9 p. m.	NW	. 1. 3	3. 22	29.657	6	l	3 p. m. 9 p. m.	SW	1.3	3.80	29. 509	ľ
7	7 a. m.	W.NW		3. 25	29. 664	···i·	19	7 a. m.	N. NE		3. 33	29. 800	ľ
	9 a. m. 2 p. m.	W.NW.			29. 626	16		9 a. m. 2 p. m.	N. NE		3.52	29, 937	ŀ
	3 p. m.	W.8W.	0.9	3.24	1	8		3 p. m. 9 p. m.	N. NE	3.9	3. 40	1	ŀ

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WINI) .	r pe-	reduced.	ed.	1		WINI).	or be-	reduced.	ed.
Date:	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer red to 320.	Therm'r detached.	Date.	Hour of day.	Course,	Relative ve-	Height of water low 1838.	Barometer red to 32°.	Therm'r dutached
18 62. Dec. 2	0 3 a. m	E. SE	1.6	3. 15			1863. Jan. 1	3 a. m.	8W	2.3	3. 87	11	
Dec. 2	7 a. m	.	1.3		30, 039	24	Jan. 1	7 a. m. 9 a. m.	8. 8W.	2.9	3. 93	29. 508	2
	2 p. m 3 p. m	. S. SE	2.6	3. 22	29. 970	28		2 p. m. 3 p. m.	8.8W.	3.5		29. 361	4
2		. South	1.8		29. 805	26	2	9 p. m. 3 a. m.	8.8W	3.5 4.3 3.7	3. 96 3. 65	29. 248	3
	7 a. m	. s. sw	2.3	3. 50	29, 571	29		7 a. m. 9 a. m.	8.8W.	3.0	· • • • • •	29. 152	3
	2 p. m 3 p. m	. W.NW.	1.2	3, 36	29. 471	28		2 p. m. 3 p. m.	8.8W.	4.6	3. 64	28.945	1
2		. W. SW	0. 0 1. 0		29. 513	29	3	9 p. m. 3 a. m.	8.8W.	3.2 1.7	3.54 3.44	29.031	• •
	7 a. m	. W.NW.	2.6	3. 57	29. 439	36		7 a. m. 9 a. m.	8. SE	0.6	3. 43	29. 053	1
	2 p. m	. W. NW.	2.6	3. 44	29. 371	40		2 p. m. 3 p. m.	8E	0. 2 0. 0	3. 36	29. 013	1.
2		. NE	0.7	3. 45 3. 31	29. 499	35	4	9 p. m. 3 a. m. 7 a. m.	N. NW	2.2	3.29 3.28	28. 897	.
	7 a. m	. East	1.5	3.38	29. 596	36		9 a. m.	W. NW	3.8	3.94	28. 808	. 3 1 4
	2 p. m 3 p. m 9 p. m	. 8E	2.2	3. 23 3. 18	29. 532	36		2 p. m. 3 p. m. 9 p. m.	West	2.2 2.0	3.46	28. 978 28. 955	
9	4 3 a. m	. South	1.7	3. 16	29. 209		5	9 p. m. 3 a. m. 7 a. m.	w.sw	1.5	3, 51 3, 58		1.3
	9 a. m	. W.8W.	3. 1	3, 30	29. 249	43 50		9 a. m.	w.sw	0.3	3.51	28, 998	ŀ
	3 p. m	. sw	3, 6	3.21	29. 318	45		3 p. m.	NW	0. 1	3. 49 3, 51	28. 985	
2		. W. NW			29. 347	36	6	3 a. m.			3. 46	29. 072 29. 197	i
	9 a. m	. West	a i	3, 60	29. 229	41		9 a. m. 2 p. m.	NW	3.7	3. 55	29. 334	1
	3 p. m 9 p. m	. Calm	0.0		29. 161	37		3 p. m. 9 p. m.	NW W.NW	3. 0 1. 4	3.58 3.67	29. 471	j-:
2		. NE	1.7		28. 911	41	7	3 a. m.	W. NW			29.696	!
	9 a. m	. N. NE	3.3	3. 17	28. 953	43	ļ	9 a. m. 2 p. m.	W. NW	1.4	•••••	29.632	 -
	3 p. m 9 p. m	. N. NW	3.4		29. 166	36	1	3 p. m. 9 p. m.	West	0. 5 0. 1	· • • • • • •	29. 618	ri
2		. w. nw			29. 217	30	8	3 a. m.	Calm	Õ. o		29. 578	١٠;
	9 a. m	. NW	1.5	3. 35	29. 203	31		9 a. m. 2 p. m.	Calm	0.0		29. 539	-
	3 p. m 9 p. m	. W. NW	2.4 1.5	3, 43 3, 44	29. 213	29		3 p. m. 9 p. m.	NW	0. 1 0. 0		29. 636	: 1
2		. sw	i. i		29. 181	31	9	3 a. m. 7 a. m.	Calm	0.0		29. 595	-
	9 a. m 2 p. m	. 8.8W	0.8	3. 53	28. 996	44		9 a. m. 2 p. m.	Bouth	0.3	3, 37	29. 452	
	3 p. m 9 p. m	. 8W	2.4 1.9	3, 54 3, 46	29. 000	39		3 p. m. 9 p. m.	8. SE	1,0 1,1		29. 229	1-3
2		. N. NW	0.9		29. 349	35	10	3 a. m. 7 a. m.	8W		3. 35	·	. 3
	9 a. m 2 p. m	. North	1,4	3, 44	29. 506	36		9 a. m. 2 p. m.	W.NW	1.9		28. 986	. 9
	3 p. m 9 p. m	. N. NE	0. 6 2. 2		29. 436	33		3 p. m. 9 p. m.	W.NW W.NW	2.6	3. 45 3. 46	29. 073	- 9
3		. N. NE	4.3		29, 462	28	11	3 a. m.	Calm	0.0	3.51	29. 155	ì
	9 a, m	. N. NE	3.5	3. 27	29. 457	29		9 a. m. 2 p. m.	NW	0.2		29, 269	
	3 p. m	. N. NE	2.3 0.4	3. 29 3. 29	29. 574	23		3 p. m.	N. NW Calm	0, 1	3. 53 3. 49	29. 409	2
3	1 3 a. m	. W. NW			29. 609	21	12	3 a. m. 7 a. m.	Calm	0.0	2 49 3 45	29. 369	
	9 a. m	. sw	1.6	3. 44	29. 604	36		9 a. m. 2 p. m.	Calm	0.0		29. 416	3
	3 p. m		2.8	3, 60	29. 594	31	İ	3 p. m. 9 p. m.	NE			1	3

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WIND	.	<u>\$</u>	reduced.	å.			WIND).	<u>\$</u>	reduced	Ę
Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer red to 320.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer red to 22°.	Therm'r detached.
1863.							1963.						
Jan. 13	3 a. m. 7 a. m.	East	1.7	3. 30	29. 354	35	Jan. 25	3 a. m. 7 a. m.	Calm	0.0		29. 595	2
	9 a. m.	SE	2.4	3. 30	1			9 a. m.	8.8 W	0. 1		. 	
	2 p. m. 3 p. m.	8.8W	0.3	3. 19	29, 048	40		2 p. m. 3 p. m.	South	a.i		29. 552	4
14	9 p. m. 3 a. m.	W.NW.	3.2	3. 29 3. 31	29. 022	45	26	9 p. m. 3 a. m.	Calm	0.0		29. 558	30
	7 a. m.	W.NW.	2.9	3. 53	29. 220	27		7 a. m. 9 a. m.		0.7		29. 484	3
	2 p. m.		1 ' 1		29. 161	25		2 p. m.	N.NW			29. 424	30
	3 p. m. 9 p. m.	W,NW.	2. 1 0. 7	3. 54 3. 53	29, 271	19		3 p. m. 9 p. m.	N.NW	0.7		29, 436	2
15	3 a. m. 7 a. m.	N.NW		3, 71	29. 245	16	27	3 a. m.	N.NE	0.9		24, 472	2
	9 a. m.	N.NW.	1.3	3. 58	l			7 a. m. 9 a. m.	N.NW	1.0			
	2 p. m. 3 p. m.			3. 34	29. 143	20		2 p. m. 3 p. m.	North	0.8	- 	29. 467	2
16	9 p. m.	N.NE North	4.3	· : -	29. 189	18	28	9 p. m. 3 a. m.	North NW	0. 2 0. 1	•••••	29.612	21
	7 a. m.			• • • • • •	29. 236	10		7 a. m.		0. 1		29. 484	19
	9 a. m. 2 p. m.	N.NW	20	• • • • • • • • • • • • • • • • • • •	29. 394	14		9 a. m. 2 p. m.	W.SW	0.2		29.354	3
	3 p. m. 9 p. m.	NW	0.8	• • • • • •	29, 479	10		3 p. m. 9 p. m.	W.SW.	0.5		29. 251	2
17	3 a. m. 7 a. m.	Culm	0.0		29. 640	ii	29	3 a. m.	W.SW.	0.4		29. 039	21
	9 a. m.	8.8W	0.6	· • • • • • • • • • • • • • • • • • • •			,	9 a. m.	W.NW.	0.5			
	2 p. m. 3 p. m.	8.8W	1.5	· • • • • •	29. 678	24		2 p. m. 3 p. m.	West	1.5		28. 928	40
18	9 p. m. 3 a. m.	8.8W 8.8W	1.3		29. 725	18	30	9 p. m. 3 a. m.	W.NW.	0.8		29. 016	3
10	7 a. m.		l l.	• • • • • •	29.704	17	30	7 a. m.				29. 288	2
	9 a. m. 2 p. m.	8.8W	1.7	. 	29. 657	37		9 a. m. 2 p. m.	NW	1.7		29. 396	3
	3 p. m. 9 p. m.	8.8W 8.8W	0.8	•••••	29, 692	31		3 p. m. 9 p. m.	W.NW. West	1.0 0.6		29. 314	2
19	3 a. m.	Calm	0.0		29. 706		31	3 a. m.	8W	0. 2		29. 305	
	7 a. m. 9 a. m.	Calm	0.0	· • • • • • • • • • • • • • • • • • • •		28		9 a. m.	W.NW.	1.0			20
1	2 p.m. 3 p.m.	Calm	0.0	• • • • •	29. 656	36		2 p. m. 3 p. m.	North	0.1		29. 264	3
20	9 p. m. 3 a. m.	E.SE	0.3		29, 617	34	Feb. 1	9 p. m. 3 a. m.	Calm 8.8W	0.0		29. 231	35
	7 a. m.			· • • • • • •	29. 407	36	Feb. 1	7 a. m.				29. 911	3
	9 a. m. 2 p. m.	E.SE	1.4	. .	29. 374	37		9 a. m. 2 p. m.	West	3.4		28. 988	i
	3 p. m. 9 p. m.	East Calm	0.1		29, 414	36		3 p. m. 9 p. m.	W.NW. W.NW. W.NW.	5.8 4.0		29. 371	-
21	3 a. m.	N.NW.	0.1	• • • • • • •	29. 518		2	3 a. m.	W.NW.	2 2			
	7 a. m. 9 a. m.	North	0.1	3. 22		33		7 a. m. 9 a. m.	NW	2. 3		29. 627	. [
	2 p. m. 3 p. m.	w.sw .	0. i	3. 29	29. 569	34		2 p. m. 3 p. m.	NW	2.0		29. 659	!
22	9 p. m.	8.8W	0.3	3. 44 3. 53	29. 563	34		9 p. m.	NW	1.0		29.697	- :
222	3 a. m. 7 a. m.	8.8W			29. 421	36	3	3 a. m. 7 a. m.	NW	1.3		29, 805	-10
	9 a. m. 2 p. m.	8W	0.7	3. 70	29. 394	28		9 a. m. 2 p. m.	NW	0.7		29, 832	1
	3 p. m. 9 p. m.	Calm N.NE	0. 0 0. 1	3. 47 3. 54	29. 421	37		3 p. m.	N.NW	0.1		29, 892	·
23	3 a. m.	E.NE	0.9	3. 46			4	3 a. m.	Calm	0.0			
	7 a. m. 9 a. m.	E.NE		3. 42	29. 422	35		7 a. m. 9 a. m.	8.8W	0.3		29. 857	1
	2 p. m. 3 p. m.	8E	l ! .	3. 07	29. 284	48		2 p. m. 3 p. m.	8.8E	1.4		29. 753	2
	9 p. m.	8.SE	2.7	3. 18	29. 162	38	_	9 p. m.	S.8W	0.3		29. 657	2
24	3 a. m. 7 a. m.	sw	2.5		29 237	35	5	3 a. m.	Calm	0.0		29. 490	2
	9 a. m. 2 p. m.	w.sw.	2.5	•••••	29. 362	36		9 a. m. 2 p. m.	N.NW.	0. 1		29. 520	2
	3 p. m.	West	2.1 0.5	· • • • • •	29. 515			3 p. m.	N.NW	0.3		23.020	1.5

TABLE AA .- Showing the wind, water, barometer, &c. - Continued.

		WIND		above uge.	reduced.	.peq.			- 4	WINI		above	duced	Pad.
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer red to 32°.	Therm'r detached.	Date		Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 32º.	Thorn's detached.
1863. ab. 6	3 a. m.	NW	0. 1				1863. Feb.		3 a. m.	Calm	0.0			١.,
. ·	7 a. m. 9 a. m.	West	0. 1		29. 486	17			7 a. m.	8.8W	0.1		29. 290	
	2 p. m. 3 p. m.	8W	0. 3		29. 352	27	1		2 p.m. 3 p.m.	Calm	0,0	•••••	28. 177	
7	9 p. m. 3 a. m.	8.8W	0.5		39. 332	25	ļ.	19	9 p. m. 3 a. m.	Calm	0.0		29, 169	٠.
7	7 a. m.	w.sw			29. 386	23			7 a. m. 9 a. m.	N,NE	0.6		29.086	
	9 a. m. 2 p. m.		0.8		29. 480	35			2 p. m.	N.NE	5.0		38.94 3	
	3 p. m. 9 p. m.	W.NW.	0.2		29. 326	30			3 p. m. 9 p. m.	N.NE	2.4		29.088	1
8	3 a. m. 7 a. m.	N.NE	0.3		29. 738	25	}	20	3 a. m. 7 a. m.				29, 418	
	9 a. m. 2 p. m.	NE	1.5		29. 697	30			9 a. m. 2 p. m.	N.NE	0.9		29. 549	• •
	3 p. m. 9 p. m.	NE E.NE	0.3		29. 531	30			3 p. m. 9 p. m.	Calm	0.0		29. 657	
9	3 a. m.	E.SE	1. 1		29. 405	382	!	81	3 a. m. 7 a. m.	N.NE	20		29.836	• •
	9 a. m.	862	1.2		29.083	39			9 a. m. 2 p. m.	E.NE	2.2		29, 812	
	2 p. m. 3 p. m.	W.NW.	0.3		29. 474	27			3 p. m. 9 p. m.	E.NE	20		29.689	.;.
10	9 p. m. 3 a. m.	W.NW.	1.4				!	22	3 a. m.	E.NE	2.7		29.545	
	7 a. m. 9 a. m.	W.NW.	0.4		29.695	19			9 a. m.	NE	1.7		29, 605	
	2 p. m. 3 p. m.	Calm	0.0		29. 733	26			2 p. m. 3 p. m.	NE	3.0			٠'.
11	9 p. m.	Calm	0.0	·····	29. 697	25		23	9 p. m. 3 a. m.	Calm	0.0		29.688	
••	7 a. m. 9 a. m.	Calm	0.0		29. 529	26			7 a. m. 9 a. m.	8W	0.1		29, 702	٠.
	2 p. m. 3 p. m.	North	0. 1			••••			2 p. m. 3 p. m.	8.8E	0.9		29, 606	
	9 p. m.	N.NE	0.6		29. 461	25	١.	94	9 p. m. 3 a. m.	Calm	0.0		29, 525	1
12	3 a. m. 7 a. m.	Calm			29. 443	25	,	~	7 a. m.	Calm	0.0		29. 515	
	9 a. m. 2 p. m.	N.NE	2.3		29. 612	27			9 a. m. 2 p. m.				29. 514	
	3 p. m. 9 p. m.	N.NE	1.6 0.0		29. 616	21			3 p. m. 9 p. m.	Calm	0.0		29. 517	• •
13	3 a. m. 7 a. m.	Calm	0.0		29, 668	24	!	25	3 a. m. 7 a. m.	N.NW.	0.1		29, 491	
	9 a. m. 2 p. m.	8M	0.1	,	29. 710	31			9 a.m. 2 p.m.	Calm	0.0		29, 439	
	3 p. m. 9 p. m.	8.8E	1.2		29. 472	34	}		3 p. m. 9 p. m.	SE	0.3		29. 236	. •
14	3 a. m.	8.8E	29		29, 174	36	1	26	3 a. m. 7 a. m.	8E	0.3		28. 879	·
	7 a. m.	8.8E	27		1	44			9 a. m. 2 p. m.	8W	2.1		28.922	
	2 p. m. 3 p. m.	West	5.3		28. 811				3 p. m.	West	2.7 0.4		29, 143	
15	9 p. m. 3 a. m.	West	6.0 3.1		29. 118	31	,	27	9 p. m. 3 a. m.	W. NW	0.7		29, 355	
	7 a. m. 9 a. m.	W.NW.	1.9		29. 506	18			7 a. m. 9 a. m.	NW	0.2			
	2 p. m. 3 p. m.	W.NW.	1.0		29. 597	30	}		2 p. m. 3 p. m.	E.SE	0.2		29, 481	٠.
16	9 p. m. 3 a. m.	W.NW.	0.1		29. 752	26	١,	28	9 p. m. 3 a. m.	Calm	0.0		29, 566	
10	7 a. m.				29. 861	13			7 a. m. 9 a. m.	E.SE	0.1		39. 530	٠.
	9 a. m. 2 p. m.	Calm	0.0		29. 750	31			2 p. m. 3 p. m.	8.8E	0. 1			
	3 p. m. 9 p. m.	8.8E	0.1		29. 642	33	No	,	9 p. m.	Calm	0.0		29, 943	3
17	3 a. m. 7 a. m.	8.8E	i		29. 505	32	Mar.	1	3 a. m. 7 a. m.	Calm			29, 151	3
	9 a. m. 2 p. m.	8.8E	0. 2		29. 426	41			9 a. m. 2 p. m.	W.NW.	0.4		99, 905	35
	3 p. m. 9 p. m.	S.SE Calm			29. 389	35	1		3 p. m. 9 p. m.	W.NW.			99. 941	3

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

	ļ	WINI) .	abov.	reduced .	3			WIN) .	abov	reduced	3
Date.	Hour of day.	Course.	Relative ve-	Heig't of water aboverser of tide-grange.	Barometer red to 39°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Heig't of water above	Barometer red to 32°.	Themp's detached
1963.							1863,						Γ
March 2	3 a. m.	E. SE	0.1		29.096	34	March 14	3 a. m. 7 a. m. 9 a. m.	Calm	0.0		29. 318	2
	9 a. m.	E. NE	5.3		29.098	33		9 a. m. 2 p. m.	8. SE	1.9		29, 161	3
	3 p. m.	N. NE N. NE	6.7					3 p. m.	8. SE	0.4			
3	3 a. m.	N. NE	5.5 1.1		29, 309	30	15	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	8E	0.2		29. 116	3
	7 a. m., 9 a. m.	North	0.9		29. 525	11		9 a. m.	East	0, 1		29 072	3
	2 p. m. 3 p. m.	Calm			29. 554	27		2 p. m. 3 p. m.	NE	0.3	•••••	29. 198	3
4	9 p. m. 3 a. m.	Calm	0.0		29. 516	21	16		Calm	0.0	•••••	99. 39 1	3
•	7 a. m.	North	a i		29. 782	12	10	7 a. m.	8E	0. 1	• • • • • • • • • • • • • • • • • • • •	29. 519	3
	2 p. m.				29. 800	25		2 p. m.			•••••	29. 601	4
	3 p. m. 9 p. m.	Calm	0.0	•••••	29.740	17			SE	0.3 0.0	••••	29. 376	3
5	3 a.m.	8.8W	0.8	•••••	29. 484	96	17	9 p. m. 3 a. m. 7 a. m. 9 a. m.	Calm	0.0		29. 291	4
	9 a. m. 2 p. m.	5. 8W	2.2		39 . 184	34		9 a. m. 2 p. m.	NE	0. 5	•••••	29. 359	3
	3 p. m. 9 p. m.	8.8W 8.8W	2.9		28. 972	37		3 p. m.	N. NE N. NE	3.0 4.8		29. 490	. 3
6	3 a. m.	W.NW.	2.5 2.4	•••••			18	9 p. m. 3 a. m. 7 a. m.	n. ne	14. 6	•••••	 	١
	7 a. m. 9 a. m.	NW	1.6	•••••	29. 260	39		9 a. m.	N. NE	3.2	•••••	29.601	2
	2 p. m. 3 p. m.	NW	0.5	•••••	29. 434	98		2 p. m. 3 p. m.	NE	3.8	•••••	29.711	3
7	9 p. m.	Calm	0.0	•••••	29. 466	25	19	9 p. m. 3 a. m.	NE E. SE	3.8 2.1 1.3	••••	29.719	3
•	7 a. m. 9 a. m.	East	0.2		29. 393	222		7 a. m. 9 a. m.	E. 8E	1.2	•••••	29. 705	3
	2 p. m.			•••••	29. 309	39		2 p. m.			•••••	29. 649	3
	3 p. m. 9 p. m.	Calm	0.0	• • • • • •	29. 216	33		3 p. m. 9 p. m. 3 a. m.	E. NE E. SE	1.0 1.8 3.9		29. 694	3
8	3 a. m. 7 a. m.	8. 8W	0. 1	•••••	29. 078	33	90	7 a. m.	8E	1	•••••	29. 481	3
	9 a. m. 2 p. m.	W.NW.	0.6		39.068	36		9 a. m. 2 p. m.	E. SE	2.9	•••••	29. 467	.3
	3 p. m. 9 p. m.	W.NW. W.NW.	1.7 2.0		29. 196	91		3 p. m. 9 p. m. 3 a. m.	B. SE	3.6 3.6		29. 413	. 3
	3 a. m.	West	1.1	•••••	• • • • • • • •		201		E.SE	0.8	•••••		١
	7 a. m. 9 a. m.	w.nw.	1.6	•••••	99. 365	18		7 a. m. 9 a. m.	Calm	0.0	•••••	29.37 0	3
	2 p. m. 3 p. m.	W.NW.	1.6		29. 464	25		2 p. m. 3 p. m.	Calm	0.0		29. 394	4
10	9 p. m. 3 a. m.	Calm SW	0.0	•••••	29. 477	22	22	9 p. m. 3 a. m.	E. SE E. SE	0.1	•••••	29, 453	3
	7 a. m.	W. 8W.	i. i		29. 325	90		7 a. m. 9 a. m.	E. 8E	0.9		29, 387	3
	2 p. m.				29. 173	41		2 p. m.	•••••			29. 261	3
	3 p. m. 9 p. m.	West W.NW.	0. 9 0. 1	•••••	29. 218	35		3 p. m. 9 p. m.	8E	0.4	•••••	29. 236	3
11	3 a. m.	NW	0. 2		29. 395	29	23	3 a.m.	Calm	0.0		29. 187	.3
	9 a.m. 2 p.m.	NW	0.4	•••••	29, 429	30		9 a. m. 2 p. m.	SE	0. 3		29, 281	
	3 p. m.	NW	0.6		29. 548			3 p. m.	8. SE	0.3			
12	3 a. m.	Calm	0.0		•••••	91	94	3 a. m.	N. NW . N. NW .	1. 0 1. 0		29, 196	3
	7 a. m.	NW	1.7		99. 635	16		7 a. m. 9 a. m.	NW	1.5		29.068	3
	2 p. m. 3 p. m.	NW	i. 9		29. 694	22		9 p. m. 3 p. m.	W.NW.	2.7		29. 052	4
13	9 p. m. 3 a. m.	Calm	0.0		29.691	19	95	9 p. m. 3 a. m.	W.NW. NW	1.4 0.5		29. 119	3
	7 a. m.				29. 651	18		7 a. m.				29 . 156	3
1	9 a. m. 2 p. m.	NW	0. 2		29. 616	29		9 a. m. 2 p. m.	N. NW .	1. 1	::::::	29, 177	3
	3 p. m.	Calm	0. 1		29. 520	97		3 p. m.	N. NW . N. NW .	1.2		29. 280	3

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

			WIND	,	sbove uge.	reduced	bed.	7 9 = 1		WIND		er be-	duced	
Date	е.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer rec to 320.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1836.	Barometer reduced to 320.	
1863	3. 26	3 a. m.	N. NW .	0.8				1863. April 7	3 a. m.	N. NE	2.1	3. 30	-11-11	 .l.
		7 a.m. 9 a.m.	N. NW	0.8		29. 378	94		7 a.m. 9 a.m.	n. ne	4.6	3, 33	29, 664	
		2 p. m. 3 p. m.	NW	0.3		29.374	35		2 p. m. 3 p. m.	NE	4. 3	3. 27	29, 672	.
	27	9 p. m. 3 a. m.	8. 8W Calm	0.0	•••••	29. 437	31	8	9 p. m. 3 a. m.	NE E. SE	2.8 1.9	3. 32 3. 33	29.712	٠.
		7 a. m. 9 a. m.	8. SW	1.2		29. 435	31		7 a. m. 9 a. m.	E. SE	ì. 3	3. 50	29.729	
		2 p. m. 3 p. m.	8. SE	1.6		29, 329	40		2 p. m. 3 p. m.	E. 8E	1.0	3. 47	29. 701	
	28	9 p. m. 3 a. m.	N. NE.	0.0 1.6		29. 231	36	9	9 p. m. 3 a. m.	Calm	0.0	3. 50 3. 45	99.666	
		7 a. m. 9 a. m.	N. NE	3.3		29. 237	31		7 a.m.	8E	2.0	3, 54	29. 630	٠,
		2 p. m. 3 p. m.	N. NW N. NW	3.3		29. 337	32		2 p. m. 3 p. m.	8. SE	2.6	3. 49	29. 497	
	29	9 p. m. 3 a. m.	N. NW . North	4. 2 2. 5		29. 482	9 5	10	9 p. m. 3 a. m.	8. SE	1.7 0.3	3.39 3.39	29. 344	
		7 a. m. 9 a. m.	NW	1.0		29. 634	922		7 a. m. 9 a. m.	8. SE	0.5	3. 45	99. 130	.!
		2 p. m. 3 p. m.	E. NE.	0.6		29. 676	31		2 p. m. 3 p. m.	s. sw	0.7	3.37	29.656	٠,
	30	9 p. m. 3 a. m.	Calm	0.0		29.721	98	11	9 p. m. 3 a. m.	8.8W 8W	3.4 2.8	3. 35 3. 41	29,006	
		7 a. m. 9 a. m.	8w	i. i	. .	29.681	28		7 a.m. 9 a.m.	w. sw.	3.9	3.41	28.988	
		2 p. m. 3 p. m.	8w	2. i		29. 467	45		2 p. m. 3 p. m.	w. nw.	3.6 3.7	3. 46	29, 080	١
	31	9 p. m. 3 a. m.	W. NW. North	1. 4 3. 4		29, 274	39	12	9 p. m. 3 a. m.	N. NE N. NE	3.7	3.23 3.38	29, 336	
		7 a.m. 9 a.m.	North	5. 2		29. 340	30		7 a. m. 9 a. m.	N. NE	3.6	3.37	29, 559	
		2 p. m. 3 p. m.	N. NW	3.0		29. 462	30		2 p. m 3 p. m.	NE	4. 1	3. 35	99, 564	
pril	1	9 p. m. 3 a. m.	E. NE 8. SW	0, 1 0, 1	3. 68	29. 496	27	13	9 p. m. 3 a. m.	N. NE	0.4	3.35 3.42		
-		7 a. m. 9 a. m.	sw	3.3	3.71	29. 305	29		7 a.m. 9 a.m.	E. SE	Q. 5	3.41	29. 555	
		2 p. m. 3 p. m.	w.sw.	4. 2	3.84	28.971	50		2 p. m. 3 p. m.	SE	0.6	3. 40	99. 527	
	2	9 p. m. 3 a. m.	W. NW.	3. 0 4. 8	3.90 3.71	28. 768	49	14	9 p. m. 3 a. m.	E. NE East	0. 3 0. 2	3, 35 3, 33	29, 499	
		7 a. m. 9 a. m.	NW	4.4	3. 86	28. 863	47		7 a. m. 9 a. m.	E. NE	1.6	3. 32	29.478	
		2 p. m. 3 p. m.	NW	4. 2	3. 83	29.010	50		2 p. m. 3 p. m.	NE	3.7	3. 30	29, 416	. 1
	3	9 p. m. 3 a. m.	NW	1.8 3.5	3. 72 3. 52	29, 138	49	15	9 p. m. 3 a. m.	NE N. NE	3.2	3.25 3.25	29, 392	
		7 a.m.	N. NE.	6.1	3. 42	29. 452	30		7 a.m. 9 a.m.	NE	0.3	3. 30	29.370	.!
		2 p. m. 3 p. m.	NE	3. 5 0. 2	3. 39	29. 684	33		2 p. m. 3 p. m.	NE	3.3	3. 34	29. 394	.!
	4	9 p. m. 3 s. m.	NE	0.2	3. 46 3. 43	29.722	30	16	9 p. m. 3 a. m.	NE Calm	0.2	3. 31 3. 36	29.368	
		7 a. m. 9 a. m.	8E	0,8	3. 48	29. 746	30		7 a. m. 9 a. m.	E. NE	0.7	3. 35	29, 382	. 1
		2 p. m. 3 p. m.	SE	2.2	3. 45	29. 644	42		2 p. m. 3 p. m.	SE	0.9	3. 36	29. 384	٠,,
	5	9 p. m. 3 a. m.	8. SE Calm	0.2	3. 49 3. 45	29. 556	37	17	9 p. m. 3 a. m.	Calm	0.0	3. 31 3. 37		
		7 a. m. 9 a. m.	8W	0.6	3. 52	29. 429	38		7 a. m. 9 a. m.	8E	1.8	3. 39	99, 390	
		2 p. m. 3 p. m.	NE	1. 2	3, 43	29. 319	35		2 p. m. 3 p. m.	8E	1.3 0.5	3. 39	29, 276	
	6	9 p. m. 3 a. m.	N. NE North	3.0 2.9	3. 50 3. 41	29. 433	41	18	9 p. m. 3 a. m.	8E 8. SE	0.5 1.6	3.34	29. 228	
		7 a. m. 9 a. m.	N. NE	5. 4	3. 45	29. 450	38		7 a.m. 9 a.m.	8E		3, 30		
		2 p. m. 3 p. m.	NE	6.9	3, 30	29. 536	40		2 p. m. 3 p. m.	SE	2.8 0.6	3. 93 3. 11	25, 110	5

TABLE AA.—Showing the wind, water, barometer, &c -Continued.

		WIND		r be-	nced	peq.			WINE	٥.	r be-	need	bed.
Date.	Hour of day.	Соитие.	Relative ve-	Height of water low 1838.	Barometer reduced to 32°,	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer reduced to 320.	Therm'r detached.
1863. April 19	3 a. m. 7 a. m. 9 a. m.	W. NW.	1.6	3, 14	29. 259	48	1863. May 1	3 a.m. 7 a.m. 9 a.m.	Calm	0.0	3, 30	29. 327	56
` 90	2 p. m. 3 p. m. 9 p. m. 3 a. m.	SE SE E. NE	1.8 0.5 1.0	3. 12 3. 22	29, 312 29, 391	52 43	2	2 p. m. 3 p. m. 9 p. m. 3 a. m.	SE S. SE NE	2.0 0,1 1.3	3. 30 3. 30 3. 35	29. 236 29. 253	63
	7 a. m. 9 a. m. 2 p. m.	NE	2.3	3, 37	29, 418 29, 409	42 45	-	7 a. m. 9 a. m. 2 p. m.	NE	4.1	3, 11	29, 306 29, 319	4
21	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	NE SE	1.7	8 19 3, 15	29, 436 29, 371	42 45	3	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	NE	3.0 2.0 1.5	2.96 2.95 2.95 2.95	29. 304 29. 270	4
22	9 p. m. 3 p. m. 9 p. m. 3 a. m.	SE E. SE NE	1.7	3.00	29, 402 29, 459	47	4	9 p. m. 3 p. m. 9 p. m. 3 a. m.	NE NE	2.0 2.9 2.5	2.85 2.88 2.88	29, 222 29, 229	4
**	7 a. m. 9 a. m. 2 p. m. 3 p. m.	NE	1.6	3.06	29. 400 29. 376	46		7 a. m. 9 a. m. 2 p. m. 3 p. m.	NE	2.4	2.84	29. 175 29. 171	4
23	9 p. m. 3 a. m. 7 a. m. 9 a. m.	NE N. NE	1.3	3.11	29, 388 29, 402	42 47	5	9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE N. NE N. NE	2.9 4.1 6.7	2.91 2.83 2.74	29, 224 29, 225	4
24	2 p. m. 3 p. m. 9 p. m. 3 a. m.	N. NE N. NE	4.1	3.25 3.27 3.25	29. 351 29. 356	60 57	6	2 p. m. 3 p. m. 9 p. m. 3 a. m.	N. NE NE E. NE	7.9 5.1 2.2	2.60 2.72	29, 344 29, 424	3
	7 a. m. 9 a. m. 2 p. m. 3 p. m.	N. NE	2.7	3, 32	29. 387 29. 434	54 61		7 a. m. 9 a. m. 2 p. m. 3 p. m.	NE	6.9	2.92	29, 400 29, 429	4
25	9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE N. NE N. NE	4.5 5.5 4.0		29. 459 29. 537	45	7	9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE N. NE	3.9	3, 02	29, 432 29, 452	4
26	2 p. m. 3 p. m. 9 p. m. 3 a. m.	NE NE Calm	3.6 0.5 0.0	3.16	29. 567 29. 546	47	8	2 p. m. 3 p. m. 9 p. m. 3 a. m.	N. NE NE N. NE	4.2 0.4 0.1		29. 407 29. 394	173
	7 a. m. 9 a. m. 2 p. m. 3 p. m.	E. NE	0.5	3. 20	89. 577 29. 577	44		7 a. m. 9 a. m. 2 p. m. 3 p. m.	W. NW.	1.5	3, 18	29. 450 29. 318	
27	9 p. m. 3 a. m. 7 a. m. 9 a. m.	Calm Calm	0.0	3. 17 3. 22	99. 511 99. 467	45 56	9	9 p. m. 3 a. m. 7 a. m. 9 a. m.	W. NW. N. NE.	0.8 1.0	3. 15	29, 448 29, 384	
28	2 p. m. 3 p. m. 9 p. m. 3 a. m.	NE Calm NE	1.0 0.0 0.7	3. 28	29. 374 29. 293	56 49	10	2 p. m. 3 p. m. 9 p. m. 3 a. m.	E. NE NE SW	1.0 0.4 0.8	3. 13 3. 07 3. 05	29. 301 29. 264	
	7 a. m. 9 a. m. 2 p. m. 3 p. m.	NE	3. 4	3.11	29. 233 29. 196	5 <u>9</u> 51		7 a. m. 9 a. m. 2 p. m. 3 p. m.	8w	9. 4	3.09	29. 177 29. 214	
29	9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE N. NE N. NE	7. 1 7. 0 5. 9	2. 95 3. 01 3. 05	29. 216 29. 272	59 44	11	9 p. m. 3 a. m. 7 a. m. 9 a. m.	NE N. NE NE	4. 0 2. 1	2.98 3.00	29. 286 29. 391	
30	2 p. m. 3 p. m. 9 p. m. 3 a. m.	N. NE	3.4	3. 14 3. 15 3. 23	29. 308 29. 344	55 49	12	2 p. m. 3 p. m. 9 p. m. 3 a. m.	E. NE N. NE Calm	C. 8 0. 3	3. 03 2. 98	29. 369 29. 431	
30	7 a. m. 9 a. m. 2 p. m.	E. NE	0, 9	3. 90	29. 401 29. 389	50 56	•	7 a. m. 9 a. m. 2 p. m.	w. nw	1.5	3.06	29. 477 29. 449	
	3 p. m. 9 p. m.	East		3. 23 3. 16	29. 339	51		3 p. m. 9 p. m.	W. W	9. 0 0. 1	3. 11 3. 04	29, 459	ŀ

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND).	r be	reduced	ۇ			WIND	.	8	faced	١,
Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer rec to 32°.	Therm'r detached.	Date,	Hour of day.	Course.	Relative ve- locity.	Height of water low 1636.	Barometer red	
1863.							1863.						Ī
May 13	3 a. m. 7 a. m.	NW	0.3	3, 12	29, 392	54	May 25	3 a.m.	W.NW.	0.5	2.96	29, 293	٠١.
	9 a. m. 2 p. m.	E. NE	1.0	3.04	29, 367	55		9 a. m. 2 p. m.	NE	1.9	2.98	20, 264	1
	3 p. m.	NE	C. 9	3.02				3 p. m.	NE	2.4	2.98		J.
14	9 p. m. 3 a. m.	N. NE	2.2 1.1	2.90 3.05	29. 392	49	96	9 p.m. 3 s.m.	Calm N. NE	0.0	2.95 2.93	29.311	
	7 a.m. 9 a.m.	NE	1.8	3. 01	29. 532	50		7 a. m. 9 a. m.	NE	2.4	2.94	29. 339	ı, i
	2 p. m.	E. NE.	1.7		29. 567	53		2 p.m.	NE	2, i	2.89	39. 374	
	9 p. m.	Calm	0.0		29. 554	47		9 p.m.	NE	0.9	2.88	99. 371	T
15	3 a.m. 7 a.m.	Calm	0.0		26. 480	48	27	3 a.m. 7 a.m.	NE	2.5	2.91	29. 439	.ا. ١.
	9 a.m. 2 p.m.	w.sw.	1.7		29, 214	66		9 a. m. 2 p. m.	NE	2.4	2.91	29.434	٠.
	3 p. m.	8.8W	4.5 3.2		29.006	60		3 p.m.	NE	3.5	2.92 2.89	29, 396	
16	3 a. m.	N. NW.	1.5				28	3 a. m.	NE N. NE	0.9			٠.
	7 a. m. 9 a. m.	NW	3.7		29. 182	55		7 a.m. 9 a.m.	NE	1.6	2.94	29.446	٠.,
	2 p. m. 3 p. m.	NW	4.3		29. 214	61		2 p. m. 3 p. m.	NE	2.5	2.94	29. 229) '
	9 p. m.	E. NE	0 4		29, 296	47		9 p.m.	N. NE	1.2	2.91	29. 346	,
17	7 a. m.	NW	1.2		29. 357	48	29	3 a.m. 7 a.m.	NW	0.9	l	29. 266	
	9 a. m. 2 p. m.	N. NW.	2.5		29, 449	53		9 a.m. 2 p.m.	E. NE	0.8	2, 95	29, 906	,
	3 p. m. 9 p. m.	E. 8E E. NE	0.5	3. 21 3. 14	29, 524	46	•	3 p. m. 9 p. m.	8E	0.3	9.99 2.94	20,082	اا
18	3 a. m.	Calm	ãõ	3.11			30	3 a. m.	N. NW.	23	294		
	7 a. m. 9 a. m.	NW	1.0	3. 15	29. 595	50		7 a. m. 9 a. m.	N. NE.	1.3	2.97	28, 955	
	2 p. m. 3 p. m.	8. SE	1.8	3.14	29. 554	63		2 p. m. 3 p. m.	w.nw.	0, 7	2.99	98.998	,
10	9 p. m.	8.8W	0.9	3.21	29. 522	54		9 p.m.	W.NW.	. Q. 1	2.94	28, 912	į
19	7 a. m.	8W	0.6		29. 514	60	May 31	7 a. m.	N. NE.	2.9]. 	98.895	•
	9 a. m. 2 p. m.	sw	3.3	3. 24	29, 408	76		9 a.m. 2 p.m.	NE	1.8	3.04	28, 875	! 5
	3 p. m. 9 p. m.	8W	22	3, 25 3, 25	29, 373	66		3 p. m. 9 p. m.	8W W.NW. W.NW.	1.3		28, 918	
90	3 a. m.	8w	1.6				June 1	3 a.m.	W.NW.	2.6			
	7 a. m. 9 a. m.	8W	2.9	3. 29	29. 428	6 6	t	7 a. m. 9 a. m.	w.xw.	6.4	3.25	26, 943	
	2 p. m. 3 p. m.	8W	2.8	3, 21	29. 405	82	ł	2 p.m. 3 p.m.	W.NW.	5.4	3. 18	29, 046	i
21	9 p. m.	8.8W	1.2	3.21	29. 495	70	2	9 p. m.	W.KW.	1.3		29, 159)
21	7 a. m.				29.518	69	2	7 a. m.				29, 212	
	9 a.m. 2 p.m.	8W	2.1	3. 10	29.515	73		9 a.m. 2 p.m.	W.NW.	2.5	3.94	29. 251	ij
	3 p. m. 9 p. m.	8E 8. SE	2.4 0.3	2.98 2.99	29. 511	60		3 p. m. 9 p. m.	North	2.4	3.10	29, 366	1
22	3 a. m.	Calm	ű. ö				, з	3 a. m.	Calm	0.2	3.00	29. 442	.'
	7 a. m. 9 a. m.	8E	1.2	2.99	29. 538	64		7 a.m. 9 a.m.	SE	i.i	3,07		
	2 p. m. 3 p. m.	SE	1.8	2.96	29. 461	73		2 p. m. 3 p. m.	SE	1.5	3,03	29, 409	
23	9 p.m.	Calm SE	0.0	2.96 2.92	29. 388	61		9 p. m. 3 a. m.	W.8W W.8W	1.5 0.9 0.8		29, 359	!
23	7 s. m.		1		29, 303	57	•	7 a.m.				29. 313	,
	9 a. m. 2 p. m.	8W	1.2	2.98	29. 286	70		9 a. m. 2 p. m.	W.8W	2.0	1	29, 236	
	3 p. m. 9 μ. m.	West	3.8 2.6	2.96 3.00	29, 316	63		3 p. m. 9 p. m.	West	2.6	9.97 9.96	29, 388	
24	3 a. m.	w. sw.	1.6				5	3 a. m.	E. NE.	1.0	2.93	29. 334	1.
	7 a. m. 9 a. m.	w. sw.	2.2	3.04	29, 243	61	1	7 a. m. 9 a. m.	NE	2.4	2.98		ŀ
	2 p. m. 3 p. m.	W.NW.	2.6	2.98	29. 183	70	ł	2 p. m. 3 p. m.	NE	3.1	2.97	29, 421	
	9 p. m.	W.NW.			29, 226	61	l	9 p. m.	NE	1.3		29, 499	1.

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WINI).	er be-	reduced to	bed.			WINI).	er be-	reduced io.	hed.
Date.	Hour of day.	Соигие.	Relative ve-	Height of water low 1838.	Barometer red	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer red to 32°.	Therm'r detached.
1863. June 6	3 a. m.	NE	36				1863. June 18	3 a. m.	w. nw.	0.5	3. 11		
	7 a. m. 9 a. m.	NE	3.2		29, 627	47		7 a. m. 9 a. m.	w. nw	3.0	3. 16	29. 148	72
	2 p. m. 3 p. m. 9 p. m.	NE	4. 1 2. 1	2.88 2.97	29. 662	50		2 p. m. 3 p. m. 9 p. m.	W. NW.	4. 0 1. 1	3.08 3.02	29. 077 29. 138	65
7	3 a. m.	N. NE N. NE	2.5	2.94	29, 730	51	19	3 a.m.	Calm	ō. o	2.96	29, 121	63
	9 a. m.	B. NE .	2.1	3.00	29.712	59		9 a. m. 2 p. m.	West	2.1	3.01	29.060	79
	3 p. m. 9 p. m.	NE	2.7 1.3	9. 98 3. 00	29. 674	50		3 p. m. 9 p. m.	W. NW.	3. 3 0. 4	2.98	29. 133	56
8	3 a. m. 7 a. m.	N. NE.	1.0	. 	29. 697	56	20	3 a. m.	W. NW.	2.3		29. 156	54
	9 a. m. 2 p. m. 3 p. m.	E. NE.	2.9	3.01	29. 646	64		9 a. m. 2 p. m. 3 p. m.	W. NW.	1.8	3. 03 2. 96	29. 206	61
9	3 p. m. 9 p. m. 3 a. m.	E. NE	1. 1 0. 1 0. 0	3.02	29. 541	53	21	9 p. m.	W. NW. W. NW. W. NW.	0.9	2.99	29, 228	58
	7 a. m. 9 a. m.	R. NE	0.8	.	29. 552	56		7 a.m. 9 a.m.	w. nw.	1. 5	3. 10	29. 211	57
	2 p. m. 3 p. m.	8E	1.5	3. 05	29, 459	64		3 p. m. 3 p. m.	W. NW. W. NW.	0.3	3. 02	29. 218	67
10	9 p. m. 3 a. m. 7 a. m.	Calm	0.0	3, 02 3, 05	29, 414 29, 352	57 58	222	9 p. m. 3 a. m. 7 a. m.	NW	1.8	3. 05 2. 96	29, 341 29, 306	56
	7 a.m. 9 a.m. 2 p.m.	8E	0.3	3. 00	29. 208	70		9 a. m. 2 p. m.	n. nw .	1. 5	3. 02	29. 371	6
	3 p. m. 9 p. m.	SE	0. 3 0. 0 0. 0	3.05 2.96	29. 273	60		3 p. m. 9 p. m.	SE 8. SE	1.5 0.1	2.99 3.05	29, 241	57
11	3 a. m. 7 a. m.	Calm			29. 281	65	23	3 a.m.	8. SE	0.4	3. 10	29. 514	56
	9 a. m. 2 p. m.	E. SE	0.7	2.95 2.98	29. 375	75		9 a. m. 2 p. m. 3 p. m.	East	1.3	3. 12	29. 516	63
12	3 p. m. 9 p. m. 3 a. m.	NE N. NE	1.2 0.1 0.8	2.92	29 . 341	61	94	3 p. m. 9 p. m. 3 a. m.	8E 8. SE Calm	1.6 0.2 0.0	3.09 3.01 3.00	29 . 534	57
	7 a. m. 9 a. m.	E. NE	1.9		29. 461	58	-	7 a.m. 9 a.m.	8E	1.7	3.00	29. 544	60
	2 p. m. 3 p. m.	E. SE	0.8	3. 09	29. 471	64		2 p. m. 3 p. m.	8E	2.0	3. 01	29. 491	71
13	9 p. m. 3 a. m.	Calm Calm	0.0	3. 02 3. 06	29. 498	58	25	9 p. m. 3 a. m.	Calm Calm	0.0	2. 98 3. 01	29. 481	62
	7 a.m. 9 a.m. 2 p.m.	8E	0.7	3. 01	29. 518 29. 510	73		7 a. m. 9 a. m. 2 p. m.	E.8E	0.9	3. 02	29. 489 29. 441	79
	2 p. m. 3 p. m. 9 p. m.	SE	0.8	3. 01 3. 00	29. 508	68		2 p. m. 3 p. m. 9 p. m.	SE Calm	1. 1 0. 0	3. 03 3. 00	29. 443	64
14	3 a. m.	Calm	0.0		29. 531	70	96	3 a.m.	Celm	0.0	3.04	29. 449	66
	9 a.m. 2 p.m.	8. SW	0.9		26. 469	89		9 a.m. 2 p.m.	E. 8E	(L.7	2. 99	29. 398	73
1.5	3 p. m. 9 p. m.	8W W. 8W.	1.9 0.9 1.3	3.08	29. 407	76	~	3 p. m. 9 p. m.	SE Calm	0.9	3.01 2.94	29. 378	70
15	3 a. m. 7 a. m. 9 a. m.	W. NW.	3.3	3. 20 3. 15	99. 445	79	27	3 a. m. 7 a. m. 9 a. m.	Calm E. SE	0.0	2.99 2.97	29. 398	- 66
	2 p. m. 3 p. m.	NE		3.03	29. 448	71		2 p. m. 3 p. m.	8E	1.9	3.00	29. 345	74
16	9 p. m. 3 a. m.	N. NE NE	3.1 0.5 0.4	2.97 2.88	29. 440	65	28	9 p. m.	Calm	0.0	2.96 2.98	29. 335	71
	7 a. m. 9 a. m.	8E	0.7	2.89	99. 485	65		7 a. m. 9 a. m.	8E	1.0	2.99	29. 293	79
	2 p. m. 3 p. m.	8E	1. 4 1. 0	2. 83 2. 88	29. 368 29. 264	79		2 p. m. 3 p. m.	8E	1. 3 0. 7	2.98	29. 905	77
17	9 p. m. 3 a. m. 7 a. m.	W. 8W. W. 8W.	1.8	2.89	29, 242	79	29	9 p. m. 3 a. m. 7 a. m.	W. NW. N. NW.	0.7	2. 95 2. 90	29. 155 29. 085	79
	9 s. m. 2 p. m.	North	2.4	2.97	29. 252	83		9 a.m. 2 p.m.	NE	1.0	2.88	29.063	77
	3 p. m. 9 p. m.	N. NW. W. NW.	2. 1 1. 0	3. 04 3. 10	29, 245	71		3 p. m. 9 p. m.	E. NE E. NE	1. 1 1. 8	2.86 2.83	29, 113	64

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WIND	٠.	er be	reduced	bed.			WINI).	9	reduced o	1
Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer red to 333	Therm'r detached	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Berometer red to 32°.	"Pharm" - dutm.had
1863. June 30	3 a. m.	NE	0. 3	2.91			1863. July 12	3 a. m.	NE	4.4	2. 72		Ī
иде о	7 a. m. 9 a. m.	E.SE	0.2		29. 191	67	3413 12	7 a. m. 9 a. m.	NE	3.4	2.72	29.604	
	2 p. m. 3 p. m.	E. SE	l		29. 221	69		2 p. m. 3 p. m.	NE	4.7	2.74	29. 601	
uly 1	9 p. m. 3 a. m.	8E	1. 2 0. 5 0. 7	3.04	29. 26:1	64	10	9 p. m. 3 a. m.	NE	1. 2 0. 1	2.78	29. 554	
uly 1	7 a. m.	8.8E			29. 233	66	13	7 a. m.				29, 481	
	9 a, m. 2 p. m.	8E	1.3		29. 155	78		9 a.m. 2 p. m.	E. 8E	0.9	2.87	29. 408	
	3 p. m. 9 p. m.	West W. SW	1.8 0.5	2. 85	29. 244	73		3 p. m. 9 p. m.	SE	1.7		29. 336	٠.
2	3 a. m. 7 a. m.	8W	0, 9		29. 268	71	14	3 a.m.	W. NW.	0.1	2.99	29, 299	
	9 a. m. 2 p. m.	W. NW.	2.3		29. 312	79		9 a.m. 2 p.m.	8 W	1.9	2. 99	29. 908	.ļ.
	3 p. m. 9 p. m.	NW W. NW.	1.8		29. 375	72		3 p. m. 9 p. m.	West N. NE	3. 0 4. 1	2.92 2.82	29. 343	٠,٠
3	3 a.m.	w. Nw.	0. 5	· • • • • •	29. 428	70	15	3 a.m.	N. NW	1.5	2.05		
	7 a. m. 9 a. m.	w. nw.	2.4	2.97				7 a.m.	NE	27	2.94	29. 483	ļ.
	2 p. m. 3 p. m.	w. nw.	1.4		29. 428			2 p. m. 3 p. m.	8E	1.3	3.00	29. 594	٠'.
4	9 p. m. 3 a. m.	Calm	0.0		29. 445	72	16	9 p. m. 3 a. m.	SE	0.0	3.01	29.58	
	7 a. m. 9 a. m.	8E	0.7	2.96	29. 435	72		7 a.m. 9 a.m.	8E		3.11	29. 675	
	2 p. m. 3 p. m.	8E	1. 2	2.86	29. 415	82	1	2 p. m. 3 p. m.	8E	2.2		29. 659	,1
	.9 p. m.	Calm	0.0	2.97	29. 405	72		9 p. m.	E. SE	0.4	3.07	29. 546	٠,
5	3 a. m.	Calm			29. 408	73	17	3 a.m.	Calm	0.0		29. 549	.!
	9 a. m. 2 p. m.	8E	1.2		29. 385	78		9 a.m. 2 p. m.	E.NE.	1. 2	. 	29. 638	1
	3 p. m. 9 p. m.	Calm	1. 1 0. 0	2. 92 2. 92	29. 455	71		3 p. m. 9 p. m.	E. NE E. NE	1.5 0.6	2.92 2.93	29. 601	٠,
6	3 a. m. 7 s. m.	N. NE	0. 2		29. 332	73	18	3 a.m.	Calm	0. 0	2.89	29. 624	ł
	9 s. m. 2 p. m.	E. NE	2.3	2.92	29. 352	81		9 a.m.	SE	2.0		. 	å
	3 p. m.	NE	1.9			[]		2 p. m. 3 p. m.	8E	2.3		29, 586	
7	9 p. m. 3 a. m.	E. NE	0.2	2.87 2.80	29. 325	74	19	9 p. m. 3 a. m.	E. SE	0.8	· • • • • • • • • • • • • • • • • • • •	29, 508	٠,
	7 a. m. 9 a. m.	NE	2.2		29.343	74		7 a. m. 9 a. m.	8E	1.7		29. 456	.]
	2 p. m. 3 p. m.	E. NE	0.8		29. 335	81		2 p. m. 3 p. m.	SE	j. i	3.14	29. 392	j
8	9 p. m. 3 a. m.	East Calm	0.1		29. 320	72	90	9 p. m. 3 a. m.	8. SE	0.3 2.3	3. 14	29, 308	
	7 a. m. 9 a. m.		0.9		29. 335	75	~	7 a. m.	 .			29, 181	1
	2 p. m.	East			29, 289	88		9 a.m. 2 p.m.	N. NE	6.6		29, 353	•
_	3 p. m. 9 p. m.	S.SE	1. 1 0.5	3. 15 3. 10	29. 304	75		3 p. m. 9 p. m.	NE	6.2	2.88	29. 446	·
9	3 a. m. 7 a. m.	N. NE	0.2	3. 13	29. 320	68	21	3 a.m. 7 a.m.	N. NE	2.7	2.94	99, 498	1
	9 a.m. 2 p.m.	N. NW	0.4	3.07	29. 282	77		9 a.m. 2 p. m.	NE	2.7	3. 02	29. 178	.!.
	3 p. m. 9 p. m.	E. NE	0, 1 0, 0	3.06 3.03	29. 256	68		3 p. m. 9 p. m.	NE	2.0	3. Ol	29. 444	
10	3 a. m. 7 a. m.	Calm		3. 11	29, 251		22	3 a. m.	NE		3.09	29, 456	١.
	9 a. m.	w.sw.		3. 09		66		7 a.m. 9 a.m.	E. SE		3. 14		1-
	2 p. m. 3 p. m.	W. NW.	0.8	3. 12	29. 204	86		2 p. m. 3 p. m.	8E	2.0	3.09	29, 431	١.
3	9 p. m. 3 a. m.	W. NW. N. NE	0.1	3. 02 2. 89	29, 207	76	23	9 p. m. 3 a. m.	SE 8. SW	0. 2	3.19	29, 391	
	7 a. m. 9 a. m.	NE			29. 395	65		7 a.m. 9 a.m.	W. NW.	l l	3 ii	29, 296	
	2 p. m.	NE			29. 491	59		2 p. m.	l	1	1	39, 227	8
	3 p. m.	NE			29. 556	57		3 p. m.	W. NW. W. NW.	0.1	3.09	29.346	7

TABLE AA .- Showing the wind, water, barometer, &c. - Continued.

			WIND) .	3	reduced	jed.			WINI) .	ģ	reduced	ij
Date	B.	Hour of day.	Course.	Relative ve-	Height of water low 1828.		Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer red	Therm'r detached
1863	,						_	1863.						_
July	24	3 a.m.	W.NW.	0.2	3, 14		٠ <u>٠.</u> .	Aug. 5	3 a. m.	8. 8W	1.8	3.04		ļ. <u></u> .
		9 a.m.	NE	0.8	2 14	99. 355	70		9 a. m.	W.NW.	1.3	3. 11	29. 288	71
		2 p.m. 3 p.m.	NE	1.3	3. 13	29. 355	67		2 p. m. 3 p. m.	W.NW.	1.5	3. 10	29. 334	82
	25	9 p. m. 3 s. m.	NE	1.3 0.7 0.3	3. 19 3. 07	29. 328	62	6	9 p. m. 3 a. m.	Calm N. NW	0. 0 1. 0	3.06	29. 374	75
	~	7 a. m.				29. 208	64		7 a. m.			. 	98. 480	67
		9 a.m.	M. NE	1.6			70		9 a.m. 2 p.m.	NE	3, 2		29. 463	71
		3 p. m. 9 p. m.	NE	1.8 0.3		29, 346	63		3 p. m. 9 p. m.	NE	2.3 0.6	2.99	29, 435	66
	26	3 a, m.	w.sw.	0. 6	3.08	29. 188	67	7	3 a. m. 7 a. m.	8. SE	0. 2	2. 95	29, 268	66
		9 s. m.			3. 16	29. 163	74		9 a. m.	w.sw.	0.2	2. 93	29. 230	76
		3 p. m.			3.25			}	2 p. m. 3 p. m.	NW	1.7	2.94	1	
	27	9 p. m. 3 a. m.	W.NW.	2.5 2.1	3. 23 3. 34	29. 193	63	8	9 p. m. 3 a. m.	SE	0.2	2.93 2.96	29. 230	72
		7 a. m. 9 a. m.	NW	2.8	3.28	29. 218	62		7 a.m. 9 a.m.	8W	1.9	3.09	29. 198	74
		2 p. m. 3 p. m.	NW		3. 28	29. 315	76		2 p. m. 3 p. m.		4.0	.	29.069	90
		9 p. m.	I NW	10.6	3.28	29, 295	67		9 p. m.	8. SE	1.4	3, 10	29. 155	72
	28	7 a. m.	8W	0.6		29, 326	66	9	3 a.m.	8.8W	1.4		29. 203	70
		9 a.m.	W.NW.	2.0	3. 16	29, 328	74		9 s. m. 2 p. m.	w. sw.	1.5	3. 09	29, 190	80
		3 p. m. 9 p. m.	W.SW.	1.9 0.6	3. 11 3. 11	29. 331	64	į	3 p. m. 9 p. m.	W.NW. W.SW.	21	3.09 2.98	29. 262	74
	2 9	3 a. m.	8. sw	0.5	3.08			10	3 a.m.	West	0.2 0.3	2.99		
		7 a. m. 9 a. m.	8E	1.0	3. 07	29. 269	68		7 a. m. 9 a. m.	SE	0.9	2. 93	29. 293	72
		9 p. m. 3 p. m.	8. SE	1.3	3. 07	29. 235	75		2 p. m. 3 p. m.	8. SE	1.6	2.92	29. 232	87
	30	9 p. m. 3 a. m.	SE	0.2	3, 07 3, 09	99. 290	67	111	9 p. m. 3 a. m.	8. SE SW	1.3 4.2	2.78 2.91	29. 145	73
	-	7 a. m.	E. 8E	0.5		29. 298	68		7 a. m.	W.NW.	5.0	2.95	29.063	70
		2 p. m.				29. 318	77		9 a. m. 2 p. m.				29. 275	77
		3 p. m. 9 p. m.	SE	1. 2 0. 5	2.98	29. 363	69	ľ	3 p. m. 9 p. m.	W.NW.	3.2	2.98 2.99	99. 366	69
	31	3 a.m.	8. 8W	1.1	2.98	29, 173	70	12	3 a.m.	Calm	0.0	3. 13	29. 538	63
		9 a.m. 2 p.m.	W. 8W.	1. 9	3.08	29, 169	87		9 a. m.	8E	0.8	3. 09	29. 525	74
	-	3 p. m.	W.NW	1.9	3. 01			ļ	3 p. m.	8. SE	1.7	3. 12		68
Aug.	1	9 p. m.	8E	0. 2 3. 3	3.05 3.00	29. 187	74	13	9 p. m. 3 a. m.	Calm	0.0	2. 92 2. 93	29. 495	
		7 a. m. 9 a. m.	w.sw.	1.8	3. 11	29. 265	76	•	7 a.m. 9 a.m.	sw	1.8	2.93	29. 43 1	70
		2 p. m. 3 p. m.	. 	2.6	2.99	29. 281	89		2 p. m. 3 p. m.	8.8W	3.0	2.94	90. 399	86
	2	9 p. m.	8W	2.1 1.9		29. 247	82		9 p. m.	w.sw	0.9	2.89	29. 369	75
	*	3 a. m. 7 a. m.	8W		•••••	29. 317	80	14	3 a.m. 7 a.m.]	· · · · · · ·	29. 370	72
		9 a.m. 2 p. m.	w.sw.	2.8		29. 374	86		9 a. m. 2 p. m.	West	2.1	· · · · · ·	29. 397	82
		3 p. m. 9 p. m.	NW	9.3 0.1	3. 34	29, 480	71		3 p. m. 9 p. m.	8. SE NE	1.8 1.9	· • • • • •	29. 377	77
	3	3 a. m.	N. NE.	0.2	3. 17	29, 608	69	15	3 a. m.	NE	1.4	3. 02	29. 480	66
		9 a. m.	E.NE	1.4	3. 19	1			7 a. m. 9 a. m.	E. 8E	1.2	2.92		
		2 p. m. 3 p. m.	E.NE	1.3	3. 09	29. 585	76		2 p. m. 3 p. m.	8E	0.8		29. 413	75
	4	9 p. m. 3 a. m.	E. NE	0.3	3. 11 3. 01	29. 538	68	16	9 p. m. 3 a. m.	8W	0. 4 0. 5	3.08 3.19	29. 388	72
	•	7 a. m.	8E		3. 09	29. 443	71	1	7 a. m. 9 a. m.	North	0.4	3.18	29. 398	74
		2 p. m.				29. 317	90	1	2 p. m.		l		29. 360	80
		3 p. m. 9 p. m.	8W 8. 8W	2.1 0.9	3. 02 3. 01	29. 254	83	1	3 p. m. 9 p. m.	NE	1.7 3.3	3. 20 3. 20	29, 440	67

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WINI	0.	er be-	reduced	bed.			WIND),	er be	duced
Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer re to 32°.	Therm'r detuched.	Date,	Hour of day.	Сопти.	Refutive ve-	Height of water low 1838.	Barometer reduced to 35°.
1863. aug. 17	3 a. m. 7 a. m. 9 a. m. 2 p. m.	NE	1.8	3. 2 5 3. 17	29, 538 29, 565	67	1863. Aug. 29	3 a. m. 7 a. m. 9 a. m. 2 p. m.	NW	2.2	3. 13 3. 11	. 29. 300 29. 501
18	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	E. NE 8E 8.8W	1.4 0.7 0.8	3. 24 3. 19 3. 27 3. 16	29. 608 29. 583	68 71	30	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	NW Calm Calm W. 8W.	3.7 0.0 0.0		29. 579 29. 691
19	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	SE S.SW Calm W.NW.	1.8 0.4 0.0	3. 14 3. 03 3. 05 3. 07	29. 523 29. 438 29. 473	72 73	31	2 p.m. 3 p.m. 9 p.m. 3 a.m. 7 a.m.	8. SE Calm SW	2.5 0.0 0.4	2.96	29. 646 29. 626 29. 604
20	9 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	8. SE N. NE N. NE	1. 9 1. 2 1. 5	3. 14 3. 09 3. 21 3. 25	29. 407 29. 395 29. 415	85 69 70	Sept. 1	9 p. m. 9 p. m. 9 p. m. 3 a. m. 7 a. m.	8. SE 8. SW SW	2.8 0.7 0.5	2.97	29. 536 29. 526 29. 500
2 l	2 p.m. 3 p.m. 9 p.m. 3 a.m. 7 a.m. 9 a.m.	East Calm Calm W. SW.	0.7 0.0 0.0	3. 24 3. 15 3. 14 3. 10	29. 426 29. 363 29. 381	74 66 68	2	2 p.m. 3 p.m. 9 p.m. 3 a.m. 7 a.m. 9 a.m.	8. 8E 8. 8E N. NE	1.6 0.2 4.3	3. 45 3. 68	29. 408 29. 363 29. 499
22	9 p.m. 3 p.m. 9 p.m. 3 a.m. 7 a.m. 9 a.m.	SW SW SW	2.4 0.8 1.4	3.06 3.07 3.00	29. 232 29. 232 29. 238	85 72 68	3	2 p.m. 3 p.m. 9 p.m. 3 a.m. 7 a.m. 9 a.m.	NE NE NE	6.5 4.5 2.6		29, 573 29, 601 29, 649
23	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	NE E. NE SE	2.4 0.5 1.4	3. 16 3. 16 3. 19	29. 333 29. 396 29. 376	8 8 8	` 4	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	ne Ne S. se	3. 1 0. 4 0. 1	3. 67 3. 57 3. 56	29. 576 29. 516
94	9 p.m. 3 p.m. 9 p.m. 3 a.m. 7 a.m.	E. NE SW NW	2.8 1.8 4.0	3. 33 3. 18 3. 28	29, 221 28, 975 29, 241	61 68 53	5	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	8. SE 8. SE W. NW.	3.0 0.7 1.2	3. 81 3. 78	29, 366 29, 253 29, 268
25	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	N. NW NW NW N. NW.	9.4 0.7 0.9	3. 53 3. 36 3. 24 3. 16	29. 569 29. 655	60 59 49	6	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	W.NW. N. NW. N. NW. NE	2.9 0.9 2.5	3. 67 3. 75 3. 69 3. 73	29, 435 29, 566
26	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	8E 8W W.8W.	1.5 0.1 1.2	3. 94 3. 90 3. 19 3. 10	29. 629 29. 596 29. 560	62 53 55	7	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.		1.6 0.6 0.5	3. 70 3. 69 3. 72 3. 76	29, 578 29, 558 29, 544
27	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	West SW W.SW.	3. 4 0. 8 1. 5	3. 14 3. 19 3. 14	29. 446 29. 423 29. 391	74 62 61	.8	7 a.m.	8. SE SE N. NW	1.7 0.1 0.2	3. 74 3. 64 3. 75 3. 68	29, 306 6 29, 296 6 29, 488 6
28	9 a. m. 2 p. m.	W. SW. S. SE S. SW. W. NW.	22	3 16	29. 258 29. 108 28. 971	69 57 58	9	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	NE NE E. NE	4.7 2.7 2.2	3.64 3.60	29. 636 5 29. 707 5 29. 745 55
	9 a. m. 2 p. m. 3 p. m.	W. NW. NW N. NW.	4. 2 3. 9	3. 24	29. 094	58		9 a. m. 2 p. m.	E. NE Calm		3.66	99.704 58

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WINI).	ğ	luoed	hed.			WINI), .	r pe-	T Cod	1
Date.	Hour of day.	Course.	Relative ve-	Height of water low 1638.	Barometer reduced to 330.	Therm'r detached	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer reduced to 320.	The state of the s
1863.	3 a. m.	Calm	0.0	3.70			1863. Sept. 22	3 a. m.	8E	0.3	3. 52		
Sept. 10	7 a.m.				29, 612	55	Sept. 22	7 a. m.				29. 828	1
	9 a. m. 2 p. m.	s. sw	1.7	3. 69	29. 508	63		9 a.m. 2 p.m.	8. SE	2.9	3. 49	29. 759	5
	3 p. m. 9 p. m.	S. SE S. SW	1.6 2.5	3.61	29. 348	65		3 p. m. 9 p. m.	8. SE 8. SE	3. 1 2. 7	3. 44 3. 47	29. 576	1.
11	3 a.m.	w.sw.	2.9	3.63	29. 306	69	23	3 a.m.	8. 8W	3.1	3. 3 0	29. 456	1
	9 a.m. 2 p.m.	W.NW.	2.9	3. 63	29. 368	76		9 a.m.	8W	3.8	3. 34		١.,
	3 p. m.	W.NW.	2.5 1.7	3. 59				3 p. m.	w.sw.	2.8	3, 42	29. 358	1.7
12	9 p. m. 3 a. m.	N.NW	3.9	3. 63 3. 64	29. 425	66	94	9 p. m. 3 a. m.	N. NE N. NE	3.8 3.8	3. 66 3. 58	29. 478	5
	7 a.m. 9 a.m.	NE	4.3	3. 74	29. 496	57		7 a. m. 9 a. m.	N. NE	3.8	3.74	29.652	4
	2 p. m. 3 p. m.	NE	2.3	3. 73	29. 486	61		2 p. m. 3 p. m.	N. NE	4.0	3. 74	29. 679	4
13	9 p. m. 3 a. m.	Calm	0.0	3. 71	29. 443	53	25	9 p. m. 3 a. m.	N. NE E. NE	3.7 2.4	3. 75 3. 70	29. 729	4
20	7 a.m. 9 a.m.				29. 429	52	~	7 a. m. 9 a. m.		2.2		29.730	4
	2 p. m.	8W	1.8	3. 67	29. 341	73		2 p. m.	E. NE		3.72	29. 699	5
	3 p. m. 9 p. m.	8W	2.9 0.7	3. 53 3. 56	29. 328	64		3 p. m. 9 p. m.	E. NE	1.7 0.1	3. 57 3. 56	29. 636	14
14	3 a.m.	s. sw	0.1	3. 52	29. 378	61	26	3 a.m. 7 a.m.	Calm	0.0	3, 48	29, 582	1.4
	9 a.m. 2 p.m.	s. sw	2.5	3. 48	29. 315	82		9 a.m. 2 p.m.	8.8W	2. 1	3. 50	29. 478	
	3 p. m. 9 p. m.	8.8W 8W	4.0 2.5	3. 50 3. 44	29. 343	71		3 p. m. 9 p. m.	8. 8E 8. 8E	3. 2 1. 4	3. 43 3. 47	29. 351	١.,
15	3 a.m.	sw	23	3. 42			27	3 a. m.	8. 8E	1.3	3. 45		
	7 a.m.	sw	2.5	3. 41	29. 393	67		7 a. m. 9 a. m.	8W	2. i	3. 47	29. 306	
	2 p. m. 3 p. m.	SE	2.0	3, 43	29. 339	86		2 p. m. 3 p. m.	8w	2. i	3. 50	29. 225	
16	9 p. m. 3 a. m.	8. 8W 8. 8E	1. 2 0. 2	2.36 3.46	29. 342	71	28	9 p. m. 3 a. m.	w.sw.	1. 5 1. 1	3. 49 3. 51	29. 265	9
	7 a.m. 9 a.m.	South	1.3	3. 49	29. 318	66		7 a.m. 9 a.m.	w.sw.	0.7	3. 52	29, 383	1
	2 p. m.				29. 207	85		2 p. m. 3 p. m.	SE			29. 379	
	9 p. m.	8.8W	3.0 3.0	3. 57 3. 44	29. 140	73		9 p. m.	8.8W	0. 7 0. 2	3. 44 3. 34	29. 375	1
17	3 a.m. 7 a.m.	8. SW	2.2	3. 57	28. 995	69	29	3 a.m. 7 a.m.	w.sw	0, 1	3. 38	29. 441	1
	9 a.m. 2 p.m.	W.NW.	2.9	3. 47	29. 095	67		9 a.m. 2 p.m.	8E	0.4	3, 41	29, 398	-:
	3 p. m. 9 p. m.	W.NW. N. NE	3.6 3.5	3. 61 3. 60	29. 278	47		3 p. m. 9 p. m.	8. SE Calm	1. 5 0. 0	3. 47 3. 46	29. 415	-
18	3 a.m. 7 a.m.	N. NW.	3.6	3. 45	29. 462	38	30	3 a. m. 7 a. m.	Calm	0. 0	3. 53	29. 383	١.,
	9 a.m.	N. NW	3. 5	3. 54				9 a.m.	8E	0. 7	3. 55		
•	2 p. m. 3 p. m.	N. NW	2.6		29. 476	43		2 p. m. 3 p. m.	8. SE	1.5	3. 63	29, 266	
19	9 p. m. 3 a. m.	N. NW . North	1.3 1.6	3. 65 3. 60	29. 494	40	Oct. 1	9 p. m. 3 a. m.	West W. 8W.	2.0 1.6	3. 49 3. 41	29. 246	
	7 a.m. 9 a.m.	NW	1.9	3. 65	29. 509	34		7 a.m. 9 a.m.	8. 8E	1.8	3.41	29. 301	1
	2 p. m. 3 p. m.	w.sw.	i. 8	3. 66	29. 413	48		2 p. m. 3 p. m.	8. SE	2.9	3, 40	29. 196	ľ
- 20	9 p. m. 3 a. m.	8. SW	1.0	3, 48	29. 381	47	2	9 p. m. 3 a. m.	W.NW.		3. 38 3. 40	29. 164	1
20	7 a. m.]			29. 324	50	 	7 a.m.				29. 207	1
	9 a.m. 2 p. m.	8W		3. 35	29. 268	67		9 a.m. 2 p.m.	W.NW.	۱ ا	3. 41	29. 279	ŀ
	3 p. m. 9 p. m.	NE	3. 4 1. 4	3. 36 3. 55	29. 328	58	1	3 p. m. 9 p. m.	West Calm	I O. O	3. 39 3. 32	29. 312	ŀ
21	3 a. m. 7 a. m.	N. NE	1.4	3. 61	29. 534	51	3		8W	0.6	3.41	29. 147	ŀ
	9 a. m.	NE	2.6	3. 67				9 a. m.	8E	2.8	3, 57		ŀ
	3 p. m.	E. NE	2.0	3.65	00.744		1	3 p. m.	8.8W West	2.5	3.64		١.
		E. NE V			29,744	51	i	ap.m.	West	5.3	3. 56	28. 976	1 4

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND		r be-	need	.peq.			WIND	.	r be	peon	
Date.	Hour of day.	Course.	Relative ve-	Height of water be- low 1838.	Barometer reduced to 320.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer reduced to 32°.	my desirable
1863.		W NW		0.55			1863.		a -1				Ī
Oct. 4	3 a. m.	W.NW.	4.2		29. 084	38	Oct. 16	3 a. m.	Calm	0.0		29. 141	,'. !
	9 s. m. 2 p. m.	W.NW.	4.0	. .	29. 211	50		9 a. m. 2 p. m.	Calm	0.0		29.00 1	1
_	3 p. m. 9 p. m.	W.NW.	3.6 2.4	3. 55	29. 344	41		3 p. m. 9 p. m.	E. NE	0.4	3.73	28.970	
5	3 a. m. 7 a. m.	W.NW.	2.1		29. 450	39	17	3 a. m. 7 a. m.	E. SE	0. 1	. 	28.905	
	9 a. m. 2 p. m.	W.NW.	2.3	3.42	29. 469	45		9 a. m. 2 p. m.	8. SE	2.5		1	٠.
	3 p. m. 9 p. m.	W.NW. West	20		29. 494	37		3 p. m. 9 p. m.	8.8W 8.8W	2. 1 3. 3		98. 738 98. 878	٠'.
6	3 a. m. 7 a. m.	w.sw.	0.1		29. 392	35	18	3 a. m. 7 a. m.	8.8W	4. 8		28, 823	٠.
	9 a. m. 2 p. m.	W. 8W.	0. 6		29. 331	51		9 a. m. 2 p. m.	8W	5. 8	3. 25	28. 978	٠.'.
	3 p. m. 9 p. m.	SE	1.3 1.0	3.76 3.84	29. 382	48		3 p. m.	sw	5. 1 2. 2	2.98 3.09	29. 161	٠.
7	3 a. m.	NE	2.1				19	3 a. m.	8. 8W	0. 9		l	
	9 a. m.	NE	2.4	3.90	29.417	40		7 a. m. 9 a. m.	8. SW	1.6	3. 18	29. 190	
	3 p. m.	NE	1.3	3.78	29. 417	49	}	2 p. m. 3 p. m.	8. SE	3.8		29.081	٠.,
8	9 p. m. 3 a. m.	8.8W	0. 1 1. 3		29. 403	43	20	9 p. m. 3 a. m.	South S.SW	4. 1 3. 0	3. 23 3. 16	29. 071	
	7 a. m. 9 a. m.	8.8W	2.0	3. 66	29. 264	45		7 a. m. 9 a. m.	w.sw.	2.9	3.43	29.034	1
	2 p. m. 3 p. m.	N. NW.	2.0	3. 65	29. 153	58	1	2 p. m. 3 p. m.	. 	3. 0	1	29, 201	i '
9	9 p. m. 3 a. m.	N. NW . North	1. 1 3. 0	3.79	29, 279	44	21	9 p. m. 3 a. m.	W.SW. W.SW.	1. 1		29.384	ĺ,
·	7 a. m. 9 a. m.	N. NE	4.7	4.06	29. 454	44		7 a. m. 9 a. m.				29. 59	1
	2 p. m. 3 p. m.	NE	3.8		29. 552	48	ļ	2 p. m.	W.NW.		3.20	29.70	i
10	9 p. m.	NE	3.9	3.98	29. 638	46		9 p. m.	W.NW.	1.8	3. 26	29. 831	ì
10	3 a. m. 7 a. m.	N. NW .	1.8		29. 745	34	22	3 a. m. 7 a. m.	NW	0.9		29. 871	ï
	9 a. m. 2 p. m.	N. NW	2.3		29.725	47		9 s. m. 2 p. m.	NW	1.0	3, 53	29. 76	; ;
	3 p. m. 9 p. m.	NE E. NE	1.6 0.1	3.76	29. 721	37		3 p. m. 9 p. m.	N. NW . Calm	1.0 Q.0		29, 754	
11	3 a. m. 7 a. m.	Calm	0.0	3. 73	29.714	32	23	3 a. m. 7 a. m.	W.NW.	0.8		29. 40	
	9 a. m. 2 p. m.	8.8W	1.5	3. 79	29. 601	5i		9 a. m. 2 p. m.	w.nw.	2.7		29. 415	
	3 p. m. 9 p. m.	S. SE SE	2.8 1.9	3.77 3.80	29. 496	48		3 p. m.	W.NW	3.7			
12	3 a. m. 7 a. m.	8. SE		3. 83			24	3 a. m.	N.NW	21		29.621	
	9 a. m.	8E	3.8	3. 90	29. 372	49		7 a. m.	North	2.6	3.63	29, 75	
	3 p. m.	E.NE	3.2	3.92	29, 356	52		2 p. m. 3 p. m.	N. NE	2.9	3. 58	29.817	
13	9 p. m. 3 a. m.	E. NE E.SE	2.5 2.3	3. 87 3. 86	29. 378	52	25	9 p. m. 3 a. m.	North N. NW .	1.7	3, 58 3, 54	29, 899	
	7 a. m. 9 a. m.	E. 8E	1.6	3.79	29. 412	49		7 a. m. 9 a. m.	N. NE	1.9	3.61	29, 967	
	2 p. m. 3 p. m.	E. NE	1.6	3.77	29. 369	58	1	2 p. m. 3 p. m.	NE	1. o	. .	29, 924	
14	9 p. m. 3 a. m.	NE	0.3	3.67	29. 371	49	26	9 p. m. 3 a. m.	Calm NE	0.0	3.56 3.57	29, 919)
- -	7 a. m. 9 a. m.	N. NW .	- -		29. 374	43	-5	7 a. m. 9 a. m.	NE	0. 1		29, 910	•
	2 p. m. 3 p. m.	SE		3. 67	29. 326	58	1	2 p. m.				29. 874	
15	9 p. m.	Calm	0.0	3. 53	29. 353	51	0~	3 p. m. 9 p. m.	E. NE		3.57	29. 845	
ii.	3 a. m.	Calm		3.59	29. 371	44	27	3 a. m. 7 a. m.	NE		3. 56	29.824	4
	9 a. m. 2 p. m.				29. 264	58		9 a. m. 2 p. m.	N. NE		3. 58	29. 767	
	3 p. m.	SE	1.9	3.67	29, 226	52	1	3 n. m.	SE	1 1 3	3.56	99 729	

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WIND		r pe-	laced	peq.			WIND		er be-	reduced.	Peg
Date,	Hour of day.	Course.	Relative ve-	Height of water low 1838.	Barometer reduced to 32°,	Therm'r detached	Dates	Honr of day.	Course.	Relative ve-	Height of water low 1836.	Barometer red to 320.	Therm'r detached.
1863. Oct. 28	3 a. m. 7 a. m. 9 a. m.	Calm	0.0	3. 5 0	29. 663	37	1863. Nov. 9	3 s. m. 7 s. m. 9 s. m.	NW	2.2 	3. 94 3. 79	29. 771	26
29	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	SE S.SE S.SE	3.2 2.3 0.9	3. 44 3. 35 3. 39	29, 557 29, 509 29, 360	46 46	10	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	N.NW NW NW	3.1 0.9 0.5	3. 85 3. 79 3. 87	29. 820 29. 877 29. 728	26 23
30	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	South 8.8W W.NW .	2.6 2.4 2.4	3. 56 3. 52 3. 72	29, 115 28, 967 29, 111	46 53 36	` 11	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	88W 8W 8W	4. 1 4. 2 3. 8	4. 30 4. 20 4. 34	29. 564 29. 357 29. 117	36 37 41
31	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	N.NE	2.0 2.1 2.9 1.9	3. 69 3. 79 3. 57 3. 58	29, 126 29, 163 29, 502	38 34 30	12	9 s. m. 2 p. m. 3 p. m. 9 p. m. 3 s. m. 7 s. m.	8W 8W 8W	2. 8 1. 2 0. 6	4. 41 4. 27 4. 12 3. 95	29. 088 29. 115 29. 096	59 48 45
Nov. 1	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	NW Calm	1. 1 0. 6 0. 0 0. 0	3. 44 3. 35 3. 48	29. 654 29. 732 29. 747	36 31 33	13	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	8.8W 8.8W 8.8W	3.6 2.0 1.0 0.3	3. 93 3. 84 3. 77 3. 68	28. 995 29. 052 29. 136	56 49 54
2	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	E.SE SE S.SE	1.8 2.7 3.6 2.6	3. 59 3. 68 3. 76 3. 65	29, 664 29, 508 29, 240	41	14	9 s. m. 2 p. m. 3 p. m. 9 p. m. 3 s. m. 7 s. m.	N.NE N.NE N.NW	1.6 3.6 5.7	3. 60 3. 67 3. 35	29. 209 29. 045 29. 028	4
3	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	W.SW. West	3.3 4.4 3.8 2.0	3. 64	29. 161 29. 342 29. 560	56 41 33	15	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	N.NW N.NW N.NW	3.0 2.7 1.6	3. 45 3. 47 3. 47	29. 130 29. 244 29. 330	30
4	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	W.NW . W.SW . S.SW S.SW	3.0 2.3 0.4 1.1		29. 566 29. 465	48 39	16	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	N.NW N.NW N.NW	2.4 2.7 1.5 2.0	3. 49 3. 52 3. 59	29. 381 29. 429 29. 455	3
5	7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	8.8W 8.SE E.NE W.SW .	1. 7 1. 9 1. 6 4. 1	4. 38 4. 16 3. 93 3. 97	29. 256 28. 957	46	17	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	N.NE N.NE N.NE	1. 5 1. 2 0. 1 0. 1	3. 66 3. 65 3. 63 3. 61	29. 391 29. 372	3
. 6	7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	W.NW. W.SW. West	3.3 2.4 3.8 2.7	3. 89 4. 05 4. 14 4. 19	29. 252 29. 256	45 55 45	18	7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	8W 8.8W 8.SE	0. 5 0. 6 0. 7 1 5	3. 66 3. 67 3. 73 3. 79	29. 337 29. 254 29. 186	4
	7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	W.8W 8.SW 8.SW	1.8 2.1	4. 21 4. 24 4. 15	29, 402 29, 386 29, 098	34 48 44		7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	8.8W W.8W .	2. 0 1. 1 0. 6	3. 91 3. 92 3. 91 3. 89	28. 933 28. 843 28. 920	3: 5: 4'
7	3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	W.NW. W.NW. North.	3.5 4.0 3.9 1.9	4. 13 3. 95 3. 76 3. 71	29, 107 29, 216 29, 518	45 51 40	19	3 a. m. 7 a. m. 9 н. m. 2 p. m. 3 p. m. 9 p. m.	W.NW.	1. 4 2. 6 3. 4 3. 6	3. 89 3. 89 3. 87 3. 90	28. 918 29. 052 29. 176	3
8	3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m.	NW	3. 2 3. 3 2. 7	3. 89 3. 87 4. 01	29. 681 29. 701	32	20	3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m.	West 8W	2.4 2.1 3.0 2.6	4. 03 4. 13 4. 31	29. 322 29. 219	3

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND		r be-	luced	bed.			WIND	,	ride-	luced	hed.
Date.	Hour of day.	Course.	Relative ve-	Height of water be- low 1838.	Barometer reduced to 32°,	Therm'r detached.	Date,	Hour of day.	Course.	Relative ve-	Height of water low zero of the	Barometer reduced to 63°.	Therm'r detached.
1863.		-7					1863.						1
Nov. 21	3 a. m.	8W	2.1	4. 19	29. 212	35	Dec. 3	3 a. m. 7 a. m.	S.SE	2.6		29, 450	3
	9 a. m.	S.SW	1.5	4.01				9 a. m.	8.8W	1.9			
	2 p. m. 3 p. m.	W.NW	3 0	4.02	29. 216	37		2 p. m. 3 p. m.	8.SW	2.8		29, 261	١.
	9 p. m.	W.NW.	3.2	3. 95	29, 427	32		9 p. m.	W.SW.	3.2		29, 226	
22	3 a. m. 7 a. m.	w.sw	1.3	4.07	29. 705	26	4	3 a. m.	W.SW	3. 5		29, 370	
	9 a. m.	West	1.7	4.04				9 a. m.	W.NW	2.9			
	2 p. m. 3 p. m.	w.sw.	1.2	4.21	29, 780	36		2 p. m. 3 p. m.	w.sw.	2.4		29, 481	
	9 p. m.	Calm	0.0	4.07	29. 849	34	1	9 p. m.	NW	1.2		29, 518	
23	3 a. m.	N.NE	2.0	3. 97	29, 859	33	5	3 a. m.	N.NE	3. 1		29, 720	
	9 a. m.	NE	2.3	3, 85				9 a. m.	North	2.5			
	2 p. m. 3 p. m.	NE	3.7	3. 73	29, 712	36		2 p. m. 3 p. m.	NE	1.2		29, 779	
	9 p. m.	E.NE	3.5	3, 53	29: 531	36		9 p. m.	NE	0.1		29, 854	
24	3 a.m.	E.NE	3.0	3. 51	29. 088	35	6	3 a. m. 7 a. m.	Calm	0.0		29, 852	
	9 a. m.	E.NE	2.4	3. 47				9 a. m.	8.8W	2.0			
	2 p. m. 3 p. m.	NW	3. 5	3, 59	29, 098	31		2 p. m. 3 p. m.	S.SE	2.4	*****	29, 792	ľ
	9 p. m.	W.SW .	2.7	3, 52	29, 328	26		9 p. m.	8.8W	1.4		29, 760	ĺ
25	3 a. m.	W.SW	1.4	3. 72	29. 360	21	7	3 a. m. 7 a. m.	S.SE	0.7		29, 734	
	9 a. m.	E.NE	1.3	3.91				9 a. m.	8.SE	1.7			٠,
	2 p. m. 3 p. m.	w.sw.	0 0	4.02	29, 271	33		2 p. m. 3 p. m.	S.SE 1	0.9		29. 612	
	9 p. m.	W.8W	2.2 2.9 1.2	4. 07	29. 358	24		9 p. m.	S.SE	0, 1		29.619	
26	3 a. m.	S.SW	1.2	4.07	29. 341	31	8	3 a. m. 7 a. m.	Calm	0.0		29, 578	
	7 a. m. 9 a. m.	8.SW	2.8	4. 28		31		9 a. m.	W.SW	0.7			١.
	2 p. m. 3 p. m.	s.sw	2.0	4. 05	29, 266	41		2 p. m. 3 p. m.	w.sw.	1.3		29. 544	
	9 p. m.	S.SW	2.0	4. 12	29, 306	38		9 p. m.	W.NW.	1.1		29, 591	
27	3 a. m. 7 a. m.	8W	1.1	3. 93	29, 260	32	9	3 a. m. 7 a. m.	N.NW	0.6	******	29, 685	
	9 a. m.	sw	1.9	3.93				9 a. m.	N. NW	0.6			
	2 p. m.	w.sw.	1.3	3. 80	29. 141	42		2 p. m. 3 p. m.	NE	9 4		29,723	٠
	3 p. m. 9 p. m.	W.NW	2,9	3.82	29, 196	31		9 p. m.	NE	2.4 1.4		29, 727	
28	3 a. m.	W.NW	3.0	3, 72	29.230	95	10	3 a. m.	SE	2. 1		29, 680	
	7 a. m. 9 a. m.	W.NW	3. 1	3. 78		25		9 a. m.	SE	2.8			٠,٠
	2 p. m.	N.NW	3.5	3, 82	29. 318	23		2 p. m. 3 p. m.	S.SE	2.8		29, 541	1
	3 p. m. 9 p. m.	W.NW	2.6	3.93	29. 456	19		9 p. m.	S.SE	2.9		29, 474	
29	3 a. m.	W.NW.	2, 9	3. 92	29, 535	7	11	3 a. m. 7 a. m.	8.SE	1.9		29, 282	
	9 a. m.	W.NW	3.3	4. 03	20.000			9 a. m.	SW	0.7			
	2 p. m.	W.NW	2 0	3. 99	29. 542	11		2 p. m. 3 p. m.	w.sw .	1.4	*****	29, 140	
	3 p. m. 9 p. m.	West	2.0	3. 99	29. 649	9	- 75	9 p. m.	W.SW .	0.4	******	29. 274	
30		w.sw	1.8		29, 720	9	12	3 a. m.	Calm	0.0		99, 253	
	7 a. m. 9 a. m.	West	2.2					9 a. m.	Calm	0.0		Acres .	۲.
	2 p. m.	w.sw.	1.8		29, 707	21		2 p. m. 3 p. m.	N. NE	1.8	*****	29. 166	
	3 p. m. 9 p. m.	SW	1.8		29, 646	25		9 p. m.	N. NE	4 3		29, 181	
Dec. 1	3 a. m.	w.sw .	2.1		29, 500	27	13	3 a. m. 7 a. m.	N.NE	5. 1	*****	29, 119	Ġ
	7 a. m. 9 a. m.	w.sw.	3.7				8.,	9 a. m.	N. NE	6.3			
	2 p. m.				29. 402	43		2 p. m. 3 p. m.	N. NE	7.9	*****	29.023	
	3 p. m. 9 p. m.	West	3, 6	*****	29. 337	43		9 p. m.	N. NE	7.7		28, 915	
2	3 a. m.	West	4. 0				14	3 a, m.	North	5.3		28, 919	9
	7 a. m. 9 a. m.	W.NW	2.7		29, 460	32		7 a, m. 9 a, m.	NW	3.6		******	٠.
	2 p. m.				29, 571	42		2 p. m.	NW	3.3	*****	20,093	2
	3 p. m. 9 p. m.	NW	0, 1		29. 626	32		3 p. m.	NW	1.4		29, 296	3

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WINI),	whov	duced	ped.			WINI).	abov suge.	reduced o.	5
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above	Barometer reduced to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer rec to 32°.	Therm'r detached
1863. Dec. 15	3 a.m. 7 a.m. 9 a.m.	NW	0.9		29. 555	10	1863. Dec. 27	3 a. m. 7 a. m. 9 a. m.	E. NE	3.5		29. 510	3
	2 p. m. 3 p. m. 9 p. m.	NW	0.5 0.6		29. 657 29. 725	7	,	2 p. m. 3 p. m. 9 p. m.	E. NE	3.5 3.5 4.7	•••••	29, 369 29, 134	30
16	3 a.m. 7 a.m. 9 a.m. 2 p. m.	E. NE	3.7		29. 582	26 29	28	3 a. m. 7 a. m. 9 a. m. 2 p. m.	E. SE	2.5	•••••	28. 698 28. 607	3
17	3 p. m. 9 p. m. 3 a. m. 7 a. m.	E. NE E. NE	5.3 6.3 5.3		29. 314 28. 961	29 31	29	3 p. m. 9 p. m. 3 a. m. 7 a. m.	West W. SW.	1. 2 4. 5 3. 7		28. 735 28. 966	30
••	9 a. m. 2 p. m. 3 p. m. 9 p. m.	8. 8W 8. 8W	4.0 2.7 0.7		28. 858 28. 898	30	-	9 s. m. 2 p. m. 3 p. m. 9 p. m.	West	3.8 4.0 2.1		29. 000 29. 314	2
18	3 a. m. 7 a. m. 9 a. m. 2 p. m.	NW	1.6		28, 956 29, 174	16	30	3 a. m. 7 a. m. 9 a. m. 2 p. m.	w.sw	0.6		29. 606 29. 647	2
19	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	W. NW. W. NW.	2.0 2.2 2.2 2.1		29. 406 29. 583	24 5	31	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	W. SW. W. NW. N. NW.	0. 4 0. 5 1. 9	•••••	29. 630 29. 380	1
*20	2 p. m. 3 p. m. 9 p. m. 3 a. m.	W. AW.		•••••	29. 642 29. 643	13	1864. Jan. 1	2 p. m. 3 p. m. 9 p. m. 3 a. m.	N. NW NW	5. 5 6. 3		29. 056 28. 990	1
	7 a. m. 9 a. m. 2 p. m. 3 p. m.			•••••	29. 604 29. 550	3 18		7 a. m. 9 a. m. 2 p. m. 3 p. m.			•••••	29. 261 29. 361	2
21	9 p. m. 3 a. m. 7 a. m. 9 a. m.			•••••	29. 450 29. 266	17 22	2	9 p. m. 3 a. m. 7 a. m. 9 a. m.	West West	3. 1 3. 0 2. 8		29. 526 29. 619	2
22	2 p. m. 3 p. m. 9 p. m. 3 a. m.	NW N. NW. N. NW.	1. 2 0. 7 0. 5		29. 163	22	3	2 p. m. 3 p. m. 9 p. m. 3 a. m.	W. 8W W. 8W W. 8W	1. 3 1. 3 1. 1		29. 669 29. 636	1
	7 a. m. 9 a. m. 2 p. m. 3 p. m.	N. NW . North	0.9		29. 423 29. 467	32		7 a. m. 9 a. m. 2 p. m. 3 p. m.	w. 8w	1.8		29. 601 29. 525	
23	9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NW . N. NW . E. NE .	1. 1 1. 1 0. 7		29. 538 29. 607	30	4	9 p. m. 3 a. m. 7 a. m. 9 a. m.	W. NW. Calm W. NW.	0.7 0.0 0.2		29. 615 29. 673	10
24	2 p. m. 3 p. m. 9 p. m. 3 a. m.	E. SE SE S. SW	1. 3 1. 7 1. 3		29. 628 29. 649	34	5	2 p. m. 3 p. m. 9 p. m. 3 a. m.	NW NW W. NW.	0. 3 0. 9 1, 1		29, 576	
	7 a. m. 9 a. m. 2 p. m. 3 p. m.	8. 8W	1. 7 1. 1		29. 639 29. 658	25 31	,	7 a. m. 9 a. m. 2 p. m. 3 p. m.	W. NW. W. NW. W. NW.	2.0		29. 443	
23	9 p. m. 3 a. m. 7 a. m. 9 a. m.	South S. SE	0. 4 1. 0 0. 9		29. 556 29. 541	30	6	9 p. m. 3 a. m. 7 a. m. 9 a. m.	W. NW. W. NW.	0. 5 0. 1 0. 7		29. 650 29. 743	19
26	2 p. m. 3 p. m. 9 p. m. 3 a. m.	S. SE Calm South	0.9 0.0 0.2		29. 524	37	7	2 p. m. 3 p. m. 9 p. m. 3 a. m.	W. NW. Calm Calm	0. 4 0. 0 0. 0		29. 868	
İ	7 a.m. 9 a.m. 2 p.m.	E. 8E	1. 2	•••••	29. 527 29. 435	37 36		7 a.m. 9 a.m. 2 p.m.	West	0.1		29. 828 29. 800	

^{*} Anemometer out of order from December 20 to 21, inclusive.

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WIND	٠,	above uge,	need	.ped.			WINE		above	peon	j
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge,	Barometer reduced to 320,	Therm'r detached.	Date.	Hour of day.	Course,	Relative ve-	ater le-gr	Barometer reduced to SNo.	Themas datashed
1864.		W. NW.	0.5			Ш	1864.		sw	0.3			ĺ
an. 8	3 a, m. 7 a. m.	W. AW.	0.5		29. 503	10	Jan. 20	3 a. m.		J		29, 340	
	9 a. m.				29, 433	ï		9 a. m. 2 p. m.	NW	0.3		29. 455	
	2 p. m. 3 p. m.	W. NW.	1.0					3 p. m.	NW	0.5			
*9	9 p. m. 3 a, m.	W. NW.	0.8		29. 493	7	21	9 p. m. 3 a. m.	N. NW	0.7	'- -	29. 615	•
	7 a. m.				29. 425	4		7 a. m.				29.682	
	9 a. m.				29, 325	13		9 a. m. 2 p. m.	8. SW	1. 4		29, 594	٠.
	3 p. m.							3 p. m.	8. SW	2.9	,	l .	
10	9 p. m. 3 a. m.				29. 347	11	92	9 p. m. 3 a. m.	S. SW	2.4	•••••	29. 542	:
20	7 a. m.				29.372	5	-	7 a. m.	********			29, 507	1
	9 a, m. 2 p. m.				29. 369	14		9 a. m. 2 p. m.	W. SW.	0.9		29. 484	
	3 p. m.							3 p. m.	8. SW	0. 2	,		
11	9 p. m. 3 a. m.				29. 373	7	23	9 p. m. 3 a. m.	8. SW	0.1		29. 474	
	7 a. m.				29. 214	2	1	7 a. m.	sw	2.1		29. 160	١,
	9 a. m. 2 p. m.					21		9 a. m. 2 p. m.				29.073	,
	3 p. m.				29. 106	15		3 p. m. 9 p. m.	W.SW.	0.8		29. 152	
12	9 p. m. 3 a. m.						24	3 a. m.	w.sw.	1.5	\		
	7 a. m. 9 a. m.				29.003	21		7 a. m. 9 a. m.	West	1.5		29.060	
	2 p. m.				28, 877	31		2 p. m.				29.007	
	3 p. m. 9 p. m.		****		28. 925	25		3 p. m. 9 p. m.	W.SW.	1.9	 	29.007	
13	3 a. m.						25	3 a. m.	W.SW.	0.5		29. 150	•
	7 a. m. 9 a. m.				28.970	26		7 a. m. 9 a. m.	W.SW.	0.5		. 	
	2 p. m. 3 p. m.				29.003	28		2 p. m.	s. sw	2.9		28.967	
	3 p. m. 9 p. m.				29. 037	28		3 p. m. 9 p. m.	West	3.8		28.95	
14	3 a. m.				28. 985	34	26	3 a. m. 7 a. m.	W. SW.	0.7		29. 23	
	9 a. m.							9 a. m.	W.SW.	0.3	,		
	2 p. m. 3 p. m.				29, 078	30		2 p. m. 3 p. m.	8. SW	i. 7	·	29. 177	
	9 p. m.				29. 213	23		9 p. m.	West W. SW.	3.5 1.2		29.084	ŀ
15	3 a. m.				29. 356	18	27	3 a. m. 7 a. m.			·	29, 139	, .
	9 a. m.				29. 363	26		9 a. m.	W. SW.	0.1		29. 169	
	2 p. m. 3 p. m.							2 p. m. 3 p. m.	Calm	0.0			
16	9 p. m. 3 a. m.				29. 388	15	28	9 p. m. 3 a. m.	Calm	0.0		29. 296	;
20	7 a, m.				29, 303	15	~0	7 a. m.	*******			29. 236	•
	9 a. m. 2 p. m.				29, 252	33		9 a. m. 2 p. m.	North	0.7		29, 404	•
	3 p. m.							3 p. m.	N. NE	6.4		29.550	
17	9 p. m. 3 a. m.				29. 185	34	29	9 p. m. 3 a. m.	N.NE.	7.0 4.5			٠.
	7 a. m. 9 a. m.				29. 167	34		7 a. m. 9 a. m.	NE	3.5	•••••	29. 498	
	2 p. m.				29.377	32		2 p. m.	********			29, 387	
	3 p. m.				29. 542	26		3 p. m. 9 p. m.	N. NE	3.7		29, 395	
18	3 a.m.						30	3 a. m.	E. NE	2 1			٠.;
	7 a. m. 9 a. m.				29. 564	22		7 a. m. 9 a. m.	NE	0.8		29. 198	,
	2 p. m.	N N1327	3.0		29. 510	28	1	2 p. m.			•••••	29, 306	
	3 p. m. 9 p. m.	N. NW.	2.6		29, 511	19		3 p. m. 9 p. m.	Calm	0.0		29, 439	
19	3 a. m. 7 a. m.	NW	1.8		29. 424	17	31	3 a.m. 7 a.m.	Calm	0.0		29. 534	1
	9 a.m.	W. NW.	1. 2				1	9 a. m.	E.NE.	0. 1			
	2 p. m. 3 p. m.	West	0.7		29. 303	22		2 p. m. 3 p. m.	E. NE	20		29, 354	
	9 p. m.				29. 295	21	I	1 2 5	NE	177		AA 049	3

^{*} Anemometer out of order from January 9 to 18, inclusive.

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WINI) .	above uge.	luced	bed,			WINI),	above age.	luced	
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 32°.	Therm'r detached,	Date.	Hour of day.	Course.	Relative ve-	Heig'tof waterabove zero of tide-gauge.	Barometer reduced to 320.	
1864.							1864.					iri	
Feb. 1	3 a. m. 7 a. m.	W.NW.	4.8		29.040	33	Feb. 13	3 a. m. 7 a. m.	W. 8W.	9.9		29. 115	1
	9 a. m. 2 p. m.	w.sw.	4.0		29. 237	34		9 a. m. 2 p. m.	8W	1.4		28. 910	1
	3 p. m. 9 p. m.	W. 8W.	3.3		29, 291	27		3 p. m. 9 p. m.	8.8W. West	1.6		28, 935	ŀ
2	3 a. m. 7 a. m.	w.sw.	2.2		29. 165	25	14	3 a. m. 7 a. m.	W.NW.	2.7		29. 327	ŀ
	9 a. m.	West	3.0		29, 220	24		9 a.m.	NW	0.6			١.
	3 p. m.	W. NW	3. 4					2 p. m. 3 p. m.	Calm	0.0		29, 432	
3	9 p. m. 3 a. m.	W. NW.	1.4		29. 372	23	15	9 p. m. 3 a. m. 7 a. m.	S.SW.	0.0		29. 367	1.
	7 a. m. 9 a. m.	w. sw	0.4	 	29. 360	14		7 a. m. 9 a. m.	West	3.3		29, 107	ŀ
	2 p. m. 3 p. m.	8. 8W	0.5	•••••	29. 195	29		2 p. m.	West	5.4	••••	29. 055	
4	9 p. m.	S. SW.	21		28.867	33	16	9 p. m.	W.NW.	6.5		29. 133	
•	3 a. m. 7 a. m.				28. 637	35	10	9 p. m. 3 a. m. 7 a. m.	• • • • • • • • • •			29. 433	ŀ
	9 a. m. 2 p. m.	NW	2.7		28. 833	33		9 a. m. 2 p. m.	W.NW	5.7	· • • • • • • • • • • • • • • • • • • •	29. 457	ŀ
	3 p. m. 9 p. m.	W.NW.	1.9		29. 005	32		3 p. m. 9 p. m.	W.NW.	4.6 1.7		29. 549	ŀ
5	3 a. m. 7 a. m.	8. SW	0.1	•••••	28. 810	35	17	3 a. m. 7 a. m.	NW	1.2		29. 722	ŀ
	9 a. m. 2 p. m.	s. sw	0. 1		28. 782	37		9 a. m.	W.NW.	2.0			ŀ
	3 p. m.	Calm	0.0		l. 			2 p. m. 3 p. m.	West	2.5		29.762	ļ.
6	9 p. m. 3 a. m.	NE	3.0		29.017	35	18	9 p. m. 3 a. m.	W.SW.	0. 6 0. 5		29. 784	l.
	7 a. m.	N. NW	1.7		29. 275	30		7 a. m. 9 a. m.	W.NW	1.4	· • • • • • • • • • • • • • • • • • • •	29. 788	١.
	2 p. m. 3 p. m.	W.NW.	0.4		29. 312	34		2 p. m. 3 p. m.	W.NW.	1.5		29. 862	ŀ
7	9 p. m. 3 a. m.	Calm SW	0.0 2.3		29. 264	23	19	9 p. zo.	8W 8.8W	0. 1 0. 2		29. 913	ľ
•	7 a. m.				28.960	29	19	3 a. m. 7 a. m. 9 a. m.				29.766	ľ
	9 a. m. 2 p. m.	8. 8W	2.5		28. 820	37		2 p. m.	8.8W	3.3		29. 349	ŀ
	3 p. m. 9 p. m.	W.NW.	2.6 2.3	! !	28. 965	31		3 p. m. 9 p. m.	8.8W 8.8W	4.3 2.2		29, 301	ŀ
8	3 a. m. 7 a. m.	NW	0. 5		29. 113	22	20	3 a. m.	8.8W	0.8	••••	29. 253	ŀ
	9 a. m. 2 p. m.	W.NW	1.7		29. 195	22		9 a. m. 2 p. m.	8.8W	2.2		29, 080	ŀ
	3 p. m.	W. NW.	2.5					3 p. m.	8.8W	2.2			ŀ
9	9 p. m. 3 a. m.	w.sw	0.9		29, 340	16	21	9 p. m, 3 a. m.	8.8W W.NW.	1.3 0.6		28, 873	
	7 a. m. 9 a. m.	W. NW	2.0		29. 443	6		7 a. m.	8w	0. 1		29. 117	
	2 p. m. 3 p. m.	NW	i.o		29. 486	13		2 p. m. 3 p. m.	8.8W.	0.3		29. 142	l.
10	9 p. m. 3 a. m.	NW	0.2		29. 581	6	22	9 p. m. 3 a. m.	8.8W	0. 4 0. 8		29.047	ľ
20	7 a. m.	Calm	a o		29, 667	3		7 a. m.				28.972	ľ
	9 a. m. 2 p. m.				29. 596	27		2 p. m.	W.NW.	1.7	· · · · · · · ·	29.075	ľ
	3 p. m. 9 p. m.	8. SE 8. SE	1.7		29. 485	25		3 p. m. 9 p. m.	8W	1.3 0.5		29. 119	ŀ
11	3 a. m. 7 a. m.	South	1.2		29, 268	32	23	3 a. m. 7 a. m.	8W	1.2		28. 969	ŀ
	9 a. m. 2 p. m	8. 8W	1.6		29. 173	40		9 a. m. 2 p. m.	West	3. 1		28.987	ŀ
	3 p. m.	W.NW	1.6					3 p. m.	West	3.3			-
12		W.NW.	0.8		29. 340	32	24	9 p. m. 3 a. m.	W. SW.	1.6 1.0		29. 117	
	7 a. m. 9 a. m.	W. NW	1.4		29. 407	24		7 a. m.	West	1.6		89. 316	l.
	2 p. m. 3 p. m.	w. sw			29. 367	28		2 p. m. 3 p. m.	West	1.0		29. 301	ľ
	9 p. m.	w. sw	3.7		29. 244	27	il	9 p. m.	SE			29, 193	ŀ

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

			WIND).	above	reduced	ped.			WINI).	above	reduced	Dod.
Dat	e.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer re to 32°.	Therm'r detached	Date.	Hour of day.	Course.	Relative ve-	Helg't of water above zero of tide-gauge.	Barometer re to 520.	Thorn'r detached.
186- Feb.		2.0 m	West	0.2	U			1864.	2	w ww	0.3			
reb.	25	3 s. m.	West			29, 002	38	March 8	3 a. m.	N. NW.			29.215	3
		9 n. m.	West	3, 6		29, 292	36		9 a. m. 2 p. m.	N. NE	1. 2		29, 270	13
		3 p. m.	North	4.6		******			3 p. m.	NE	0.8			J.
	26	9 p. m. 3 a. m.	N. NW	0, 1		29, 519	33	9	9 p. m. 3 a. m.	Calm	0.0		29, 322	ľ
		7 a. m.				29, 587	30		7 a. m.	*******			29, 339	1
		9 a. m. 2 p. m.	E. SE	0, 8		29. 411	36		9 a. m. 2 p. m.	E.SE.	0, 5	******	29, 237	t
		3 p. m.	SE	2.2					3 p. m.	E. NE.	3.0		*******	
	27	9 p. m. 3 a. m.	S. SE	1.5		29, 336	38	10	9 p. m. 3 a. m.	NE	2.6	*****	29, 152	j.,
		7 a. m. 9 a. m.	S. SE	1.0		29. 191	38		7 a. m.	N. NE.	2,9		28, 910	E
		2 p. m.				29. 095	39		9 a. m. 2 p. m.	N. Ab.			28, 807	f
		3 p. m. 9 p. m.	W. SW.	0.9		29, 195	36		3 p. m. 9 p. m.	North N. NE	0.6		28, 807	t
	28	3 a. m.	West.	3.9				11	3 a. m.	NW				3.
		7 a. m. 9 a. m.	West	4.8		29, 407	21		7 a. m. 9 a. m.	NW	0.1		28, 697	ľ
		2 p. m.				29, 425	21		2 p. m.	*******			28, 737	Ţ
		3 p. m. 9 p. m.	West W.NW.	2.9		29.528	17		3 p. m. 9 p. m.	Calm			28, 917	
	29	3 a. m.	W.NW.	1, 1		*******		12	3 a. m.	Calm	0.0			Į.
		7 a. m.	W.NW.	0.6		29. 633	11		7 a. m. 9 a. m.	Calm	0.0		29, 102	
		2 p. m.				29, 601	24		2 p. m.				29, 160	q
	U	3 p. m. 9 p. m.	Calm	0.8		29.575	22		3 p. m. 9 p. m.	NE	0.6		29, 274	ŧ
March	1	3 a. m. 7 a. m.	s.sw	0. 2		29, 400	25	13	3 a. m.	N. NE	3. 0		29, 358	d
	- 11	9 a. m.	sw	1.2					7 a. m. 9 a. m.	N. NE	2.5			ł
	111	2 p. m. 3 p. m.	West	3.1	•••••	29, 292	37		2 p. m.	NE	1.0		29, 418	1
	13	9 p. m.	West	1.8		29, 382	32	45	9 p. m.	Calm	0,0		29, 470	ï
	2	3 a. m.	W.NW.	0.8		29. 568	23	14	3 a. m.	W.NW.	2.2	*****	29, 508	r
		9 a. m.	NW	0.6					9 a. m.	NW	3.5			d,
		2 p. m. 3 p. m.	E.SE	1.6		29. 592	31		2 p. m. 3 p. m.	NW	3.1	*****	29, 538	1
	6	9 p. m.	S. SE	0.3		29, 517	31		9 p. m.	N. NW.	0,9		29, 598	9
	3	3 a. m. 7 a. m.	South	0.5		29, 387	35	15	3 a. m.	N. NW.	2.3		29, 591	ï
		9 a. m.	South	3.0		29, 196	52		9 a. m.	N. NW.	3.1		29, 581	d
		2 p. m. 3 p. m.	South	4.3					2 p. m. 3 p. m.	NW	1.7	*****	*****	
	4	9 p. m. 3 a. m.	S.SW	0.6		29. 129	43	16	9 p. m. 3 s. m.	W.SW.	0.8		29, 550	4
	•	7 a. m.				28. 987	39	10	7 a. m.				29, 471	ı
		9 n. m. 2 p. m.	W. NW.	0.6		29.007	34		9 a. m. 2 p. m.	West	2.1		29, 345	å
		3 p. m.	North	1, 9					3 p. m.	West	2.9	*****		4
	5	9 p. m. 3 a. m.	N. NW.	1.7		29, 165	27	17	9 p. m. 3 a. m.	8W	0, 4		29, 208	I
		7 a. m.	****** **			29, 210	17	131	7 a. m.	*******	1000		29, 093	H
		9 a. m. 2 p. m.	NW	0.1		29, 160	35		9 a. m. 2 p. m.	w.sw.	2.4		98, 917	r
		3 p. m.	Calm	0.0					3 p. m.	W.SW.	2.7	****	28, 747	ß
	6	9 p. m. 3 a. m.	Calm	0.0		29, 155	30	18	9 p. m. 3 s. m.	W.NW.	4.7		*****	Įį,
		7 a. m. 9 a. m.	SE	0, 4		29, 067	33		7 a. m. 9 a. m.	W.NW.	MARK!		28, 901	1
		2 p. m.				29, 057	40		2 p. m.	********	4.7		29, 994	ľ
		3 p. m. 9 p. m.	S. SE E. NE	0.7		29. 125	35		3 p. m. 9 p. m.	W.NW.	4.3		29, 214	t
	7	3 a. m.	N. NW.	0.5				19	3 n. m.	W.NW	0.7	******		ы
		7 a. m. 9 a. m.	North	1.7		29. 157	34		7 a. m. 9 a. m.	W.NW.	Acres		99, 217	0
		2 p. m.	*******			29, 142	38		2 p. m.	*******	2004		29, 200	1
		3 p. m. 9 p. m.	N. NE.	2.8		29, 195	35		3 p. m. 9 p. m.	W.NW.			39. 222	i

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WINI		above tuge.	duced	hed.			WINI).	above	reduced.	ped.
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 32°.	Therm'r detached,	Date.	Hour of day.	Course.	Relative ve-	Heig't of waterabove zero of tide-gauge.	Barometer re to 32°.	Therm'r detached.
1864. Mar. 20	3 a. m. 7 a. m.	w.nw.	1.8	-	29. 396		1864. April 1	3 a. m. 7 a. m. 9 a. m.	N. NE		4.32	29. 145	35
	9 a. m. 2 p. m.	NW	3.3		29. 446	9		2 p. m.	N. NE	1.1	4.34	29.077	4
	3 p. m. 9 p. m.	W. NW. W. NW. W. NW.	3.0 0.4 0.7		29. 468	8		3 p. m. 9 p. m.	Calm	0.8	4. 32 4. 25	29.112	3
21	3 a. m. 7 a. m. 9 a. m.		1.3		29. 533	10	2	3 a. m. 7 a. m.	Calm	l l	4. 32	29. 092	3
	9 a.m. 2 p.m. 3 p.m.	W. NW.			29. 535	20		9 a. m. 2 p. m. 3 p. m.	E.SE.	0.4	4. 32	29. 137	4
22	9 p. m. 3 a. m.	NW	1. 2 0. 4 1. 2		29. 632	16	3	9 p. m. 3 a. m.	Calm	0.0	4. 29	29. 233	3
**	7 a. m. 9 a. m.	E. NE.	1.4		29. 664	21		7 a. m. 9 a. m.	N. NE	0.6	4. 29	29. 318	3
	2 p. m. 3 p. m.	NE	1.1		29. 641	26		2 p. m. 3 p. m.	NE	1. 9	4. 29	29. 318	4
23	9 p. m. 3 a. m.	Calm NW	0.0		29, 585	20	4	9 p. m. 3 a. m.	N. NE N. NE	1.2	4. 18 4. 16	29. 306	3
	7 a. m. 9 a. m.	E. SE	0.6		29. 578	19	_	7 a.m. 9 a.m.	North	4. i	4. 12	29. 132	3
	2 p. m. 3 p. m.	SE	2. i		29. 538	33		2 p. m. 3 p. m.	North	4.9	4.06	29.067	3
24	9 p. m. 3 a. m.	South 8. 8W	0. 1 0. 1	· · · · · ·	29. 500	29	5	9 p. m. 3 a. m.	N. NW . N. NW .	3. 2 1. 7	4. 10 4. 10	29. 107	3
	7 a. m. 9 a. m.	s. sw	i. 8		29. 472	38		7 a. m. 9 a. m.	N. NW .	1.9	4. 13	29. 197	3
	2 p. m. 3 p. m.	8E	2.0	•••••	29. 324	44		2 p. m. 3 p. m.	N. NE	1.0	4. 15	29, 202	4
25	9 p. m. 3 a. m. 7 a. m.	SE Calm	0.4 0.0		29. 249 29. 030	37	6	9 p. m. 3 s. m. 7 s. m.	North N. NE	0. 4 1. 4	4. 16 4. 19	29, 409	
	9 a. m. 2 p. m.	E. 8E	0. 2		28, 940	42		9 a. m. 2 p. m.	NE	2.0	4. 19	29. 514	. 3
	3 p. m. 9 p. m.	SE	0. 2		29. 045	34		3 p. m. 9 p. m.	N. NE N. NE	2. 5 1. 3	4. 16 4. 14	29. 497	
26	3 a. m. 7 a. m.	Calm	0.0		29. 262	35	7	3 a.m. 7 a.m.	East	1. 3 0. 1	4. 05	29. 392	
	9 a.m. 2 p.m.	N. NE	1.7		29. 452	37		9 a.m.	E. 8E	0. 1	4. 09	29. 291	- 4
	3 p. m. 9 p. m.	E. NE N. NW	0. 1 0. 3		29. 394	34	ļ.	3 p. m. 9 p. m.	NE N. NE	0.3 1.7	3. 98 3. 84	29. 321	. 3
27	3 a. m. 7 a. m.	N. NE	1.8		29. 494	32	8	3 a.m. 7 a.m.	N. NE	2.0	3.91	29. 245	3
	9 a. m. 2 p. m.	N. NE	2.7		29. 414	35		9 a. un. 2 p. m.	N. NE	3. 5	3.88	29. 097	4
28	3 p. m. 9 p. m.	N. NE N. NE E. SE	2.7 2.1 1.7		29. 182	37		3 p. m. 9 p. m. 3 s. m.	N. NE NE	3.3 1.9	3. 75 3. 74 3. 80	29. 060	4
20	3 a. m. 7 a. m. 9 a. m.	E. SE	2.0		28. 897	42	9	3 a. m. 7 a. m. 9 a. m.	N. NE	2. 1	3.91	29.077	3
	2 p. m. 3 p. m.	E. 8E	1.3		28. 689	42		2 p. m. 3 p. m.	North	4. 0	3. 85	29 062	3
29	9 p. m. 3 a. m.	8W E. SE	0.2		28. 677	40	10	9 p. m. 3 a. m.	N. NW .	1.9	3.90	29. 087	3
	7 a. m. 9 a. m.	E. SE	1.4		28. 741	37	10	7 a.m. 9 a.m.	NW	1.9	4. 10	29. 090	3
	2 p. m. 3 p. m.	E. 8E	1.2		28. 784	42		2 p. m. 3 p. m.	North		4. 09	29. 152	4
30	9 p. m. 3 a. m.	N. NE W. NW.	0. 1 0. 7		28. 889	45	11	9 p. m. 3 a. m.	Calm	0.0	4. 11 4. 12	29. 224	3
	7 a. m. 9 a. m.	N. NW	1.7		28. 932	36		7 a. m. 9 a. m.			4. 21	29. 275	3
	2 p. m. 3 p. m.	N. NE	2.4		28, 977	37		2 p. m. 3 p. m.	NE W. NW.	0. 2	4. 13	29. 225	•
31	9 p. m. 3 a. m.	N. NE	0. 9 0. 6		29.075	36	12	9 p. m.	W. NW.	0. 1 0. 0	4. 10 4. 03	29, 204	4
	7 a. m.	N. NE	1.4		29, 067	35		7 a. m.	West	0. 5	4. 01	29. 190	4
	2 p. m. 3 p. m. 9 p. m.	NE	0.8		29. 117 29. 155	40 35		2 p. m. 3 p. m. 9 p. m.	W. NW.	2.9 1.2	3. 96 3. 94	29, 208 29, 274	-

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND		abov	duced	hed.			WINI),	abov	reduced	1000
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 32°.	Therm'r detached.	Date.	Hour of day.	Соптие,	Relative ve-	Heig't of water above zero of tide-gange.	Barometer re-	to de la constante de la const
1864.		** *****	1.0				1864.						ŀ
April 13	3 a. m.	N.NW	1.0	4. 02	29, 359	39	April 25	3 a. m. 7 a. m.	North	1.8	3, 65	29, 175	†.
	9 a. m.	W.NW.	1.9	4.11				9 a. m.	N.NW.	0.5	3. 81		ļ.
	2 p. m. 3 p. m.	W.NW	1.6	4. 13	29, 371	48	11	2 p. m. 3 p. m.	N.NW.	1.6	3.78	29, 177	J
	9 p. m.	Calm	0, 0	4. 12	29.408	41	10	9 p. m.	NW	0.2	3.88	29, 271	Ï
14	3 a. m. 7 a. m.	NE	0.1	4. 12	29. 398	37	26	3 a. m. 7 a. m.	Calm	0.0	3, 88	29, 332	•
	9 a. m.	N.NE	1, 3	4.11				9 a. m.	E.SE	0.9			. .
	2 p. m. 3 p. m.	N.NE	2.7	4. 08	29. 394	41		2 p. m. 3 p. m.	E.SE	1.8	3.82	29, 291	
	9 p. m.	North	1.7	3.94	29.369	36		9 p. m.	North	6, 2	3, 76	29, 200	
15	3 a. m. 7 a. m.	N.NE	2.5	3.99	29, 340	33	27	3 a. m. 7 a. m.	North	3. 1	3, 65	29, 582	
	9 a. m.	N.NE	2.8	3.96	23. 340	33		9 a. m.	N.NE	6. 6	3, 66	22. 352	.!
	2 p. m.	N. M.			29, 257	39		2 p. m.				29, 600	1
	3 p. m. 9 p. m.	N.NE.	1.5	4. 02 3. 98	29, 259	36		3 p. m. 9 p. m.	N.NE	7.0	3.66	29.585	
16	3 a. m.	N.NE	1. 2	4.00			28	3 a. m.	N.NE	1.7	3, 64		-1-
	7 a. m. 9 a. m.	N.NW.	0.6	4. 01	29. 230	34	1 67	7 a. m. 9 a. m.	NE	2.2	3, 69	29, 650	
	2 p. m.	*******			29. 224	39		2 p. m.				29, 629	ď
	3 p. m. 9 p. m.	E.NE Calm	0.1	4.07	29. 284	33		3 p. m. 9 p. m.	N.NE	2.9	3, 69	29. 634	
17	3 a. m.	Calm	0.0	4.11			29	3 a. m.	E.SE	0. 5	3, 70		
	7 a. m.	sw	0.8	4.00	29, 299	35		7 a. m.	n an		0.70	29, 637	1
	9 a. m. 2 p. m.	DW	0.0	4. 09	29, 274	46		9 a. m. 2 p. m.	E.SE	1,4	3.70	29, 594	١,
	3 p. m.	E.SE	0.3	4. 13				3 p. m.	N. NE	2.1	3, 75		-1
18	9 p. m. 3 a. m.	Calm	0.0	4.06	29. 366	40	30	9 p. m. 3 a. m.	North	2.5	3, 69	29, 539	j
. 40	7 a. m.	*******			29, 394	38	30	7 a. m.				29, 422	i:
	9 a. m. 2 p. m.	N.NE	1. 4	4.03	29, 464	43		9 a. m. 2 p. m.	N.NE	0.7	3.70	29. 389	-1
	3 p. m.	NE	0.9	4. 05				3 p. m.	NE	0.7	3.81		
19	9 p. m. 3 a. m.	Calm	0.0	3.96 4.00	29. 536	36	May 1	9 p. m. 3 a. m.	8.8W	0.5		29, 346	i
13	7 a. m.				29. 599	37	May 1	7 a. m.				29, 184	i
	9 a. m.	N.NE	1.7	3, 96	00 556	45	1	9 a. m.	West	3. 5		29, 156	
	2 p. m. 3 p. m.	N.NE	2.0	4. 03	29, 556	45		2 p. m. 3 p. m.	W.NW	4. 1	3, 59		٠.
	9 p. m.	Calm	0.0	3.92	29, 491	37	15	9 p. m.	Calm	0.0	3, 59 3, 51	29, 187	1
20	3 a. m. 7 a. m.	Calm	0, 0	3, 96	29. 466	40	2	3 a. m.	Calm	0.0	3, 63	29, 904	1
	9 a. m.	E.SE	0, 4	3, 96				9 a. m.			3.56		.,
	2 p. m. 3 p. m.	E.SE	0.7	4.00	29. 383	46		2 p. m.	Calm	0.0	3.55	29, 341	1
	9 p. m.	Calm	0,0	2.93	29. 361	41		3 p. m. 9 p. m.	Calm	0.0	3, 58	29, 331	
21	3 a. m. 7 a. m.	Calm	0.0	3.97	29. 356	44	3	3 a. m.	W.NW.	0, 2	3. 73	29, 373	٦
	9 a. m.	E.SE	1.0	3, 96	22. 350	**		7 a. m.	NW	1. 2	3. 69		
	2 p. m.	E.SE			29, 301	49		2 p. m.				29, 366	
	3 p. m. 9 p. m.	E.SE	1, 5	3. 93	29, 271	44		3 p. m. 9 p. m.	E.SE	1.3	3.71	29, 407	•
22	3 a. m.	N.NE	0. 2	3.84			4	3 a. m.	8.8W	0.3			.1
	7 a. m. 9 a. m.	Calm	0.0	3. 84	29, 201	43		7 a. m. 9 a. m.	SE	2.2	3. 51	29, 416	
	2 p. m.				29, 203	52		2 p. m.				29, 435	,
	3 p. m. 9 p. m.	Calm	0.0	3.82	29, 298	47		3 p. m.	8.8E	0, 9	3, 52	29, 300	•
23	3 a, m.	North	2.9	3. 81			5	9 p. m. 3 a. m.	S.SW	0,9	3. 43		
	7 a. m.	North	5. 4		29.418	39	1	7 a. m.				29, 325	
	9 a. m. 2 p. m.	********		3. 90	29. 446	41		9 a. m. 2 p. m.	s.sw	2.7	3, 46	29, 909	1
	3 p. m.	N.NE	4.2	3.94				3 p. m.	8.8W	4.2	3, 46	29, 224	1
24	9 p. m. 3 a. m.	N.NE	1.6	3, 92	29. 457	37	6	9 p. m. 3 a. m.	W.SW .	3.0	3. 43	29, 271	١.
	7 a. m.				29. 332	38		7 a. m.	1			29, 367	ŀ
	9 a. m. 2 p. m.	N.NE	4.1	3. 81	29, 217	36		9 a.m. 2 p.m.	N.NE	4.8	3. 40	29, (23	•
	3 p. m.	N.NE	5.3 3.0	3. 66				3 p. m.	N.NE	3.0	3.40		
	9 p. m.	N.NE	13.0	3. 61	29. 195	38		9 p. m.	N.NE	1.6	3, 46	29, 386	ı

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

	ŀ	WIND		above uge.	reduced.	Ped.			WIND).	above	duced	led.
Date.	Hour of day.	Course.	Relative ve- locity.	Heig't of water above zero of tide-gauge.	Barometer rec to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve- locity.	Heig't of wuter above zero of tide-gange.	Barometer reduced to 32°.	Therm'r detached.
1864.							1864.						
May 7	3 a. m.	North	0.6	3.38	29. 276	42	May 19	3 a. m.	Calm	0.0	3. 45	29. 379	63
	9 a. m. 2 p. m.	NE	1.4	3. 32	29. 213	42		9 a. m. 2 p. m.	w.sw.	0. 4	3. 42	29. 336	6
	3 p. m.	NE N.NE	1.0					3 p. m.	E.SE		3, 43 x		١
8	3 a. m.	N. NE	0. 8 0. 6		29. 207	41	20	9 p. m. 3 a. m.	Calm	0.0		29. 363	59
	7 a. m. 9 a. m.	N.NE	1.6	3, 39	29. 196	42		7 a. m. 9 a. m.	w.nw	1.7	3. 52	29. 341	6:
	2 p. m. 3 p. m.	NE	0.2		29. 218	50		2 p. m. 3 p. m.	West	1.8	3. 49	29. 273	86
	9 p. m.	Calm	0.0	3.66	29. 179	44		9 p.m.	w.sw.	0.5	3.51	29, 225	74
9	3 s. m.	8W	0.9		29. 156	54	21	3 a. m. 7 a. m.	w.sw	1.2		29. 253	7:
	9 a. m. 2 p. m.	8W	1.4	3. 57	29. 016	70	1	9 a. m. 2 p. m.	w.sw.	3. 0	3. 58	29. 175	88
	3 p. m.	8.8W	2.5 5.6	3. 52				3 p. m.	W.SW.	3.0			ļ
10	9 p. m. 3 a. m.	8.8W N.NE	9. 2	3. 19 3. 12	29. 983	62	22	9 p. m. 3 a. m.	W.8W .	0.7	3. 50 3. 32	29. 165	70
	7 a. m. 9 a. m.	N.NE	7. 2		29. 277	34		7 a. m. 9 a. m.	NE	1.4	3. 28	29. 278	5
	2 p. m. 3 p. m.	N.NE	4.0		29. 355	38		2 p. m. 3 p. m.	NE	0, 1	3, 19	29. 241	6
	9 p. m.	N.NE	0.7		29.377	34		9 p.m.	Calm	0.0	3.17	29. 146	5
11	3 a. m.	Calm	0.0	l. .	29. 374	38	23	3 a. m. 7 a. m.	Calm	0.0	3. 16	29.065	7
	9 a. m. 2 p. m.	8E	1. 2	•••••	29, 256	47		9 a. m. 2 p. m.	w.sw.	2.9	3. 31	29. 062	7
	3 p. m.	E.SE	2.3 0.6					3 p. m.	W.SW	4, 2			١
12	9 p. m. 3 a. m.	8E 8.8W	1.0		29. 189	47	24	9 p. m. 3 a. m.	W.8W.	0.9	3. 40 3. 36	29. 135	6
	7 a. m. 9 a. m.	N.NE	1.6		29. 084	53		7 a. m. 9 a. m.	w.sw	1.2		29. 115	6
	2 p. m. 3 p. m.	N.NE	4.8		29. 179	47		2 p.m.	E.8E	0.4		29. 125	6
	9 p. m.	North	2.6 1.8		29. 237	46		3 p. m. 3 p. m.	Calm	0.0		29. 140	5
13	3 a. m.	North			29. 328	46	25	3 a. m.	N.NE	2.1		29. 189	4
	9 a. m. 2 p. m.	N.NE	3.1		29. 303	50		9 a. m. 2 p. m.	North	5. 2	3. 30	29. 217	5
	3 p. m.	N. NE	3.8 2.8					3 p.m.	N.NE	4.2			
14	9 p. m. 3 a. m.	North North	24		29. 323	49	26	9 p.m. 3 a.m.	N.NE	2.2	3. 39 3. 35	29. 245	5
	7 a. m. 9 a. m.	N.NE	4.0		29.411	48	ļ.	7 a. m. 9 a. m.	N.NE	2.8	3. 39	29. 270	5
	2 p. m. 3 p. m.	North	5.6		29. 333	58	1	2 p. m.	NE	0.8		29. 252	6
	9 p.m.	North	3.5	3.50	29.310	56		3 p. m. 9 p. m.	Calm	0.0	3. 45	29, 240	5
15	3 a. m. 7 a. m.	N.NE	1.3	3. 53	29. 335	54	27	3 a. m. 7 a. m.	NW	0.3	3. 45	29. 317	5
	9 a. m. 2 p. m.	N.NE	3. 7	3, 47	29. 355	 58		9 a. m. 2 p. m.	NE	2.3	3. 43	29. 392	5
	3 p. m.	N.NE	3.3					3 p. m.	NE	3. 1	3.41		١
16	9 p. m. 3 a. m.	North	0.5		29. 377	52	28	9 p. m. 3 a. m.	North Calm	3.5 0.0	3. 43 3. 59	29. 487	4
	7 a. m. 9 a. m.	N.NE	3.3	3, 47	29. 392	58		7 a. m. 9 a. m.	E.SE	3.7	3.41	29. 590	5
	2 p. m.			3. 53	29. 342	67		2 p. m.	 -			29. 535	6
	3 p. m. 9 p. m.	N.NE North	2.4	3. 41 3. 46	29. 337	58	Ì	3 p. m. 9 p. m.	E.SE	0.9		29. 437	5
17	3 s. m. 7 s. m.	N.NE		3. 46	29. 387	56	29	3 a. m. 7 a. m.	s.sw		3. 56	29. 207	6
	9 a. m.	N.NE		3.43		62	Ì	9 a. m.	8W	3.7	3. 43	29. 108	
	2 p. m. 3 p. m.	North	3. i	3.46	29. 354			2 p. m. 3 p. m.	NE	2.1	3. 41	l	7
18	9 p. m. 3 a. m.	N.NE	3. 1 0. 6	3. 37 3. 46	29, 337	59	30	9 p. m. 3 a. m.	Calm		3. 39 3. 44	29. 181	5
	7 a. m. 9 a. m.	N.NE		3. 40	29. 367	53		7 a. m. 9 a. m.	8W			29. 098	6
	2 p. m.	N.NE			29. 351	72		2 p. m.	l	1		29. 997	8
	3 p. m.	N.NE	0.0	3.44	29. 324	60		3 p. m.	8W	5.0	3, 41	29. 035	7

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

			WIND		erabove gange.	reduced io.	bed.			WINI		above	reduced	paq.
Da	te.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide gange.	Barometer red to 320.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide gauge.	Barometer re- to 322.	Therm'r detached.
186			Wanth		2 25			1864.	2	N. NW	1.6	3. 45		1
May	31	3 a. m. 7 a. m.	North	2.4		29, 241	53	June 12	3 a. m. 7 a. m.				29, 653	
		9 a.m. 2 p.m.	North	5. 9	3, 24	29, 291	54		9 a. m. 2 p. m.	NE	1.5	3, 40	29. 635	5
		3 p. m.	North	6. 2	3. 30				3 p. m.	N. NE		3.35	29. 635	1
June	1	9 p. m. 3 a. m.	North N. NE	0, 5	3, 28 3, 47	29. 410	48	13	9 p. m. 3 a. m.	N. NE	0,0	3.32	29. 610	
	- 6	7 a. m.				29, 460	51	1	7 a. m.	E.NE	1 3	3. 40	29, 689	5
	8	9 a. m. 2 p. m.	N. NE		3.41	29, 462	61		9 a.m. 2 p. m.				29. 642	5
		3 p. m. 9 p. m.	Calm	0.0	3, 50	29. 417	54		3 p. m. 9 p. m.	Calm		3.44	29, 550	4
	2	3 a. m.	W.SW.	0.4				14	3 a. m.	Cahn		3.46		
		7 a. m. 9 a. m.	NE	1,0	3, 48	29, 427	57		7 a. m. 9 a. m.	E. SE	0. 9	3. 43	29. 592	
		2 p. m.				29, 444	58		2 p. m.	E. SE		3, 43	29, 484	
		3 p. m. 9 p. m.	Calm	0. 2		29. 477	50		3 p. m. 9 p. m.	Calm	0.0	3. 43	20, 432	
	3	3 a. m.	Calm	0.0		29, 457	58	15	3 a. m. 7 a. m.	Calm	0.0	3. 47	29, 454	
		7 a. m. 9 a. m.	sw	0.3	3. 53				9 a. m.	E. SE	0.6	3.44		
		2 p. m. 3 p. m.	sw	0.3	3.50	29. 351	74		2 p. m. 3 p. m.	E.SE	1.4	3. 51	29, 421	
		9 p. m.	Calm	0.0	3, 54	29. 340	65		9 p. m.	Calm	0.0	3.47	29, 401	
	4	3 a. m. 7 a. m.	Calm	0.0	3, 53	29, 298	68	16	3 a. m.	Calm	0,0	3. 54	29, 454	Ü
		9 a. m.	8. SW	1.1	3. 55				9 a. m.	E. SE	0.6	3, 50	29, 423	٠.
		2 p. m. 3 p. m.	S. SE	1.7	3, 53	29. 212	71		2 p. m. 3 p. m.	E. SE	0, 6	3, 56		4.50
	5	9 p. m. 3 a. m.	S. SE S. SE	0. 2	3, 52	29. 132	66	17	9 p. m. 3 a. m.	Calm	0.0		29, 328	
		7 a. m.				29, 035	70		7 a. m.				29, 456	
		9 a. m. 2 p. m.	sw	2.5	3. 53	28. 965	81		9 a. m. 2 p. m.	NE	0, 3	3. 47	29, 408	
		3 p. m.	W. SW.	0.3					3 p. m.	E. SE		3. 48	29, 355	
	6	9 p. m. 3 a. m.	W.NW .	2.9		29. 037	75	18	9 p. m. 3 a. m.	Calm	0,0	3, 42		2 3
	-17	7 a. m.				29, 429	53	1	7 a. m. 9 a. m.	E.SE	0.5		29. 341	9
		9 a. m. 2 p. m.	NE	3.4	3. 57	29. 554	52		2 p. m.				29, 363	
		3 p. m. 9 p. m.	N. NE	2.1	3, 64	29. 560	47		3 p. m. 9 p. m.	NE	0.1	3, 42	29, 308	
	7	3 a. m.	Calm	0.0				19	3 a. m.	N. NE		3, 39		2 1
		7 a. m. 9 a. m.	E.SE	0.9	3. 56	29, 642	51		7 a. m.	E. NE	0,8	3. 42	29. 313	
		2 p. m.				29. 592	64		2 p. m.		1.0		29, 345	
		3 p. m. 9 p. m.	E. SE	1.0		29. 512	50		3 p. m. 9 p. m.	E. SE	0.0		29, 360	0
	8	3 a. m.	Calm	0.0		29. 369	59	20	3 a. m. 7 a. m.	Calm	0,0	3, 41	29, 470	
		7 a. m. 9 a. m.	Calm	0.0	3. 46				9 a. m.	E. NE	0.6	3.47		
		2 p. m. 3 p. m.	s. sw	2, 2	3.43	29. 234	71		2 p. m. 3 p. m.	E. NE	1.4	3.41	29, 435	
		9 p. m.	S. SW	1.1	3. 55	29, 085	70		9 p. m.	NW	0.3	3.44	29, 435	7
	9	3 a. m. 7 a. m.	w.sw.	1.2	3, 54	29, 030	72	21	3 a. m. 7 a. m.	Calm	0, 1	3.42	29, 488	*
		9 a. m.	N. NE	2.8	3. 53				9 a. m.	East	0, 6	3, 47	29, 495	
		2 p. m. 3 p. m.	North		3, 53	29. 194	57		2 p. m. 3 p. m.	E.SE		3, 41		
	10	9 p. m.	North	3.8	3, 49	29. 247	48	22	9 p. m. 3 a. m.	Calm		3.44	29, 500	
	10	3 a. m. 7 a. m.				29, 403	47	~~	7 a. m.				29, 547	1
		9 a. m. 2 p. m.	North	4.4	3. 42	29, 440	51		9 a. m. 2 p. m.	E. SE		3.48	29, 497	. 00
		3 p. m.	North	3. 2	3, 41				3 p. m.	E. SE	1.5	3. 43 3. 51	29, 452	7
	11	9 p. m. 3 a. m.	N. NE	2.3	3, 42	29, 411	45	23	9 p. m. 3 a. m.	Calm	0.0	3, 50	*****	1
		7 a.m.				29. 561	46		7 a. m.			3. 60	29, 524	
		9 a. m. 2 p. m.	N. NE		3. 44	20. 563	56		9 a. m. 2 p. m.	West	l l		99. 472	
		3 p. m.	N. NE N. NW .	3.0	3.49		50			E. SE	1.7	3.52	99, 427	7

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

			WINI).	above	hood	ped.			WIND		above	reduced o.	bed.
Dat	te.	Hour of day.	Course,	Relative ve-	Beig't of water above zero of tide-gange.	Barometer reduced to 32°,	Therm'r detached.	Date,	Hour of day.	Course,	Relative ve-	Heig't of water above zero of tide-gange.	Barometer re-	Therm'r detached.
1866 June	1. 24	3 a. m. 7 a. m. 9 a. m. 2 p. m.	Calm	0.0	3. 52 3. 60	29. 499 29. 399	94	1864. July 6	3 a. m. 7 a. m. 9 a. m. 2 p. m.	Calm N. NE	0.0	3. 52 3. 57	29. 213 29. 240	66
	25	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	8.8W 8W 8W	2. 5 0. 6 0. 2 4. 2	3. 55 3. 65 3. 64 3. 74	29. 339 29. 401	83 82	7	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NE N. NE NW	2.3 0.6 0.4 1.7	3. 51 3. 50 3. 47 3. 57	29. 243 29. 211	6
	26	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	W. 8W. SW SW	4.3 1.8 1.8	3. 74 3. 82 3. 71	29. 306 29. 329 29. 447	94 75 82	8	9 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	W.NW . Calm Calm	2.3 0.0 0.0	3. 49 3. 53 3. 54 3. 65	29. 260 29. 318 29. 473	7
	27	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	West N. NE N. NE	4.8 4.8 4.7	3. 62 3. 45 3. 56	29. 291 29. 430 29. 590	92 53 50	9	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 a. m.	SE Calm Calm	0. 6 0. 0 0. 0	3. 64 3. 65	29, 495 29, 465 29, 443	7
	28	9 a. m. 2 p m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	N. NE E. NE Calm	3. 1 0. 1 0. 0	3. 47 3. 44 3. 49	29. 617 29. 587 29. 610	56 51 56	10	2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	E. SE SW W. SW.	1. 7 0. 8 1. 8	3, 58 3, 52 3, 58	29. 370 29. 302 29. 265	
	29	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	S.SW SE S.SW	0.3 0.1 0.2	3. 50 3. 41 3. 44	29. 509 29. 416 29. 301	61 60 76	11	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	W. 8W. 8.8W. W.NW	2.4 2.3 0.7 1.8	3. 64 3. 64 3. 68	29. 230 29. 207 29. 292	
	30	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	8.8W 8W	1. 2 2. 2 0. 3 0. 2	3. 49 3. 41 3. 53	29, 258 29, 222 29, 245	83 83 75	12	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m. 7 a. m.	NW N. NE N.NE	2.3 0.1 0.9	3, 70 3, 65 3, 66 3, 65	29, 300 29, 340 29, 495	
uly	1	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	N.NW N. NE SW	0. 5 3. 4 0. 3	3. 48 3. 53 3. 48 3. 57	29. 328 29. 378	65 59	13	9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	NE NE N.NE Calm	1.7 1.1 0.2 0.0	3. 65 3. 64 3. 56 3. 56	29. 510 29. 505	
•		7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	N. NE	1. 8 0. 0	3. 51 3. 52 3. 48	29. 216 29. 189 29. 279	53 58 58	14	7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m. 3 a. m.	E.SE	0. 6 1. 9 0. 1	3. 55 3. 55 3. 50	29. 531 29. 510 29. 483	
		3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	W.NW W.NW Calm	0.0 1.1 2.2 0.0	3. 50 3. 60 3. 64 3. 66	29. 294 29. 293 29. 331	58 75 65	14	3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m. 9 p. m.	E.SE Calm	0. 0 1. 9 1. 7 0. 0	3. 52 3. 59 3. 53	29. 518 29. 493 29. 444	
	3	3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m.	W.NW.	0.0	3. 75 3. 73 3. 66	29. 408 29. 371	70 74	15	3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m.	E.SE SE	0.9	3. 59 3. 55 3. 64	29. 420 29. 398	
	4	9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m.	Calm S. SW	0.0	3. 63	29, 381 29, 433 29, 405	65 68 81	16	9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m. 3 p. m.	Calm E.SE	l	3. 59	29. 360 29. 378 29. 370	
	5	3 p. m. 9 p. m. 3 a. m. 7 a. m. 9 s. m. 2 p. m.	S.SE Calm South	0. 1 0. 0	3. 59 3. 59 3. 61	29. 373 29. 413 29. 333	68 72 75	17	9 p. m. 3 a. m. 7 a. m. 9 a. m. 2 p. m.	SE S. SE	2.5	3. 57 3. 55 3. 59 3. 55	29. 330 29. 335 29. 350	
		3 p. m. 9 p. m.	E. SE	0. 2 0. 7	3. 60 3. 59	29. 308	65		3 p. m.	8.8E	1.8 1.2	3.52		ŀ

TABLE AA.—Showing the wind, water. barometer, &c.—Continued.

		WIND	la l	apow	reduced	bed,			WIND		abov	meed	bed.
Date.	Hour of day.	Course.	Relative ve-	Helg't of water above zero of tide-gauge.	Barometer red to 320.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 320.	Therm'r detached.
1864.	2	0.00	1 .	0.40			1864.		w ow	0.4	2.09		
July 18	3 a.m.	8E	1.6		29, 550	66	July 30	3 a. m. 7 a. m.	w.sw	0.4		29, 242	7
	9 a. m. 2 p. m.	South	1.8	3. 59	29, 498	73		9 a.m. 2 p.m. 3 p.m.			3. 66	29, 194	
	3 p. m. 9 p. m.	8W	0. 9 0. 5	3.63	29. 468	71		9 p. m.	sw s. sw	1.2	3, 68	29, 196	i
19	3 a. m. 7 a. m.			3.61	29. 483	70	31	3 a. m.	General Control	0. 6		29, 217	1
	9 a. m. 2 p. m.	South	2.0		29, 420	74		9 a. m.	E.8E	1.0		29, 184	
	3 p. m. 9 p. m.	8W	1.6 0.7	3. 66 3. 76	29, 415	70	100	2 p. m. 3 p. m. 9 p. m.	s. sw	1.8	49 67.8	29, 139	
20	3 a. m. 7 a. m.	W. SW.	1.2		29, 488	68	Aug. 1	3 a. m. 7 a. m.	w.sw .	1.4	3. 69	29, 190	
	9 a. m. 2 p. m.	N. NE	2.9	3. 79	29, 538	69		9 a. m.	W. NW.	1.5	3. 65	29, 215	١.
	3 p. m. 9 p. m.	NE N. NE	2. 6 3. 0	3. 70 3. 68	29. 581			2 p. m. 3 p. m. 9 p. m.	E.NE N. NE	0. 9		29, 282	
21	3 a. m.	N. NE	3.1	3. 54		::::	2	3 a. m.	N.NW.	0.7	3. 64		
	7 a. m. 9 a. m.	N. NE	3.9	3. 62	29, 688	57		7 a. m. 9 a. m.	N.NE	2.9	3. 60	29. 345	٠.
	2 p. m. 3 p. m.	N. NE	3.0	3. 58	29. 674	67		2 p. m. 3 p. m.	N.NE	2.0	3, 55	29.300	ь.
22	9 p. m. 3 a. m.	N.NW	0. 5 0. 0	3. 65 3. 59	29. 654	59	3	9 p. m. 3 a. m.	N.NE	0.7	3. 59 3. 56	29, 465	
	7 a. m. 9 a. m.	8W	0.7	3. 69	29. 696	63		7 a. m. 9 a. m.	NE	1.0		29, 335	Ť.
	2 p. m. 3 p. m.	E. SE	1.3	3. 67	29, 638	75		2 p. m.	N. NE	1.0		29. 280	
23	9 p. m. 3 a. m.	8W W. 8W.	0. 5 1. 5	3. 76	29, 603	66		9 p. m.	N. NE	0.3	3.54	29, 248	
23	7 a. m.	-		3.74	29, 621	66	4	3 a. m. 7 a. m.				29. 247	
	9 a. m. 2 p. m.	W. NW.	2.4	3. 79	29. 578	78		9 a. m. 2 p. m.	NE,	1.4		29. 230	1
	3 p. m. 9 p. m.	W.SW.	2.7 0.6	3. 76 3. 74	29. 533	68		3 p. m. 9 p. m.	N. NE N. NW	1.2	2.53 3.56	29, 278	İ,
24	3 a.m. 7 a.m.	w.sw.	0.8	3.70	29, 540	67	5	3 a. m. 7 a. m.	North	1.5	3. 49	29. 347	
	9 a.m. 2 p. m.	w. sw.	2.2	3.71	29. 46C	81		9 a. m. 2 p. m.	N. NE	3. 4		29, 368	
	3 p. m. 9 p. m.	W. SW.	1. 0 0. 0	3. 6 9 3. 76	29, 428	-69		3 p. m. 9 p. m.	N. NE	2.6	3. 48	29.398	٠.
25	3 a.m.	w.sw.	0.3	3. 66			6	3 a. m.	N. NE	0.1	3. 44	29, 475	
	7 a. m. 9 a. m.	w.sw.	1.4	3. 65	29, 353	69		7 a. m. 9 a. m.	N. NE	1.3	3. 50		
	2 p. m. 3 p. m.	8. 8W	0.9	3.61	29, 295	82		2 p. m. 3 p. m.	N.NE	1.6	3.52	29, 483	٠.
26	9 p. m. 3 a. m.	8.8W N. NE	0.3		29. 300	73	7	9 p. m. 3 a. m.	N. NW .	0.1	3, 56	29, 508	٠.
	7 a. m. 9 a. m.	N. NE.	2.0	3. 71	29, 380	66	1	7 a. m. 9 a. m.	NE	0.7	3.61	29, 585	di Ma
	2 p. m. 3 p. m.	E. NE.	0.8		29, 440	67		2 p. m. 3 p. m.	E. SE	0.5		29, 595	ď.
27	9 p. m. 3 a. m.	Calm	0.0	3.68	29. 470	66		9 p. m.	Calm	0.0	3, 63	29, 555	
41	7 a. m.			. 	29. 515	64	8	3 a. m. 7 a. m.	Calm			29. 570	Ē
	2 p. m.	E. SE	0.4	3. 66	29. 558	80		9 a.m. 2 p. m.	SE	0.6		29, 490	ï
	3 p. m. 9 p. m.	8E 8. SE	1.5 0.3		29, 422	74		3 p. m. 9 p. m.	SE	0.0	3.69	29, 414	٠,
28	3 a. m. 7 a. m.	sw	0.8	3.70	29. 395	74	9	3 a. m. 7 a. m.	Calm	0.0	3. 66	29, 385	ì
	9 a. m. 2 p. m.	sw	1.8	3. 69	29. 344	90		9 a. m. 2 p. m.	sw	1,0	3. 69		1
	3 p. m. 9 p. m.	8W W.8W	1.5 0.7	3.75 3.69	29, 304	79		3 p. m. 9 p. m.	8E 8.8W	1.3	3.69	29, 242	į
29	3 a. m.	w. sw.	0. 3	3.74			10	3 a. m.	W.SW	0.5	3, 73	20, 267	
	9 a. m.	w.sw	1.8	3.68	29. 324	75		7 a.m.	w.sw.	1.2	3.75		ļ.,
	2 p. m. 3 p. m. 9 p. m.	NE W. SW.	0.8	3, 67 3, 60	29. 267	81		2 p. m. 3 p. m.	E. SE S. SW	1.6	3.74 3.72	29, 224 29, 224	8

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WINI		abow uge.	duced	ped.			WIND		above	reduced.	1
Date.	Hour of day.	Course,	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer red to 32°,	Tributania Jahanhard
1864. Aug . 11	3 a. m.	Calm	ao	3. 73	90 987		1864. Aug. 23	3 a. m.	8. SE	1.5	3. 80		
	7 a. m. 9 a. m. 2 p. m.	E. SE	1.0	3, 71	29, 257	79 85		7 a. m. 9 a. m. 2 p. m.	8. SE	3.0	3. 85	29. 258 29. 143	6
	3 p. m. 9 p. m.	8E E, 8E	1.9 0.2	3.68 3.63	29. 241	78		3 p. m. 9 p. m.	8. 8E 8. 8W	2. 8 0. 6	3. 90 3. 86	29, 158	
12	3 a. m. 7 a. m.	8. 8W	0. 2 0. 5	3.68	29. 227	77	24	3 a.m. 7 a.m.	8. SW	1. 2	3, 89	29. 163	
	9 a. m. 2 p. m.	8. 8W	1.8	3. 71	29. 141	88		9 a.m. 2 p.m.	s. sw	1.5	3.88	29, 120	٠.
	3 p. m. 9 p. m.	8. 8W 8. 8W	2. 4 1. 4	3. 75 3. 73	29. 152	74		3 p. m. 9 p. m.	8. 8W South	1.4 1.0	3. 91 3. 84	29. 072	1
13	3 a. m. 7 a. m.	8w	1.7		29. 187	75	25	3 a.m. 7 a.m.	w. 8w.	2.9	3. 95	29. 148	6
	9 a. m. 2 p. m.	NW	2.2	3.77	29, 282	83		9 a.m. 2 p.m.	West	4.5	3.98	29. 220	١٠,
	3 p. m. 9 p. m.	E. SE	1. 1 0. 0		29. 332	74		3 p. m. 9 p. m.	West W. SW.	3. 6 0. 2	4. 04 4. 08	29. 218	
14	3 s. m. 7 s. m.	NW	0, 3		29, 415	73	26	3 a.m.	w.sw.	0.6	4. 11	29. 117	1
	9 a. m. 2 p. m.	NE	0.7	3. 78	29. 400	79		9 a.m. 2 p.m.	W. NW.	3. 3	4. 09	29. 025	1
15	3 p. m. 9 p. m.	Calm	1.7 0.0		29. 382	72		3 p. m. 9 p. m.	West W. 8W.	4. 2 1. 3	4. 14 4. 03	29. 007	F
10	3 a. m. 7 a. m. 9 a. m.	Calm	0.0		29. 345	67	27	3 a.m.	w.sw.	1.8	4. 16	29. 095	1
	2 p. m. 3 p. m.	E. SE	1.3	3. 70 3. 72	29. 320	79		9 a. m. 2 p. m.	W. NW.	4.0	4. 12	29. 143	ŀ
16	9 p. m. 3 a. m.	Calm	0.0	3.72	29. 327	74	28	3 p. m. 9 p. m.	W. NW. W. NW. W. NW.	3. 6 0. 3	4. 18 4. 08	29. 198	ŀ
	7 a. m. 9 a. m.	North	4.1	3. 60	29. 360	71	20	3 a. m. 7 a. m. 9 a. m.	NW	0. 6 2. 1	4. 14	29. 228	ŀ
	2 p. m. 3 p. m.	N. NE	4.3		29. 443	69		2 p. m. 3 p. m.	W. NW.	2. 2	4. 02	29. 241	
17	9 p. m. 3 a. m.	N. NE N. NW .	3.9 2.3	3.46	29. 488	66	29	9 p. m. 3 s. m.	NW	0. 2 0. 4	3. 95 4. 00	29. 291	1
	7 a. m. 9 a. m.	N. NE	3.8	3. 59	29. 661	58	_	7 a. m. 9 a. m.	N. NE	1.3	3. 95	29. 393	1
	2 p. m. 3 p. m.	N. NE	2.7	3. 58	29. 568	68		2 p. m. 3 p. m.	E. NE .	1.0	3. 99	29. 403	9
18	9 p. m. 3 a. m.	NW Calm	0.3	3.64	29, 566	58	30	9 p. m. 3 a. m.	Calm	0. 0 0. 0	3. 97 3. 98	29. 436	
	7 a. m. 9 a. m.	E. NE	0.4	3. 77	29. 584	60		7 a. m. 9 a. m.	East	0. 9	3.91	29. 519	
	2 p. m. 3 p. m.	East	i. i		29. 566	74		2 p. m. 3 p. m.	E. NE	1. 2	3.91	29. 498	
19	9 p. m. 3 a. m.	Calm	0.0		29. 568	60	31	9 p. m. 3 a. m.	NE	0. 1 0. 0	3. 89 3. 93	29. 508	
	7 a. m. 9 a. m.	E. NE	0.6	3.78	29. 566	57		7 a. m. 9 a. m.	E. SE	1. 2	3. 90	29. 554	
	2 p. m. 3 p. m.	E. SE	0.5	3.68	29, 513	76	ŀ	2 p. m. 3 p. m.	E. 8E	1.0	3. 89	29. 533	
20	9 p. m. 3 a. m.	Calm	0. 0 0. 0		29. 495	61	Sept. 1	9 p. m. 3 a. m.	'E. SE Calm	0. 4 0. 0	3. 84 3. 95	29. 511	
	7 a. m. 9 a. m.	N. NE	1.7	3.68	29. 481	58		7 a. m. 9 a. m.	8E	i. 8	3. 94	29. 456	
	2 p. m. 3 p. m.	N. NE.	1.7	3.61	29. 406	74	Ì	2 p. m. 3 p. m.	8E	2. 1	3.91	29. 324	1.7
21	9 p. m. 3 a. m.	N. NW.	0. 1 0. 1		29. 363	62	2	9 p. m. 3 a. m.	SE	0. 3 0. 0	3. 90 3. 87	29. 281	
	7 a. m. 9 a. m. 2 p. m.	NE		3. 65	29. 348	58		7 a. m. 9 a. m.	E. 8E	0.7	3. 91	29. 278	١.,
	3 p. m. 9 p. m.	NE Calm	1.0	3. 66 3. 74	29. 328 29. 368	73		2 p. m. 3 p. m.	NE		3.89	29, 240	
22	3 a. m.	Calm	0.0	3.77	29. 406	63 59	3	9 p. m. 3 a. m. 7 a. m.	N. NE N. NE	1.5	3. 86 3. 83	29. 271	
	9 a.m.	E. SE	0.7	3.85	29. 353	74		9 a. m. 2 p. m.	NE		3. 82	29. 141	
	3 p. m. 9 p. m.	8E	1. 9	3, 80			1		N. NE		3. 78	29. 168	

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND		above	Inced	hed.	11 - 7		WINI),	above inge.	reduced	4
Date.	Hour of day.	Course,	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 32º.	Therm'r detached.	Date,	Hour of day.	Course,	Relative ve-	Heig't of water above zero of tide-gunge.	Barometer red to 392.	The state of
1864. Sept. 4	3 a. m.	E. NE	2.5	3. 71			1864. Sept. 16	3 a, m.	w.sw.	1.0	4. 27		
•	7 a. m. 9 a. m.	NE	3.5	3.70	29. 223	65		7 a. m. 9 a. m.	NW	1. 3	4. 18	29. 494	٠١
	2 p. m. 3 p. m.	N. NE.	5. 3	3. 64	29. 256	65		2 p. m. 3 p. m.	E. SE	0.7	4.08	29. 404	٠,٠.
5	9 p. m. 3 a. m.	NE	3. 5 3. 4	3. 56 3. 50	29. 251	62	17	9 p. m. 3 a. m.	Calm	0.0	4.07	29. 374	٠.
	7 a. m. 9 a. m.	N. NE.	3.2	3. 52	29, 276	61		7 a.m. 9 a.m.	South	2.2	4.14	29. 279	٠,٠
	2 p. m. 3 p. m.	N. NE.	2.7	3. 52	29. 364	63		2 p. m. 3 p. m.	8. 8W	3.8	4 14	29.098	
6	9 p. m. 3 a. m	E. NE			29. 419	63	18	9 p. m. 3 a. m.			4. 20 4. 12	29.022	
	7 a. m. 9 a. m.	E. SE	2.9	3. 76	29. 494	60		7 a.m. 9 a.m.	w. nw.		4.14	29.083	٠,.
	2 p. m. 3 p. m.	East	1.5		29. 491	66		2 p. m. 3 p. m.	W. NW.		4. 15	29, 162	ľ
7	9 p. m. 3 a. m.	E. SE	2. 1 2. 6		29. 431	65	19	9 p. m., 3 a. m.	Calm		4. 25	29. 312	j.
	7 a. m. 9 a. m.	SE		3, 87	29.458	64		7 a.m. 9 a.m.	8w		4.38	29, 390	
	2 p. m. 3 p. m.	SE	i. 0		29. 428	63		2 p. m. 3 p. m.	8. SW		4. 36		1
8	9 p. m. 3 a. m.	Calm W. 8W.	0.0	3.88	29. 458	63	20	9 p. m. 3 a. m.	SW	0. 1		29, 272	
·	7 a. m. 9 a. m.	w. 8w	1.5		29. 533	63		7 a. m. 9 a. m.	E. SE		4. 22	20. 427	1
	2 p. m. 3 p. m.	s. sw		4.06	29. 506	70		2 р. ш. 3 р. m.	SE	l	4. 19	29. 425	1
9	9 p. m. 3 a. m.	8. 8W 8W	0.4		29. 451	67	21	9 p. m. 3 a. m.	Calm	0.0	4. 29	29. 399	
•	7 a. m. 9 a. m.	w.sw.		4. 10	29. 376	68	21	7 a. m.	N. NE		4. 13	29, 282	ĺ
	2 p. m. 3 p. m.	N. NE	2.7		29. 295	81		9 a. m. 2 p. m.	. 	1.9	. 	29, 211	
10	9 p. m. 3 a. m.	S. SW	1.8	3.98	29. 243	73		3 p. m. 9 p. m.	N. NE	0.3	3.89	29. 184	
10	7 a. m.	8. 8W	0.3		29. 285	65	22	3 a. m.	South	1.1		28.993	
	9 a. m. 2 p. m.	N. NE	1.2		29. 298	69		9 a. m. 2 p. m.	South	2.7	- 	28. 935	
	3 p. m. 9 p. m.	N. NE	1. 3 0. 0	3. 94	29.311	62		3 p. m. 9 p. m.	South	1. 1 2. 7		28.940	
11	3 a. m. 7 a. m.	Calm	0.0		29. 386	60	23	3 a.m. 7 a.m.	sw	1.6		29.062	
	9 a. m. 2 p. m.	N. NE	3. 5	. .	29. 457	63		9 a.m. 2 p.m.	W. NW.		4. 12	29. 143	i
	3 p. m. 9 p. m.	N. NE N. NE	3.6 2.6	3. 94 3. 78	29. 466	 59		3 p. m. 9 p. m.	8W W. SW	0. 6 1. 1	4.32	29. 128	
12	3 a. m.	NE	2.0	3. 87	29. 486	61	94	3 a.m. 7 a.m.	w.sw.	1. 5	4. 37	29. 217	٠,٠
	9 a. m. 2 p. m.	E. 8E	12.7	3. 82	29. 464	64		9 a.m. 2 p.m.	W. NW.	4.2	4. 33	29, 347	÷
	3 p. m. 9 p. m.	E. SE	1.9 1.9	3. 92	29. 429	62		3 p. m. 9 p. m.	W. NW. W. SW.		4.32	29, 485	- -
13	3 a. m. 7 a. m.	S. SE	1.0	4.05	29. 374	57	25	3 a.m. 7 a.m.	Calm		4, 40	29. 533	إ.
	9 a. m. 2 p. m.	8E		3.98	29. 224	62		9 a. m. 2 p. m.	8E	0. 5	4. 42	29. 345	٠
	3 p. m. 9 p. m.	8E	3.5	4. 07 4. 01	29. 154	64		3 p. m. 9 p. m.	South S. SE	1.3	4. 21 4. 15	29, 322	٠.
14	3 a. m. 7 a. m.	West		4. 13	29. 143		26	3 a.m.	8. SE		4. 15 4. 20	29, 240	١.
	9 a.m.	W. NW.		4.11		63		7 a. m. 9 a. m.	8. sw	3.5	4. 11	29.098	
	2 p. m. 3 p. m.	W. SW.	3. 2	4. 21	29. 188	71		2 p. m. 3 p. m.	South	3.6	4.11	29,090	١.
15	9 p. m. 3 a. m.	W. SW. West		4. 21 4. 32	29. 201	57	27	9 p. m. 3 a. m.	8. SW 8. SW	2.7	4.07 4.22		١.
	7 a. m. 9 a. m.	W.NW.		4.37	29. 342	54		7 a. m. 9 a. m.	8W	2.9	4. 26	28.988	٠.
	2 p. m. 3 p. m.	W. NW.	2.4	4. 39	29. 376	67		2 p. m. 3 p. m.	NW	116	4. 23	29. 151	
	9 p. m.	w.sw.	0.3	4. 34	29. 406	58	l	9 p. m.	Calm	0.0	4. 31	29, 379	

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

		WINI).	apove uge.	Inced	red.			WINI).	above uge.	nced	1
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gange.	Barometer reduced to 32°,	Therm'r detached.	Date,	Hour of day.	Course,	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 32°.	
1864. ept. 28	3 a. m.	n. nw .	1. 2	4. 33			1864. Oct. 10	3 a. m.	w.sw.	1, 4	4. 60		
	7 a. m. 9 a. m. 2 p. m.	N. NE	1.0	4. 38	29. 582 29. 557	43 54		7 a. m. 9 a. m. 2 p. m.	N. NW	1. 2	4. 54	29. 121 29. 270	
•	3 p. m. 9 p. m.	E. SE N. NE		4. 28 4. 12	29, 495	51		3 p. m. 9 p. m.	Calm	0.0	4. 54 4. 40 4. 45	29. 450	
29	3 a. m. 7 a. m. 9 a. m.	N. NE	2.3	3. 99 4. 02	29. 320	49	11	3 a.m. 7 a.m. 9 a.m.	South		4. 36	25. 535	
	2 p. m. 3 p. m.	N. NE	1.8	3. 99				2 p. m. 3 p. m.	SE	0.8	4. 38	29. 470	
30	9 p. m. 3 a. m. 7 a. m.	Calm	0.0	4. 10 4. 17	29. 475 29. 557	50 47	12	9 p. m. 3 a. m. 7 a. m.	8. SW W. NW.		4. 33 4. 38	29, 390	. -
	9 a. m. 2 p. m.	NE		4. 18	29. 550	50		9 a.m. 2 p. m.	N. NW	1.9		29, 445	ŀ
et. 1	3 p. m. 9 p. m. 3 a. m.	E. SE East E. SE	0.8	4. 22 4. 09 4. 11	29. 507	50	13	3 p. m. 9 p. m. 3 a. m.	Calm	1.4 0.0 0.0	4. 33 5. 38 4. 51	29. 503	1
-	7 a.m. 9 a.m.	E. 8E		4. 11	29. 432	50		7 a.m. 9 a.m.	w. sw		4. 52	29. 488	
	2 p. m. 3 p. m. 9 p. m.	NE	0. 6 0. 0	4. 06 4. 06	29, 380 29, 375	56 		2 p. m. 3 p. m. 9 p. m.	8W	1. 0 0. 0	4. 59 4. 44	29. 329 29. 255	
2	3 a.m. 7 a.m.	Calm	0.0	4. 11	29. 447	42	14	3 a. m. 7 a. m.	8W	0, 3	4. 36	29. 149	1
	9 a. m. 2 p. m. 3 p. m.	SE	1.2	4. 24	29. 452	56		9 a. m. 2 p. m. 3 p. m.	8W	0.4	4. 18	29. 044	
3	9 p. m. 3 a. m.	Calm	0. 0 0. 0	4. 19	29. 487	54	15	9 p. m. 3 a. m.	Calm		4.00	29. 061	
,	7 a. m. 9 a. m. 2 p. m.	N. NE	0. 5	4. 25	29. 492 25. 527	51 55		7 a. m. 9 a. m. 2 p. m.	NW	2.0	4. 17	29. 079 29. 144	
	3 p. m. 9 p. m.	N. NE	0. 7 0. 0 0. 3	4. 18	29. 560	51	16	3 p. m. 9 p. m.	W. NW. W. NW. W. SW.	2.4 0.4 0.2	4. 28	29. 265	1
4	3 a.m. 7 a.m. 9 a.m.	N. NE	2.2	4. 17	29. 592	53	10	3 a. m. 7 a. m. 9 a. m.	w. sw.	0.4		29. 308	
	2 p. m. 3 p. m.	N. NE.	2. 8 1. 4	4. 14	29. 537 29. 422	57 		2 p. m. 3 p. m. 9 p. m.	W. NW.	1. 2 0. 0		29. 293 29. 361	1
5	9 p. m. 3 a. m. 7 a. m:	SW	0.8		29. 334	51	17	9 p. m. 3 a. m. 7 a. m.	Calm	0.0	4. 59	29. 423	1
	9 a. m. 2 p. m. 3 p. m.	W. SW.		4. 24	29. 294	55		9 a.m. 2 p.m. 3 p.m.	Calm	0.0	4. 60	29. 330	1
6	9 p. m. 3 a. m.	Calm South	0.0	4, 18 4, 17	29. 179	51	18	9 p. m. 3 a. m.	N. NE W. NW. W. NW.	0.8		29, 373	
	7 a. m. 9 a. m. 2 p. m.	s. sw	1.8	4. 34	28. 979 28. 933	49 54		7 a. m. 9 a. m. 2 p. m.	West	1.8	4. 52	29. 374 29. 296	1
_	3 p. m. 9 p. m.	W. 8W.	2.9 4.0	4. 29 4. 34	29. 078	50		3 p. m. 9 p. m.	W. 8W	0. 2	4, 50 4, 55	29. 273	ŀ
7	3 a.m. 7 a.m. 9 a.m.	8W W. NW.	0. 5 3. 3	4. 42	29. 021	51	19	3 a.m. 7 a.m. 9 a.m.	W. NW.		4. 51 4. 48	29, 221	1
	2 p. m. 3 p. m.	w. nw.	4.5	4. 37	29. 141	47		2 p. m. 3 p. m.	E. 8E	0.4	4. 37	29. 218	1
8	9 p. m. 3 a. m. 7 a. m.	NW N. NW.		4. 25 4. 19	29. 305 29. 487	38	20	9 p. m. 3 a. m. 7 a. m.		0.0		29. 315 29. 373	1
	9 a.m. 2 p.m.	N. NW .	2.4	4. 10	29. 479	44		9 a.m. 2 p.m.	N. NE	1.0	· · · · · ·	29. 421	1
9	3 p. m. 9 p. m. 3 a. m.	NW Calm Calm	0.0	4. 30 4. 23 4. 34	29. 494	34	21	3 p. m. 9 p. m. 3 a. m.	N. NE NE	0.7		29. 431	1
	7 a. m. 9 a. m.	8. 8W	1.5	4. 42	29. 429	32 51		7 a. m. 9 a. m.	E. NE	0.6	· • • • • • • • • • • • • • • • • • • •	29, 381	1
	2 p. m. 3 p. m.	8. 8W	3. 4	4. 68	29. 189	51		2 p. m. 3 p. m.		0.4	4.34	29. 353	

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

	1	WIND	N.	above	duced	hed.			WIND		above	reduced	. Post
Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer reduced to 32°.	Therm'r detuched.	Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer re-	Thorna's detached.
1864. Oct. 22	3 a. m.	West	1.3	4.50			1864. Nov. 3	3 a. m.	Calm	0.0	4. 62		1
	7 a. m. 9 a. m.	********		4. 55	29. 381	39	Nov. 3	7 a. m. 9 a. m.	North		4.71	29, 368	
	2 p. m.	W OW			29. 328	50		2 p. m.				29, 174	
-	3 p. m. 9 p. m.	W.SW.	1.5	4. 65 4. 65	29. 248	46		3 p. m. 9 p. m.	NE	1.3	4.65	29. 017	
23	3 a. m.	sw		4. 62	29. 091	41	4	3 a. m.	N. NW	0.6	4.56	28.915	e e
	9 a. m. 2 p. m.	West	0. 6	4. 47	29. 006	53	(1)	9 a. m. 2 p. m.	N. NW.	3. 7	4. 42	28, 961	
	3 p. m. 9 p. m.	N. NE	0.8	4.35	29. 132	47		3 p. m.	NW Calm	2.1	4.71		
24	3 a. m.	NW	1.6				5	9 p. m. 3 a. m.	Calm		4.79	29, 105	
	7 a. m. 9 a. m.	NE	0.6	4. 26	29, 318	35		7 a. m. 9 a. m.	8. SE	1.1	4.87	29. 207	
	2 p. m. 3 p. m.	East	0.1	4. 35	29. 326	47		2 p. m. 3 p. m.	8. SE	1.5	4.91	29, 160	
25	9 p. m. 3 a. m.	Calm	0.0	4. 39	29. 361	39	6	9 p. m. 3 a. m.	S. SE	2.2	4, 95	29. 096	
~~	7 a. m.				29. 383	33		7 a. m.		2270		28, 905	
	9 a. m. 2 p. m.	s.sw			29. 355	51		9 a. m. 2 p. m.	S. SE		4. 79	28.767	1
	3 p. m. 9 p. m.	E. SE	0.7	4. 56	29. 345	50		3 p. m. 9 p. m.	8W	1.6	4.78	29. 191	i
26	3 a. m. 7 a. m.	E. SE	1.1		29, 203	50	7	3 a. m. 7 a. m.	8.8W	0, 2	4.93	29. 233	-
	9 a. m. 2 p. m.	E. SE	1.5		29, 101			9 a. m.	8.8W.	1.1	4.97	29. 315	0 .
	3 p. m.	E. SE	1.1	4. 58		51		2 p. m. 3 p. m.	8. SW		5.05		
27	9 p. m. 3 a. m.	SE	1.7		28. 956	51	8	9 p. m. 3 a. m.	N. NE.	0.3	5. 00 4. 85	29, 456	
	7 a. m. 9 a. m.	8.8W.	2.2	4.54	28, 836	49		7 a. m. 9 a. m.	NE	1	4.74	29. 343	
	2 p. m. 3 p. m.	sw	2.0		28. 841	47		2 p. m.		4.1		29, 184	4
	9 p. m.	8.8W	1. 2	4. 59	28.849	44		3 p. m. 9 p. m.	N. NE N. NW.	2.9	4.50	29, 102	2
28	3 a. m.	sw	1. 2		28. 874	44	9	3 a. m. 7 a. m.	North	3.9	4. 39	28, 927	7)
	9 a. m. 2 p. m.	w. sw.	2.0	4. 76	28. 971	46		9 a. m. 2 p. m.	N. NE	5.3	4. 25	28, 344	
	3 p. m. 9 p. m.	W.SW.	1.5		29,076	44	10 00	3 p. m.	8.8W W.SW.		4. 66	28. 607	
29	3 a. m.	W. NW.		4.85			10	9 p. m. 3 a. m.	W. SW.	5.8 4.6	5. 03 4. 89		
	7 a. m. 9 a. m.	W.NW.	0, 1	4. 82	29. 205	44		7 a. m.	w.sw.	3.7	4. 85	29,000	
	2 p. m. 3 p. m.	West	0.3	4.81	29, 245	47		2 p. m. 3 p. m.	w.sw.	3. 1	5. 18	29. 072	
30	9 p. m. 3 a. m.	Calm	0.0		29, 343	45	11	9 p. m. 3 a. m.	West W.SW.	2.4	5. 17 5. 11	29, 155	ř.
00	7 IL ID.				29. 428	42	**	7 a. m.				29, 107	
	9 a. m. 2 p. m.	Calm		4. 80	29, 465	47		9 a. m.	West		5. 04	29. 077	
	3 p. m. 9 p. m.	Calm	0.3	4.85	29. 518	44		3 p. m. 9 p. m.	N. NW.	1.1	4.99	29, 147	
31	3 a. m. 7 a. m.	Calm	0.0	4.89	29. 595	41	12	3 a. m. 7 a. m.	NW	1.0		29, 305	1
	9 a. m.	Calm	0.0	4. 89				9 a. m.	NW	2.4			Al.
	2 p. m. 3 p. m.	N. NE	0, 7	4.83	29, 628	45		2 p. m. 3 p. m.	NW	9.2		29, 361	
Nov. 1	9 p. m. 3 a. m.	Calm	0.0	4.84	29. 696	41	13	9 p. m. 3 a. m.	W.NW.	0.9		29, 480	110
	7 a. m. 9 a. m.	Calm		4.72	29, 757	37		7 a. m. 9 a. m.	NW			29, 540	
	2 p. m.				29. 731	44	1	2 p. m.				29, 552	T.
- 2	3 p. m. 9 p. m.	NE	0.0	4. 69 4. 67	29.711	41	11	3 p. m. 9 p. m.	Calm				2
2	3 a. m.	NW	0, 1	4. 61	29, 666	36	14	3 a. m.	Calm	0.0		29, 567	1
	9 a. m. 2 p. m.	NE	0.6	4. 65	29, 561	46		9 a. m. 2 p. m.	8E	1.8		29, 414	
	3 p. m. 9 p. m.	NE		4. 62	23, 301	40		3 p. m.	E. SE	2.8		29, 359	

TABLE AA.—Showing the wind, water, barometer, &c.—Continued.

			WINI).	above age.	reduced.	jed.			WIND		above	reduced	pod
Date	e.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer rec to 32°.	Therm'r detached.	Date.	Hour of day.	Course.	Relative ve-	Heig't of water above zero of tide-gauge.	Barometer re to 32º.	Therm'r detached
1864 Nov.	i. 15	3 a. m.	NW	0. 9		29, 457	26	1864. Nov. 27	3 a. m.	Calm	0.0		29. 221	31
		9 a. m.	W.NW.	0. 6		29. 54G	37		9 a. m. 2 p. m. 3 p. m.	8₩	0.9		29. 097	4
		3 p. m. 9 p. m.	W.8W.	0. 4 0. 0		29. 591	30		9 p. m.	8W	1.5 0.5		29. 044	4
	16	3 a. m. 7 a. m.	8.8W	0.4		29. 582	32	28	3 a. m. 7 a. m.	Calm	0.0		28. 947	4
		9 a. m. 2 p. m.	8.8W	1.1		29. 520	39		9 a. m. 2 p. m.	Calm	0.0		28. 896	5
		3 p. m. 9 p. m.	8W	0.3	4. 43	29. 469	36	29	3 p. m. 9 p. m.	Calm Calm Calm	0.0		28. 871	4
	17	3 a. m. 7 a. m.	W.NW.	0.1	4.38	29. 474	33	209	3 a. m. 7 a. m. 9 a. m.	sw	1.3		28. 804	5
		9 a. m. 2 p. m.	W.NW.	1. 2	4: 49	29. 516	34		2 p. m. 3 p. m.	8W	1.8		28. 876	5
		3 p. m. 9 p. m. 3 a. m.	W.NW. W.NW. 8W	1.0	4. 69 4. 76	29. 639	31	30	9 p. m. 3 a. m.	W.8W	5.4		29.056	4
	18	7 a. m.	w.sw	1.7	4.88	29. 634	25	"	7 a. m. 9 a. m.	W.8W .	0.3		29. 445	3
		2 p. m. 3 p. m.	w.sw	1.7	4. 85	29. 606	37		2 p. m.	w.sw.	0.7	· • • • • • • • • • • • • • • • • • • •	29. 479	5
	19	9 p. m.	West Calm	0.2	4. 90 4. 83	29. 607	32	Dec. 1	9 p. m. 3 a. m.	Calm	0.0		29. 528	4
	10	7 a. m. 9 a. m.	8.8W	0. 5		29. 502	96		7 a. m. 9 a. m.	E.8E	0. 1		29. 545	4
		2 p. m. 3 p. m.	1	0.6	4.70	29. 299	41		2 p. m. 3 p. m.	E. 8E	0. 9		29, 393	4
	20	9 p. m. 3 a. m.	8.8W 8.8W 8.8W	0.7	4. 70 4. 69	29. 200	36	2	9 p. m. 3 a. m.	E.SE	1.0 1.1		29. 333	1
		7 a. m. 9 a. m.	Calm	0.0	4. 54	29. 052	33		7 a. m. 9 a. m.	8E	0.1		29, 104 28, 956	1.4
		2 p. m. 3 p. m.	w.sw.	1.9	4. 56	29. 957	37		2 p. m. 3 p. m. 9 p. m.	Galm West	0.0		28, 966	1
	9 1	9 p. m. 3 a. m.	W.SW.	4.0 3.7	4. 66 4. 47	29.027	26	3	9 p. m. 3 a. m. 7 a. m.	West	1.4		29, 102	
		7 a. m. 9 a. m.	w.ww	4. 0	4. 55	29. 220	17 23		9 a. m.	West	2.0		29. 179	
		2 p. m. 3 p. m.	W.NW.	3. 5 3. 5	4. 61	29, 263 29, 285	15		9 p. m. 3 p. m. 9 p. m.	West	2.5 1.5		29. 287	
	22	9 p. m. 3 a. m.	W.NW.	3.5	4. 61 4. 58	29, 276	12	4	3 a. m. 7 a. m.	W.NW.	0.6		29. 342	
		7 a. m. 9 a. m.	W.NW.	4. 2	4. 53	29. 341	16		9 a. m. 2 p. m.	Calm	0.0		29, 270	
		9 p. m. 3 p. m.	West	3.8	4 47	29. 441	9		3 p. m. 9 p. m.	Calm E. SE	0.0		29. 130	3
	23	9 p. m. 3 a. m. 7 a. m.	W.NW.	0. 2	4. 44 4. 57		7	5	3 a. m. 7 a. m.	E.8E	0.9		28. 970	3
		9 a. m.	South	0.4	4. 58	29. 448	29		9 a. m.	Calm	0.0		29.067	3
		2 p. m. 3 p. m. 9 p. m.	8.8E 8.8E	0.4 0.1		29. 483	29		3 p. m. 9 p. m.	W.NW.	0.0		29. 100	3
	94	3 a. m.	Calm	a o		29.600	29	6	3 a. m. 7 a. m.	Calm	0.0	•••••	29. 107	9
		9 a. m. 2 p. m.	w.sw	0, 1		29. 642	36		9 a. m. 2 p. m.	West	0.8		29. 137	. 8
		3 p. m. 9 p. m.	Calm	0.0		29. 697	29		3 p. m. 9 p. m.	Calm	0.0		29, 147	8
	25	3 a. m.	Calm	0.0		29. 610	32	7	3 a. m. 7 a. m.	NW	0.1		29. 078	i
		9 a. m.	8E	0.4		29. 394	40		9 a. m. 2 p. m.	W.NW.	1.8		29. 123	i
		3 p. m.	8E	1.5 1.0		29. 279	42	_	3 p. m. 9 p. m.	West W. SW.	2.9 3.7 3.9		29. 301	
	26	3 a. m. 7 a. m.	8W	0.2		29. 226	37	8	3 a. m.	West	2.5		29. 664	i
		9 a. m. 2 p. m.	W.8W.	1.1		29. 289	39		9 a. m. 2 p. m.	West			29, 836	•••
		3 p. m. 9 p. m.	W.SW.	0.3		29, 316	38		3 p. m. 9 p. m.	W.SW.	0. 9 0. 0		29. 918	i

TABLE AA .- Showing the wind, water, barometer, &c .- Continued.

		WIND	•	erabove gauge.	Inced	je g				WIND		above	laced	Ď.
Date.	Hour of day.	Course.	Relative ve- locity.	Heig't of water above zero of tide-gauge.	Barometer reduced to 32°.	Therm's detached.	Dat	e.	Hour of day.	Course.	Relative ve-	Helg't of water above zoro of tide-guuga.	Barometer reduced to 32°.	Therm'r detached.
1864.							186	4						1
Dec. 9	3 a. m.	Calm	0.0		29. 970	12	Dec.	290	2 p. m. 3 p. m. 9 p. m. 3 a. m.	8W	3.0 1.3 1.6		29. 233	
	9 a. m. 2 p. m.	Calm	0.0		29.779	6		21	9 p. m. 3 a. m. 7 a. m. 9 a. m.	N. NW	1. 6		29. 100	
	2 p. m. 3 p. m. 9 p. m. 3 a. m.	E.SE	0.3 0.4		29, 528	22			7 a. m.	N. NW	5.4	- 	26.983	9
10	3 a. m. 7 a. m.	Calm	0.0		29, 407	19			2 p. m.	l			29, 104	1
	9 a. m.	Calm	0.0		l. 				9 p. m.	N. NW .	4. 0 4. 0 3. 4		29. 305	1
	2 p. m. 3 p. m.	Calm	0.0	· · · · · ·	29. 951	23	ļ	22	7 a. m.	NW			29. 610)
11	3 p. m. 9 p. m. 3 a. m.	8W W.8W.	0.2	••••	29. 276	20			9 a. m. 2 p. m.	W.NW.	2.1		29.673	
**	7 a. m.			•••••	29.073	4			3 p. m.	W. SW W. SW	1.9 0.1 0.1			
	9 a. m. 2 p. m. 3 p. m.	South	1.1		29, 149	19		23	9 p. m. 3 s. m. 7 s. m.	8. 8W	Q I		29.720	
	3 p. m. 9 p. m.	NW West	3. 5 1. 1		29. 479	5			9 a. m.	8. 8W	i. 7		29. 507	
12	9 p. m. 3 a. m. 7 a. m.	W.NW.	0. 5		29. 625	7			2 p. m. 3 p. m.	!	l [.]		29. 172	!
	9 a. m.	W.NW.	0. i			l l			9 p. m.	8W W.8W. W.NW.	1.5			
	2 p. m. 3 p. m.	Calm	0.0		29. 635	8		24	9 p. m. 3 a. m. 7 a. m.	W.NW.	5.3		29. 356	
13	9 p. m. 3 a. m.	Calm 8. SE	0. 0 0. 6	•••••	29. 504	8			9 a. m.	8.8W	1. 5		29. 330	٠.
10	7 a. m.				29. 214	5			3 b. m.	8. SE	1.9			
	9 a. m. 2 p. m. 3 p. m.	8. SE	0.4		29. 041	91		25	9 p. m. 3 a. m.	South 8. 8E	2.8 2.5		29. 290	
	3 p. m. 9 p. m.	Calm W.NW. W.NW.	0.0			23	•		7 a. m. 9 a. m.	8.8W	2.2		29. 147	' .
14	3 a. m.	w.nw.	0.3						2 p. m.	W. 8W.			29.055	
	9 a. m.	Calm	o.o		29. 393	2			3 p. m. 9 p. m.	8W	1.3 0.8 1.3		29. 097	
	2 p. m. 3 p. m.	Calm	0.0	•••••	29. 549	9		26	9 p. m. 3 a. m. 7 a. m.	South	1. 3		29.060	'n,
15	9 p. m.	Calm	0.0		29. 603	2			9 a. m.	8. SE	2.8		28.897	
13	7 a. m.	Calm	0, 0		28. 401	17	ĺ		3 p. m.	8. SE 8. SE	3. 0 1. 7		l	
	2 p. m.	Calm	0.0		29, 240	28		27	9 p. m. 3 a. m.	8. SE	1.7 1.6		28.772	
	3 p. m.	W. 8W.	0. 3 0. 0		29, 380	13		-	7 a. m. 9 a. m.	w.sw.	3.7		98. 552	
16	3 a. m.	Calm	0.0						2 p. m.				28.647	
	7 a. m. 9 a. m.	Calm	0.0		29.400	12		ı	3 p. m. 9 p. m.	W.8W. W.8W. W.8W.	3.5 3.0 2.3		28. 839	
	2 p. m. 3 p. m.	l	Ö. 0		29. 273	33	}	28	3 a. m. 7 a. m.	w.sw.	2.3	• • • • • •	19 . 050	
	9 p. m. 3 a. m.	Calm SW	1.1		29. 290	31			9 a, m.	W.NW.	2.3			٠.
17	7 a. m.	w.sw.			29. 472	30			2 p. m. 3 p. m.	NW	2.5		29. 128	. •
	9 a. m. 2 p. m.	w.sw.	1.6	•••••	29. 630	33		29	9 p.m. 3 a.m.	NW W.NW. W.NW.	1.6	••••	29. 251	
	3 p. m.	NW E.NE	0.2					_	7 a. m.				29. 166	
18	3 a. m.	E.SE	1. 1 1. 5		29. 734	34			9 a. m. 2 p. m. 3 p. m.	8.8W	0.3	· • • • • • • • • • • • • • • • • • • •	23, 968	Ţ
	7 a. m.	SE	1.8		29. 569	33		- 1	9 D. m.	8. 8E 8W	1.5 1.5		28.841	•
	2 p. m.		0.6		29. 396	36		30	3 a. m.	w. 8w.	21		98. 901	
	9 p. m.	8.8W W.8W. W.NW.	1.6		29. 247	34			9 a. m.	W.NW.	3.8		. 	
19	3 a. m.	w.nw	4. 1		29, 312	15			2 p. m. 3 p. m.	W.NW.	3.1		29. 141	
	9 a. m.	West	3.4		29, 462	15		31	9 p. m.	W.NW. W.NW. W.NW.	1.7		29. 316	į.
	3 p. m.	West	3.6					31	7 a. m.				29. 531	,-•
20	9 p. m. 3 a. m.	W.8W .	3.6 1.9 1.2		29. 480	10		- 1	9 a. m.	W.NW.	1.3	·	29. 581	i
	7 a. m.		1.6		29. 401	4		- 1	3 p. m.	W.NW. W.NW.	1.8		29. 646	٠.,

Table B. B.—Showing the duration of the wind at the several points of the compans for each hour, the stormy and cloudy days being eliminated, for the following months, at Milwaukee, Wisconsin.

Points.	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	101	ıı
JUNE, 1861.	-			_						_			_		-	-		_		1	1	+	†
North N. NE	2	1 2	1 2	 2	3	2 3	1 3 1	1 4	1 4	3		 1	 1	 1	 1	1	1 2 1	3		1 2	1	2	. 2
North N. NE N. NE E. NE E. SE SE				· · · · · ·	••••	 i	 1	3 1 1	 2	1 1 3	3 1 3	1	3	3	1			٠.	2				
60. OED				· · · ·	· • • • • • • • • • • • • • • • • • • •		· • • • • • • • • • • • • • • • • • • •	i	4	3	3 4 1	3 4	5 3 1	7	1 3 5	2 7	1 1 6		3	1 1	1	i	i.
South S. 8W &W	 i	i	·••• ••••	···;	···i	 i	 1	 i	:::: ::;:	2	1 1	1 1	i	 1	1 1 1	i	1 2	1	1 2	2	1	1 1	1 2
W. 8W West	2	3 1	2	1 2	· i	i	i	• • • • • • • • • • • • • • • • • • •		••••			••••		1 3	1	22	1	::				1
N. NW		1		1	3	4	2	1				:			::	::	-:	-:	\exists		-	-⊦	i :
Sums	10	10	8	<u>9</u>	10	12	12	14	16	16	16	16	16	16	16	=	=	=	=	3	7	7	9 1
Calms	6	6	8	-7 	6	4	3	2	_			_	_	0	0	0	0	0	3	8	9	9	7
JULY, 1861.																							
North N. NE NE		· · · · ·		1	i	ï	2 1	22	2 2	1 2	1 1 2			····i	::		••	·· ·i	i				1
E. NE East E. SE	i		···i·			 	 i	i	1 	 1 2		 1 2		 1 9	2 4 6	2 3) 1	2	i i	:: :	
E. SE							 i		<u>-</u> -	1	3 1 1	4	3 1	2 4 2 1	6		 3 2 5 2	8	6 2	1 2	ا:٠		
S. SW SW W. SW West	i	 2 2	1 1 1	2 1	 2 1	1	1 3 1	1 2 1	1 3 1	1 4	1 2 1	3 2 1	1 5	 3	i	i	2	١	2	1		2	i i
West W. NW NW	1 1	 i	2 1			1 1 	i 	i	 2 1	2 1	i 				i								i
N. NW	1		1	ī	j	1	<u></u>	i							<u></u>	::	Ŀ	<u></u>	1	<u>::</u>	-	1	1
8ums	7	5	8	7	7	9	13	12	15	15	15	15	15	15	14	=	=	=	14	8	8	6	8
Calms	7		6	7	7	6	1	<u> </u>	<u> </u>	_	Ľ		Ë	Ľ	Ľ	=	=	=	Ľ	=	4	-	=
AUGUST, 1861.	İ	ŀ				l										١.					1	1	-
North N. NE NE	1	3	3	2	1	2	2	1 2	3	 4 1	1 3 1	4	 i	3		1		1	١١	1	1 1 2	2 1 1	1
E. NE East E. SE					 	:::: :::::		1	 	 1	1 	 1 3	2	6	1 2 4 5 1	2 8	1 				2	i	i
SE	i				 i				ī	2	3	4		5		8	1		2 2 4 1	200			:: :
8. 8W	1 2	2 3	1 3 2	1 2 1	1 1	1		1 1 1	2	3		1 2	3	2	3	i	1	 2 1	 2		1	1 2	: -
W.8W West W. NW	2 1	3	1 1	3	2	2 2 2	2 2 3	1 3 3	2	3 1 1	2 1 1	1 1 1	 1 1	1	1 	1	1 1	١	i	ij		i	1
NW	1 1	<u>i</u> .	1 1	 1 1	1 1 2	1 1	2	2 1	3 1 1	1	2	i	i	ï	:: ::	:: ::	.	: . : .	-:	::			:: :
Sums	11	11	14	11	11	14	15	17	18	19	19	19	19	19	19	19	19	18	16	12	14	9	7
Calms	8	8	5	7	7	3	4	1	0	0	0	0	0	0	0	0	0	1	3	7	5	10	12

TABLE B B .- Showing the duration of the wind, &c .- Continued.

Points.	1	2	3	4	5	6	7	8	9	10	11	19	1	2	3	4	5	6	7	8 9	1	0 11
JUNR, JULY, AND AUGUST, 1861.															Ī	-			Ì	†		
North N. NE	3	2	4 3	1 4	2	4 6	2 6	5 8 3	4	7	2 6	2		;·	ŀi	1	13	4	1	9	3	5 4 4 4
NE			·			.		3	5	7	7	9	6	i	3	' З	2	20	11	Ŀ	2	ì,
E. NE East	···;·				1	i	2	2 2	1 3	1	i'i'	1 3	2	i	2	2	2	2		9:	3	1 1
e. se			i			ī	1	. 1	4	1 7 6	7	3 8	13	12	12	5	6	2	Š	78	ĭ.	
3E 		·		.		.	·	1	5		10	111	11	17	16	24	18	21	11	5	2,	
l SE	1		•		i	• • • • •	i		i	1	1	1	2	1	1	٠-	3	3	3	3	5	' 1
.8W	2	ï	2	2	i	i	l î	2	2	2	7	4	î	î	2	2	2	2	4	2	2	2.
w	4	4	5	4	3	3	6	4	1 4	8	7	7	9	5	4	3	4	3	3	3	4	5,4
V. 8W	7 3	7	5	3 2 2	3	3	3	2 3	4	3	2	2	<u>:</u> -	2	2	3	2	2	!	-:	1	1 2
Vest V. NW	1	9	3	2	3	3	1	3	5 2	2	i	li	1	1	i	1	'n		ı	- 1		1 1
W	2	2	3	6	5	6	5	3	2	2	3	î	i	ï			ļ. <u>-</u>				٠.,	1
W и	2	ĩ	2	3	2	2	4	2	ĩ	1		• • • •				• •			1		••!	11
Sums	28	26	30	27	28	35	40	43	49	50	50	50	50	50	49	50	49	48				24 2
Calms	21	23	19	21	20	13	8	6	0	0	0	0	0	0	0	0	0	<u> </u>	7	21	20:	37 = =
JUNE, 1862.			١.			•											١.				1	
orth	. .						- -		ļ		ļ					١	١	ا…ا				
. NE	2	4	2	2	2	2	6	5	5	4	3	2	2	2	3	3	4	3	3	3	3	2 5
E	 						1	1	··i	1	1	2	i	2	3	2	×	1	I		٠-,	٠٠,٠٠
ast								l . .					2				1				•••	
ast								1	1	1	2	3	2	3	3	3	3	1 1	1		1	1
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outh	• • • •	••••		••••			ļ		· · · ·		••••			ï		١٠٠	::				••	• • • •
8W												ï	1	1	2	ï	١					
W		:-		1		·· <u>·</u> ·			·- <u>-</u> -	····	1	1	1	1		1	3	9	1	1	1	1 1
7.8W	1	1	2		1	1	2	3	3	4	8	2	2	··i·	i	• •	. 5	-:	1	1	ŀ	1.5
'. NW	·i	l . .					l i		i	ï		i.	i		i	i		ı	ì	1		
w	••••			1		1		l	1	1		1	1	2	1	1						1.
. NW	••••		• • • •				1	1	1		••••			• • • •	$ \cdot $	• •	••		1		1.	I
Sums	4	6	4	4	3	5	12	14	13	13	13	14	14	14	14	14	13	10	=	6	=':	6 7
Calms	10	8	10	10	10	9	2	0	1	0	0	0		0	0	0 =	1	4	7 =,	8	<u>و</u>	ê. ===
JULY, 1869.																	l				1	
orth				:			٠٠		2	1 2		1		٠٠	٠.:	٠.	٠.	اينا	اي-		ا	
NE	ĭ	1	1	1		••••	2	i	3	1	×	1	*	*	*	1		1	ျ	2	Ĭ.	
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18t	··i		• • • •				2		<u>-</u>		8	· · <u>· ·</u> ·		•••		٠:				-	-:,	1
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TABLE B B .- Showing the duration of the wind, &c .- Continued.

Points.	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	1 12
AUGUST, 1862.																				П		Ī	
North N. NE NE NE E. NE E. SE SE South S. SW SW West W. NW NW N. N. NW	1 1 1 1 2 2 2	1 2 1 3 3 3 2 1 1	1 1 1 3 2 1	1 1 2 3	2 2 1	2 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1 1 2 2 2 3 2 2 1	1 1 3 3 1 3	31 4 1 2 1 1 1 1	1 2 1 1 5 2 4	2 2 1 6 2 1	9 1 2 6	1 2 2 1 5 3 2 2 1 1	1 3 2 2 5 1 1 3	32 171 33	1 3 5	1 2 1 2 3 1 1 1 5 2 2	 4 1 1	 2 1 1 1 	:	 1 3	2	3 2 3 3 1 1 2 2 1 1 1 1
Sums	13	15	12	11	10	12	18	20	21	21	21	21	21	22	22	21	20	17	10	9	13	161	15 12
Calms	9	7	10	11	12	9	4	2	== 1	0	0	0	0	_	0	1	2	= 5	11	= 12	9	6	7 10
JUNE, JULY, AND AUGUST, 1862.		-		_						_							_		=	_		= =	
North N. NE NE E. NE E. NE E. SE SE SSE SSE SOuth S. SW SW W. SW West W. NW	5 2 1 1 3 2 6	2 1 1 5 3 2 4	1 4 1 1 1 4 4	5 1 2 4 3 1 5	2 1 1 2 1 5	1 1 2 2 3 5 3 2	10 4 1 3 1 1 2 6 6 3 3 5	29 33 14 1 25 7 25 5	4 11 2 3 5 1 6 6 3	9835 47 36523	65426811273253	1 5 3 3 10 3 8 3 7	5 2 1 4 5 14 3 6 4 2 6	5 5 7 14 2 6 2 1 6	4 367 1424 512			4 1 77 22 72 24	63 1143 343			2	2 5 6 1 1 1 1 1 1 5 4 4 3 7 5 4 2 1 4 3 2
W. NW NW N. NW	3	2	5 1	3	3	7	3 5 4	5 5	5 3 3	8	3	6	3	4	3	1 2	2		4	1	2 2		3 2
NW	1 3	2	5 1 26	3	3	7	3 5 4	5 5 5 55	3 3 54	57	3 2 57	6	3		3	1 2	-	=		1 	 	_!_	3 2 37 31
N. N.W	3	1	5 1			7	4	5	3	8	8	6 1	3	1	3	1 2	56 =	 49 =-	 36	1 28	33	34 3	
Sums Calms JUNE, 1663.	3 1 30	32	26 26	25	3 21	30	49	5 55 4	3 54 2 2	57	2 57	6 1 58	3 2 58	4 1 59	3 2 59	1 2 58	56 =	 49 =-	 36	1 28	33	34 3 25 2	37 31
Sums Calms JUNE, 1863. North N. NE NE E. NE East E. SE SE SOUTH SUM SUM West W. NW N. NW N. NW	30 29 3 1	2 1 32 27 4 1 3	2 1 1	25 34 25 1 1	37 37 1	30 28 31 2 1 1	1 4 2 1 3 2	5 55 4 2 1 3 2 1 1 1 	3 54 9 1 2 3 9 4 3 	1 1 3 1 6 4 1 2 1	2 57 0 1 1 2 4 8 1 1 1	58 0 	3 9 58 0 	1 59 0 2 3 10 	3 2 59: 0 	1 2 58 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		 49 = 10 = 2 2 2 7 1 1 1 1 1		1 28 = 300 = 1 1 1 1	33 26 26 1 2 1 1 2	1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	2 2 2 3
Sums Calms JUNE, 1663. North N. NE NE E. NE E. SE SE SOUTH SUM SUM SUM SUM SUM SUM SUM SUM SUM SUM	3 1 30 29 3 1 1 3 3	2 1 32 27 	2 1 1 	25 34 25 1	37 37 1	30 28 31 2 1 	1 4 2 1 3 1 2	5 55 4 2 1 3 2 1 1 1 	3 54 2 3 2 3 2 3 2 4 3 1	1 1 3 1 6 4 1	2 57 0 	58 0 	3 3 58 0 0 1 2 3 10 1	1 59 0 2 3 10 	3 2 59 0 = 1 2 1 1 1 1	1 2 58 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				1 28 2 2 2 1	333 26 26 1 2 1	34 3 25 9 1 2 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	2 2 2 2 2 1 1

TABLE B B .- Showing the duration of the wind, &c .- Continued.

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W. NW IW	2	3		2	1	i	3	1	2	••••	1	i	2	8		2	7	i		::	ίį	i i
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AUGUST, 1863.																	_				1	
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Sums	19	18	14	18	20	18	23	22	24	22	94	24	24	24	24	24	24	24	21	18	17	17 18 1
Calms	5	6	10	6	4	6	1	2	0	2	0	0	0	0	0	0	0	0	3	6	7	7 6
UNE, JULY, AND AUGUST, 1863.																					1	i
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ast L SE E SE outh . SW	 2 1 9	1 3 7	1 2 5	1 2	1 3 1 4	2 2	 3 4	6 2 1 4	10	12 2 5	19 2 	25 1 2 4	25 2 1	24 4 3 3	4 20 5	12 12	13	7	3	4	2 3 2 1 5	4 3 1 4 . 1 4 1 4 8
Cast 2. SE 11E 3. SE 4. SE 4. SW 5. SW 7. SW 7. SW 8. SW 8. SW 9.	2 1 9 5	ï	1 2 5	1 2 1 2 4	1 3 	22 21 62	3 4 3	6 2 1 4	10 20 20 20 7 20	12 2 5 6	19 2 1 4 4	25 1 2 4 4 4	25 2 1 1 6	24 4 3 3 1	4 22 5 .0 3 1	12	19	7	3	3 4	2321552	4 3 1 4 . 1 4 1 4 2 3 5 1
Last	2 1 9 5 1	3 7 4 1	1 2 5 4 2	1 2 1 2 4 4 2 5	1 3 1 4 6	2 2 1 6 2 3	3 4 3 7	6 2 1 4 2 7 4 4	10 22 22 27 22 5	12 2 5 6 1	19 2 1 4 4 4 5	25 1 2 4 4 4 3	25 2 1 6 6	24 4 3 3 1 2	4 22 5 .0 3 1	12	19	7	3	3 4	2321552	43 14 14 48 35 1
Cast Cast Cast Cast Cast Cast Cast Cast	1 9 5 1 2	3 7 4 1 4 2	1 2 5 4 9 3	1 2 1 2 4 4 4 2 5	1 3 1 4 6	22 21 62 34	6 3 4 3 7 5	6 2 1 4 2 7 4 4 1	10 22 22 27 22 5 22	12 2 5 6 1 4 1	19 8 1 4 4 4 5 4	25 2 4 4 4 3 3	25 2 1 6 2 6 2	24 4 3 3 1 2	4 22 5 .0 3 1	12	19	7	3	3 4	2321552	43 14 14 48 35 1
ast	1 9 5 1 2 2 4	3 7 4 1 4 2 3	1 2 5 4 9 3 1 4	1 2 1 2 4 4 2 5 2 4	1 3 1 4 6 4 5 2	22 21 62 34 3	6 3 4 3 7 5 2	6 2 1 4 2 7 4 4 1 5	10 22 22 27 22 5 22 4	12 2 5 6 1 4 1 5	19 % 1 4 4 4 5 4 1	25 2 4 4 4 3 3	25 2 1 6 2 6 2 3	24 4 3 3 1 9 5 4 1	4 2 3 1 4 5 3 2	12 4 3 5 7	19	1 1 1 2 3 3	3 1 4 2 5	341	2321552 421	43 14 1 41 43 35 1 13 3
.: NE cast .: SE .: SE .: SE .: SE .: SW .	1 9 5 1 2	3 7 4 1 4 2	1 2 5 4 9 3	1 2 1 2 4 4 4 2 5	1 3 1 4 6	22 21 62 34	6 3 4 3 7 5	6 2 1 4 2 7 4 4 1	10 22 22 27 22 5 22	12 2 5 6 1 4 1	19 8 1 4 4 4 5 4	25 2 4 4 4 3 3	25 2 1 6 2 6 2	24 4 3 3 1 2 5 4 1	4 2 5 3 1 4 5 3 2 65 E	12 4 3 5 7	19 13 52 7 2 64	1 1 1 2 3 3	3 1 4 2 5 	3 4 1 2 2 1 43	2321552 421	4 3 1 4 . 1 4 1 4 8 3 5 1

TABLE B B .- Showing the duration of the wind, &c .- Continued.

Points.	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	101	11
JUNE, 1864.																		1					
North	3			1 2			 2 3	3 2		 3 2	 2 2	 2 2	2	2 2 1	1 2 2	1	1	1 2 1	1 1 1	2	1		1
NE			••••	••••	1	3	3	2	2 2		3	3	3	1	2	1	i		- 1		2	1	1
ust					''i'			2		2	2			2				::	i	::		:: :	
. SE								1	5	5	ī	7	7	7	8	7	6	5	2				
E	··i·	··;·	;.				••••	1	••••		:-		••••	••••	1	2	2	1				•;∤•	٠٠ ٠
onth		. 1	1	[••••	''i'	4		ا٠٠٠	••••		::				••)	1	:: :
. 8W					i	2	2	2	2	î	i	i	i	i	i		1	1	i	::1			
W	1	1	1				1 3		2	2	2	1	2	2	11	9	2	1	1 1 2	i	2	2	1
V. SW	ī	8	2	2	1	1	3	3	1	1	1	2	1	1	2	2	2	2	2	4			1
V. NW	::::			••••		"i"	i.			::::								::	::	.1	i	il.	
i W	2	1	1	1	1		1	1									- 1				1		1
1. NW						1	1			1				• • • •	• •		••		$\cdot \cdot $		••	1	1
Sums	8	8	8	6	9	10	15	16	19	18	18	18	18	18	18	18	17	14	10	7	8	7	7
Calms	11	11	11	12	8	7	4	3	0	0	ī	1	1	1	1	1	2	3	7	10	11	12	12 1
JULY, 1964.																							
iorth	1	1	2					1														1	
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South	• • • •	··i		••••	••••	••••	1	··i	ï	"i"	"i"		"i		i	1	1	$\cdot \cdot $	1	41		1	5
9 W		. .		1					, î	î					1	i	i	i	ĩ	è	i	٦į.	
sw	3	1	1		1	1	1	2	1	2	3	2	1	2	1	1 2 2	1 1 3	2	1 2	1	2	6	6
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NW	2	1	2	ï	··i	₁	1	ĩ		i				i.	2	1		2					
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Sums	13	12	11	9	10	10	17	22	21	24	24	24	24	24	23	22	23 =	21	_	_	=	11	-1:
Calms	11	18	12	14	13	12	6	2		_			0	0	0	1	0	2 =	5 =	11	11 =	13	10
AUGUST, 1864.																							
North	1			2	1	1			<u>-</u> -	<u>:</u> -					4	٠.,		1	5	اي.	 3	٠.	ا.
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E. SE	• • • •	• • • •			• • • •	• • • •	1	1	3	3	4	7	7	8	8 2	5	3 6	l	4	.:	2	1	1
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South	1		 .	i	1	1	1			î	2	î	3	l .			1 5		آا	 2 1 2	3	1 3	i
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NW N. NW	1 3	2	2 4	4 2	5	3	3	2	2	3	2	1	2	1	::	::	i	1	3	1 3	5	2	3
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8ums	16	14	16	15	14	16	19	20	24	24	25	25	25	25	25	25		20	17	17	15	11	14

TABLE B B .- Showing the duration of the wind, &c .- Continued.

Points.	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9 1	0,111
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JUNE, JULY, AND AUGUST, 1864.																						1!
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E. NE			••••				3	2	4 2	3	9	10	6	3	4	5	5	1	3	3		1
East	••••	••••		• • • • •	1	• • • •	··i	2	10	3	10	···· 22	24	3 21	نذا	3		1,1				
E. NE East E. SE		• • • •				ï	2	2 2 5 2 1	2	13	3	2	1	6	6	9	12	12	5 7	5	26 92	1, 1.
8. SE South	2	2	2	1	1	1	1	1	1	1	5	2	1		1	1	3	1	3	2	2	2 3
6. 8 W	3	3		1	1 2	1 3	1 1 4	1 4	2 1 1 5	3	3 5 3 2	1	4 1		14	3	1	12	3	3	4	2: 3 1: 9 4: 3!
8W	3 4	2	3	2	1	1 7	4	4	6	13 2 1 3 2 7	4	4	3	5 2	3	4	4 5	3	3 3 3 3	2	5	8 7
W. 8W West	5 2	6	8	8	7		4 7 1	8	6 7 1	1	4	4 3	2 5	5	3	4	5	4	3	2	'	2
W. NW	3	3	2		l i	3	5	5	5	4	6	3		3	3	2	9	::	:		i	1 1
NW	5	4	5	6	2	2	5	4	2	4	2	1	2	2	1	1	١	3	ï	i	3	2 2
N. NW	3	2	4	3	6	5	3	1		1	=	627	1				1	 	3	_	3	
Sums	37 31	34	35 34	30	33	36 92	51 12	58 	67	66	67	1	67	67	1	65 =- 2	=ا	=	=	=	=	29 33. = = 39 33 .
Cams	=	<u></u>	-	=	=	==		=		-		<u> </u>	<u> </u>	<u> </u>	Ë	=	=	=	_	=	=	==
JUNE, 1862, AT CLEVELAND, OHIO.																						
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East		··i·	1			ï								1	1			1				!'
E. 8E	i	••••	••••		••••		··i	••••					··i	••••	ļ		١	١	•••			
a a c						::::			2	2	2	3					::	::	ï		2	2 9
South	• • • •	••••				;-	···i·			···•	3	3	•	••••	٠-	٠-	١٠;	i	i	ī	i	3 3
South		ï	"i	i	1	1		1	1	3	i			ï	ï	۱ï	1:	1.	1.		i	
W. 8W												••••		2					١			,1
West W. NW	··i	ï		··i	"ï	··;·	2	··i	··i	··i·	··i·	"i"	··i	i	2	l'i	i	ŀ;	9	1	i	1 1
NW	1	i		2	l	1		l . .	l				1	î		2	1 9	1 2				
N. NW	2	ļ ·		1	3	2	1					• • • •	2		٠-	1	3	2	1			'
Sums	8	7	6	6	6	6	5	2	5	7	8	7	8	7	6	6	6	6	5	2	5	7.8
Calms	4	5	6	6	6	6	7	10	7	5	4	5	4	5	6	6	6	6	7	10	7	5 4
JULY, 1962.															ŀ							
North	··;·	· · · ·		ļ	····	 -	 		···i·	2	3	2	1 3	1 3	١٠;	2	i	'i	٠-		i	1 i
N. NE NE	l							"i					3	3	2	1	2	3	l'i	 1		
E. NE East	··i		ļ		ļ	ļ	ļ		ļ		ļ	ļ			i		١		٠.			
East E. SE	1	1						• • • •							1	::	١				••	!1
8E			l	 	::::	::::			 						::	1	::		::			
S. SE	8	10	9	7	8	8	6	4	2	1		··i	···i·	1	ì	2	2	2	2	8	6	10 9 ' l
South 8. 8W		2	5	5	4	4	5	1	1	••••		L	1		·:	:-	::	::	1 1	3	2	2, 2
8W		1	l	2	3	4	2	6	1 3 3	2	i	i	2	2	9	9		1 1	ī	·i	1	2 1.
W. 8W West	1	1	1		····	i	1	3	3 2	4	3	2		··i	1:	1	i	1	••	1		- 1.1
W. NW	l''i'		l <u>.</u> .	<u> </u>		l			i	3.	3	4	6	3	1 9	3	9	ï	::		i	
NW	!	1	l'i	1	l'i		i	1	1		1	2	l	l	١	١.,	١					''
N. NW	1	1	1	1	1		10	1	1		1	4	3	3	3	٠.	-	ij	1	<u> </u>		15 16 !
	15	17	17	15	16	16	16	17	16	13	12	16	16	14	(13	13	11	9	6	7	120	: 10 س
Calms	4	2	2	4	3	3	3	2	3	6	7	3	3	5	6	_	=	10	1	-	=	4 3

TABLE B B .- Showing the duration of the wind, &c .- Continued.

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Points.	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	101	1 15
AUGUST, 1862.																-							
North	··¡·	i							··;·	i.	3	3	3	1	 5	3	i		:			:: :	
NE E. NE East	••••		1	::::						••••				1	-1	3	5	3	4				
East E. SE	··i·		··i												1 1		-		:	1 2	2		
8E	12	1 12	12	1 12	112	1 12	10	7	2	2	2	··i		••••	1	1	1	2		3	1	2 .	1,12
S. SE							ì	1	2			1	··i	ï	ï		·i			4	. 1	2	
8. 8W 8W	1	1	1	··i	1	1	1 1	2		1	1	1	3	3	3	1	5	1	3	4	3	2	3 1
W. 8W West								ī	2	3	1	1			1	2	1	1			•-]		2
W. NW								··i·	1	··i	1 2	2	1	••••					::				i
NW						· · · ·				••••	···i·	1	1 2	3	2	2	2	2					
Sums	15	15	15	14	14	14	-14	13	10	8	11	13	14	15	14	14	14	13	!	<u>.</u> '-	12	16	6 15
Calms	1	1	1	2	2	2	2	3	6	8	5	3	2	1	2	2	2	3	4	4	4	0	0 0
september, 1962.																							
North		1	i	i	<u>.</u>	 1	i	···i	1	1	4	3 1	2	3		4	1	2	i	i		:: :	i
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E. SE SE		1	2		i	i	···i	1	• • • •	• • • • •	••••		••••			::	::	::	1	i	::	1	
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8. SW	.:::		:	ï		••••			••••		i	··i	••••		'n			i	ï		- 1		i
South S. SW SW W SW Wost W. NW NW NW NW NW NW NW NW NW NW NW NW NW											1	··i·	 1 1	i	.1	::	.1	::	::			:: .	1
West	''i'					·		• • • •		• • • •	• • • •	····	···i					$ \cdot\cdot $::	
NW	. .		••••							ï	i				i	i							
M. M W	<u> </u>						=		<u></u>	····	<u> </u>	<u> </u>	2	3	1	2	1		-		-!-		
Sums	8	8	9	- 1	8	8	8	8	<u>*</u>		8 	8	8	8 1	7	7	7	6	3	8	2	='=	6 7 2 2
Calms	<u> </u>	-				<u> </u>	-	-	1	<u> </u>	÷			=	2	2	=	_	=	4	=	= -	= =
JUNE, 1864, AT THUNDER BAY, MICH.																							
North	4	5	5	5	5	5	4	5	5	5	7	6	4	5	4	4	4		- 1		3		4 9
N. NE	5	"i	3.		3	··i			"i"			3,	5		::		i					::	1 2
E. NE East	··i·	···i·	<u> </u>	··i·	<u></u>	;-				3			···i		i	از:	i	٠i	i	i		3	i
E. SE	۱		l	l	۱			l							١١	ا		[…]	1				
SE	1		1	1	1	1	1	1	1	2	1	4	6	5	6	5	4	4	2	1	1	l.	1 2
South	5	5	5	3	4	3	3	4	5	6	6	5	5	8	6	8	6	8	8	6	5	6	6 6
8.8WW8.8	::::	ï	ï	"i		'n	ï	"ï	ï			"ï	::::	 ::::	2	3	2	2	2	i	i		: i
W. 8W West	4		3			1			2		3		••••			i	i	-:			٠i		1 1 2 2
W.NW		2	1	1	3	8	1	1	1			;-	i	"i"	1 1		• •	i	i		i	ı.	
NW N. NW	2	1	2	2	3	4	6	4	2	2		1	1	::::		::	::	::	::	ï			
Sums	19	18	20	18	20	19	20	20	21	21	21	22	22	21	20	-	19	_	19	\-	ļ.	<u></u> ¦-	6 19
Calms	3	4	2	4	2	3	2	2	1	1	1	0	0	1	2	1	3	2	3	8	8	6	6 3
	<u> </u>		<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>		<u> </u>	<u> </u>	<u> </u>	<u> </u>			1 !	_!	_	_		. 1	. 1		

TABLE B B .- Showing the duration of the wind, &c .- Continued.

Points.	1	2	2	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6 7	7 8	9	10 11
JULY, 1864.															П		Ī	- 		-	
or b	3	4	4	2	1	2 1	4	4	5	4	3	4	3	5			7	7	6		6 4
Œ		2	•••	3	2	i	i			i i	i	i	1			:-	:-			1::	1
. NE										ļ						٠.,١	1				'
ast	1		••••		2	1	1	1	1	2	2	4	5	3	3	1	1	1	1	1 1	1
E						1		2	2	3	7	6	6	8	8	9	8	8	7	5 3	1 5
SEouth	4	3	2	3	 . 3		4	5	6	5	5	5	5	5	4	3	٠.	5	6	7	7 :
. sw							٠٠			١					.]	1		5	1.		J. J.
W 7. ≋W	1	5	2	2	2	1	1	1	••••	1	• • • •			····	2	1	ij	::	•• •		1 1
Vest	4	3	3	2	2	2	3	2	2	i	i	i	i	ï	ì				!.	i i	i 1
V. NW			٠٠٠٠.	1 9	10	1 9	6	6	1				;-			٠į	-:		٠٠,		
ww	6	7	8		10	۳			5	6	3	1	1				1	1	1		1 1 3
8ums	19	21	22	22	23	23	21	22	22	23	23	23	23	23	23	23	 23	23	22.5	'_ 21,19	18 1
Calms	4	2	1	1	0	0	2	1	1	0	0	0	0	0	0	0	_ 0		=¦: 1	='= 2 4	5 8
		-		_	_	_		_	_	_				_	Ϊ	=' 	=	='	= -	= =	
AUGUST, 1864.							_									ı]	
orth	7	6	3	3	2	2	2	3	3	4	4	4	3	3	3	4		4	4	3 :	3 3
E		i	'i'	i	ï				i	i i	i	8	3	5	i	٦	i		::	i	i,
NE			 							١	•••			··· <u>·</u> ·							
ант	••••		1	1	1	2	2	2	1	1	1	1	1	2	2	3	2	1	1	1	-:
E	1	'n	ï							ì	i	ì	2	2	3	2	3	3	2		1
. 8E	5.	3	3	3	3	3.	3	3	i	ï	4	6	5	5	5	5	5	5	5	4	3 j.
. 8 W W	1	1	1		Ĭ	ì	ì	1	1	1	1					1		ا: ٠	1	.].;	. 1
v. sw	1	1	1	••••			i		1	1	••••		1	1					.1	1 1	11
/est	3	3	3	4	4	4	3	2	2	4	2	2	1	1							. 1, :
V. NW W		2	3	4	1 5	1 5	1 6	1 6	6	3	1 3	2		2	3	3	3	3	2	3	4 4
. NW																			.	.].	
Sums	17	18	17	16	18	18	18	18	18	18	18	18	18	18	18	18	18	17	15	5 1	111
Calms	1	0	1	2	0	0	0	0	0	0	0	0	0	0	0	0	0	1	3	3 4	4 7
UNE, JULY, AND AUGUST, 1864.																					1-:-
orth	14	15	12	10	8	9	10	12	13	13	14	14	10	13	12	15	15	15	14	4,1:	3 13 1
NE	2		5	6	6	1 2	i				4	6	9	2	i	i	2		i	i	
. NE			ļ		 	ļ													٠.١.		
ast	2	1	1	2	3	4	5	5	4	6	5	7	7	7	6	5	4	3	3	3 3	3 4
E	2	i	2	i	i	2	, i	3	3	6	9	ıi	14	15	17	16	15	15	ii	8 6	5 3
. SE				l	 -			l	1	1				 .	1					<u>.</u> !.:	٠
outh	11	11	10	9	10	10	10	12 1	12	12	15	16	15	18	15	16 1	17,	18 I 1	19,1	7 14	131
W	2	1	4	3	4	2	2	2	2	. 2	l	i i	i i	i	4	3		3	3	2 3	2 2
V. 8W Vert	1		1			1			1	7		3	2	2	$ \cdot_{\mathbf{i}} $:		-	٠١.		1, 1
V. NW	11	8	9	8	6	6	'8 3	6 3	6 3	i	6	3	×	ı	2	.,	1	ï	i.	. 1	1 1
W	10	10	13	15	18	18	18	16	13	11	Ĝ	4	4	ş	3	4	4	4	3	3 4	
Sums	55	57	59	56	61	60	59	60	61	62	62	63	63	62	61	62	60	60.	1_	_!_	45,4
Calms	8	6	-		:	_	=	_	_	_	-	<u> </u>	<u> </u>		1	1		_'-	₽=	==	===
			4	. 7	2	1 3	4	3	; 2	1	1	0	U	1	2	•	3	•			18.18

TABLE B B .- Showing the duration of the wind, &c .- Continued.

Points.	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
JUNE, 1865.																				- 				_
North	2	3	3	2	2	2	2	3	4	5	4	4	3	2	2	2	1	1	3	4	4	2	3	3
NE	2	i			i					ï	2	i i	2	ï	2	2	5	4	2	il	i	2	1	•
E. NE												 .			١					1	11	11	1	1
Enut		1	2	2	2	2	2	2	1	1	1	2	2	1	1	1	٠.	1	3	1	i	1	1	2
E. SE	••••		;-					;-	:	:-		:-		:	٠:	!	1	٠:	ان	.:	1		•:	٠:
SE	1	2	1	1	1	1	1	1	2	2 1	2	2	3	5 l	5 1	1	3		2 2 4	2	2	2	1	1
South	4	4	5	5	4	3	6	6	7	8	8	8.	7	7	6	5	1 5	4	4	4	3	3	3	3
8.8W						i																		
8W	1	ì	2	2	2	3	3	1													i	1	1	
w. sw											 											٠.		٠.
West	3	1	1	2	1	1		••••								1	1	2	1	2			1	2
W. NW	•••			3	:-		1	1	··ːː·			• • • •		• • • •	١٠٠	• -	• •		٠-	٠;	3	٠:	-:	2
NW N. NW	3	3	3	- 1	2	2	2	2	2	••••					::	::	:	:	• •	1		4	4	
*** ** ** *********			••••	• • • • •	••••								••••		<u></u>	<u>::</u>	_						_!	•
Sums	16	16	17	17	15	15	17	16	17	18	18	18	18	17	17	17	17	17	17	16	16	16	15	14
Calms	2	2	1	1	3	3	1	2	1	0	0	0	0	1	1	ī	1	1	1	2	2	2	3	4
	==		-			_	==	==	==	_	-	-		=	=	=	=	=	-	=	=	=j:	=	=
JULY, 1865.																					1		- 1	
North	7	6	4	4	3	ı	2	4	6	7	7	5	5	5	6	7	7	7	7	7	8	7	8	7
N. NE.	•		•	, T	3	•	- 3	3	0	•	'	٥	١٠		٥	'	'	'	1	٠,	٩	.1	٩	•
NE					i	i	2	i	i	2	4	5	5	5	3	3	2	2	2	i		i		•
E. NE						1															1			
Eust	1	1	1	1	1	1																		٠.
E. SE	•••	••••	••••	:-			1	1	1	1	1	1	1		<u>ا:</u> ا	٠:	٠:	.:	٠.:	اذ	اي-	٠: ا	اد-	٠:
8E	1	••••	• • • • •	1	1	2	2	2	1		1	2	2	3	2	1	1	1	2	2	2	2	2	l
South	ï	2		··i	i		••••		i	2	4	5	5	5	7	7		7	6	4	4	4	3	· ;
8. 8W			. .														١]]		
8W			2	2	2	3	3	3	2	1										1	1	1	1	
W. 8W						,.	,.															-:	1	٠.
West	2	3	2	1	1	1	1	1	3	4	2	1	1	:-	·:	٠:	٠.	.:		٠;	::	2		3
W. NW		1	3 5	3 6	2	2 7	8	5	4	2	··i	···i	··i	1	1	1	1		1	1	1	1	1	
N. NW		•				lí	ı	i	i	li					,	•	*	•	•	•			1	i
Sums	15	17	19	19	19	20	20		20	20	20	20	20	20	20			-		17	_	19	10	- 16
Calms	======================================	3	1		=	0	0	0	0	0	0	0	0	0	= 0	=	0	=	=	 3	=	1	= !	4
Carms	===		<u> </u>					_						_	=	=	=	='	=	=	=	=	=	_
AUGUST, 1865.																								
North	5	6	4	2	3	4	3	4	4	5	7	8	8	9	10	9	9	7	7	6	٥	9	5	5
N. NE.				lî	î	i	2	3	3	2	Ιí					i	1	.'I		."	- 1	.]	ĭ	
NE	i	i	i	î	i	i.	ĩ	ĭ	ĭ	ĩ	li	i	ï	i		i	i	1	i	i				
E. NE			١																				- 1	٠.
East	1	1	2	2	2	1	1			1					·:	<u>.</u> ا	<u>.</u> ن	ايرا	1	1		1	1	• •
E. SE	••••		;.	;		2	••••	2	•••	;-					23	2	2	2 5	2	1	14	1 2	2	• ;
8E	1	1	1	1	1	, X	1	z	Z	3	•	6	6	6	1	1	4			7				-2
South	5	2	2	3	4	5	4	4	5	5	6	7	9	7	7	9	8	7	6	6	6	6	6	8
8.8W		ĩ					1	1	ļ	1	l i	<u>.</u> .		ļ . .		اا]]].	
8W	2	4	4	2	2	2	3	2	1	1	ì	1			1									1
W. 8W				••••			;.		1	1		1											٠;١٠	٠:
West	2 1	3	4	4	3	2	1	3	3	1	;-	••••	1									1	1	1
W. NW	4	5	7	9	7	6	5	5	3	2	1 2	···i	··i·	1	i	i	ij	i	•••	••		••	-	i
N. NW	i	ĭ	í	1	i	ì	ĭ					i	i	'n	i	i		2	3	2			i	i
								-		-		_	_		_	_	_	_	-	-	_!	-	-	_
8ums	23	25	26	26	25	25	23	26	24	24	24	26	27	26	26	27	27	25	23	21	21	20	18:	w

TABLE B B .- Showing the duration of the wind, &c .- Continued.

Points.	1	2	3	4	5	6	7	8	9	10	11	12	1	2	. 3	4	5	6	7	g	9 10	0 11 1:
JUNE, JULY, AND AUGUST, 1865.											_		ĺ		1		_			;- 		!
North	14	15	11	8	8	7	7	11	14 3 2	17	18	17	16	16	18	18	17	15	17	17	21 1	8 16 1
N. NE	. .	١	l	1	1	1	2	3 2	3	2	1	l		l		1	1	٠١		- 1	'.	. 1
NE	3	: 2	1	2	3	. 2	3	2	2	4	7	7	8	7	. 5	6	8	7	- 5	. 3d	ľ	3 1.
E. NE	l	ļ			••••	1			١	l				l				٠١		1:	ı	11
East	2	3	5	5	5	4	3	2	1	2	1	2	2	1	1	1		1	4	2	2	2 2
E. 8E	. .		l			١	1	1	1	1	1	1	1	l	2	3	3	2	2	1	ı	1,
8E	3	3	5	3	3	5	4	5	5	5	7	10	11	14	10	7	8	10	8	8.	8	6 5
8. SE		i							1	i	1	1	1	2	2	2	1	l!	2			11.
Bouth	10	8	9	9	.9	8	10	10	13	15	18	20	21	19	20	21	21	18	16	141	131	3 19 1
8. 8W		1				ì	1	1		1	1								٠.'	1	· .	
8W	3	5	6	6	6	' 8	9	6	4	2	1	1			1			'		1.	2	2 2
w. sw		l					١		1	ì		i				!				1		
West	7	7	7	7	5	4	9	4	6	5	2	1	3			1	1	2	1	2	!	2 4
W. NW	i	i	3	3	2	2	3	4	i	i i	ī			9	1	1	1	1	1	1	1	11
NW	10	12	15	18	16	15	13	12	9	4	3 '	2	2	1	2	2	2	2	1	2	4	5 5
N. NW	1	1	1	1	1	2	2	1	1	1		1	1	1	1	1	1	2	2	2		. 1
Sums	54	58	62	62	59	60	60	62	61	62	62	64	65	63	63	64	64	61	 59	54	54 5	3 52 5
Calms	11	7	3	3	6		-5	3	4	3	3		_	9	=	٦	╗	=	=:	1	= = 11 1	e = = = 0 13 1

TABLE C.C.—Showing the duration of the wind at the several points of the compass for each hour of the day, the stormy and cloudy days being eliminated for the following months:

	ī	í	ī	ī	ī	ī	ī		-	1	Г	ī	1	Г	Т-	ī	T	τ-	_		1	_	_	
Points.	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12
JUNE, JULY, AND AUGUST, 1861, 1862, 1863, AND 1864, AT MILWAUKEE, WIS. NOTH N. NE NE E. NE E. SE S. SE S. SE S. SE S. SW W. SW W. SW W. NW N. NW	22 6 9 12 10	21 7 3 1 4 13 16 20 4 14 9 8	15 3 1 2 2 3 17 21 3 12 13	5 1 1 4 3 5 15 18 5 12 17 9	24 4 4 1 1 4 4 5 10 18 4 11 15 10	6 4 1 3 3 3 3 9 9 21 8 11 19	27 21 6 5 29 20 20 13 16 21	34 19 6 13 10 3 4 12 15 24 11 18 12 14	36 19 12 6 21 4 4 10 18 24 8 20 9	27 25 15 6 34 26 4 3 10 26 18 5 13	222 19 10 31 39 4 4 21 13 8 17 12 3	24 22 3 45 47 2 11 22 12 8 15 10 3	15 23 15 5 51 49 4 7 6 24 6 11 12 8	7 48 59 5 9 19 7 8 11	48 56 8 1 12 15 6 7 15 7 4	22 13 12 28 61 16 4 7 19 8 5 15	31 57 10 2 8 21 7 7 9	18 56 16 3 9 14 12 3 6 10	11 11 2 8 5	7 4 5 19 7 12 11 6 3 5 4	11 55 55 13 10 6 18 15 6 8 7 6	1 4 8 4 1 6 20 5 4 3 10 4 .	7 3 1i 24 16 1 5 9	2 : 23 4 4 2 13 24 17 4 8 6 10 -
Sams	132	129	120	119	121	144	193	204	234	236	239	240	240	241	239	238	234	212	170	35,	38	23	29 1	24
Calms	109	112	122	118	114	85	42	25	4	3	1	1	1	1	1	3	6	25	66		02		12	17
8 W	4 3 3 1 1 25 3 1 3 1 3 46	1 2 1	1 1 2 26 1 6 1 1 2 1	1 2 24	26 3 26 3	2 26 3 6 3	1 3 22 27 7 3 1 1	11 11 11 11 16 22 37 74 42	3 2 1	3 2 1 1 1 6 2 4 4 4 7 7	4 6 1 5 3 4 1 6 2 2 2 3 9	56 1 4 2 5 1 4 2 7 3 4	1 2 6 1 1 9 2 9	1 1 6 1 1 4 3 8	10 5 2 2 1 1 1 1 1 1 4 1 6	1 3 4 3 7	1 2 10 2 2 3 3 3 1 1 6	2 1 6 6 1 1 2 2 2 1 4	7 6 2 2 2 2 2 2	332218116	2	3 3 7 4	1 29 3 3 3	
Sums		'	!	':	'.			='=		=='=		'_	!_	44	_!	-	_	==:'=	!_		='=	1=	=	
Calms	10	9	9	13	12	12	13	16	17	20	17	12	10	12	16	16	18	22	27 2	7 2	0 1	11 3		,

TABLE C C .- Showing the duration of the wind, &c .- Continued.

Points.	1	2	3	4	5	6	7	8	9	10	u	12	1	2	3	4	5	6	7	8	9	10	11	12
JUNE, JULY, AND AU- GUST, 1864 & 1865, AT THUNDER BAY ISLAND, MICH.																								
North	₅	6		18 1 8	1		4	3	4	2	1		26 17	 .	6	7	39 1 10			4	2		1 2	3
E. NEEastE. SE	 4 		6	7	 8 	8 	8 1 5	1	5 1 8	 8 1 11	6 2 16	عة ا	 9 2 25	8	7 2 27	6 3 23	3	2	2	1	5 1	6 1	4	1 3
8. SE South S. SW	2i 2i	19			19 1 1 10	2	20	22 22	2	1 27	1	36	36	37	2 35	37 1	38	36 1	35	31	27	1 26 1	1	
8W W.8W West W. NW	18	15 3	1 16	15 5	11	1 10 6	10	10	12 4	1 12 2	 8 2	1 4	4	2	3 5	 2 1	 2 1	 2 2	2 4 2		2	1 4 2	1	1
NW	1	1	1	33 1	1	2	2	1	1	1	•••	_1	• • •		L	1	1	6 2	2	_	•••		1	2
		_		118		_	_	==	==		124			_		_	_	_	_=			_	=	=
Calms	19	13		10	-8	8	9	6	6	4	4	1	<u></u>	3	4	2	4		13	24	27	28	31	96
JUNE, JULY, AND AU- GUST, 1864 & 1865, AT THUNDER BAY ISLAND, MICH.																								
North NE East SE	29 5 4 5 22 5	30 6 4 4	6	7.	9 8 4	7	19 4 9 5 22	2 8 10	30 4 6 10 27	6 9 12	8	13 11 22	17 11	29 9 31 37	9 29	7 9 24	10	7	31 6 9	5	3 6 14	4 7 10	3 4 10	4 3 9
South	5 19 21	21 9 18 23	11 20	9 20 34	20 10 17 35	16	11 16	8 17	8 16 23	5 14	1 10 9	30 4	1 4	1 5	5 4 6	38 3 3 7	24 38 38 37	34	19 36 3 3 6	34 48	5 4 8	5 6	5 11	5 16 14
Sums	110	115	119	119	120	120	119	123	124	123	122	127	128	125	124	124	124	122	113	104	101	101	97	102
Calms	18	13	9	9	8	8	9	5	4	5	6	1	0	3	4	4	4	6	15	24	27	28	31	26

APPENDIX D.

Return of Charts issued at Detroit and Buffalo from July 1, 1866, to June 30, 1867.

,			186	56.					186	7.		1	÷
Charts of—	July.	Angust	September.	October.	November.	December.	January.	February.	March.	April.	May.	Jupe.	Total of each.
Lake Erie West end of Lake Erie Keily's and Bass Islands Straits of Mackinac East Neebish Rapids Head of Green Bay St. Clair Flats Buffalo Harbor Tawas Harbor Beaver Group Eagle Harbor St. Mary's River, No. 1 St. Mary's River, No. 2 Maumee Bay Eagle River Ontonagon Harbor Saginaw Bay Thunder Bay Marquette Harbor Presque Isle and Middle Island.	25 18 21 21 19 26 21 24 21 14 17 20 14 14 28 25 27	25 20 28 19 26 25 24 25 19 18 18 28 19 19 24 19 24 25 19	12 16 18 13 9 13 12 14 13 15 9 10 10 10 11 15 10	18 17 18 17 14 19 20 16 14 13 14 20 17 15 20 19	10 7 8 8 5 8 7 9 8 9 5 5 5 5 5 9 5 5 5 6 10 10 10 10 10 10 10 10 10 10 10 10 10	111111111111111111111111111111111111111	1	221131211222222222222	655656555555555556656	33 31 22 27 16 21 32 26 17 16 16 17 28 17 19 33 33	44 41 39 34 27 35 39 37 37 34 29 27 31 30 40 28 42 36 38	36 34 37 34 29 23 37 36 34 29 31 31 29 30 37 32 31	212 193 194 191 144 172 900 196 200 187 144 140 150 199 147 148 213 207 151 192
Lake Huron	30 19 17 18 32	20 29 27 28 34	11 23 14 18 18	20 18 24 25 28	12 13 9 9	1 2 2 2	1	3 4 5 5 5	5 5 5 9	40 38 24 40 45	37 41 35 36 36	30 42 35 38 38	210 235 197 225 246
Bays. North end of Green Bay Copper Harbor L'Anse and Keweenaw Bay Portage Lake	16	24	23	31 9	15 7 1	2 2 2	1 1	5 5 5	11 5 5	48 44 46	42 45 45 38	98 38 37 35	945 156 142 73
Total in each month	548	641	364	511	217	37	6	77	162	846	1079	976	5, 461

Table showing the annual issue prior to July 1, 1867.

Charts issued—	Number.	Charts issued—	Number.
Prior to October 1, 1857	2, 600 4, 890 3, 254 5, 245	October 1, 1863, to October 1, 1864 October 1, 1864, to October 1, 1865 October 1, 1865, to October 1, 1866 July 1, 1866, to July 1, 1867 Total to July 1, 1867	2,58

APPENDIX V.

Engineer Department, Washington, May 20, 1867.

SIR: The letter of Samuel Adams respecting his examinations of the Colorado River of the West, referred to this department for report, confirms other information in possession of this office regarding the importance of resuming the explorations of that river to its source, beginning at the locality where Lieutenant Ives, of the topographical engineers, terminated his survey, a few miles below the Mormon settlement of Callville, the present head of navigation, over 500 miles from the mouth of the river.

Between Callville and the sources of the river there are larger portions of the Colorado entirely unexplored. At two points in Great Cañon region, 100 and 150 miles, respectively, above Callville, Lieutenant lves succeeded in getting down to the surface of the river with his land party, and considered it at these points unnavigable in the low-water stage, owing to rocks and rapids. The marks of high water were 50 feet above the low-water.

So far as known, the duration of the high water in this part of its course is

brief, and the time of its occurrence uncertain.

It has been stated that during the last winter the river was ascended some 50 or 80 miles in a skiff from Callville, and found to be navigable for steamers at that time.

When Colonel Macomb in his exploration reached the vicinity of the junction of the Green and Grand rivers, the principal tributaries of the Colorado, he was

unable to descend to the surface of the river, owing to the great canons.

About midway between the junction of the Green and Grand rivers and the highest point at which Lieutenant Ives reached the river surface, Escalaute in 1776 crossed the river, but only after a search of several days for a crossing place. It is probable that the Mormon crossing mentioned by Captain Adams is at this point.

It will be perceived from the foregoing statement that the Colorado river is virtually unknown between the Mormon settlement of Callville and the junction of the Green and Grand rivers, which form the Colorado, in latitude 38°,

a distance of about 400 miles.

Green river, the chief branch, rising in latitude 43°, is also but little known in its length of more than 700 miles, so far as relates to its adaptation to pur-

poses of navigation.

The lower portions of Green river run through canons; with that exception it is a valley stream, and should be explored at a different season and in a different manner from the Colorado.

The exploration of the Colorado should be commenced from Callville during the low-water stage, early in December, and should be made in a flat-bottomed, iron stern-wheel steamer, not drawing more than 12 inches, as recommended by Lieutenant Ives.

. To this should be added light skiffs enough for the whole party, so that the expedition may be continued in them beyond the highest point the steamer may be able to reach in the event of her being disabled by sunken rocks, or other accident.

The party should be commanded by an officer of engineers, and composed

in the manner usual for exploring parties.

Green river, except the lower part, should be examined in the summer or fall, by a party provided with land transportation and boats, and the examinations may commence in the upper part of its course.

The only appropriation applicable to exploration and survey of the interior

is that made for surveys for defence.

An examination of the Colorado simply for military purposes might be made by a small engineer party and escort at a cost of \$15,000 or \$20,000 to the fund for military defences, provided the quartermasters' department could furnish the transportation for the escort—that is, pay for one-half the cost of the steamer and skiffs.

The examination of the Green river should not cost more than \$10,000.

In addition to these two explorations the necessity of the removal of the obstructions to the navigation of the Colorado below Callville will soon attract attention, as there seems to be a growing trade upon it, employing some seven or eight steamers.

Very respectfully, your obedient servant,

A. A. HUMPHREYS,

Brig. Gen. and Chief Engineer, Major General Vols.

Hon. E. M. STANTON, Secretary of War.



V 1.

ENGINEER DEPARTMENT, Washington, March 21, 1867.

SIR: In accordance with directions of the Secretary of War of this date you are appointed to take charge of the explorations provided for in section 3 of the act of Congress, approved March 2. 1867, authorizing the Secretary of War to direct a geological and topographical exploration of the territory between the Rocky mountains and the Sierra Nevada mountains, including the route or

routes of the Pacific railroad.

The object of the exploration is to examine and describe the geological structure, geographical condition and natural resources of a belt of country extending from the 120th meridian eastward to the 105th meridian along the 40th parallel of latitude, with sufficient expansion north and south to include the lines of the "Central" and "Union Pacific" railroads, and as much more as may be consistent with accuracy and a proper progress, which should be not less than five degrees of longitude yearly. The exploration will be commenced at the 120th meridian, where it will connect with the geological survey of California, and should, if practicable, be completed in two years.

You will examine all rock formations, mountain ranges, detrital plains, mines,

coal deposits, soils, minerals, ores, saline and alcaline deposits.

You will also collect material for detailed maps of the chief mining districts coal fields, salt basins, &c., as well as material for a topographical map of the region traversed, and conduct a systematic series of barometric and thermometric observations, with constant study of the atmospheric conditions bearing upon the subject of refraction and evaporation.

You will also make collections in botany and zoology, with the view to a memoir on these subjects, illustrating the occurrence and distribution of plants

and animals.

You are authorized to employ the following assistants, at the monthly rates of compensation set opposite to each respectively, namely:

One assistant geologist	\$200
One assistant	150
One topographical assistant	200
One topographical assistant.	150

One botanical collector	\$ 50
One zoological collector.	50
One photographer Six laborers, at rates of locality.	100
Six laborers, at rates of locality.	
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REPORT OF THE SECRETARY OF WAR.

Your own compensation as geologist in charge of the exploration will be at the rate monthly of \$250.

You are authorized to subsist the employes, including yourself while on duty in the field, as is usual in like surveys, and you are authorized by the War Department to purchase subsistence stores from the subsistence department of the army, when practicable. You are also authorized by the War Department to call upon the commanding general of the division of the Pacific to furnish an escort of twenty mounted men, (California cavalry if possible,) with the proper number of non-commissioned officers and the necessary camp equipage, subsistence and transportation therefor.

You are authorized to make the outfit for your employes, embracing camp equipage, subsistence, and transportation, to be paid from the funds applicable

to the exploration.

You will make requisition upon the engineer department for funds as they may be needed for outfit, and for the current expenditures for the month succeeding. All funds expended by you must be in accordance with the rules and regulations prescribed for the disbursement of public funds, with which you will acquaint yourself before proceeding upon your exploration.

You will be required to enter into bonds for the faithful expenditure of such funds, in the amount of \$20,000, with two sureties, according to the form here-

with.

You will make reports monthly, or more frequently if occasion requires it, of the progress of the exploration, stating in general terms the duties upon which the employes have been engaged and the results obtained. If not within reach of the line of mail communication at the time of making up your monthly reports, you will transmit them as soon thereafter as the means at your disposal will admit, either through a messenger or other safe conveyance to the nearest mail station.

Very respectfully, your obedient servant,

A. A. HUMPHREYS.

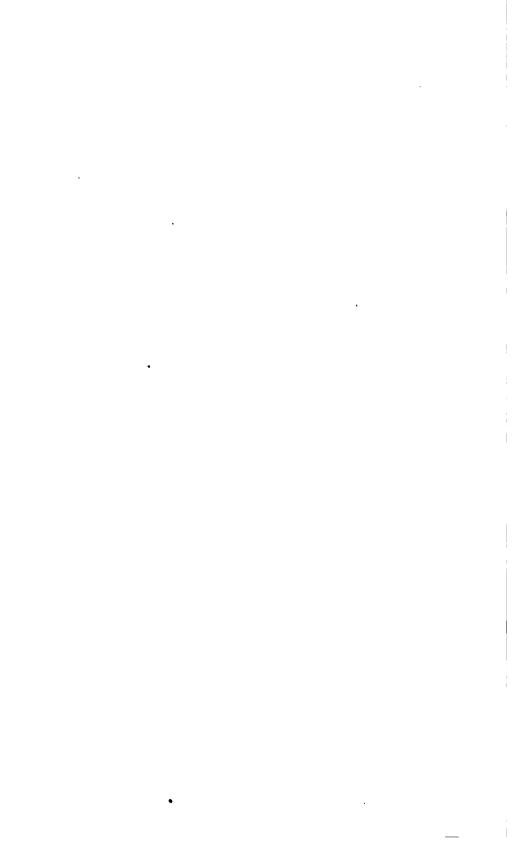
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Brig. Gen. and Chief of Engineers, Major General Vols.

Mr. CLABENCE KING, Washington.







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